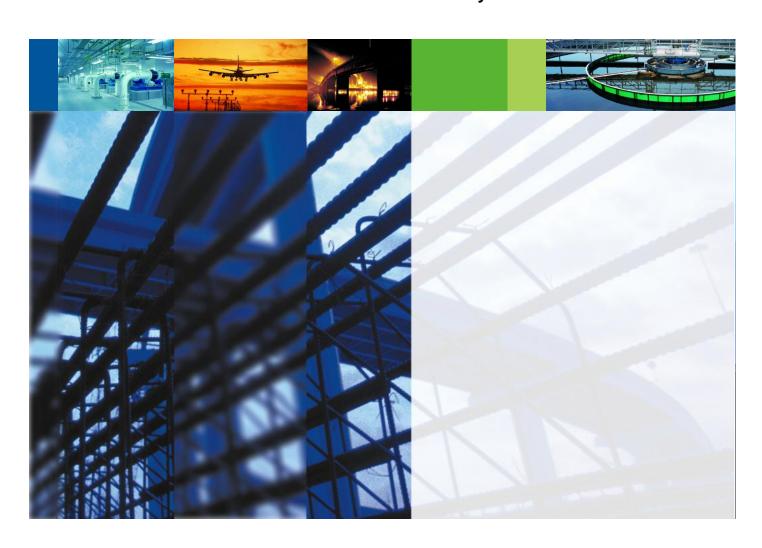
# **Final Report**

# **The City of Cold Lake**

**Cold Lake Transportation Study** 

## February 2012



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## **FINAL REPORT**

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## Introduction

The City of Cold Lake (City) is located 287 km northeast of the City of Edmonton and was formed by merging three municipalities, namely Cold Lake, Grand Centre and Medley (Canadian Forces Base W4) in 1996. Grand Centre was subsequently renamed Cold Lake South (CLS) and the original Cold Lake is now known as Cold Lake North (CLN).

The City has experienced noticeable growth in recent years. According to municipal census the City had a population of 11,991 in 2006 and 13,924 in 2009. This corresponds to a 5.4% linear growth annually. Current transportation improvements within the City have been based on the previous transportation study completed in 2000 and is no longer considered representative of the actual transportation network required to address current and future transportation needs.

In light of the continuing accelerated pace of development in the region and the need to rationalize and identify the transportation network requirements for the City, including surrounding rural municipalities and counties, the existing transportation plan requires a comprehensive update.

#### 1.1 PROJECT BACKGROUND

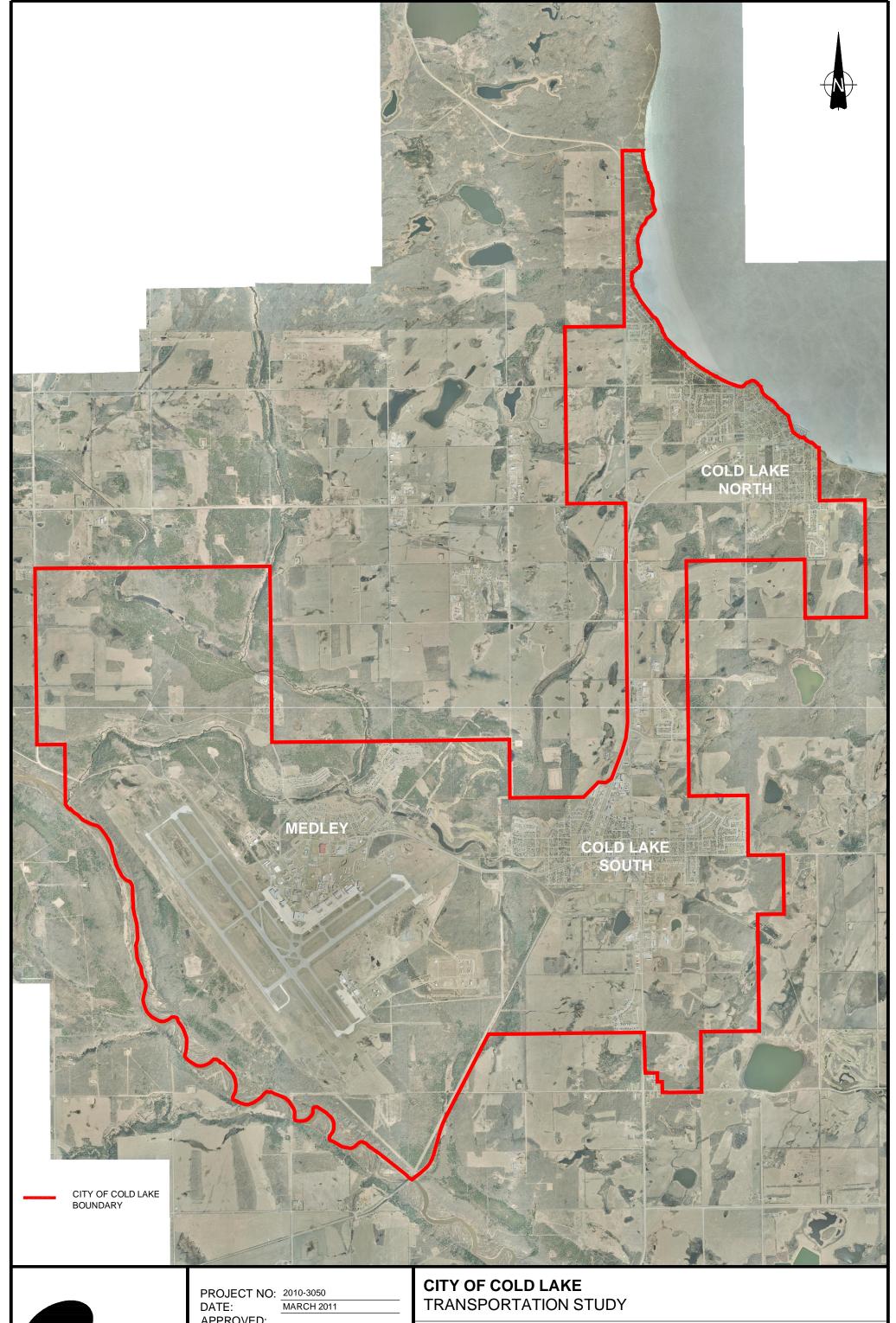
Associated Engineering (AE) was retained by the City to update the 2000 transportation study. The purpose of the transportation study is to provide a comprehensive long range plan that integrates the transportation infrastructure requirements of the existing and future land uses. The transportation study will provide the City with a blueprint on which to plan and implement specific transportation network improvement projects over the next 20 years, in 5-year, 10-year, 15-year and 20-year planning horizons. The transportation study will consider municipal roads, traffic calming, parking, traffic safety, traffic signal coordination, school zones, transit, truck routes, traffic management, and transportation system operations.

This report compiles the major findings from the different components of the transportation study; detailed information regarding the analysis and results can be referenced in the full technical memorandum.

#### 1.2 PROJECT STUDY AREA

The transportation study encompassed the area bounded by the current City limits, including Cold Lake North, Cold Lake South and Medley. Figure 1.1 presents the study area.







APPROVED: SCALE:

DWG NO:

NTS

FIGURE 1.1 TRANSPORTATION STUDY - STUDY AREA

### 1.3 PROJECT COMPONENTS

The transportation study was broken down into the following three stages:

- Stage 1 Study preparation, model development and traffic volume forecasting
- Stage 2 Operational analysis and various transportation studies
- Stage 3 Reporting.

Table 1.1 summarizes the various tasks that were proposed and approved as part of the transportation study.

Table 1.1
Transportation Study Tasks

Project Stage	Project Task Number	Project Task Description
	1A.1	Project Initiation Meeting
	1A.2	Data Collection and Review
	1A.3	Stakeholder Discussions/Meetings
	1A.4	Supplemental Data Collection (Optional)
	1A.5	Land Use Data
	1A.6	Planned Developments
	1A.7	Traffic Analysis Zones
	1A.8	Base Road Network
1	1A.9	Traffic Volumes
	1A.10	Study Review Meetings
	1B.1A	Model Development - VISUM Model
	1B.1B	Model Development - EMME Model
	1B.1C	Model Development - Spreadsheet Model
	1B.2	Existing (2010) Traffic Operations
	1B.3	Traffic Volume Forecasting
	1B.4	Transportation Policies and Strategies
	1B.5	Study Review Meeting
	2A.1	Capacity Analysis for Planning Horizons
	2A.2	Development and Evaluation of Roadway Improvements
	2A.3	Traffic Calming
	2A.4	Parking Management
2	2A.5	In-Service Safety Reviews
	2A.6	Coordination of Traffic Signals
	2A.7	Highway 28 Functional Review
	2A.8	Truck and Dangerous Goods Routes Review
	2A.9	School Zone Safety Analysis
	2A.10	Transit Services



Project Stage	Project Task Number	Project Task Description
	2A.11	Sustainable Transportation
	2A.12	Develop Cost Estimates
	2A.13	Prioritize Transportation Infrastructure Capital Improvements
	2B.1	Determine Land Use and Utility Conflicts
	2B.2	Study Review Meeting
	2B.3	Acquire/Purchase Software
	2B.4	Provide Training to City Staff
	2B.5	Integration of Software to other Applications
	2B.6	Transportation Levy
	2B.7	Public Consultation Process
	3.1	Draft Report
	3.2	Plan Submission
3	3.3	Study Review Committee Meeting
	3.4	Final Report
	3.5	Presentation to Council

Tasks 1A.1 through 1B.1 were predominantly preparation work for the transportation study. Where the information was relevant and utilized for completion of a task, it was included as part of the appendix in the relevant technical memorandum. Therefore, Tasks 1A.1 through 1B.1 were not documented in a separate report and will not be summarized in this report.

At project initiation, some tasks were determined to be not applicable as a result of the traffic volume forecast model selected. The spreadsheet model was selected by the City; therefore Task 1A.7, Task 2B.3 and Task 2B.5 were not applicable. Additionally, as the study progressed, several tasks were determined to be unnecessary by the City. These tasks include Task 1B.4, Task 2A.2, Task 2A.13, Task 2B.1 and Task 3.2.

A technical memorandum was prepared for each task completed above. The traffic calming and in-service safety reviews were completed and documented together, as part of the in-service road safety review. Additionally, the study review meetings (Task 1A.10, Task 1B.5, Task 2B.2, and Task 3.3) were completed in a series of telephone conversations (weekly progress meetings) and several face-to-face meetings, and will not be documented in the report.

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## **Project Tasks**

Each project task completed for the transportation study will be discussed as a separate section below. A summary of each project task will be provided along with guidance to the full report, which can be referenced for detailed information about the assumptions, methodology and complete results from each task.

#### 2.1 EXISTING (2010) TRAFFIC OPERATIONS

Task 1B.2 (existing traffic operations) was completed and documented in the technical memorandum titled Existing (2010) Traffic Operational Analysis, included in **Appendix A**.

Only the major roadways (collector and arterial roads) were analyzed for the existing traffic operations. Traffic operational analyses were completed at the intersections between two collectors, between a collector and an arterial, and between two arterials, as per the 2000 transportation study road classification.

Overall, most of the intersections within the City are currently operating above acceptable levels (LOS C or better). The intersections which are not currently operating above acceptable LOS are:

- 8 Avenue and 16 Street
- Highway 28/55 with 75 Avenue
- 28/55 with 61/62 Avenue
- Highway 28/55 and 54 Avenue
- Highway 28/55 and 52 Avenue
- Highway 28/55 and 52 Street
- Highway 28/55 and 46 Avenue
- Centre Avenue and 59 Street
- Centre Avenue and 57 Street.

Several of the intersections listed above are currently being upgraded as part of the Highway 28 twinning project. These intersections include:

- 8 Avenue and 16 Street
- Highway 28/55 with 75 Avenue
- Highway 28/55 with 61/62 Avenue
- Highway 28/55 and 54 Avenue.

With the intersection improvements that will be implemented as part of the Highway 28 twinning project, the intersections of 8 Avenue and 16 Street, and Highway 28/55 and 54 Avenue will operate above acceptable LOS and with low minimum delays. However the intersections of Highway 28/55 with 75 Avenue and 61/62 Avenue will require additional improvements.



#### The City of Cold Lake

The following intersections will require additional improvements, above and beyond the Highway 28 Twinning project improvements:

- Highway 28/55 with 75 Avenue
- Highway 28/55 with 61/62 Avenue
- Highway 28/55 and 52 Avenue
- Highway 28/55 and 52 Street
- Highway 28/55 and 46 Avenue
- Centre Avenue and 59 Street
- Centre Avenue and 57 Street.

The recommended lane configuration and traffic control are presented in Figure 6.1 through Figure 6.4 in the Existing (2010) Traffic Operational Analysis technical memorandum.

#### 2.2 COLLISION ANALYSIS

A collision analysis was completed as part of Task 1B.2 and documented in the technical memorandum titled Collision History Review and Analysis, included in **Appendix B**.

The collision history within the City was reviewed and used to identify high collision locations. AE also analyzed the collision data at the high collision locations to determine collision distribution patterns and provide the City with potential safety concerns at each location.

Between 2005 and 2009, a total of 2,071 collisions occurred within the City of Cold Lake. Intersections with 10 collisions or more within the 5-year timeframe were identified as high collision locations. The following intersections were identified as high collision locations:

- 1. Highway 28 and 54 Avenue
- 2. Highway 28 and Tri City Mall
- 3. Highway 28 and 50 Avenue
- 4. 55/55A Street and 54 Avenue
- 5. 50 Street and 50 Avenue
- 6. 50 Street and 46 Avenue
- 7. Highway 28 and 50 Street
- 8. 51 Street and 50 Avenue
- 9. 50 Street and 43 Avenue
- 10. 52 Street and 50 Avenue
- 40 Highway 00 and 40 Avany

49 Street and 51 Avenue

- 12. Highway 28 and 43 Avenue
- 13. Highway 28 and 55 Avenue

11.

At the intersections listed above, the following collision distribution patterns were analyzed:

- Temporal collision distributions By year, by month, by day, and by hour
- Type and cause distributions By type, by cause, and by severity
- Environmental distributions By weather conditions, by road surface condition, and by light condition.

The detailed collision analysis at each location is provided in Appendix B of the Collision History Review and Analysis technical memorandum. Potential safety concerns at each high collision location were identified, where possible, from the patterns identified from the collision analysis. In general, the potential safety issues identified in this section are non-conclusive since the collision data did not provide enough detail, with regards to travel direction and other factors, to identify the collision causes and the probable solutions.

The high collision locations identified in the Collision History and Analysis technical memorandum need further detailed analysis to identify the exact cause of the collisions and develop probable engineering solutions. AE recommends that the City conduct in-service safety assessments at each of the high collision locations to understand the underlying causes for the collisions. As part of the in-service safety assessments, the City should obtain the full collision reports from AT to obtain a better understanding of the collision events and to facilitate the identification of the safety issues.

The Chrysler intersection, located at Highway 28 and 50 Street, was identified by the City and by the collision analysis as a high collision location. Through discussions with the City, three options were considered to improve the operation of the Chrysler intersection: provide a roundabout at the service road intersection, provide a right-in-right-out at the service road intersection, or provide a cul-de-sac at the service road intersection. The roundabout option is not feasible due to the close proximity to Highway 28 and the limited right-of-way at the intersection. AE recommends that the City consider providing a right-in-right-out or a cul-de-sac at the service road intersection. Both the right-in-right-out and cul-de-sac would eliminate some, if not all, turning movements to and from the service road, thereby removing some conflict points and reducing driver confusion. A detailed traffic analysis should be completed at the study intersection, and the adjacent intersections, to determine the traffic impact of the right-in-right-out or cul-de-sac.

Seven of the thirteen high collision locations occurred at intersections with Highway 28 or the service roads (55/55A Street or 50 Street) that run parallel to the highway. The separation distance that are provided between Highway 28 and the service roads are typically within 30 m to 65 m. Multiple intersections located within a short distance generate more conflict points and significantly increases the driver workload; thus, resulting in a higher frequency of collisions. Similar to the Chrysler intersection, the City should consider closing or providing right-in-right-out, or cul-de-sacs, at the service road and complete a traffic analysis to determine which service road intersections to close and the impact of the road closure on the surrounding road network. Intersection closures typically shift traffic to the adjacent intersections. Traffic analysis is required to determine if any improvements would be required on the adjacent intersections to accommodate the additional traffic volumes.



#### 2.3 TRAFFIC VOLUME FORECASTING AND ANALYSIS

Task 1B.3 (Traffic Volume Forecasting) and Task 2A.1 (Capacity Analysis for Planning Horizons) were completed together and documented in the technical memorandum titled Traffic Volume Forecast and Analysis, included in **Appendix C**.

Future traffic volumes were forecasted for the next 20 years (in the 5-year, 10-year, 15-year, and 20-year planning horizons) and analyzed to determine roadway classification and number of lanes required to accommodate the future traffic volumes.

A spreadsheet model, following a four-step planning process, was used to forecast the future traffic volumes in the City. To complete the spreadsheet model, a skeletal road network was developed for each planning horizon to represent the anticipated road network. The major road network (collectors and arterials) identified for each planning horizon in the 2000 transportation study were used to represent the skeletal road network for the respective planning horizon, with modification to reflect current roadway conditions.

Future traffic within the City will be comprised of background traffic and development traffic. Background traffic represents the existing traffic expanded to reflect future growth in the surrounding areas and in the City's existing subdivisions. Background traffic was generated by applying an annual non-compounded growth of 2% to the existing (2010) traffic volumes. Development traffic represents traffic generated by new subdivisions or area redevelopment. The information about future development or redevelopment within the City was obtained from the City's Area Structure Plans (ASP), Area Redevelopment Plans (ARP) and Outline Plans and from the Municipal District (MD) of Bonnyville's Intermunicipal Department Plan (IDP). To generate the development traffic, a four-step process was used:

- Trip Generation: Estimate the number of trips generated from and attracted to each development/redevelopment
- Trip Distribution: Estimate the origin and destination of trips to and from each development/redevelopment
- Modal Split: Not within the scope of the study
- **Trip Assignment**: Select the routes to and from the developments/redevelopments and assign the development traffic volumes to the City's road network.

A simplified gravity model was used to establish the trip distribution within the City. The results from the gravity model were revised, with discussions with the City, to reflect local travel patterns.

The development trips were assigned onto the future road network with consideration for the logical routes, on the basis of convenience and travel time, which would be taken by commuters between the origin and destinations. To capture worst-case traffic scenarios, the development trips were primarily assigned to the skeletal road network established for the planning horizons.

Figure 5.1 through Figure 5.4, in Traffic Volume Forecast and Analysis technical memorandum, presents the forecasted daily traffic volumes for the 5-year, 10-year, 15-year, and 20-year planning horizons respectively.

The forecasted total traffic volumes for each planning horizon were compared with the City's daily service volumes to determine the required roadway classification. The lane volumes were also compared with the lane capacity for the given road classification, to determine the number of lanes required along each roadway. The results of the analysis are summarized in Appendix D for each planning horizon, in Traffic Volume Forecast and Analysis technical memorandum.

The 20-year (2030) road classification and number of lanes will be used by the City to determine the right-of-way that should be retained to accommodate future expansion of the road network. The major corridors in the 20-year road network were reviewed independently to establish consistent road classification and numbers of lanes along the corridor, where possible. The recommended road classification and number of lanes is presented in Figure 6.1, in Traffic Volume Forecast and Analysis technical memorandum.

Table 6.4 in the Traffic Volume Forecast and Analysis technical memorandum summarizes the major road network in the 20-year planning horizon, along with a comparison of the existing and future road classification and number of lanes. The table also summarizes the improvements required to upgrade the corridors from the existing horizon to the 20-year planning horizon.

#### 2.4 COLD LAKE NORTH PARKING STUDY

Task 2A.4 (Parking Management) was completed as two components; separate parking studies were completed for Cold Lake North (CLN) and Cold Lake South (CLS). This section presents the major findings from the Cold Lake North Parking Study, which is documented in the technical memorandum titled Cold Lake North - Parking Study, included in **Appendix D**.

From the existing parking conditions analysis, the following conclusions can be made:

- Overall, the existing on-street parking supply is able to accommodate the on-street parking demand.
- The existing off-street parking supply provided in the Marina Lot is unable to accommodate the
  off-street parking demand.
- The existing off-street parking supply provided in the 1 Avenue Lot is able to accommodate the off-street parking demand.
- The existing off-street parking supply provided in the Gravel Lot is able to accommodate the off-street parking demand but approaches capacity on July 1.
- For both on-street and off-street parking, the parking demand observed on July 1 (Canada Day)
   was higher than the parking demand on July 2 and 3, which represents a typical weekend period.
- The difference in parking demand between Canada Day and a typical weekend period was most noticeable in the areas surrounding the 1 Avenue corridor and Kinosoo Beach.



- Parking in "no parking" zones was observed at various locations along Lakeshore Drive and
   1 Avenue.
- Illegal parking in the off-street lot was observed in the Marina lot.

The following parking strategies were developed to improve the existing parking condition in Cold Lake North:

- Provide summer overflow parking for the Marina Lot
- Provide marked (painted) on-street parking stalls
- Enhance current parking zones by delineating with concrete bulbs or pavement markings
- Enforce "no-parking" zones
- Pave and paint stalls in the Gravel Lot.

Opportunities exist to integrate streetscaping and landscaping in the Gravel Lot to coordinate with the beautification efforts for the Lakeshore Commercial and beachfront areas. Landscaped islands with shrubs and trees can be provided between and at the end of aisles to improve the aesthetics of the parking lot. Additionally good illumination and a designated pathway between the parking lot and Kinosoo Beach could increase the utilization of the gravel lot.

The City indicated that complaints have been received regarding illegal parking at various locations throughout Cold Lake. AE recommends that the City monitor the parking conditions throughout the City and determine the effect on traffic operations. Areas which have been specifically identified to have illegal parking issues include: 16 Street, roadways adjacent to schools and playgrounds, and snow removal routes.

#### 2.5 COLD LAKE SOUTH PARKING STUDY

This section presents the major findings from the Cold Lake South Parking Study, which is documented in the technical memorandum titled Cold Lake South - Parking Study, included in **Appendix E**.

The City advised that the parking condition and parking policies within Cold Lake South has not significantly changed since the previous parking study conducted in 1985. For this reason, the current parking demand in the Downtown is expected to be similar to or less than the parking demand observed in 1985.

AE reviewed the 1985 Parking Study and conducted a verification study to determine if current parking utilization rates at select locations are similar to those observed in the 1985 parking study. A parking survey was conducted on October 20, 2010 to obtain current parking utilization rates at select locations.

The verification study indicated that the parking utilization has not significantly changed in the Downtown. Parking utilization along 51 Avenue and 52 Street (south of 50 Avenue) has decreased since 1985 while the parking utilization along 50 Avenue and 52 Street (north of 50 Avenue) has increased since 1985. The annual growth observed along 50 Avenue and 52 Street (north of 50 Avenue) is less than 2%. Even if the

parking demand continues to grow at an annual growth rate of 2%, the current parking supply should be able to accommodate the parking demand for the next 30 years.

The City has experienced a trend of business relocation from the Downtown to the commercial area along Highway 28, south of 43 Avenue, in recent years. The trend is expected to continue as the commercial area develops and continues to draw more businesses. With the relocation of businesses outside Downtown, an annual parking growth rate of 2% for the Downtown may not be achieved. The actual growth in parking demand will be dependent on the future land use changes within the Downtown.

The City should monitor the land uses and parking conditions periodically within the Downtown and consider a detailed parking study if there is significant land use changes that would attract more trips into Downtown Cold Lake South.

#### 2.6 IN-SERVICE SAFETY REVIEWS AND TRAFFIC CALMING

Task 2A.3 (Traffic Calming) and Task 2A.5 (In-Service Safety Reviews) were completed together and documented in the technical memorandum titled In-Service Road Safety Reviews, included in **Appendix F**.

In-service safety reviews were completed for the following four corridors:

- 1 Avenue, from the MD Campground (23 Street) to 2 Avenue/10 Street
- 10 Street, from 2 Avenue to 8 Avenue
- Lakeshore Drive, from 1 Avenue/10 Street to 8 Avenue
- 50 Avenue, from Highway 28 to 49 Street.

The purpose of the in-service road safety reviews was to identify potential safety issues along the corridors and propose improvement options that will reduce/eliminate the safety issues. The in-service road safety reviews were conducted based on the procedures outlined in the Transportation Association of Canada (TAC) Canadian Guide to In-Service Road Safety Reviews (TAC Safety Guideline). Potential safety issues were identified based on observations during the site reconnaissance and the results from the operational analysis for the Existing (2010) Traffic Operational Analysis.

No operational issues were identified as far as traffic flow and intersection capacity is concerned.

Table 2.1 summarizes the safety issues identified for each corridor and the improvement options developed to address each of the potential safety issues.



Table 2.1 Summary of Safety Issues and Improvement Options

Study Corridor	Safety Issues	Improvement Options			
	1 Avenue/2 Avenue/10 Street intersection configuration	Improve intersection configuration at 1 Avenue/2 Avenue/10 Street intersection. Roundabout option should be considered but requires further conceptual design.			
1 Avenue	Speeding problem	Conduct speed study to confirm speeding problem. Provide traffic calming measures.			
	Poor pavement conditions	Repave corridor.			
	19 Street pedestrian crosswalk	Provide pavement marking and signage at 19 Street crosswalk.			
40.00	Vertical crest curve at 3 Avenue	Provide signage.			
10 Street	Poor pavement conditions	Repave corridor.			
Lakeshore Drive	Road width and alignment	Change lane configuration along corridor. Detailed traffic analysis is required to determine the impact of lane changes prior to implementation.			
	Poor pavement conditions	Repave corridor.			
	Pedestrian crosswalks	Improve pedestrian crosswalks.			

Study Corridor	Safety Issues	Improvement Options			
	Highway 29 interportion	Complete detailed intersection analysis to review intersection geometry and lane configuration.			
	Highway 28 intersection	Close 55 Street intersection.			
		Conduct Main Street Analysis.			
50 Avenue	Angle parking	Provide back-in angle or parallel parking stalls.			
	Faded pavement markings	Repaint pavement markings.			
	Multiple driveway accesses  If 50 Avenue remains an arroadway, close unnecessar accesses.				
	Pedestrian crosswalks	Provide curb extensions and signage at crosswalks.			

Figure 7.1 and Figure 7.2, in the In-Service Road Safety Reviews technical memorandum; summarize the recommendations for the study corridors in Cold Lake North and Cold Lake South respectively.

The Lakeshore Redevelopment Plan (LRP) was finalized in March 2010 and provided a strategic direction for the revitalization of the Lakeshore Commercial District to a vibrant "urban village" that would attract residents and tourists. The LRP identified the need to improve the aesthetics of the Lakeshore Commercial District, given its beautiful setting and prominent location at the end of Highway 28. The opportunity exists to expand the scope of the LRP to include the 1 Avenue/beachfront area, to take advantage of the attractiveness of the area in the summer months.

1 Avenue and Lakeshore Drive were analyzed as part of the Cold Lake North Parking Study. The strategies presented by the parking study for these two corridors are presented in Section 2.4 above. The improvements in Table 2.1 for the 1 Avenue and Lakeshore Drive corridors should be coordinated with the visions presented in the LRP and the improvements recommended from the parking study. Integration of the recommendations from the various studies will provide for cost effectiveness and a unified vision for the revitalization/beautification of the area.



#### 2.7 COORDINATION OF TRAFFIC SIGNALS

The City currently has traffic signals at twelve intersections, with six more recommended for the future. They are located on three of the City's busiest streets, along the north-south corridor of Highway 28, the east-west corridor of Kingsway, and on Centre Avenue.

Existing traffic signals operate as separate independent units and are neither coordinated nor hard-wired interconnected for efficiency and synchronization. Although less reliable than cable connection, if necessary, existing controllers can be coordinated through their built-in TBC (time base coordination) function and by using radio transmission devices.

If deemed desirable, traffic signals can be programmed for pre-emption by emergency vehicles so that the latter will receive priority right-of-way with a through green upon activation. This will require retrofitting of hardware and software to existing signal equipment and adding onto existing timing plans an emergency procedure. Emergency vehicles will be equipped with a transponder which throws a beam onto a receiver mounted on the signal pole arm. The receiver is in turn wired into the signal controller. Commercial vendors are available to retrofit existing equipment if authorized.

For a smoother progression of traffic stream along Highway 28 for future operations when traffic volume warrants, traffic signal coordination should be considered. With some exceptions, signal spacing between the majority of intersections along the highway corridor is mostly less than one kilometer, and are ideally suited for synchronization.

We recommend that traffic signals on Highway 28 should be considered for coordination in the future. Due to the relative low traffic volumes along the highway corridor (less than 800 trips in each direction during the p.m. peak hour), the need for immediate action is not necessary. Traffic congestion and travel speeds are good indicators of whether system operation would be beneficial and this should be monitored by Cold Lake as the City grows. To prepare for a possible future progression system, we would recommend that the City consider the purchase of any new or replacement of old signal equipment, in particular traffic signal controllers, with modern compatible NEMA or 170 types.

#### 2.8 HIGHWAY 28 FUNCTIONAL REVIEW

Task 2A.7 (Highway 28 Functional Review) was completed and documented in the technical memorandum titled Highway 28 Functional Review, included in **Appendix G**.

Associated Engineering (AE) was retained by the City of Cold Lake (City) to undertake a review of the Highway 28 Functional Plan, from 52 Avenue to the south city limits. The purpose for the review was to provide the City with recommendations on intersection requirements and improvements along the Highway 28 corridor. After an in-depth discussion with the City, the scope of the Highway 28 Functional Review was revised to include the following:

- Select and analyze the traffic operations at a representative intersection under the 20-year planning horizon to determine the required traffic control and intersection configuration
- Develop a standard intersection template with designated turn lanes, storage lengths, and channelization, if required.

The intersection of Highway 28 and 43 Avenue currently experiences high traffic volumes and is expected to experience higher traffic volumes in the 20-year planning horizon; therefore, it was selected as the representative intersection. The afternoon (p.m.) traffic volumes, from the 20-year planning horizon, were analyzed to determine the required traffic control and intersection configuration. 20-year traffic volumes were forecasted as part of the future traffic volume forecasts.

Figure 4.1 in the Highway 28 Functional Review technical memorandum presents the intersection configuration required to accommodate the future traffic volumes anticipated at Highway 28 and 43 Avenue.

As per the City's requirements, a future standard intersection template was developed based on the capacity analysis results for Highway 28 and 43 Avenue. The City's design standards as outlined in the Municipal Engineering Servicing Standards and Standard Construction Specifications (2008) and the Transportation Association of Canada's Geometric Design Guide for Canadian Roads were referenced in the development of the template presented in Figure 5.1 in the Highway 28 Functional Review technical memorandum.

The intersection template was designed to accommodate the traffic volumes assumed for Highway 28 and 43 Avenue. For application at other intersection locations, the City should complete an intersection operational analysis to determine the designated turn lanes and storage lengths required, as well as geometric design to determine the actual land requirements. The template should be modified to reflect the requirements of the specific intersection. Figure 6.1 in the technical memorandum presents the 20-year road network classification along Highway 28, between 50 Avenue and 34 Avenue. The figure also highlights the potential locations for application of the intersection template and the intersection improvements required at 52 Street under the existing (2010) conditions.

#### 2.9 TRUCK AND DANGEROUS GOODS ROUTE REVIEW

Due to the low volumes of truck traffic hauling dangerous goods within or passing through the City of Cold Lake to date, the City has not yet developed an official roadway network and there are no defined or designated truck routes for this purpose. There are trucks which carry jet fuel on a regular basis originating from the Edmonton area and destined to the airfield in Medley (CFB 4 Wing), west of Cold Lake. Beyond that, other dangerous goods truck traffic is insignificant.

Delivery of jet fuels to Medley currently uses Highway 28 and Highway 55, through a "front entrance" and a "back entrance". Both of these two routes present issues that need attention. The "front entrance" option passes several schools en-route. It also goes through a few residential neighbourhoods. The "back entrance" option via Highway 55 has steep vertical curves that make it difficult and dangerous for truck traffic. There are three possible solutions to address the problem:



- 1. Build the dangerous goods route for jet fuel delivery as shown on the maps in the Municipal Development Plan. Improvements to the roadway need to be made and details have to be worked out. This option has a low probability of acceptance as residential sub-divisions have already been proposed for the area west of Highway 28 where the dangerous goods route is shown.
- 2. Allow the fuel trucks to continue to use the front entrance along Kingsway. This option requires the review of the intersection at Highway 28/50 Avenue, with particular attention paid to trucks turning left from Highway 28 northbound to 50 Avenue/Kingsway westbound. Mitigation measures may include geometric improvements and/or signalization.
- 3. Re-establish a rail line to Cold Lake. This will allow the delivery of jet fuel via rail cars. The rail line could also be used for the delivery of solid waste from the City to designations outside of the City (e.g., to Edmonton or Riley). As well, a rail line could possibly be used for tourism promotion through the use of steam engine locomotives for excursion trips in and around Cold Lake. This option needs to be carefully studied possibly requiring the development of a business plan to determine the concept's feasibility.

The absence of an acceptable truck route for the transport of dangerous goods within the City is unsatisfactory. It is recommended that a detailed study be conducted to examine the above and any other options for the safe delivery of jet fuel in the immediate near term.

Moving forward, and planning into the future, it will be necessary for the City to develop technical guidelines so that a rational and comprehensive dangerous goods route system can be established, eventually leading to the enactment of bylaws for enforcement. The law will be used to designate where, when and how hazardous materials may or should be transported on existing and future roads of the City. The resultant bylaw will also help to streamline the process, provide risk management, promulgate policies, and regulate/control the safe and efficient movement of dangerous goods within City limits.

It is not the purpose of this study to develop a comprehensive citywide dangerous goods truck route system. We recommend that the City of Cold Lake monitor the movement of dangerous goods (in volume and manner) and at the appropriate time (threshold as a function of activity level) initiate a full study process. A suggested framework for a detailed work plan is given in the Transportation Best Practices technical memorandum.

#### 2.10 SCHOOL ZONE SAFETY ANALYSIS

Task 2A.9 (School Zone Safety Analysis) was completed and documented in the technical memorandum titled School Zone Safety Analysis, included in **Appendix H**.

The following tasks were completed as part of the School Zone Safety Analysis:

- Verify the existing school and playground areas and zones
- Observe existing operations at a representative school site for safety

 Determine the need to establish school/playground zone policy and guideline, and recommend the principles and best practices for establishing the guideline for the City.

Roadways adjacent to existing schools within Cold Lake were analyzed to verify the current school/playground areas and zones. Worksheets provided in TAC's School and Playground Areas and Zones: Guidelines for Application and Implementation were used for the verification and the results are summarized in Table 3.1 and Table 3.2, in the School Zone Safety Analysis technical memorandum. It should be noted that the worksheets from TAC are identical to the worksheets published in AT's Guidelines for School and Playground Zones and Areas, which is currently used by the City.

The existing signage and pavement markings at the school sites should be compared against the TAC or AT signage and pavement marking plans to ensure compliance with the standards.

Grand Centre Middle School was selected as the representative school site to observe the morning drop-off and afternoon pick-up periods. The school site was observed to identify any safety issues related to traffic operations and pedestrian movement. Overall, the morning and afternoon discharge periods operated well. AE did not observe anything that was considered unsafe.

In the 2010/2011 school year, Grand Centre Middle School was relocated south of Centre Avenue, to the building previously occupied by Grand Centre High School. The old location for Grand Centre Middle School, on 56 Street, currently is vacant and there are no immediate plans for redevelopment of the building. With the relocation of Grand Centre Middle School, student traffic across Highway 28 will shift from the pedestrian activated crosswalk at 51 Avenue to the intersection of Highway 28 and 50 Avenue. The pedestrian crosswalks at this intersection should be maintained to ensure maximum visibility.

Grand Centre Elementary School is still located on the previously shared lot with Grand Centre Middle School. To improve safety for the elementary school, AE recommends that the crosswalk currently provided at the horizontal curve transition between 56 Street and 51 Avenue be realigned to provide a north-south crosswalk across 51 Avenue. Signage should be provided on the west side of 51 Avenue to warn southbound drivers of the crosswalk, if it is relocated.

The City should consider the removal of the existing school zones at following locations:

- Highway 28, from 52 Avenue to 46 Avenue
- Service road parallel and west of Highway 28, from 52 Avenue to 50 Avenue
- Centre Avenue, from 57A Street to service road west of Highway 28.

There are no schools located on these roadways and unwarranted school zones can lead to driver frustration and non-compliance. Prior to the removal of these school zones, the City should undertake a speed study to determine the level of driver compliance within these school zones and review the collision history. If current driver compliance level is low, the City should remove the school zones immediately. If current driver compliance level is high, the City can delay the removal of the school zones. On the other hand, if the collision history indicates the presence of pedestrian related collisions on Highway 28 and



Centre Avenue/50 Street, the City may choose to maintain the 30 km/h zones. If the removal of the above school zones is not feasible, AE recommends re-designating the school zone to a school area. With a school area, motorists are warned to be cautious of the nearby school and the associated student traffic, but are not required to reduce their travel speeds.

AE recommends that the City continue to follow the policies outlined in the AT Guideline. An established guideline would help to promote uniformity in the establishment and signing and marking of schools and playground areas and zones within the City. It should be noted that the methodology established in the AT Guideline is similar to the methodology established in the TAC Guideline.

#### 2.11 TRANSIT SERVICES

Given the size of the population, it is inconceivable that there is sufficient demand for a fully integrated bus network service to cover the entire City in the near future. Currently there are no formal or regular schedule public transit services in the City. There is a special-need bus service for para-transit that is available to senior citizens, people with temporary or long term disabilities, students and those with special needs. This service is provided on a request basis with advance booking necessary. Other minor community resources offering van and bus rides are run by various community agencies and support groups, but they are limited in nature. Taxi services are available on a commercial basis and the school board provides school bus services for those students who qualify.

Although a full scale public bus system within Cold Lake is probably not warranted now or anytime in the near future, given its green and sustainable transportation objective, the City may consider developing a limited service system focusing on specific routes or loops which may pose as viable options. At a conceptual level several routes stand out as potential candidates for bus services. These include:

- JJ Parr at 4 Wing
- Travel along Kingsway to Highway 28
- Highway 28 south to the Walmart shopping area
- Highway 28 north with stops at 50 Avenue, the Tri City Mall, the Energy Centre, the Senior's Centre/North Library, and the Marina
- Travel west along Lakeshore/1 Avenue with stops at Kinosoo Beach and the MD Campground
- 25 Street with a stop at the General Hospital
- Travel along 25 Street to access Highway 28 south, Kingsway west and return to JJ Parr.

To cover the above areas, as a start, the City may consider operating two buses along these routes in the opposite direction. Funding by the provincial government through its "GreenTrip" program should be explored.

#### 2.12 SUSTAINABLE TRANSPORTATION

Achieving a green and sustainable transportation system is an important goal of the City of Cold Lake. The City is committed to consider seriously alternate modes of transportation such as pedestrian and bicycles. Given the population size, establishing a full bus transit system in Cold Lake is not feasible in the near future. The development of a transit system and promoting its use is therefore considered as a long term transportation objective.

The areas of immediate interest to the City for now are as follow:

- Developing and linking a network of paths and trail systems for pedestrians and cyclists between parks and other points of interest with existing residential subdivisions
- Exploring a sports vehicle transportation network. Quads in the summer time and snowmobiles in winter are very popular in Cold Lake. The need to establish a policy to define hiking and cycling trails and to establish how these can be easily accessed is considered a priority.

To reach the City's goals of developing a citywide trail system, we will recommend that a technical review committee (TRC) consisting of a city planner, a landscape architect, and an engineer be set up to finalize details. To be useful, these trails have to be conveniently connected to and easily accessible from residential homes. Accessibility to these trails could be promoted by planting at strategic locations within the City parking lots to allow users to leave their cars at safe locations and staging points to continue their journey on foot, by bicycle, or by any other non-motorized and non-traditional form of travel. Connectivity to the planned trails will also consider the possible use of back alleys as driveways for this purpose.

The goal to create a sustainable transportation system will mean a significant change in land use, partially converting from what is currently a predominantly vehicle-based society to a more pedestrian and bike friendly environment throughout the City. The result will be the generation of different traffic volumes (possibly different from what is predicted by our travel demand model), and a change in travel patterns. Under this scenario, as well as the inevitable growth of background vehicle traffic, a high volume of pedestrian and bicycle activities performing conflicting movements will occur in the more urbanized communities. Neighbourhoods in this category, especially in the downtown areas and the densely developed retail areas, will need to have a traffic management plan to allow for an orderly flow of mixed-use traffic, which must address the issues of commuter drop-off and pick-up, the free and safe flow of pedestrians and bicycles, bus stops (in the future), area businesses, downtown merchants and retail facilities. Such a plan will also call for the adequate provision of traffic mitigation measures such as proper signage, rest benches and chairs for weary travelers, shelters for commuters, on-street parking, creation of a liveable community, and the design of a roadway system that exemplifies a serene and "traffic calmed" environment.

Our suggested approach to the development of a sustainable transportation framework is to create traffic solution alternatives using a context sensitive design philosophy that engages a collaborative interdisciplinary effort, and that involves all stakeholders. Design solutions should be developed as a holistic transportation facility that fits its physical settings, and preserves the aesthetic, historic, cultural and



environmental resources, while maintaining safety and mobility, and that satisfies the development needs, goals and objectives of the City of Cold Lake. Sustainable transportation solutions and management plans should be designed as place-making solutions, using transportation means as a "catalytic process" to mobilize community partnerships, turning the place around and maintaining its vibrancy, while encouraging "non-traditional", yet beneficial activities, and in general, creating better communities through enhanced planning of its roadway network.

On sustainable development and transportation planning, coordination with the public is an essential component of a traffic management plan. A workable public involvement plan satisfying needs of the City and which is acceptable to the majority of communities and stakeholders is crucial. Opinion surveys, if appropriate, will be carried out.

#### 2.13 COST ESTIMATES

The recommendations from the various project tasks were compiled to develop a cost estimate for the future roadway improvements. The cost estimate will include costs associated with the roadway upgrades required to accommodate traffic in the 20-year planning horizon, traffic signal installation, pavement markings to replace existing worn markings and delineate on-street parking stalls, Gravel Parking Lot (Kinosoo Beach) paving and painting, signage, re-paving sections of roadway (1 Street, 10 Street and Lakeshore Drive), and various pedestrian crosswalk improvements.

The City's Municipal Engineering Servicing Standards and Standard Construction Specifications (MESS, 2008) was reviewed to establish the pavement design for the roadway upgrades in the 20-year planning horizon. Unit rates were provided by the City of Cold Lake. Where City rates were not provided, 2010 weighted unit price averages were obtained from Alberta Transportation for the North Central Region and used to develop unit price rates for the recommended improvements. The unit price rates developed are summarized and also presented in Appendix I.

The conceptual cost estimates for the recommended improvements in the City of Cold Lake, for the 20-year planning horizon, are presented in Table 2.2.

Cold Lake Transportation Study Project No: 2010-3050 Date: April 14, 2011

Table 2.2: Conceptual Cost Estimates
NOTE:
- Prices do not include cost of land acquisition and utility relocation
- Prices reflect 2010 values and do not include GST

Location				Recommendation from	ltem .		Unit	Unit Rate	Total Cost
Corridor	From	To	Intersection	Recommendation from	Kelli	Qualitity	Oilit	Ollit Kate	Total Cost
8 Avenue	10 Street	Lakeshore Drive	-	Traffic Volume Forecast and Analysis	Widen to provide centre median and one additional travel lane in each direction (4-lane Divided Arterial with curb and gutter)	170	LM	\$2,131.63	\$362,377
							Mo	obilization Fee	\$10,000
								nts - Subtotal	
10% Contingency and 12% Engineering/Testing									
8 Avenue Corridor Improvements - Total									6454 200

	Location			Recommendation from	ltem .		Unit	Unit Rate	Total Cost
Corridor	From	To	Intersection	Recommendation from	Recommendation from Rem		Oilit	Ollit Kate	Total Cost
Highway 28/55			75 Avenue	Existing (2010) Traffic Operational Analysis	Provide traffic signal	1	LS	\$350,000.00	\$350,000
Highway 28/55				Existing (2010) Traffic Operational Analysis		1	LS	\$350,000.00	\$350,000
Highway 28/55			61/62 Avenue	Existing (2010) Traffic Operational Analysis	Channelize northbound right turn lane	1	LS	\$116,960.56	\$116,961
Highway 28/55			52 Avenue	Existing (2010) Traffic Operational Analysis	Provide traffic signal	1	LS	\$350,000.00	\$350,000
Highway 28/55			52 Avenue	Existing (2010) Traffic Operational Analysis	Channelize northbound right turn lane	1	LS	\$116,960.56	\$116,961
Highway 28/55			52 Street	Existing (2010) Traffic Operational Analysis	Provide traffic signal	1	LS	\$350,000.00	\$350,000
Highway 28/55			52 Street	Existing (2010) Traffic Operational Analysis	Channelize northbound right turn lane	1	LS	\$85,761.64	\$85,762
Highway 28/55	53 Avenue	52 Avenue		Traffic Volume Forecast and Analysis	Widen to provide centre median (4-lane Divided Expressway with curb and gutter)	230	LM	\$1,258.04	\$289,350
Highway 28/55	52 Street	47 Avenue		Traffic Volume Forecast and Analysis	Widen to provide centre median (4-lane Divided Arterial with curb and gutter)	430	LM	\$1,258.04	\$540,958
								lobilization Fee	\$10,000
						Highway 28/55 Corridor Im			\$2,559,991
						10% Contingency and 12% Engineering/Testing			\$563,198
						Highway 28/55 Corridor Improvements - Total			\$3,123,189

	Location			Recommendation from	Item		Unit	Unit Unit Rate	Total Cost
Corridor	From	To	Intersection	Recommendation from	Kelli	Quantity	Oiiii	Ollit Kate	I Olai Cost
1 Avenue	25 Street	10 Street	-		Provide marked (painted) on-street parallel parking stalls	240	stalls	\$4.80	\$1,153
1 Avenue	25 Street	10 Street			Pave and paint parking stalls in Gravel Lot at Kinosoo Beach. No beautification.	1	LS	\$205,273.50	\$205,273
1 Avenue	25 Street	10 Street	-	Cold Lake North Parking Study	Install parking control (RB-51, RB-52) signs	1	LS	\$988.08	\$988
1 Avenue	25 Street	10 Street	2 Avenue/10 Street	In-Service Road Safety Reviews	Improve intersection configuration at 1 Avenue/2 Avenue/10 Street. Provide roundabout after further conceptual design. 1	1	LS	\$250,000.00	\$250,000
1 Avenue	25 Street	10 Street	-	In-Service Road Safety Reviews	Repave corridor	1,220	LM	\$826.61	\$1,008,464
1 Avenue	25 Street	10 Street	19 Street	In-Service Road Safety Reviews	Provide pavement marking and signage at 19 Street crosswalk	1	LS	\$733.82	\$734
								obilization Fee	
								ents - Subtotal	\$1,476,612
10% Contingency and 12% Engineering/Testing									

Location				Recommendation from	ltem		Unit	Unit Rate	Total Cost
Corridor	From	To	Intersection	Trecommendation nom	NCIII	Quantity	0	Oint itute	10101 0001
Highway 55	28 Street	Highway 28	-	Traffic Volume Forecast and Analysis	Build pavement structure to Arterial standard (4-lane Divided Arterial with curb and gutter)	555	LM	\$2,990.14	\$1,659,525
		Mobilization Fee					\$10,000		
				Highway 55 Corridor Improvements - Subtotal \$1				\$1,669,525	
	10% Contingency and 12% Engineering/Testing						\$417,381		
Highway 55 Corridor Improvements - Total							\$2,086,906		

Location				Recommendation from	Item		Unit	Unit Rate	Total Cost
Corridor	From	To	Intersection	Recommendation from	Kelli	Quantity	Oilit	Ollit Kate	i otal cost
16 Avenue	Highway 28	16 Street	-	Traffic Volume Forecast and Analysis	Build pavement structure to Arterial standard (4-lane Undivided Arterial with curb and gutter)	1,430	LM	\$2,214.01	\$3,166,037
16 Avenue	16 Street	8 Street		Traffic Volume Forecast and Analysis	Widen to provide one additional travel lane in each direction (4-lane Collector with curb and gutter)	825	LM	\$1,201.39	\$991,146
								obilization Fee	\$10,000
	16 Avenue Corridor Improvements - Subtotal								\$4,167,183
	10% Contingency and 12% Engineering/Testi							\$916,780	
16 Avanua Carridar Improvemen									

Location				Recommendation from	ltem		Quantity	Unit	Unit Rate	Total Cost
Corridor	From	To	Intersection	Recommendation from	iteiii		Quantity	OIIIL	Ollit Kate	i otal cost
English Bay Road	Lake Avenue	Highway 28		Traffic Volume Forecast and Analysis	Build pavement structure to Arterial standard (4-lane Divided Arterial with curb and gutter)		2,345	LM	\$2,998.64	\$7,031,800
				*	•	Mobilization Fee				\$10,000
						English Bay C				\$7,041,800
	10% Contingency and 12% Engineering/Testing								\$1,549,196	
English Bay Corridor Improvements - Total							\$8,590,996			

	Local	tion		Recommendation from	Item	Quantity	Unit	Unit Rate	Total Cost
Corridor	From	To	Intersection	The Commendation Iron	NCIII	additity	0	Omit itute	10101 0001
28 Street	Lake Avenue	Highway 28	-	Traffic Volume Forecast and Analysis	Remove existing 28 Street roadway	1,165	LM	\$379.52	\$442,146
28 Street	Lake Avenue	Highway 28		Traffic Volume Forecast and Analysis	Relocate stop signs	2	signs	\$331.88	\$664
28 Street	Lake Avenue	Highway 28		Traffic Volume Forecast and Analysis	Realign 28 Street and build pavement structure to Arterial standard (2-lane Undivided Arterial with curb and gutter)	1,075	LM	\$1,094.18	\$1,176,239
								obilization Fee	\$10,000
								nts - Subtotal	\$1,629,049
					10% Continger	cy and 12%	Engine	ering/Testing	\$358,391
					28 Stre	at Corridor I	mnrove	ments - Total	\$1 007 440 22

	Loca	ation		Recommendation from	Item	Quantity	Unit	Unit Rate	Total Cost
Corridor	From	To	Intersection	Recommendation from			Oilit	Ollit Kate	I Olai Cost
16 Street	16 Avenue	75 Avenue		Traffic Volume Forecast and Analysis	Build pavement structure to Arterial standard (2-lane Undivided Arterial)	1,625	LM	\$1,295.08	\$2,104,507
Future Arterial	75 Avenue	50 Avenue		Traffic Volume Forecast and Analysis	Build out as per 20-year horizon (2-lane Undivided Arterial)	3,300	LM	\$1,052.58	\$3,473,518
								obilization Fee	\$10,000 \$5,588,025
						16 Street/Future Arterial Corridor Improvements - Subtota			
					10% Continger				
					16 Street/Future Arter	al Corridor II	mnrove	mente - Total	EC 047 300

	Loca	tion		Recommendation from	tem	Quantity	Unit	Unit Rate	Total Cost		
Corridor	From	To	Intersection	Recommendation from	item	Quantity	OIIIL	Ollit Kate	Total Cost		
10 Street	1 Avenue	8 Avenue	3 Avenue	In-Service Road Safety Reviews	Provide signage for vertical curve at 3 Avenue	1	LS	\$494.04	\$494		
10 Street	1 Avenue	8 Avenue		In-Service Road Safety Reviews	Repave corridor	850	LM	\$959.12	\$815,252 \$10,000		
	•				•	Mobilization Fe					
					10 Street Co	10 Street Corridor Improvements - Subtot					
					10% Contingency and 12% Engineering/Testing						

	Location			Recommendation from	Item		Quantity	Unit	Unit Rate	Total Cost
Corridor	From	To	Intersection	Trecommendation nom	No.		addining	0	Omit Hate	101010001
8 Street	16 Avenue	75 Avenue		Traffic Volume Forecast and Analysis	Build pavement structure to Collector standard (2-lane Collector with curb and gutter)		1,630	LM	\$1,208.12	\$1,969,240
						Mobilization Fee			\$10,000	
						8 Street Corridor Improvements - Subtota				
						10% Contingency and 12% Engineering/Testir			ering/Testing	\$435,433
						8 Street (	Corridor II	nnrove	ments - Total	\$2 414 673

	Location			Recommendation from	Item		Quantity	Unit	Unit Rate	Total Cost
Corridor	From	To	Intersection	Trecommendation from	No.		additity	0	Omit itute	10101 0001
20 Avenue	12 Street	8 Street		Traffic Volume Forecast and Analysis	Build as per 20-year horizon (2-lane Collector with curb and gutter)	550 LM \$965.62			\$965.62	\$531,092
						Mobilization Fee				\$10,000
						20 Avenue Corridor Improvements - Subtot				\$541,092
					10% Contingency and 12% Engineering/Testing				\$119,040	
						20 Avenue	Corridor I	mnrove	mente - Total	\$660 122

	Loca	tion		Recommendation from	Item	Quantity	Unit	Unit Rate	Total Cost	
Corridor	From	To	Intersection	Recommendation from	Refit	Quantity	Oilit	Oliit Kate	i otal cost	
Lakeshore Drive	10 Street	7 Street			Provide marked (painted) on-street parallel parking stalls	117	stalls	\$4.80	\$562	
Lakeshore Drive	10 Street	7 Street		Cold Lake North Parking Study	Install parking control (RB-51, RB-52) signs	1	LS	\$2,717.22	\$2,717	
Lakeshore Drive	10 Street	8 Avenue		In-Service Road Safety Reviews	Repave corridor (2-lane Local)	1,055	LM	\$781.09	\$824,046	
Lakeshore Drive	10 Street	8 Avenue		In-Service Road Safety Reviews	Improve pedestrian crosswalks (8 Avenue, midblock between 8 Avenue and 7 Avenue, 7 Avenue, 6 Avenue and 2 Avenue)	5	LS	\$9,985.34	\$49,927	
								obilization Fee	\$10,000 \$887,252	
					Lakeshore Drive Corridor Improvements - Subtotal					
					10% Contingency and 12% Engineering/Testing					

	Loca	ition		Recommendation from	Hom	Quantity	Unit	Unit Rate	Total Cost
Corridor	From	To	Intersection	Recommendation from	Item	Quantity	Oilit	Ollit Kate	I Otal Cost
75 Avenue	Highway 28	8 Street	-	Traffic Volume Forecast and Analysis	Build pavement structure to Collector standard (4-lane Collector with curb and gutter)	2,430 LM \$1,864.57			
	•			*	•	Mobilization Fee			
					75 Avenue	75 Avenue Corridor Improvements - Subtotal			
10% Contingency and 12% Engineering/Testing					\$999,001				
					75 Aug	ue Corridor I	mnrove	mente - Total	\$E E20 012

	Locat	ion		Recommendation from	ltem .	Quantity	Unit	Unit Rate	Total Cost
Corridor			Intersection	Trecommendation nom	No.	quantity	0	Omit itute	101010001
69 Avenue Arterial	Glenwood Drive	Highway 28	-	Traffic Volume Forecast and Analysis	Build out as per 20-year horizon (2-lane Undivided Arterial)	4,220	LM	\$1,052.58	\$4,441,893
	Mobilizati					bilization Fee	\$10,000 \$4,451,893		
						69 Avenue Corridor Improvements - Subtotal			
					10% Contingen				\$979,416
								monte Total	

Cold Lake Transportation Study Project No: 2010-3050 Date: April 14, 2011

- Table 2.2: Conceptual Cost Estimates
  NOTE:
   Prices do not include cost of land acquisition and utility relocation
   Prices reflect 2010 values and do not include GST

	Loca	ation		Recommendation from	Item		Quantity	Unit	Unit Rate	Total Cost
Corridor	From	To	Intersection	Trecommendation nom	i i i i i i i i i i i i i i i i i i i		quantity	0	Omi Hate	I TOTAL OUGS
54 Avenue	56 Street	49 Street		Traffic Volume Forecast and Analysis	Widen to provide one additional travel lane in each direction (4-lane Collector with curb and gutter)		1,320	LM	\$1,201.39	\$1,585,834
54 Avenue	49 Street	Future Arterial		Traffic Volume Forecast and Analysis	Build out as per 20-year horizon (4-lane Collector with curb and gutter)		1,230	LM	\$1,622.07	\$1,995,149
									obilization Fee	\$10,000
						54 Avenue Corridor Improvements - Subtotal				
						10% Contingen				
						54 Avenu	e Corridor I	mprove	ments - Total	\$4 380 999

	Location			Recommendation from	Item		Quantity	Unit	Unit Rate	Total Cost
Corridor	From	То	Intersection	Trecommendation from	nc.ii		quantity	0	Omit itute	1
52 Avenue	57 Street	Highway 28		Traffic Volume Forecast and Analysis	Widen to provide one additional travel lane in each direction (4-lane Collector with curb and gutter)		305	LM	\$1,161.39	\$354,224
						Mobilization Fee			\$10,000	
						52 Avenue Corridor Improvements - Subtotal				
						10% Contingenc				
										6444.050

	Loca	tion		Recommendation from	Item	Quantity	Unit	Unit Rate	Total Cost	
Corridor	From	To	Intersection	Recommendation from	Reili	Quantity	Oiiii	Ollit Kate	i otal cost	
51 Avenue	56 Street	Service Road	-	School Zone Safety Analysis	Relocate crosswalk	1 LS \$773.18				
					Mobilization Fee					
					51 Avenue Corridor Improvements - Subtotal					
10% Contingency and 12% Engineering/Testing					\$2,370					
					52 Avenue	Corridor I	mnrove	ments - Total	\$12.142	

	Loca	ition		Recommendation from	Item	Quantity	Unit	Unit Rate	Total Cost
Corridor	From	To	Intersection	Recommendation from	Kelli	Qualitity	Oilit	Ollit Kate	Total Cost
Centre Avenue				Existing (2010) Traffic Operational Analysis		1	LS	\$350,000.00	\$350,000
Centre Avenue			57 Street	Existing (2010) Traffic Operational Analysis	Provide traffic signal	1	LS	\$350,000.00	\$350,000
Centre Avenue	59 Street	57 Street	-		Widen to provide centre median and one additional travel lane in each direction (4-lane Divided Arterial with curb and gutter)	330	LM	\$2,017.63	\$665,818
Centre Avenue	57 Street	Highway 28		Traffic Volume Forecast and Analysis	Widen to provide centre median (4-lane Divided Arterial with curb and gutter)	440	LM	\$1,258.04	\$553,539
					•			obilization Fee	\$10,000
Centre Avenue Corridor Improvements - Subtotal \$									\$1,929,357
									\$424,458
					Centre Avenu	e Corridor I	mprove	ements - Total	\$2,353,815

Location				Recommendation from	Item	Quantity	Unit	Unit Rate	Total Cost
Corridor	From	To	Intersection	Trecommendation irom	nem		0	Omit Hate	10101 0001
50 Avenue	Highway 28	49 Street	-	In-Service Road Safety Reviews	Provide back-in angle parking stalls (back in angle parking pavement markings)	76	stalls	\$534.20	\$40,599
50 Avenue	Highway 28	49 Street		In-Service Road Safety Reviews	Repaint pavement markings	1	LS	\$8,134.70	\$8,135
50 Avenue	Highway 28	49 Street		In-Service Road Safety Reviews	Provide curb extensions and signage at crosswalks	1		\$57,834.21	\$57,834
Mobilization Fee								\$10,000	
50 Avenue Corridor Improvements - Subtotal							\$116,568		
	10% Contingency and 12% Engineering/Testing								
50 Avenue Corridor Improvements - Total						\$1.42.242			

Location				Recommendation from	Item	Quantity Uni	Unit	Unit Rate	Total Cost
Corridor	From	To	Intersection	Recommendation from	Kelli	Qualitity	Oilit	Ollit Kate	Total Cost
43 Avenue	Highway 28	45 Street	Highway 28	Highway 28 Functional Review	Provide intersection improvements at 43 Avenue	1	LS	***************************************	\$2,871,930
43 Avenue	Highway 28	45 Street		Traffic Volume Forecast and Analysis	Build pavement structure to Collector standard (4-lane Collector with curb and gutter)	625	LM	\$1,898.57	\$1,186,608
	Mobilization Fee								\$10,000
	43 Avenue Corridor Improvements - Subtotal								
	10% Contingency and 12% Engineering/Testing								
					43 Avenu	a Corridor Ir	nnrove	ments - Total	£4.000.047

Location				Recommendation from	ltem		Unit	Unit Rate	Total Cost
Corridor	From	То	Intersection	Recommendation from	Reni		Oilit	Ollit Kate	I Otal Cost
Kingsway	59 Street	Medley Gate	-		Widen to provide centre median and one additional travel lane in each direction (4-lane Divided Arterial)		LM	\$1,976.04	\$3,122,137
Kingsway	Medley Gate	Glenwood			t and Analysis Widen to provide centre median and one additional travel lane in each direction (4-lane Divided Arterial with curb and gutter)		LM	\$2,017.63	\$2,168,952
Kingsway	Glenwood	Timberline Drive		Traffic Volume Forecast and Analysis	Build pavement structure to Arterial Standard (4-lane Divided Arterial with curb and gutter)		LM	\$3,104.14	\$1,645,192
Mobilization Fee								\$10,000	
Kingsway Corridor Improvements - Subtotal									\$6,946,281
10% Contingency and 12% Engineering/Testing								\$1,528,182	
Kingsway Corridor Improvements - Total								60 474 462	

Location				Recommendation from	ltem	Quantity	Unit	Unit Rate	Total Cost		
Corridor	From	То	Intersection	recommendation from	nem		quantity	U	I CIME HALL	10101 0001	
Glenwood Drive	69 Avenue Arterial	Kingsway	-	Traffic Volume Forecast and Analysis	Build pavement structure to Arterial Standard (4-lane Undivided Arterial with curb and gutter)		485	LM	\$2,214.01	\$1,073,795.84	
						Mobilization Fee					
						Glenwood Drive Corridor Improvements - Subtotal					
10% Contingency and 12% Engineering/Testin											
Glenwood Drive Corridor Improvements - Tota								ments - Total	\$1 322 230 93		

| Improvement Costs = \$55,841,217 | 15% Contingency and 10% Engineering = \$12,335,153 | Total Improvement Costs = \$68,176,370

Several studies and detailed traffic analysis were recommended as part of the transportation study update, which could not be captured in the conceptual cost estimates. The studies and detailed traffic analysis need to be completed in order to develop recommendations that could then be assigned a cost. Following is a summary of the studies and detailed traffic analysis recommended:

- Conduct a speed study and a collision review to validate the removal of existing school zones along Highway 28 and Centre Avenue.
- Conduct a speed study to confirm the speed problem along 1 Avenue.
- Complete a traffic calming study to determine and design the traffic calming measures to implement along 1 Avenue, between 25 Street and 10 Street. Consideration should be given to provide further delineation of the on-street parking lanes.
- Complete a detailed traffic analysis to determine the traffic impact of changing the lane configuration along Lakeshore Drive to a one-way roadway.
- Complete a detailed traffic analysis at Highway 28 and 50 Avenue and review the intersection geometry and lane configuration to improve safety.
- Conduct a "Main Street Analysis" to determine and establish the future function for 50 Avenue, between Highway 28 and 49 Street (CBD).
- Conduct a detailed traffic analysis to determine the traffic impact of providing cul-de-sacs/right-in-right-out along the service roads (55/55A Street and 50 Street).
- Conduct individual in-service safety assessments at the "High Collision Locations".
- Revise the school/playground zone/area, where required, and provide signage in compliance with AT or TAC standards.
- Determine the land ownership of all the driveways along 50 Avenue and identify whether the driveways are currently being used. Unnecessary driveways should be closed.
- Monitor parking conditions throughout the City and determine the effect on traffic operation.
   Particular attention should be given to areas where complaints have been received (i.e., 16 Street, roadways adjacent to schools/playgrounds, and snow removal routes).

The following items were also not incorporated into the cost estimate, as it requires further consideration by the City:

- Enforce "No Parking" zones in Cold Lake North, along 1 Avenue and Lakeshore Drive
- Provide overflow parking for the marina lot
- Close 55 Street and 50 Avenue intersection.

#### 2.14 STAFF TRAINING

A Synchro training course was offered by AE for two (2) personnel from the City of Cold Lake, on June 23 and June 24, 2011.

Synchro is a software package for modeling, optimizing, managing and simulating transportation networks. This course and workshop covered both the Synchro signal timing and analysis software and the SimTraffic simulation and animation package. The main emphasis of this course focused on building, optimizing and



analyzing a network and simulating the results. At the end of the course, all attendees were able to:

- Create a map of street and intersections in Synchro with and without the use of a background map
- Enter the appropriate lane, volume, timing, simulation and detector information into Synchro
- Optimize individual intersection and network cycle length, splits, and offsets with Synchro
- Display and modify Synchro's time space diagrams
- Understand the results displayed in the Synchro program
- Create report to display timing information and measures of effectiveness
- Understand how Synchro performs an optimization
- Apply some workarounds for Synchro and SimTraffic.

#### 2.15 TRANSPORTATION LEVY

Transportation fees levied on proposed developments is an established means for municipalities to recover some or all of the capital costs of providing such services, including the construction of infrastructures both within site boundaries and off-site.

Methods by which transportation levy are calculated are non-uniform amongst different jurisdiction. The basic choice open to municipalities is between uniform or average cost pricing, which tends to be supported by many smaller and growing municipalities, and site-specific pricing, which is a variant of marginal cost pricing that developers prefer and often advocate.

The City of Cold Lake uses a fixed fee approach and currently has a bylaw approving the charge to developers on an average acreage assessment fee for a proposed development. The method works well in a pro-development environment and for municipalities in a growing trend.

While we recommend that current practice be upheld for the time being, as the City grows, the future of development charge practice in Cold Lake may have to be adjusted both by magnitude and by method to allow for at least the rate of inflation, and to allow for impact fees levied to have a more defensible linkage, providing a rational nexus between traffic generated by the proposed development and any roadway improvements deemed necessary.

#### 2.16 PUBLIC CONSULTATION & COUNCIL PRESENTATION

The success of any major transportation planning initiative is ultimately determined by how the public perceives value in the planned program, and how well they support it. A public consultation program was conducted to inform the stakeholders/public about the City's current and future transportation issues and requirements. In addition to the public consultation program, the draft report was reviewed with and presented to the City of Cold Lake's internal stakeholders including the Planning and Engineering departments before the public consultation.

The project team reviewed the City of Cold Lake Transportation Study draft report with the Planning as well as Engineering department to confirm the various components of the project, including:

- 20-year roadway network plan
- Existing traffic analysis results and recommendations
- Forecasted traffic volumes and analyses
- Cold Lake North parking study results
- Cold Lake South parking study results
- City of Cold Lake collision analysis and recommendations
- Highway 28 functional review
- In-Service Road Safety Review results and recommendations
- School & Playground Zone analyses and recommendations
- 20-year horizon roadway improvement cost estimates
- Truck and dangerous goods route considerations
- Transit service requirements and recommendations
- Sustainable transportation initiatives and policies
- Transportation policies and strategies
- Transportation levies.

Two public information sessions were conducted for the Cold Lake Transportation Study. During the day, on Monday, April 18, 2011, a public information session was held at Lakeland Inn in Cold Lake South. In the evening of the same day, a public information session was held at a Fire Hall in Cold Lake North. The information sessions were drop-in style format.

Public notification of the open houses was made through advertisements in the local newspaper, by street-side signs near City Hall, and/or through the City's website. Advertisements were placed well in advance of the open house so that all residents will have an opportunity to attend. AE and the City of Cold Lake project team members were present to inform the public and stakeholders about the City's current and future transportation infrastructure issues and requirements.

The information session displayed materials describing the ultimate roadway network, and the proposed roadway improvements at the four study horizons. The display boards included:

- 20-vear roadway network plan
- Existing traffic analysis results and recommendations
- Forecasted traffic volumes and analyses
- Cold Lake North parking study results
- Cold Lake South parking study results
- City of Cold Lake collision analysis and recommendations
- Highway 28 recommended intersection template
- In-Service Road Safety Review results and recommendations
- School & Playground Zone analyses and recommendations
- Truck and dangerous goods route
- Transit service requirements and recommendations.

No major concerns/comments were received from the general public about the City of Cold Lake's current

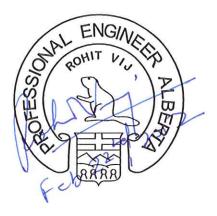


## The City of Cold Lake

transportation challenges and future transportation.

The final Cold Lake Transportation Study report was presented to the Council and Senior Management on Tuesday, April 19, 2011, at the Council Priorities Meeting. AE project team presented this report for discussion purposes and facilitate feedback from Council prior to the document being finalized and adopted by policy.

## **CERTIFICATION PAGE**



ASSOCIATED ENGINEERING

QUALITY MANAGEMENT SIGN-OFF

Signature: .

Date:

**APEGGA Permit to Practice P 3979** 



## **FINAL REPORT**



Appendix A - Transportation Study - Existing (2010)
Traffic Operational Analysis

# **Technical Memorandum**

## **The City of Cold Lake**

**Transportation Study - Existing** (2010) **Traffic Operational Analysis** 

## **April 2011**



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### **TECHNICAL MEMORANDUM**

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### TECHNICAL MEMORANDUM

1

# Introduction

The City of Cold Lake (City) is located 287 km northeast of the City of Edmonton and was formed in 1996 by merging three municipalities, namely Grand Centre, Medley (Canadian Forces Base W4) and Cold Lake. Grand Centre was subsequently renamed Cold Lake South and the original Cold Lake is now known as Cold Lake North.

The City has experienced noticeable growth in recent years. According to municipal census the City had a population of 11,991 in 2006 and 13,924 in 2009. This corresponds to a 5.4% linear growth annually. Current transportation improvements within the City have been based on the previous transportation study completed in 2000 and is no longer considered representative of the actual transportation network required to address current and future transportation needs.

In light of the continuing accelerated pace of development in the region and the need to rationalize and identify the transportation network requirements for the City, including surrounding rural municipalities and counties, the existing transportation study requires a comprehensive update.

### 1.1 PROJECT BACKGROUND

Associated Engineering (AE) was retained by the City to update the transportation study. The purpose of the transportation study is to provide a comprehensive long range plan that integrates the transportation infrastructure requirements of the existing and future land uses. The study will provide the City with a master plan on which to plan and implement specific transportation network improvement projects related to municipal roads, traffic calming, parking, traffic safety, traffic signal coordination, school zones, transit, truck routes, traffic management and transportation system operations over the next 5, 10, 15 and 20 years.

The first task of the transportation study update was to evaluate the traffic operational conditions for the existing roadway system. This technical memorandum presents the traffic operational analysis completed for the existing (2010) horizon and the recommended improvements required.

### 1.2 STUDY AREA

The scope of the transportation study update encompasses the entire City of Cold Lake, including Cold Lake North, Cold Lake South and Medley. The City's boundaries are presented in Figure 1.1.



Upon discussion with the City, only the major roadways (collector and arterial roads) were analyzed for the transportation study. The existing roadway classification from the 2000 transportation study was used to identify the study roadways. Traffic operational analyses were completed at the intersections between two collectors, between a collector and an arterial, and between two arterials. Figure 1.2 presents the collector and arterial roadways within the City, as identified in the 2000 transportation study and the study intersections. Upon further discussion, traffic operations at some additional intersections along Highway 28/55 and along 50 Avenue in the Cold Lake South business zone were also analyzed. The additional study intersections are also presented in Figure 1.2.

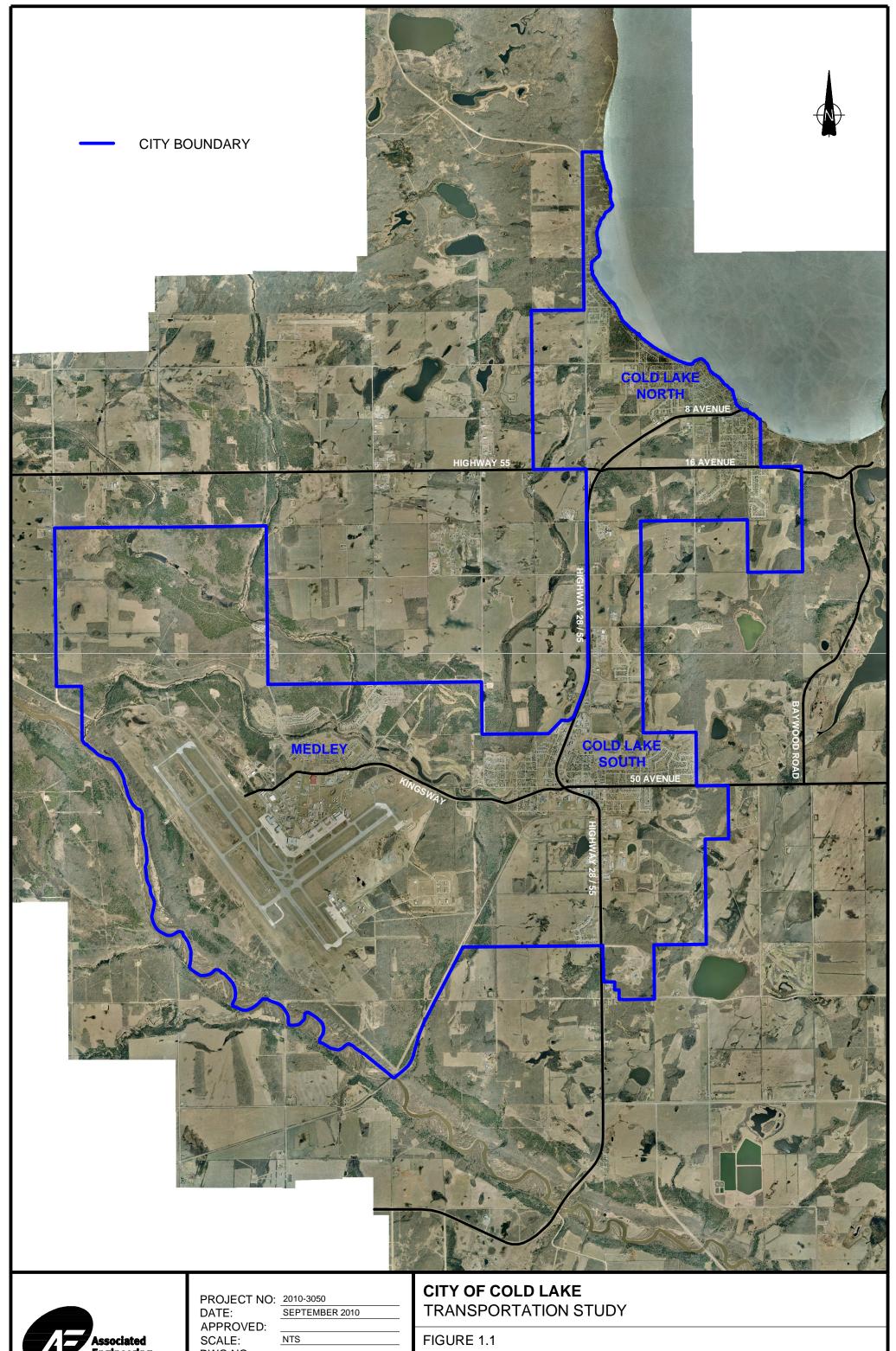
The study area was broken down into the following areas:

- Cold Lake North Encompasses all of Cold Lake North, including the intersection of Highway 28 and Highway 55
- Highway 28/55 Corridor Encompasses the intersections along Highway 28/55 between the Energy Centre Access and 61/62 Avenue
- Cold Lake South Encompasses all of Cold Lake South
- Medley Encompasses all of Medley.

### 1.3 STUDY METHODOLOGY

The existing (2010) traffic operational analysis was completed using the following methodology:

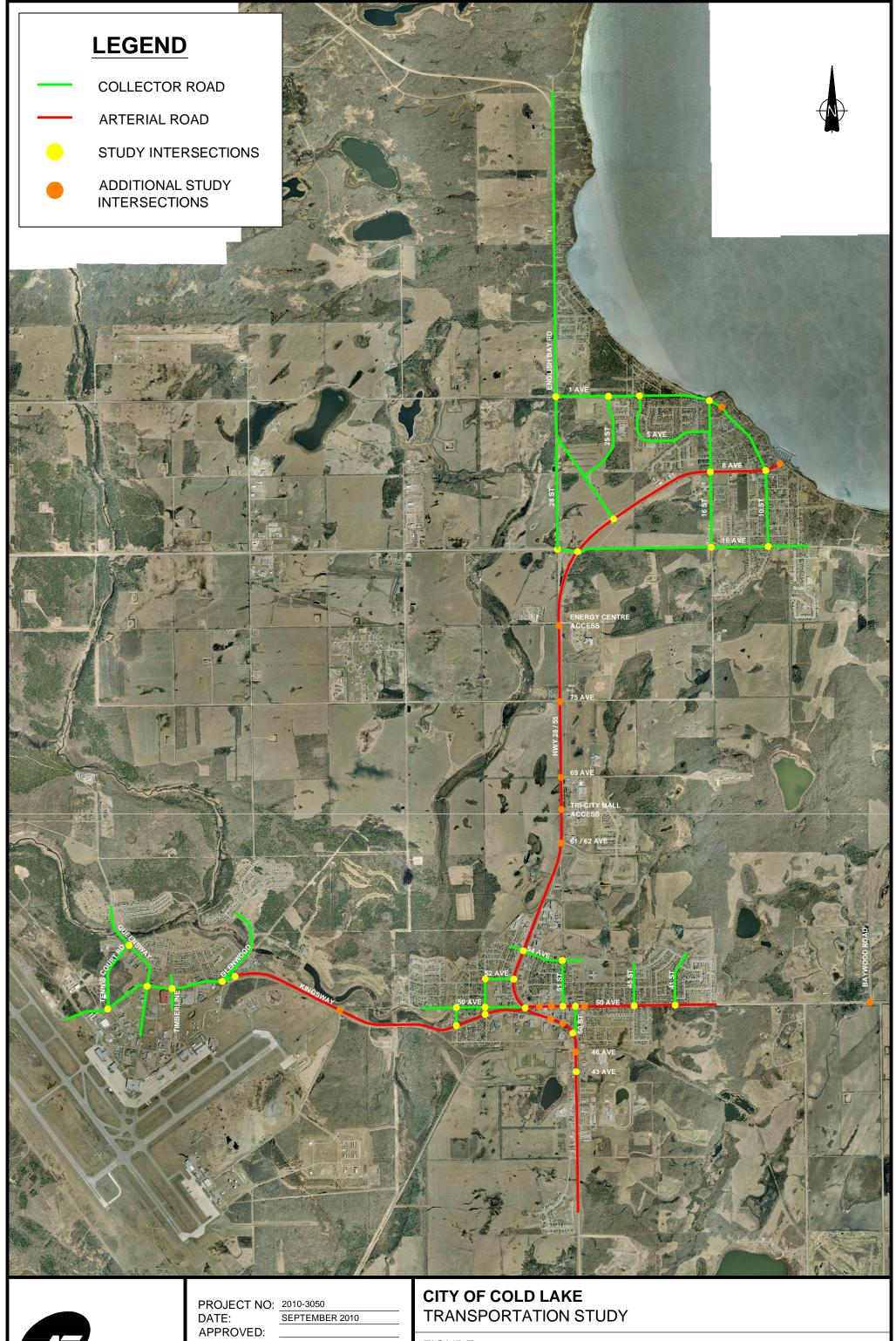
- Conduct project initiation meeting
- Conduct site reconnaissance
- Collect background information and data
- Establish existing (2010) p.m. peak hour traffic volumes
- Complete operational intersection analysis for the p.m. peak hour
- Identify required intersection improvements for the p.m. peak hour
- Conduct client review meeting
- Produce draft and final reports.



Associated Engineering

DWG NO:

FIGURE 1.1 CITY OF COLD LAKE BOUNDARY



Associated Engineering

SCALE: DWG NO:

NTS

TRANSPORTATION STUDY

FIGURE 1.2 STUDY CORRIDORS AND INTERSECTIONS 2

## **Existing Conditions - Data Collection**

### 2.1 SITE RECONNAISSANCE

A site reconnaissance was conducted by AE over a period of two days, on May 4 and 5, 2010. The prevailing weather condition was cloudy with light flurries. The following information was collected during the site visit:

- Lane configuration and traffic control at the study intersections
- Posted speed limits along the study corridors
- Photographs and video log of the study corridors and intersections.

### 2.2 EXISTING LANE CONFIGURATIONS

The existing lane configuration, traffic control, and the posted speed limits at the study intersections are presented in Figure 2.1 through Figure 2.4 for Cold Lake North, the Highway 28/55 Corridor, Cold Lake South, and Medley, respectively.

### 2.3 EXISTING (2010) TRAFFIC VOLUMES

Traffic volume data was obtained from the following sources:

- 2009 intersection turning movement counts obtained from the Highway 28 Needs Assessment Report provided by the City of Cold Lake
- ATR link volumes provided by the City of Cold Lake
- 1999 forecasted intersection turning movement volumes and link volumes obtained from the 2000 transportation study provided by the City of Cold Lake.

Where traffic data was not available from the above sources, intersection turning movement counts were completed. The turning movement counts were conducted by City staff in the month of June 2010, at the following intersections:

- 1 Avenue and 28 Street
- 1 Avenue and 25 Street
- 1 Avenue and 16 Street
- 1 Avenue/10 Street and 2 Avenue
- 8 Avenue and Lakeshore Drive
- 16 Avenue and 10 Street
- 54 Avenue and 51 Street
- 50 Avenue and 53 Street
- 50 Avenue and 52 Street
- 50 Avenue and 50 Street



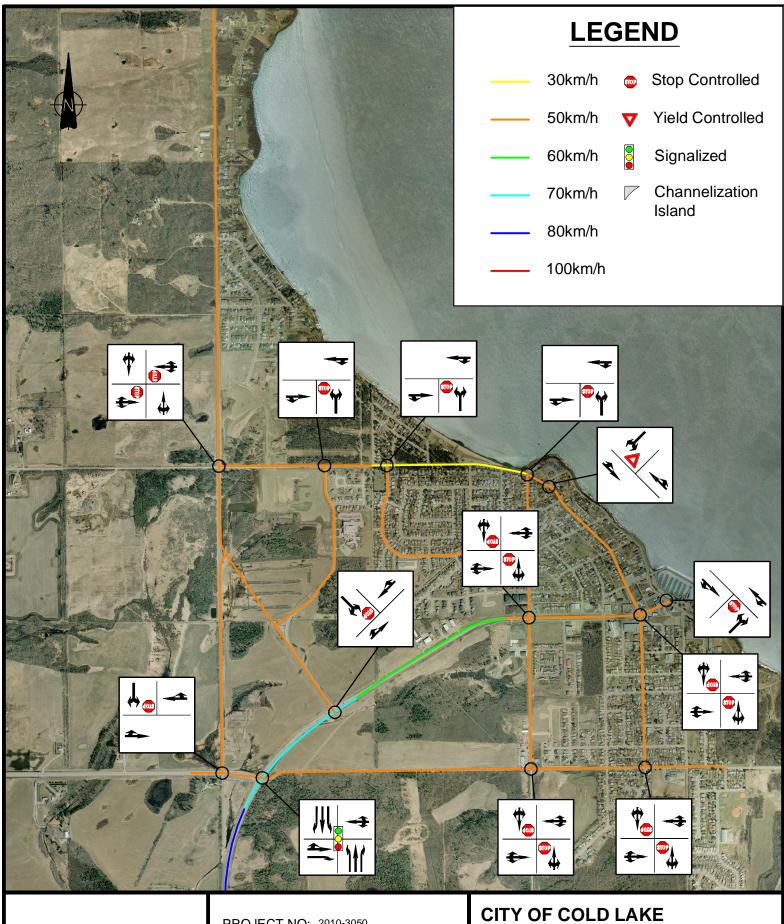
- 50 Avenue and 49 Street
- 50 Avenue and 45 Street
- 50 Avenue and 41 Street
- 50 Avenue and Baywood Road
- Highway 28 and 52 Street
- Highway 28 and 51 Street
- Highway 28 and 43 Avenue
- Kingsway and Medley Road
- Kingsway and Queensway
- Kingsway and Tennis Court Road
- Queensway and Tennis Court Road.

For clarity, traffic volumes collected through intersection turning movement counts and ATR counts will be referred to as 'field volumes' and traffic volumes generated through forecasting methods will be referred to as 'forecasted volumes'.

The existing (2010) p.m. peak hour traffic volumes were established using the following methodology:

- The most recent field volumes were used, where available. If the most recent field volumes were
  not collected in 2010, the volumes where grown using an annual growth rate of 2% (discussed in
  detail in the following section) to the 2010 horizon.
- Where field volumes were not available, the forecasted volumes were used and grown to the 2010 horizon.
- At locations where only forecasted volumes were available at the intersection and field volumes
  were available on the adjacent links, the intersection volumes were adjusted to match the link
  volumes while maintaining the intersection split from the forecast.

Figure 2.5 through Figure 2.8 present the p.m. peak hour traffic volumes used for the existing (2010) traffic operational analysis for Cold Lake North, the Highway 28/55 Corridor, Cold Lake South, and Medley, respectively.





PROJECT NO: 2010-3050
DATE: SEPTEMBE

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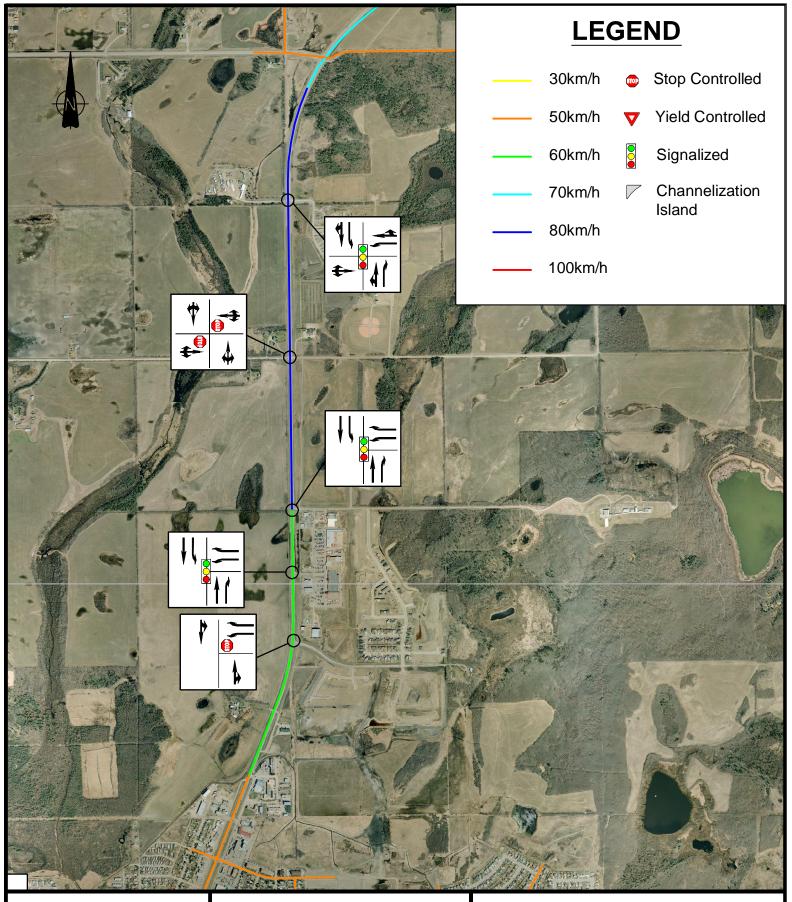
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SEPTEMBER 2010

# TRANSPORTATION STUDY

FIGURE 2.1 - COLD LAKE NORTH EXISTING LANE CONFIGURATION, TRAFFIC CONTROL AND POSTED SPEED LIMIT





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DATE: SEPTEMBER 2010

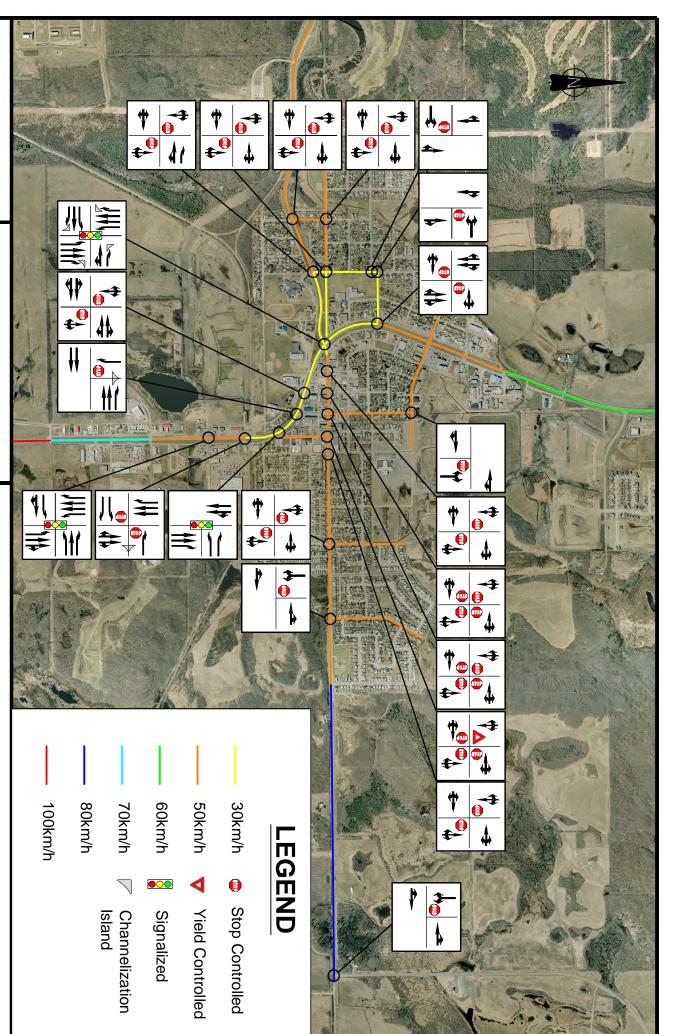
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# CITY OF COLD LAKE TRANSPORTATION STUDY

FIGURE 2.2 - HIGHWAY 28 / 55 CORRIDOR EXISTING LANE CONFIGURATION, TRAFFIC CONTROL AND POSTED SPEED LIMIT





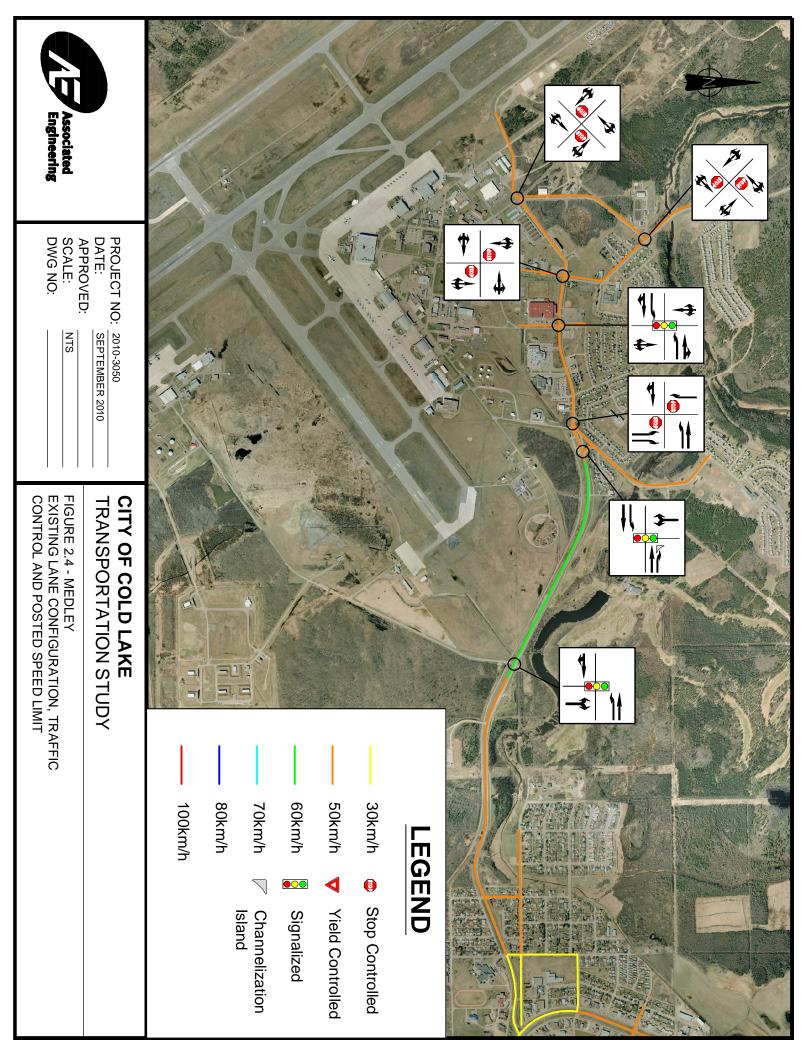
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# CITY OF COLD LAKE TRANSPORTATION STUDY

FIGURE 2.3 - COLD LAKE SOUTH EXISTING LANE CONFIGURATION, TRAFFIC CONTROL AND POSTED SPEED LIMIT



### 2.4 ANNUAL GROWTH RATE

In order to develop the 2010 traffic volumes, a growth rate was applied to some of the traffic volumes provided by the City.

Table 2.1 presents the growth rate calculations performed on the data provided by the City of Cold Lake. The following four sources were reviewed in the calculation of the growth rate:

- Source 1: Historical 2003-2004 traffic counts along Highway 28 provided by the City
- Source 2: Cold Lake Halley Test 2009 (population growth)
- Source 3: City of Cold Lake, Municipal Development Plan (Page 7)
- Source 4: MD Bonnyville, Intermunicipal Development Plan (Page 16).

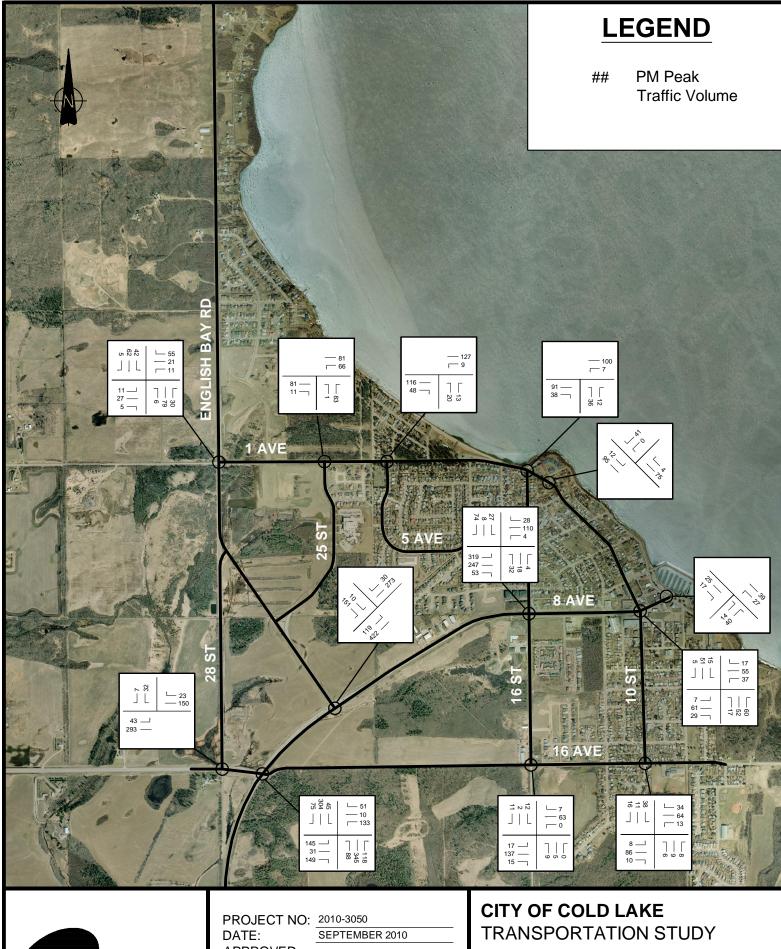
The information obtained from these sources is summarized below and is included in Appendix A.

Through discussion with the City and with consideration for the below information, an annual growth rate of 2.0% was selected and utilized for traffic growth in this report.



Table 2.1
Growth Rate Calculation

D i	1	Discoult		Source 1		0	0	0
Roadway	Location	Direction	Oct 2003	June 2004	% Growth	Source 2	Source 3	Source 4
	North of Highway 55	Northbound	3,785	4,100				
	North of Highway 55	Southbound	3,890	4,386				
Highway 28	North of Highway 55	Two-way Total	7,675	8,487	10.6%			
	South of Highway 55	Northbound	5,796	4,796				
	South of Highway 55	Southbound	5,818	5,011				
	South of Highway 55	Two-way Total	11,614	9,897	-14.8%			
Highway 55	West of Highway 28	Two-way Total	3,212	4,713	46.7%			
16 Avenue	East of Highway 28	Two-way Total	2,735	1,958	-28.4%			
	North of Imperial Park Rd	Northbound	6,012	-				
	North of Imperial Park Rd	Southbound	6,032	-			1.5% Average Annual	5.5% in Five
Highway 28	North of Imperial Park Rd	Two-way Total	12,044	12,565	4.3%		Growth (1986 - 2001)	Years (1996- 2001)
Highway 20	South of Imperial Park Rd	Northbound	5,090	-		2.0% Assumed	2.5% - High Projected	
	South of Imperial Park Rd	Southbound	6,069	-		17.0%	17.0% Annual Growth	Growth
	South of Imperial Park Rd	Two-way Total	11,159	12,821	14.9%	3.0% Annual Growth if growth continues in oil sands, tourism and service-based industries.	1.5% - Moderation Projected Growth	
	North of Mall Access	Northbound	5,860	4,678				
	North of Mall Access	Southbound	5,512	6,441			tourism and	0.5% - Low Projected Growth
Highway 28	North of Mall Access	Two-way Total	11,372	11,209	-1.4%			
g,	South of Mall Access	Northbound	7,101	7,799				
	South of Mall Access	Southbound	6,917	7,694				
	South of Mall Access	Two-way Total	14,018	15,493	10.5%			
Imperial Park Rd (Energy Access Centre)	East of Highway 28	Two-way Total	126	282	123.8%			
Mall	East of Highway 28	Northbound	2,158	2,502				
(Tri-City) Access	East of Highway 28	Southbound	2,074	3,870				
700699	East of Highway 28	Two-way Total	4,232	6,372	50.6%			
Average Annual Growth Rate					21.7%			



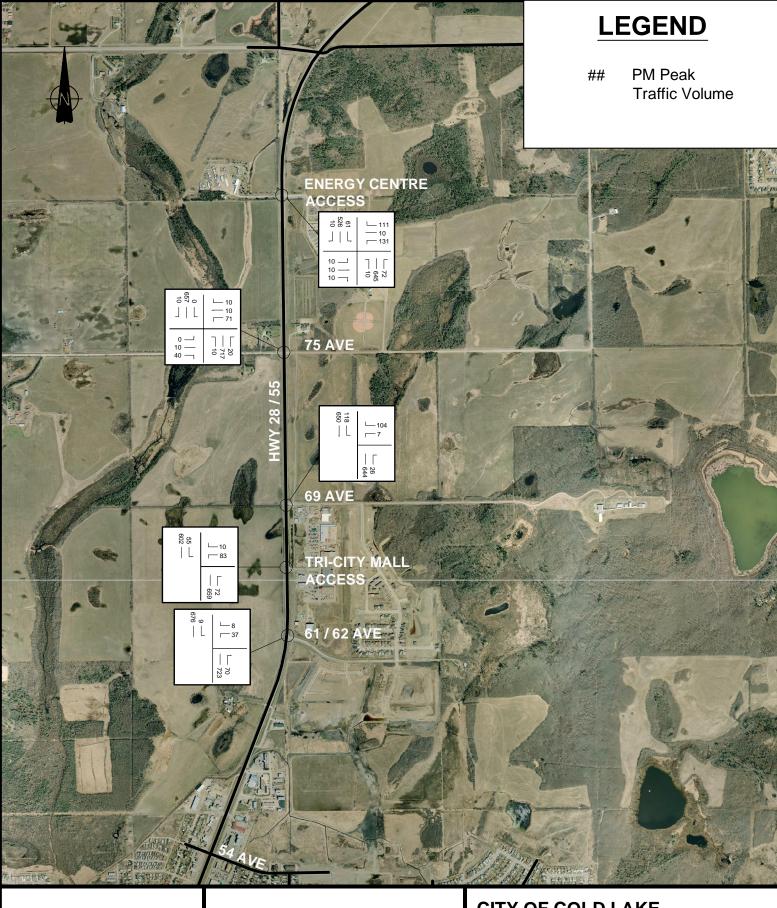


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FIGURE 2.5 - COLD LAKE NORTH EXISTING (2010) PM PEAK TRAFFIC VOLUMES





PROJECT NO: 2010-3050
DATE: SEPTEMBER 2010

APPROVED: SCALE:

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# CITY OF COLD LAKE TRANSPORTATION STUDY

FIGURE 2.6 - HIGHWAY 28 / 55 CORRIDOR EXISTING (2010) PM PEAK TRAFFIC VOLUMES



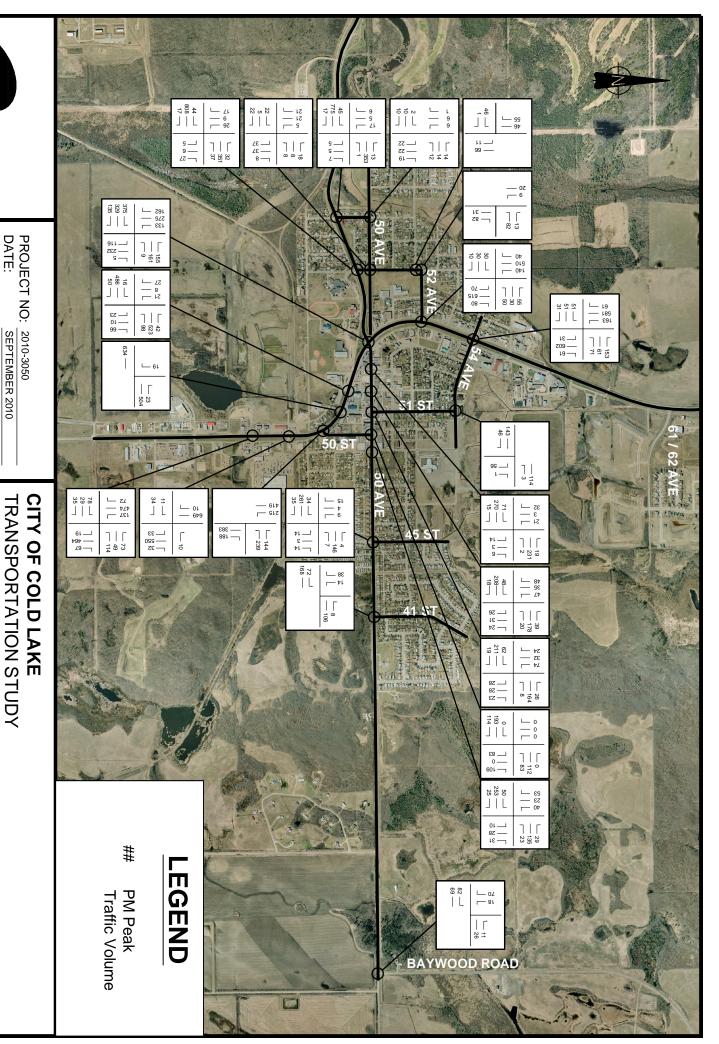
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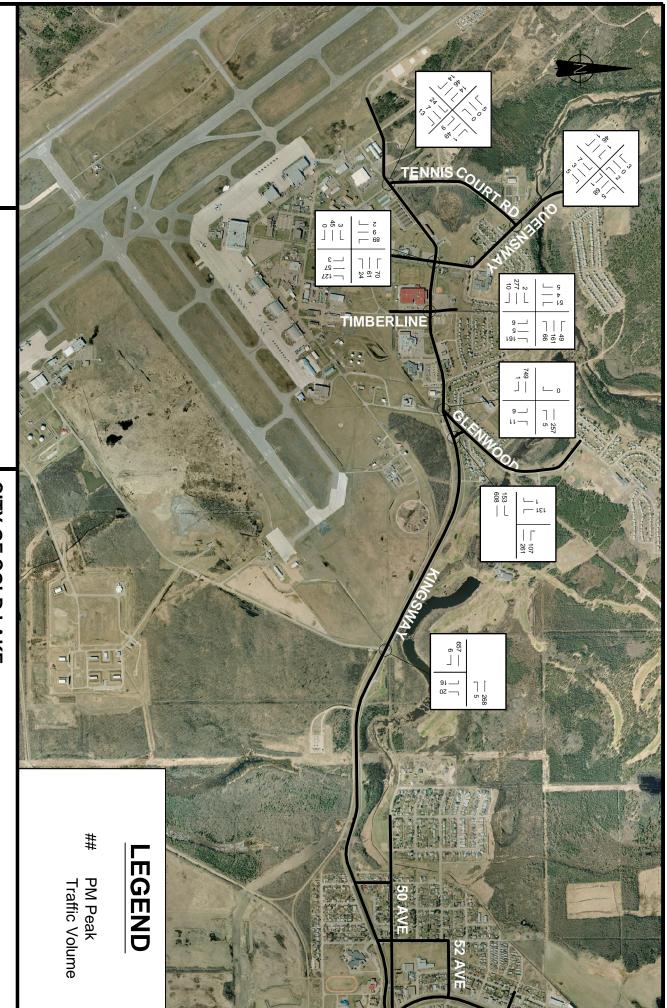
**EXISTING (2010) PM PEAK TRAFFIC VOLUMES** 

FIGURE 2.7 - COLD LAKE SOUTH

SEPTEMBER 2010







PROJECT NO: 2010-3050

DATE: SEPTEMBER 2010

DATE:
APPROVED:
SCALE:
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CITY OF COLD LAKE TRANSPORTATION STUDY

FIGURE 2.8 - MEDLEY EXISTING (2010) PM PEAK TRAFFIC VOLUMES

3

# **Traffic Operational Analysis**

The Synchro 7.0 traffic analysis program based on the Highway Capacity Manual (HCM) was used to complete the capacity analysis of the study intersections. Synchro 7.0 applies the methodology established by the HCM to output a level of service (LOS) for study intersections, given the lane configuration, vehicular volumes, heavy vehicle percentages, signal timing, etc.

A design criteria was developed by AE at project initiation to set various parameters for the capacity analysis. The design criteria was submitted and approved by the City, and is included in Appendix B. Changes to the design criteria were made as the study progressed. The peak hour factor (PHF) was revised to 0.86 from a default value of 1.00; a PHF of 0.86 reflects the average calculated PHF from the traffic counts conducted by the City in June 2010. The PHF is a measure of traffic demand fluctuation within the peak hour and is calculated by taking the hourly volume during the peak hour of the day and dividing by the peak 15-min flow rate within the peak hour. Additionally, default signal timing parameters such as minimum green time and pedestrian walk/clearance time were revised to reflect those proposed as part of the Highway 28 Twinning project.

It should be noted that existing signal timing plans were used, where available, at current signalized intersections.

The operational capability of the study intersections were assessed using capacity, which is a measure of the sustainable flow rate at which vehicles can be expected to transverse a point. The critical measures used in the assessment were:

**Volume to capacity (v/c) ratio** provides the amount of congestion for each turning movement and for each lane group for signalized intersections. A v/c value over 1.00 indicates that the movement or lane group is over capacity.

**Control delay** is the amount of delay a vehicle experiences in seconds.

**LOS** is a qualitative measure describing operational conditions within a traffic stream and is based on service measures such as delay and congestion.

A LOS C was required for both the overall intersection and the individual intersection approaches to be operating above an acceptable level. The LOS definitions for unsignalized and signalized intersections were included in the design criteria under Appendix B.

A review of the operational analysis results is presented below and the detailed summary of the Synchro results is included in Appendix C.



### 3.1 COLD LAKE NORTH

Figure 3.1 presents the overall intersection capacity results for the study intersections in Cold Lake North. With the exception of the intersection at 8 Avenue and 16 Street, all the study intersections are currently operating well, with overall intersection LOS B or higher and intersection delays of 12.0 seconds or less.

The intersection of 8 Avenue and 16 Street is currently operating with an overall intersection LOS B, maximum v/c ratio of 0.59, and intersection delays of 12.0 seconds. While the intersection is operating well overall, the northbound and southbound approaches are operating poorly. The northbound approach is operating at LOS F with delays of 77.7 seconds and the southbound approach is operating at LOS D with delays of 30.1 seconds. The poor LOS on these approaches can be attributed to the stop control provided on these approaches. Vehicles on the northbound and southbound approaches experience considerable delays while waiting for acceptable gaps in successive platoons of traffic on 8 Avenue. This intersection is currently being upgraded as part of the Highway 28 Twinning project.

### 3.2 HIGHWAY 28/55 CORRIDOR

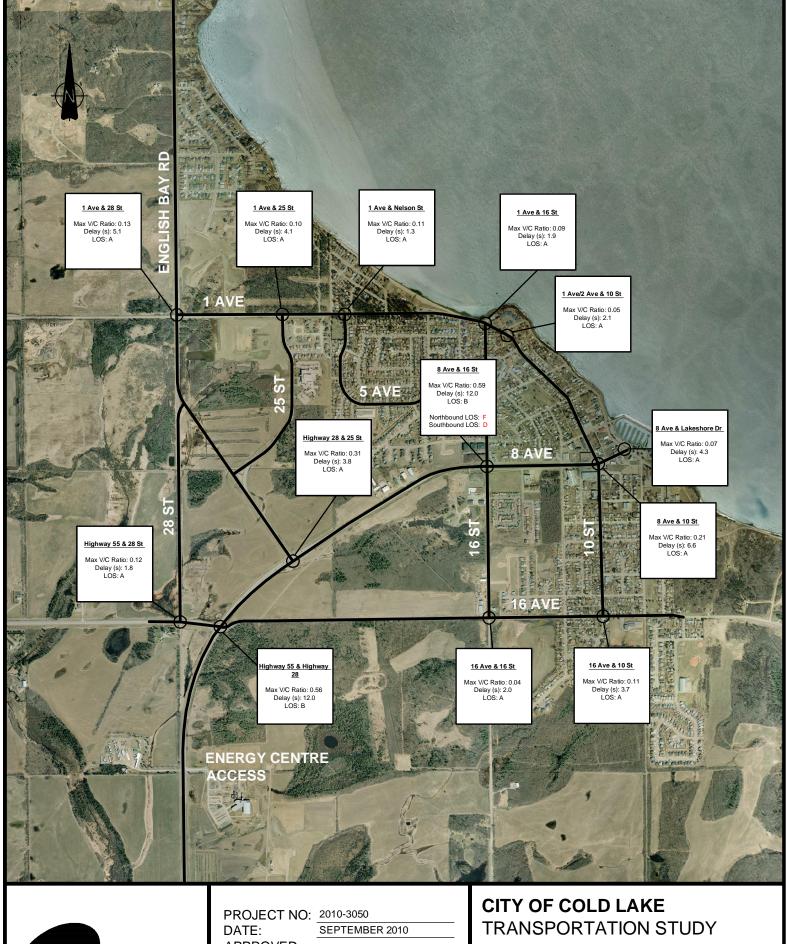
Figure 3.2 presents the overall intersection capacity results for the study intersections along the Highway 28/Highway 55 corridor.

The intersection of Highway 28/55 with the Energy Centre Access, 69 Avenue, and the Tri-City Mall Access are all operating well, with overall intersection LOS B and intersection delays of 14.3 seconds or less.

The intersection of Highway 28/55 with 75 Avenue is currently operating with an overall intersection LOS D, maximum v/c ratio of 1.61, and intersection delays of 26.8 seconds. The maximum v/c ratio is experienced by the westbound approach, which indicates that the existing volumes on this approach are exceeding the capacity of the single lane provided. The westbound approach is operating at LOS F with delays of 440.6 seconds. The long delays on this approach are compounded by the stop control provided for the eastbound and westbound approaches.

The intersection of Highway 28/55 with 61/62 Avenue is currently operating with an overall intersection LOS A, maximum v/c ratio of 0.58, and intersection delays of 2.9 seconds. While the intersection is operating well overall, the westbound left turn movement is operating poorly. This movement is operating at LOS F with delays of 106.5 seconds. The poor LOS on this movement can be attributed to the stop control provided on the westbound approach. Left turn vehicles on the westbound approach experience considerable delays while waiting for acceptable gaps in successive platoons of traffic on Highway 28/55.

Both the intersections at 75 Avenue and 61/62 Avenue are currently being upgraded as part of the Highway 28 Twinning project.





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SCALE: DWG NO:

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FIGURE 3.1 - COLD LAKE NORTH EXISTING (2010) CAPACITY RESULTS





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# CITY OF COLD LAKE TRANSPORTATION STUDY

FIGURE 3.2 - HIGHWAY 28 / 55 CORRIDOR EXISTING (2010) CAPACITY RESULTS

### 3.3 COLD LAKE SOUTH

Figure 3.3 presents the overall intersection capacity results for the study intersections in Cold Lake South.

With the exception of the following intersections, all the study intersections are currently operating well, with overall intersection LOS B or higher and intersection delays of 14.3 seconds or less:

- Highway 28/55 and 54 Avenue
- Highway 28/55 and 52 Avenue
- Highway 28/55 and 52 Street
- Highway 28/55 and 46 Avenue
- Centre Avenue and 59 Street
- Centre Avenue and 57 Street.

The intersection of Highway 28/55 with 54 Avenue is currently operating with an overall intersection LOS B, maximum v/c ratio of 0.86, and intersection delays of 19.5 seconds. While the intersection is operating well overall, the southbound left turn movement is operating below the acceptable threshold. This movement is operating at LOS D with delays of 52.5 seconds. The lower LOS on this movement can be attributed to delays while waiting for acceptable gaps in the opposing traffic on Highway 28/55. This intersection is currently being upgraded as part of the Highway 28 Twinning project.

The intersection of Highway 28/55 with 52 Avenue currently fails, with an overall intersection LOS F, maximum v/c ratio of 3.35, and intersection delays of 849.3 seconds. The intersection is failing due to the eastbound and westbound approaches. Both these approaches are operating at LOS F, with v/c ratios exceeding 1.00 (v/c = 2.25 on the eastbound approach and v/c = 3.35 on the westbound approach), and delays exceeding 812.5 seconds. The v/c ratios indicate that the existing volumes on these approaches are exceeding the capacity of the single lane provided. The long delays indicate that vehicles on these approaches are waiting a long time at the stop sign for acceptable gaps in successive platoons of traffic on Highway 28/55.

The intersection of Highway 28/55 and 52 Street is currently operating with an overall intersection LOS A, maximum v/c ratio of 0.47, and intersection delays of 5.1 seconds. While the intersection is operating well overall, the northbound and southbound approaches are operating poorly. The northbound approach is operating at LOS D with delays of 32.2 seconds and the southbound approach is operating at LOS E with delays of 42.6 seconds. The poor LOS on these approaches can be attributed to the stop control provided on 52 Street. Vehicles on the northbound and southbound approaches experience considerable delays while waiting for acceptable gaps in successive platoons of traffic on Highway 28/55.



The intersection of Highway 28/55 and 46 Avenue is currently operating with an overall intersection LOS A, maximum v/c ratio of 0.22, and intersection delays of 1.0 seconds. While the intersection is operating well overall, the eastbound left turn movement is operating below the acceptable threshold. This movement is operating at LOS D with delays of 33.2 seconds. The poor LOS on this movement can be attributed to the stop control provided on the eastbound approach. Left turn vehicles on the eastbound approach experience considerable delays while waiting for acceptable gaps in successive platoons of traffic on Highway 28/55.

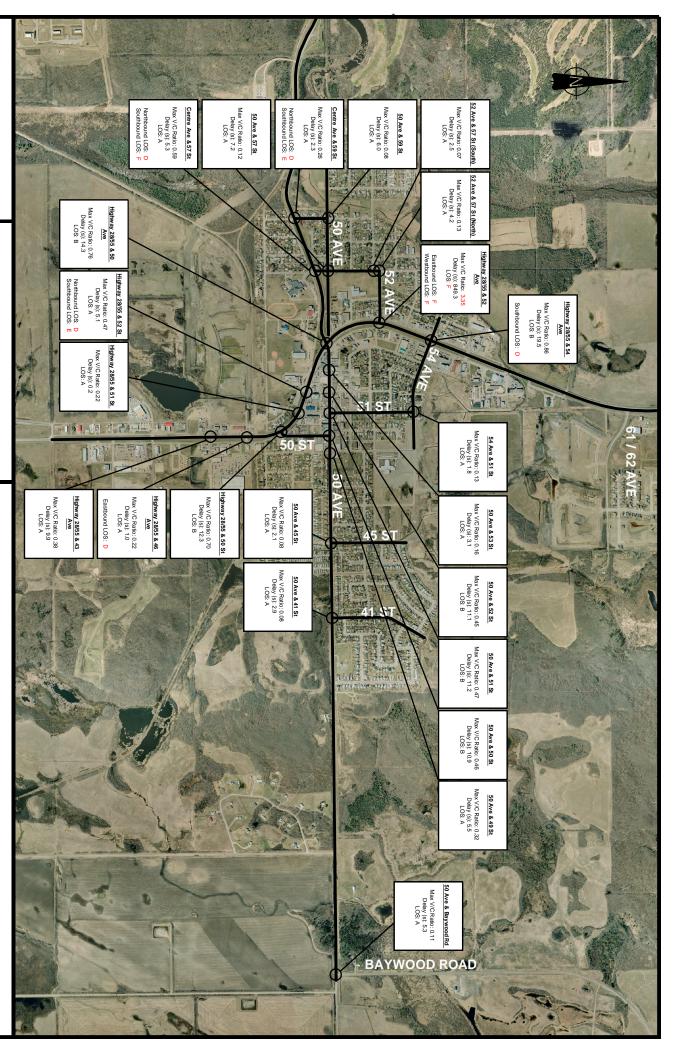
The intersection of Centre Avenue and 59 Street is currently operating with an overall intersection LOS A, maximum v/c ratio of 0.26, and intersection delays of 2.3 seconds. While the intersection is operating well overall, the northbound and southbound approaches are operating poorly. The northbound approach is operating at LOS D with delays of 32.7 seconds and the southbound approach is operating at LOS E with delays of 44.5 seconds. The poor LOS on these approaches can be attributed to the stop control provided on 59 Street. Vehicles on the northbound and southbound approaches experience considerable delays while waiting for acceptable gaps in successive platoons of traffic on Centre Avenue.

The intersection of Centre Avenue and 57 Street is currently operating with an overall intersection LOS A, maximum v/c ratio of 0.59, and intersection delays of 5.3 seconds. While the intersection is operating well overall, the northbound and southbound approaches are operating poorly. The northbound approach is operating at LOS D with delays of 33.0 seconds and the southbound approach is failing at LOS F with delays of 81.7 seconds. The poor LOS on these approaches can be attributed to the stop control provided on 57 Street. Vehicles on the northbound and southbound approaches experience considerable delays while waiting for acceptable gaps in successive platoons of traffic on Centre Avenue.

### 3.4 MEDLEY

Figure 3.4 presents the overall intersection capacity results for the study intersections in Medley.

All the study intersections are operating well with overall intersection LOS B or higher and intersection delays of 13.6 seconds or less. Additionally, all the intersection approaches are operating above the acceptable LOS C.





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# CITY OF COLD LAKE TRANSPORTATION STUDY

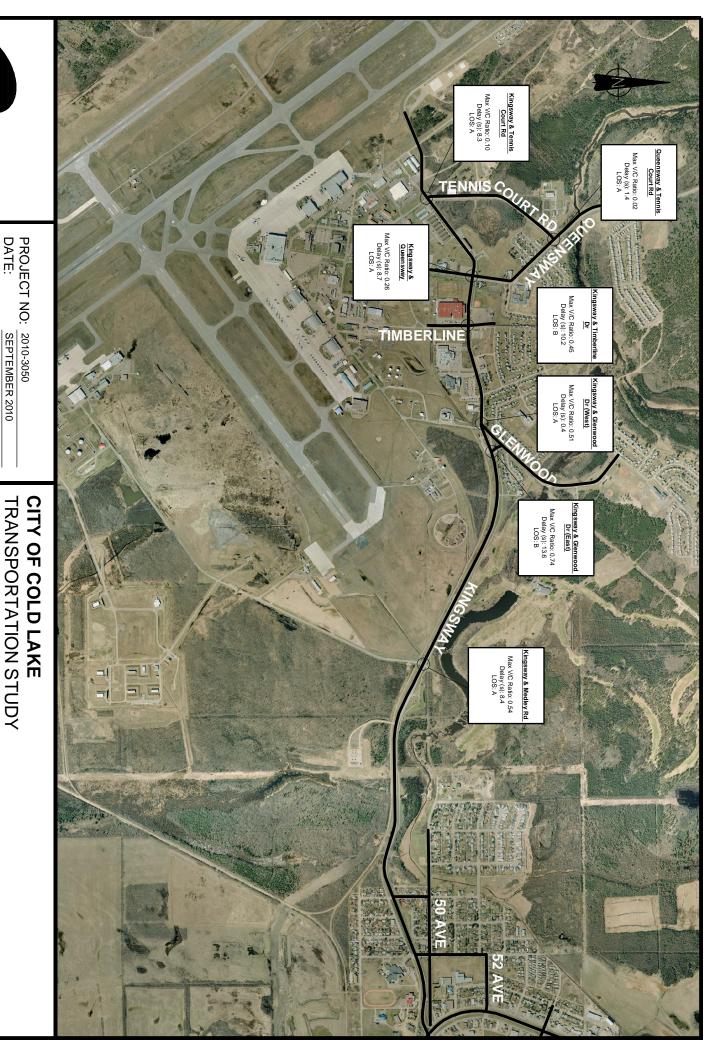
**EXISTING (2010) CAPACITY RESULTS** FIGURE 3.3 - COLD LAKE SOUTH



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FIGURE 3.4 - MEDLEY EXISTING (2010) CAPACITY RESULTS



4

# **Highway 28 Twinning**

Highway 28 is currently undergoing construction to be twinned from 10 Street in Cold Lake North to 54 Avenue in Cold Lake South. The following study intersections will be upgraded as part of the Highway 28 Twinning project including intersection configuration improvements and signalization:

### **Cold Lake North**

- 8 Avenue and 10 Street
- 8 Avenue and 16 Street
- Highway 28 and 25 Street
- Highway 28 and Highway 55/16 Avenue.

### Highway 28/55 Corridor

- Highway 28/55 and Energy Centre Access
- Highway 28/55 and 75 Avenue
- Highway 28/55 and 69 Avenue
- Highway 28/55 and Tri-City Mall Access
- Highway 28/55 and 61/62 Avenue.

### **Cold Lake South**

Highway 28/55 and 54 Avenue.

The ultimate lane configuration at the above intersections and the ultimate signal timing plans for the signalized intersections were provided by the City. The ultimate signal timing plans were revised based on optimization using Synchro. Figure 4.1 presents the ultimate lane configuration and traffic control along Highway 28/55, from 10 Street to 54 Avenue, after the Highway 28 Twinning.

The upgraded intersections along Highway 28/55 were analyzed to determine the expected operational capacity, after the Highway 28 Twinning project. Figure 4.2 presents the overall intersection capacity results. A review of the operational analysis results is presented below and the detailed summary of the Synchro results is included in Appendix C.



### 4.1 COLD LAKE NORTH

All the intersections upgraded in Cold Lake North, as part of the Highway 28 Twinning project, are expected to operate at an acceptable LOS. The upgraded intersections are expected to operate with an overall intersection LOS B or higher and intersection delays of 18.1 seconds or less. Additionally, all the intersection approaches are expected to operate at LOS C or better.

### 4.2 HIGHWAY 28/55 CORRIDOR

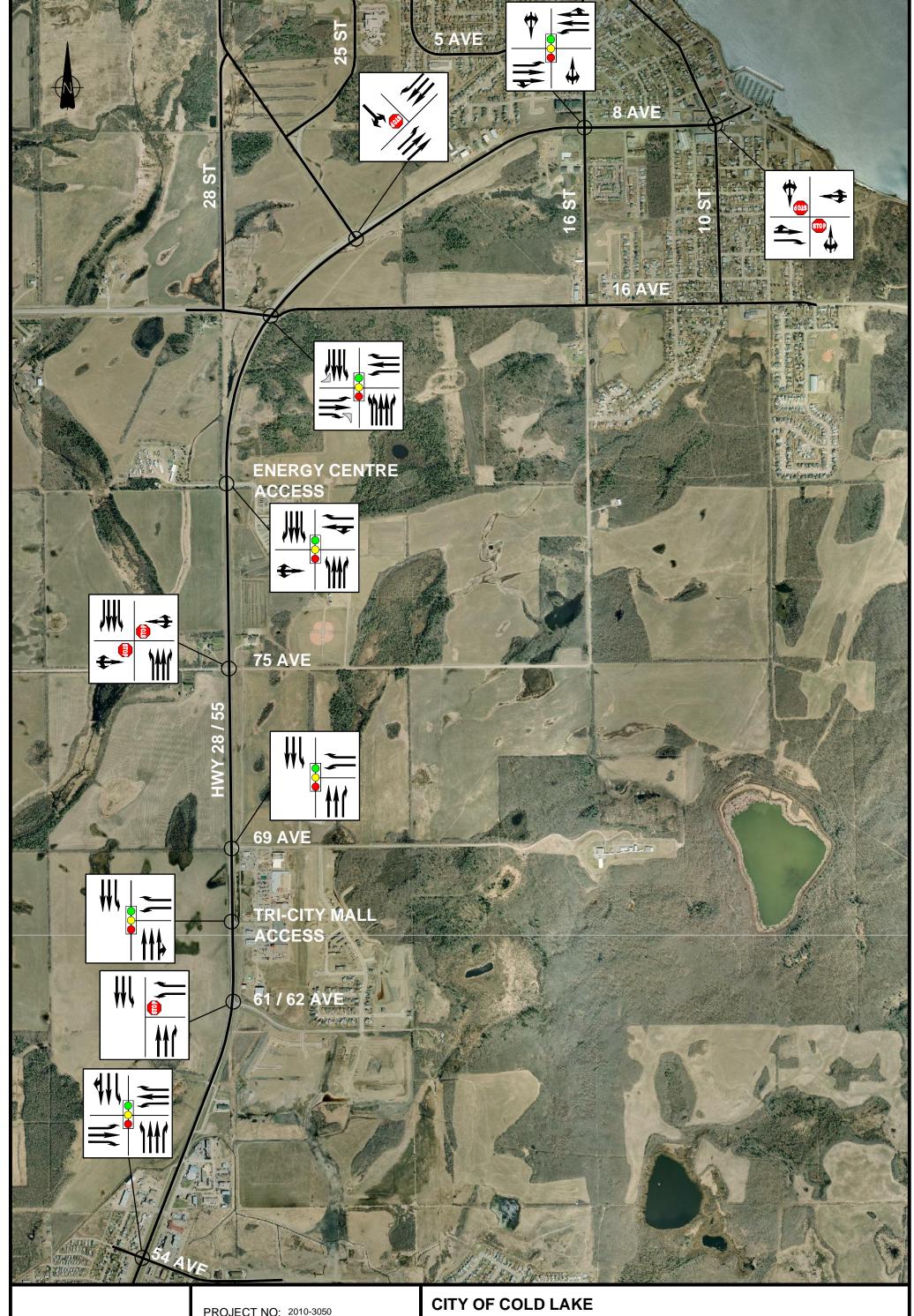
After the Highway 28 Twinning project, the intersection of Highway 28/55 with the Energy Centre Access, 69 Avenue, and the Tri-City Mall Access are expected to continue operating well, with overall intersection LOS B or higher and intersection delays of 14.9 seconds or less.

The intersection of Highway 28/55 and 75 Avenue is expected to operate at an overall intersection LOS B, with a maximum v/c ratio of 1.00 and intersection delays of 10.2 seconds. With the intersection upgrades the maximum v/c ratio is expected to improve by 0.61; however, the westbound approach continues to fail and operate at LOS F with a v/c ratio of 1.00 and delays of 161.2 seconds. Additional intersection upgrades will be required for the intersection to operate at an acceptable level.

The intersection of Highway 28/55 and 61/62 Avenue is expected to operate at an overall intersection LOS A, with a maximum v/c ratio of 0.27 and intersection delays of 1.0 seconds. While the intersection is expected to operate well overall, the westbound left turn movement is expected to operate poorly. This movement is expected to operate at LOS E with delays of 36.2 seconds. The poor LOS on this movement can be attributed to the stop control provided on the westbound approach. Additional intersection upgrades will be required for the westbound left turn movement to operate at an acceptable level.

### 4.3 COLD LAKE SOUTH

Only the intersection of Highway 28/55 and 54 Avenue was upgraded as part of the Highway 28 Twinning project. With the upgrades, the intersection is expected to operate well with an overall intersection LOS C, maximum v/c ratio of 0.67, and intersection delays of 20.6 seconds. Additionally all the intersection approaches are expected to operate at LOS C or better.





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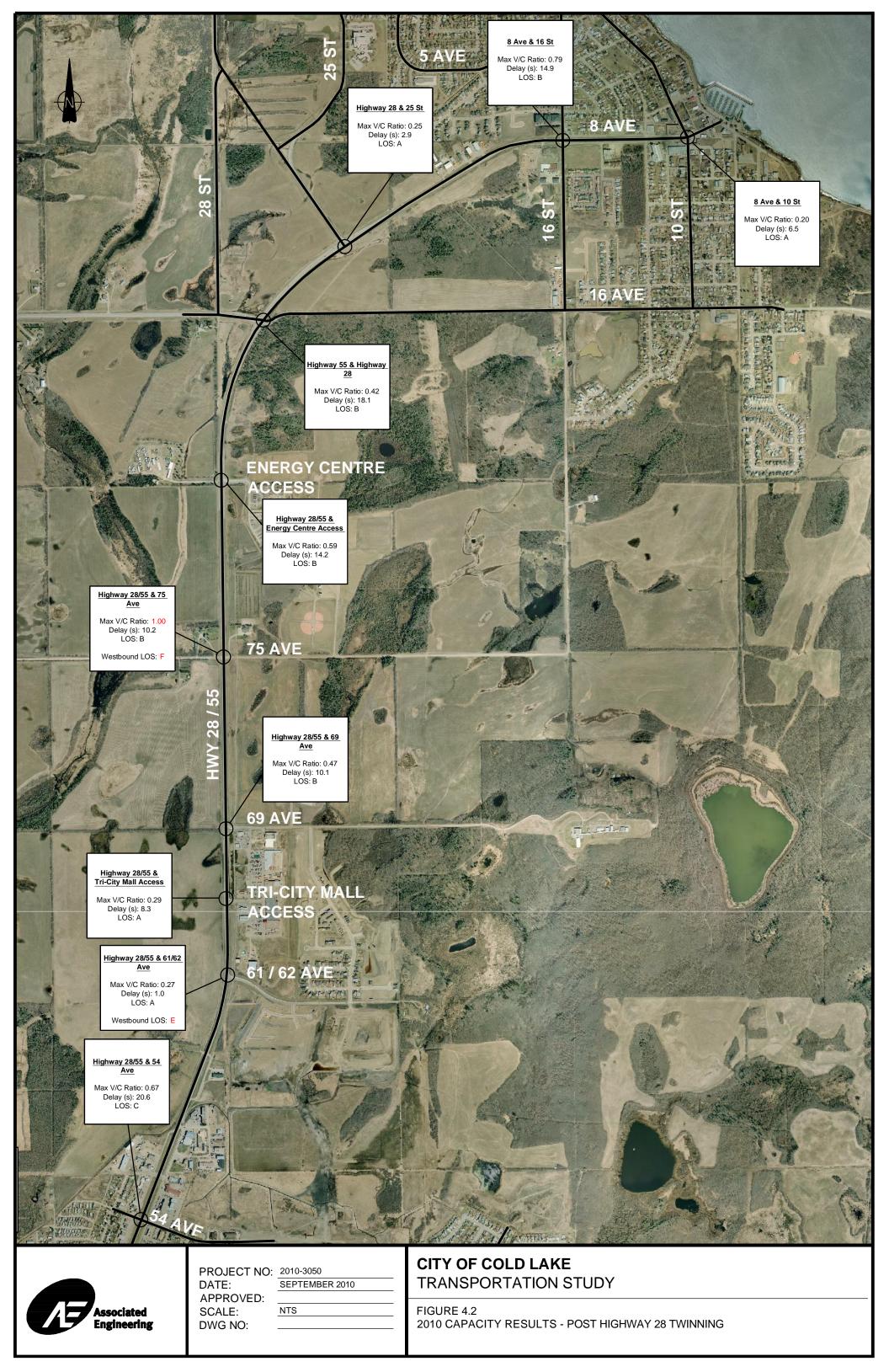
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TRANSPORTATION STUDY

FIGURE 4.1 ULTIMATE LANE CONFIGURATION AND TRAFFIC CONTROL - POST HIGHWAY 28 TWINNING



5

### **Gemetric/Operational Improvements**

Despite the improvements being implemented as part of the Highway 28 Twinning, there are a number of intersections that require additional improvements in order for the intersection to operate above the acceptable level. The intersections which require further improvements are:

### Highway 28/55 Corridor

- Highway 28/55 and 75 Avenue
- Highway 28/55 and 61/62 Avenue.

### **Cold Lake South**

- Highway 28/55 and 52 Avenue
- Highway 28/55 and 52 Street
- Highway 23/55 and 46 Avenue
- Centre Avenue and 59 Street
- Centre Avenue and 57 Street.

The above intersections were analyzed to determine the improvements required. Figure 5.1 presents the required intersection improvements at the study intersections and Figure 5.2 presents the overall intersection capacity results for the required intersection improvements.

A review of the operational analysis results is presented below and the detailed summary of the Synchro results is included in Appendix C. The improvements described below are in addition to any improvements proposed for the Highway 28 Twinning.

### 5.1 HIGHWAY 28/55 AND 75 AVENUE

Traffic signals are recommended at the intersection of Highway 28/55 and 75 Avenue, to provide sufficient gaps between the successive platoons of traffic on Highway 28 and reduce the delays on the westbound approach. With the traffic signal, the intersection is expected to operate at an overall intersection LOS A, with a maximum v/c ratio of 0.37 and intersection delays of 7.6 seconds. All intersection approaches are expected to operate at LOS B or better.

### 5.2 HIGHWAY 28/55 AND 61/62 AVENUE

Traffic signals are recommended at the intersection of Highway 28/55 and 61/62 Avenue, to provide sufficient gaps between the successive platoons of traffic on Highway 28 traffic and reduce the delays on the westbound approach. Additionally, a channelized right turn lane is recommended on the northbound approach since the right turn volumes exceed 60 vehicles per hour in the p.m. peak hour. With the recommended improvements, the intersection is expected to operate at an overall intersection LOS A, with a maximum v/c ratio of 0.31 and intersection delays of 4.6 seconds. All intersection approaches are expected to operate at LOS B or better.



### 5.3 HIGHWAY 28/55 AND 52 AVENUE

Similar to 61/62 Avenue, traffic signals are recommended at the intersection of Highway 28/55 and 52 Avenue to provide sufficient gaps between the successive platoons of traffic on Highway 28 traffic and reduce the delays on the eastbound and westbound approaches. Additionally, a channelized right turn lane is recommended on the northbound approach since the right turn volumes exceed 60 vehicles per hour in the p.m. peak hour. With the recommended improvements, the intersection is expected to operate at an overall intersection LOS A, with a maximum v/c ratio of 0.70 and intersection delays of 13.3 seconds. All intersection approaches are expected to operate at LOS B.

### 5.4 HIGHWAY 28/55 AND 52 STREET

Two options were developed for the intersection of Highway 28/55 and 52 Street. Option one recommends converting the existing two-way stop intersection to a four-way (all-way) stop intersection. Option two recommends installing traffic signals at the intersection. Both options recommend a channelized right turn lane on the northbound approach since the right turn volumes exceed 60 vehicles per hour in the p.m. peak hour.

With the recommended Option 1, the intersection is expected to operate at LOS C, with a maximum v/c ratio of 0.71 and intersection delays of 16.5 seconds. All intersection approaches are expected to operate at LOS C or better.

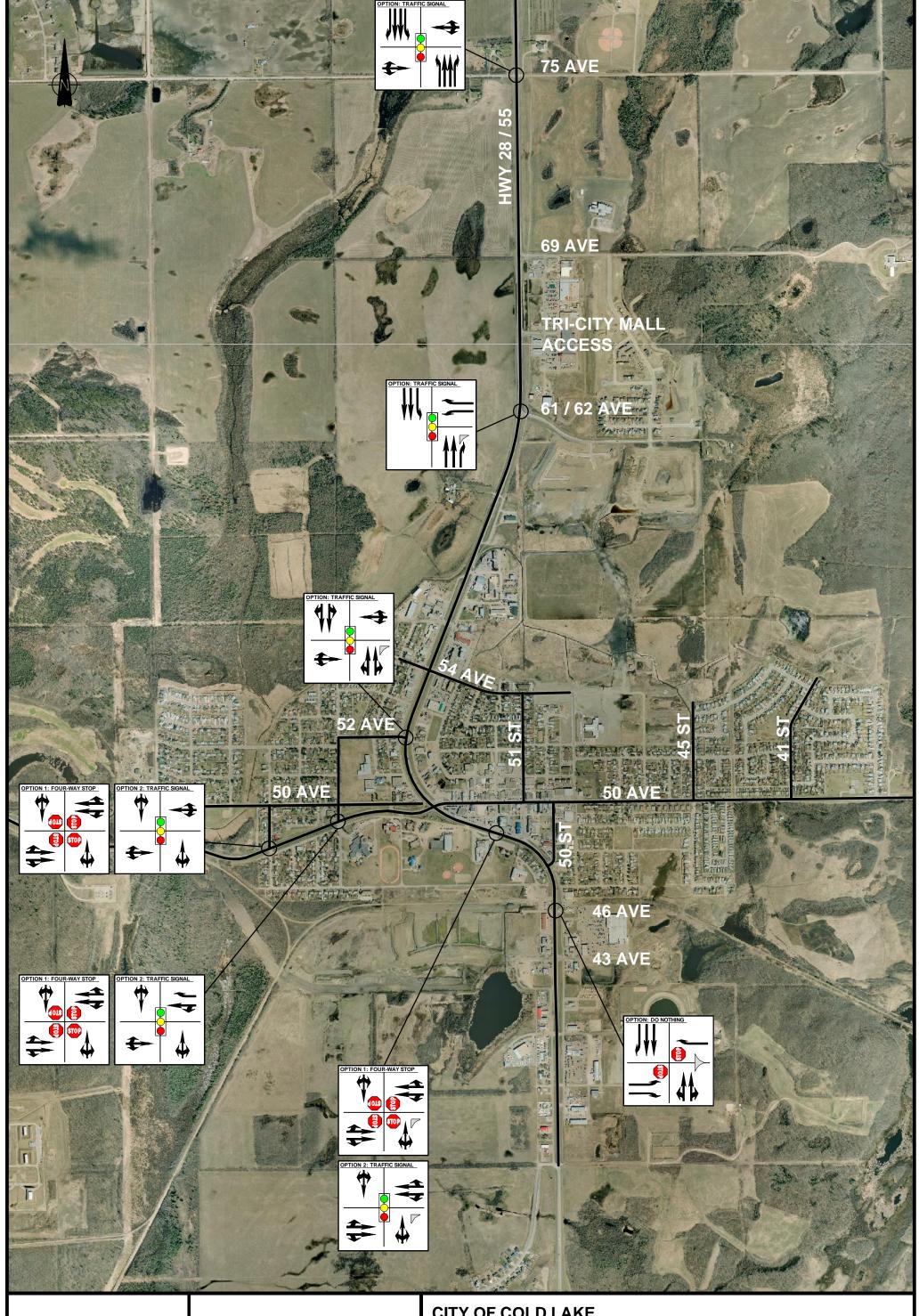
With the recommended Option 2, the intersection is expected to operate at LOS B, with a maximum v/c ratio of 0.66 and intersection delays of 11.8 seconds. All intersection approaches are expected to operate at LOS B or better.

AE recommends Option 2 because a four-way stop is undesirable along Highway 28 and because a signalized intersection will result in a higher LOS.

### 5.5 HIGHWAY 28/55 AND 46 AVENUE

Highway 28/55 and 46 Avenue is currently operating well overall, at LOS A and with a maximum v/c ratio of 0.22. The intersection requires improvement because the eastbound left turn movement is currently operating at LOS D. To improve the operation of the eastbound left turn movement, traffic signals would be required to provide the crossing opportunities required to reduce the delays on the eastbound approach.

At this point in time, AE does not recommend installing traffic signals at this intersection. At LOS D, the eastbound left turn movement is operating fairly and is not actually failing. This intersection should be monitored and traffic signals should be installed when the eastbound left turn movement deteriorates to LOS F.





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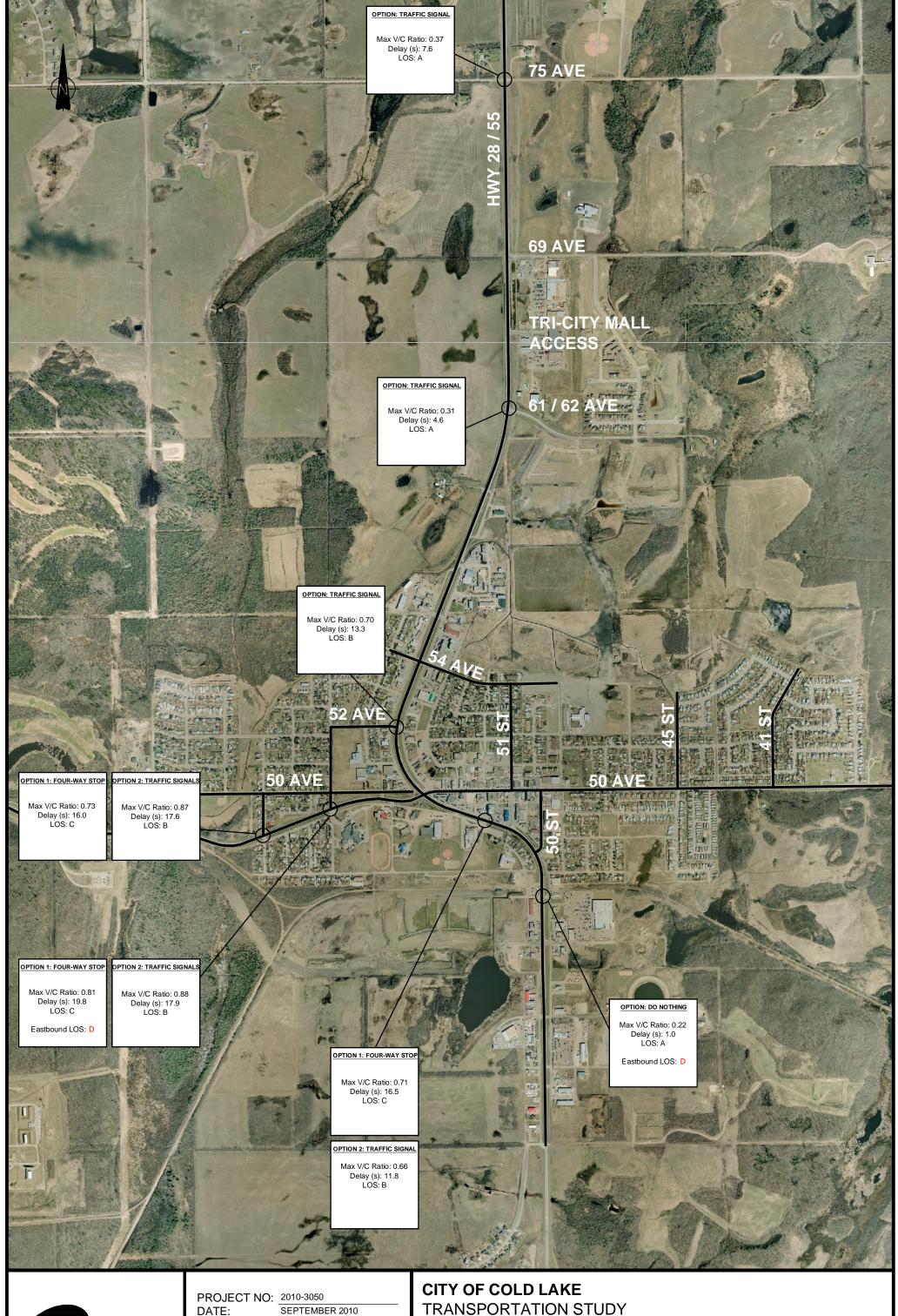
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**CITY OF COLD LAKE** TRANSPORTATION STUDY

FIGURE 5.1 REQUIRED LANE CONFIGURATION AND TRAFFIC CONTROL IMPROVEMENTS





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FIGURE 5.2 2010 CAPACITY RESULTS - REQUIRED LANE CONFIGURATION AND TRAFFIC CONTROL IMPROVEMENTS

#### 5.6 CENTRE AVENUE AND 59 STREET

Two options were developed for the intersection of Centre Avenue and 59 Street. Option one recommends widening Centre Avenue, between 59 Street and Highway 28/55, to provide two lanes in each direction and converting the existing two-way stop control to a four-way (all-way) stop control at the intersection of Centre Avenue and 59 Street. Option two recommends maintaining the existing lane configuration along Centre Avenue but providing traffic signals at Centre Avenue and 59 Street.

With the recommended Option 1, the intersection is expected to operate at LOS C, with a maximum v/c ratio of 0.73 and intersection delays of 16.0 seconds. All intersection approaches are expected to operate at LOS C or better.

With the recommended Option 2, the intersection is expected to operate at LOS B, with a maximum v/c ratio of 0.87 and intersection delays of 17.6 seconds. All intersection approaches are expected to operate at LOS C or better.

AE recommends Option 2 because a four-way stop is undesirable along Centre Avenue and because a signalized intersection will result in a higher LOS.

#### 5.7 CENTRE AVENUE AND 57 STREET

Similar to Centre Avenue and 59 Street, two options were developed for the intersection of Centre Avenue and 57 Street. Option one recommends widening Centre Avenue, between 57 Street and Highway 28/55, to provide two lanes in each direction and converting the existing two-way stop control to a four-way (all-way) stop control at the intersection of Centre Avenue and 57 Street. Option two recommends maintaining the existing lane configuration along Centre Avenue but providing traffic signals at Centre Avenue and 57 Street.

With the recommended Option 1, the intersection is expected to operate at LOS C, with a maximum v/c ratio of 0.81 and intersection delays of 19.8 seconds. With the exception of the eastbound left turn movement, all intersection approaches are expected to operate at LOS C or better. The eastbound left turn movement is expected to operate at LOS D.

With the recommended Option 2, the intersection is expected to operate at LOS B, with a maximum v/c ratio of 0.88 and intersection delays of 17.9 seconds. All intersection approaches are expected to operate at LOS C or better.

AE recommends Option 2 because a four-way stop is undesirable along Centre Avenue and because a signalized intersection will result in a higher LOS.





### **Summary and Recommendations**

The following intersection improvements are recommended at the study intersections which are operating below the acceptable LOS level under the existing (2010) horizon. The recommendations at the study intersections along the Highway 28 corridor are above and beyond the upgrades that will be implemented as part of the Highway 28 twinning project.

#### 6.1 HIGHWAY 28/55 CORRIDOR

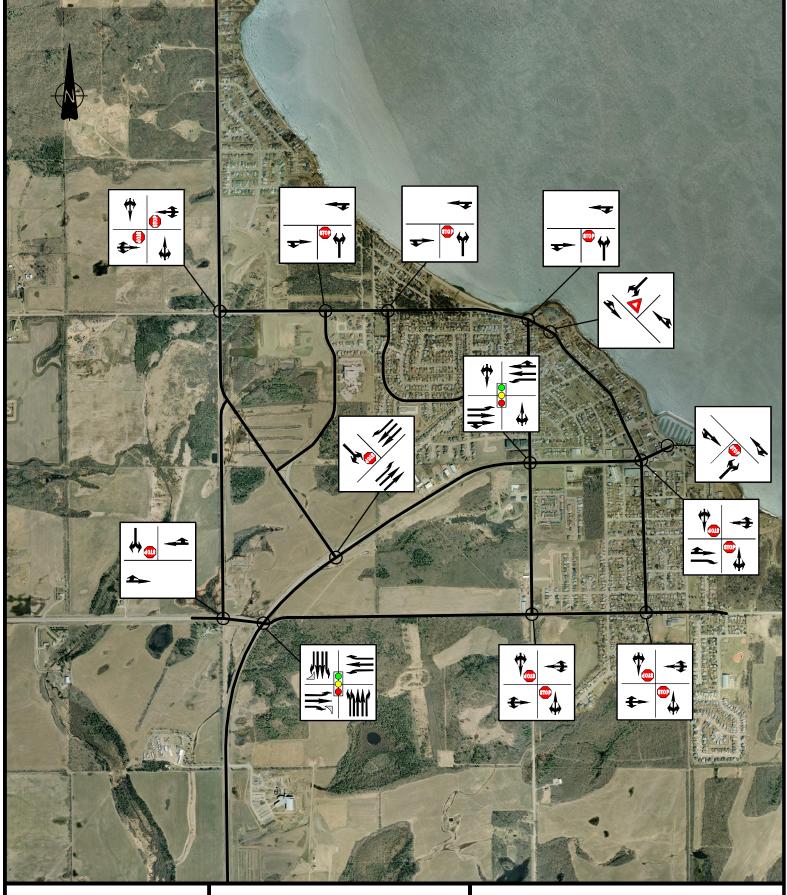
- Highway 28/55 and 75 Avenue: Signalize intersection
- Highway 28/55 and 61/62 Avenue: Signalize intersection and provide channelized northbound right turn lane.

#### 6.2 COLD LAKE SOUTH

- Highway 28/55 and 52 Avenue: Signalize intersection and provide channelized northbound right turn lane
- Highway 28/55 and 52 Street: Signalize intersection and provide channelized northbound right turn lane
- Centre Avenue and 59 Street: Signalize intersection
- Centre Avenue and 57 Street: Signalize intersection.

Figure 6.1 through Figure 6.4 presents the recommended lane configuration and traffic control for Cold Lake North, the Highway 28/55 Corridor, Cold Lake South and Medley, respectively.







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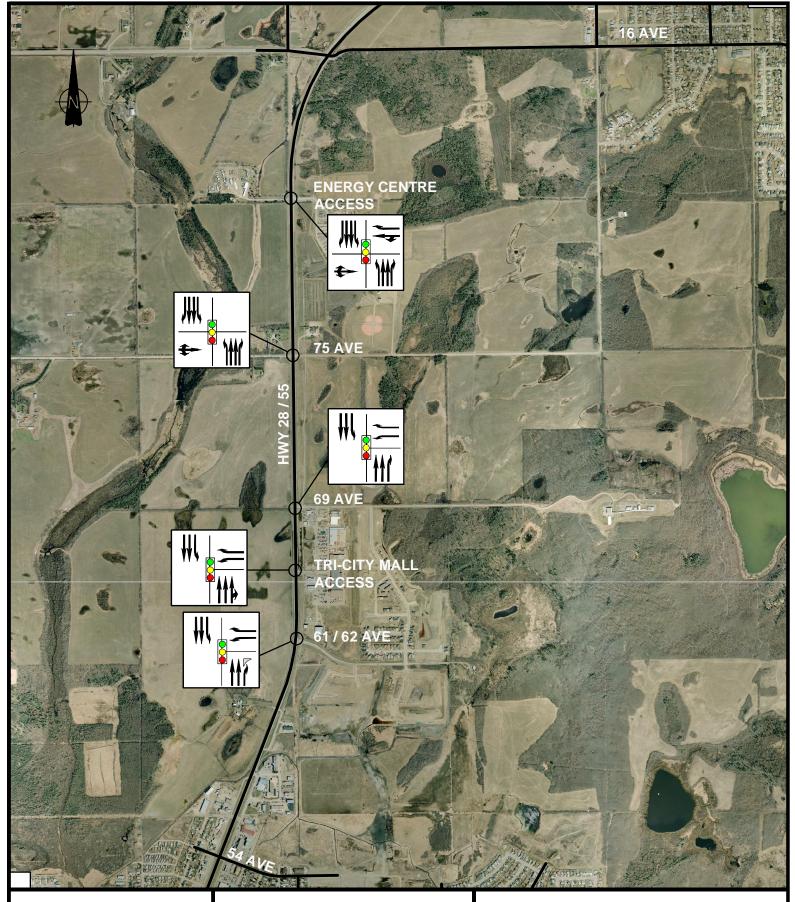
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# CITY OF COLD LAKE TRANSPORTATION STUDY

FIGURE 6.1 - COLD LAKE NORTH RECOMMENDED LANE CONFIGURATION AND TRAFFIC CONTROL





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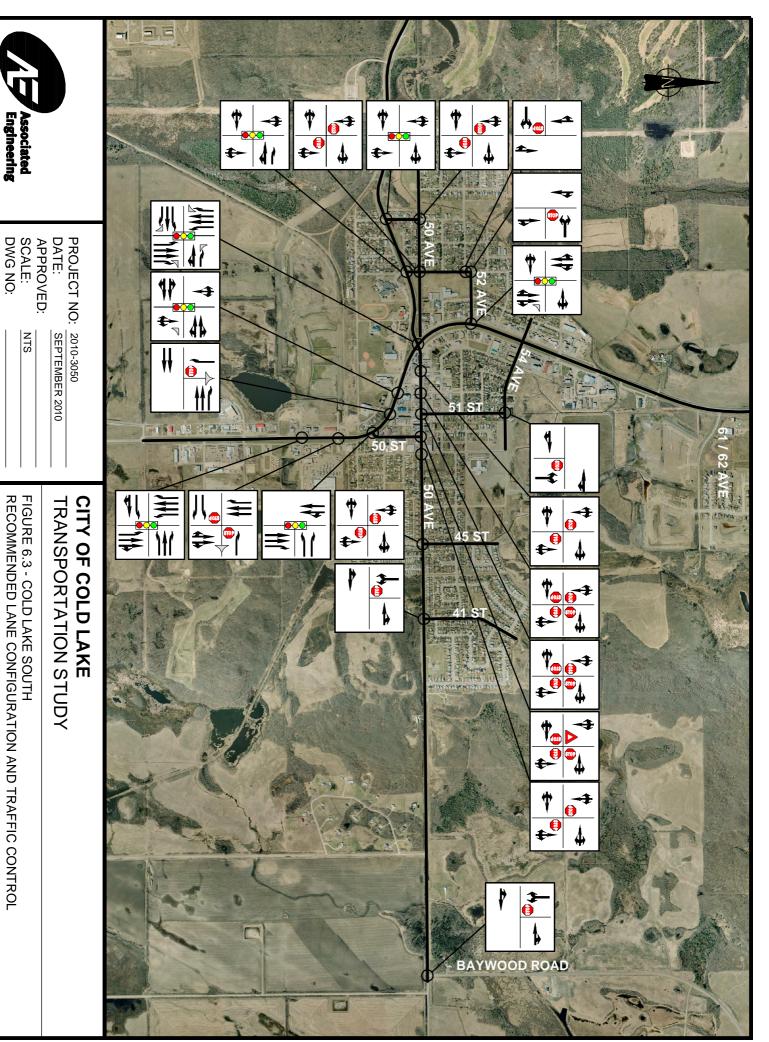
### **CITY OF COLD LAKE** TRANSPORTATION STUDY

FIGURE 6.2 - HIGHWAY 28 / 55 CORRIDOR RECOMMENDED LANE CONFIGURATION AND TRAFFIC CONTROL

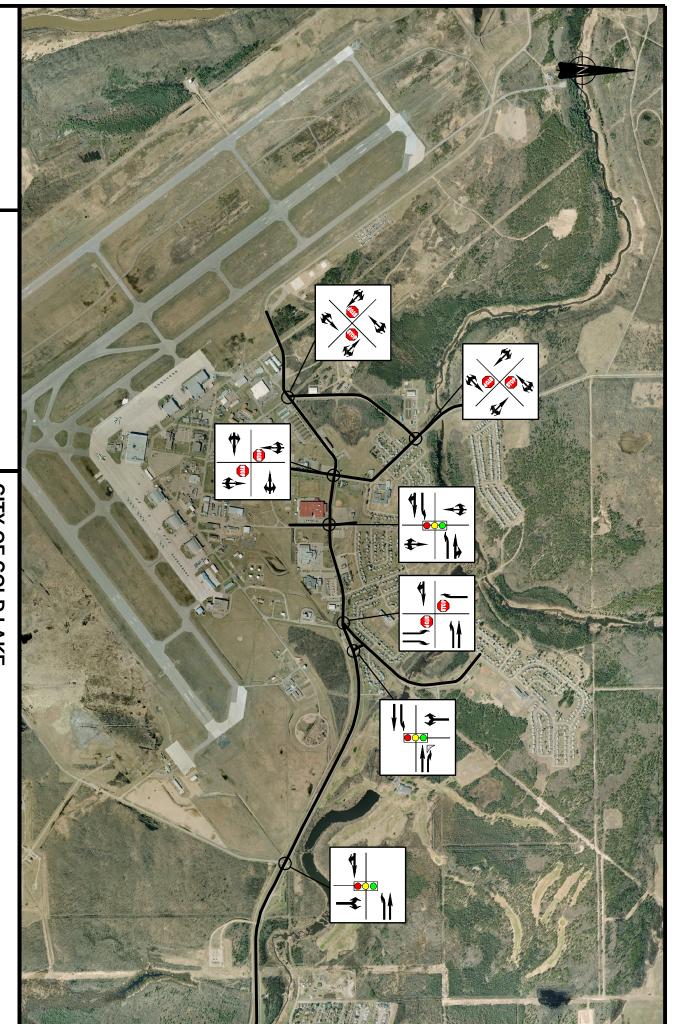


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FIGURE 6.3 - COLD LAKE SOUTH RECOMMENDED LANE CONFIGURATION AND TRAFFIC CONTROL







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CITY OF COLD LAKE TRANSPORTATION STUDY

FIGURE 6.4 - MEDLEY RECOMMENDED LANE CONFIGURATION AND TRAFFIC CONTROL

#### TECHNICAL MEMORANDUM

7

### **Conclusion**

Associated Engineering was retained by the City of Cold Lake to complete an update to the City's transportation study. The first task completed for the transportation study update was to evaluate the operational conditions for the existing roadway system. This technical memorandum presented the operational analysis completed for the existing (2010) horizon and the recommended improvements.

Only the major roadways (collector and arterial roads) were analyzed for the transportation study. Traffic operational analyses were completed at the intersections between two collectors, between a collector and an arterial, and between two arterials.

Overall, most of the intersections within the City are currently operating above acceptable levels. The intersections which are not currently operating above acceptable LOS are:

- 8 Avenue and 16 Street
- Highway 28/55 with 75 Avenue
- 28/55 with 61/62 Avenue
- Highway 28/55 and 54 Avenue
- Highway 28/55 and 52 Avenue
- Highway 28/55 and 52 Street
- Highway 28/55 and 46 Avenue
- Centre Avenue and 59 Street
- Centre Avenue and 57 Street.

Several of the intersections listed above are currently being upgraded as part of the Highway 28 Twinning project. These intersections include:

- 8 Avenue and 16 Street
- Highway 28/55 with 75 Avenue
- Highway 28/55 with 61/62 Avenue
- Highway 28/55 and 54 Avenue.

With the intersection improvements that will be implemented as part of the Highway 28 twinning project, the intersections of 8 Avenue and 16 Street and Highway 28/55 and 54 Avenue will operate above acceptable LOS and with low minimum delays. However the intersections of Highway 28/55 with 75 Avenue and 61/62 Avenue will require additional improvements.



### The City of Cold Lake

The following intersections will require additional improvements, above and beyond the Highway 28 Twinning project improvements:

- Highway 28/55 with 75 Avenue
- Highway 28/55 with 61/62 Avenue
- Highway 28/55 and 52 Avenue
- Highway 28/55 and 52 Street
- Highway 28/55 and 46 Avenue
- Centre Avenue and 59 Street
- Centre Avenue and 57 Street.

The recommended lane configuration and traffic control are presented in Figure 6.1 through 6.4 in Section 6 above.

### **TECHNICAL MEMORANDUM**



# **Appendix A - Growth Rate Information**



Source 1: Provided by City of Cold Lake, from 2006 Hwy 28 Traffic Count Summary

Table 1 –	Comparison of October 2003 a in vehicles per		ounts,	
Roadway	Location	Direction	October 2003	June 2004
	North of Highway 55	Northbound	3,785	4,100
	North of Highway 55	Southbound	3,890	4,386
Highway 28 <sup>1</sup>	North of Highway 55	Two-way Total	7,675	8,487
riigiiway 20	South of Highway 55	Northbound	5,796	4,796
	South of Highway 55	Southbound	5,818	5,011
	South of Highway 55	Two-way Total	11,614	9,897
Highway 55 1	West of Highway 28	Two-way Total	3,212	4,713
16th Avenue 1	East of Highway 28	Two-way Total	2,735	1,958
Highway 28 <sup>2</sup>	North of Imperial Park Rd.	Northbound	6,012	-
	North of Imperial Park Rd.	Southbound	6,032	-
	North of Imperial Park Rd.	Two-way Total	12,044	12,565
	South of Imperial Park Rd.	Northbound	5,090	-
	South of Imperial Park Rd.	Southbound	6,069	-
	South of Imperial Park Rd.	Two-way Total	11,159	12,821
Highway 28 <sup>1</sup>	North of Mall Access	Northbound	5,860	4,678
	North of Mall Access	Southbound	5,512	6,441
	North of Mall Access	Two-way Total	11,372	11,209
	South of Mall Access	Northbound	7,101	7,799
	South of Mall Access	Southbound	6,917	7,694
	South of Mall Access	Two-way Total	14,018	15,493
Imperial Park Rd. 2	East of Highway 28	Two-way Total	126	282
Mall Access 2	East of Highway 28	Westbound (outbound)	2,158	2,502
	East of Highway 28	Eastbound (Inbound)	2,074	3,870
	East of Highway 28	Two-way Total	4,232	6,372

<sup>&</sup>lt;sup>1</sup>counts completed using classifier counters <sup>2</sup>counts completed using pneumatic counters

Source 2: Provided by City of Cold Lake, Cold Lake Halley Test 2009

*******	*******	********	*********	******	*******
MALE 10-YEAR	FEMALE 5-YEAR	MALE 5-YEAR		***	
SURVIVAL RATE	SURVIVAL RATE	SURVIVAL RATE		***	
BY AGE GROUP	BY AGE GROUP	BY AGE GROUP		***	
			EXACT AGE	***	(Enter
sM	sF	sM	INTERVALS	***	growth rate
10 x	5 x	5 x	x to x+n	***	below in AJ13)
				***	·
				***	
				***	
ages 0-4	ages 0-4			***	GROWTH RATE
1.0000	1.0000	1.0000	0-4	***	0.170000
1.0000	1.0000	1.0000	5-9	***	
1.0000	1.0000	1.0000	10-14	***	FEMALE
1.0000	1.0000	1.0000	15-19	***	AGE DISCREPANCY
1.0000	1.0000	1.0000	20-24	***	1.366568
1.0000	1.0000	1.0000	25-29	***	
1.0000	1.0000	1.0000	30-34	***	MALE
0.9964	1.0000	1.0000	35-39	***	AGE DISCREPANCY
0.9926	0.9999	0.9964	40-44	***	1.360108
0.9853	0.9919	0.9962	45-49	***	
0.9531	0.9919	0.9891	50-54	***	TOTAL
0.9054	0.9836	0.9637	55-59	***	AGE DISCREPANCY
0.8833	0.9617	0.9395	60-64	***	2.726676
0.8249	0.9419	0.9402	65-69	***	
0.7255	0.9645	0.8774	70-74	***	
0.6838	1.0001	0.8269	75-79	***	
indeterminate	0.9982	0.8443	80-84	***	
indeterminate	indeterminate	indeterminate	85+	***	
=======================================		=======================================		====== ***	

Overall, Cold Lake's population has experienced considerable growth in the past 15 years. This growth has been driven primarily by the 4 Wing Air Force Base, oil and gas exploration as well as tourism related to the lake. Between 1986 and 2001, the population of the three communities that now comprises the City of Cold Lake increased by approximately 23%, as shown in Figure 1 below. The community's average annual growth over those years was 1.5%. Assuming that growth rates increase to an average of 2% annual growth, and assuming the figures in the 2006 Federal Census as a base, a population of 14,909 is projected by 2017. This is estimated increase to 18,174 by 2027 and 22,154 by 2037. This growth rate is approximated by the centre growth line in Figure 2 below (based on 2% growth). If growth continues in the region's oil sands, tourism and service-based industries, Cold Lake could experience an annual growth rate closer to 3%, which could result an estimated population of 30,000 people by 2037.

Figure 1.0: City of Cold Lake Historic Population Growth

	1984	1986	1988	1990	1992	1994	1996	1996*	2001*
Town of Cold Lake	2,515	3,059	3,445	3,604	3,941	4,250	4,250		
Town of Grand Centre	3,195	3,506	3,655	3,715	3,877	3,990	3,990		
4 Wing	n/a	n/a	n/a	n/a	n/a	n/a	3,550		
City of Cold Lake (after amalgamation)								11,791	11,520

Source: Ministry of Municipal Affairs, Official Population Lists

\* Statistics Canada 1996/2001 Federal Census

32,000 30,000 28,000 24,000 22,000 18,000 18,000

2017

13,378

14,909

16.598

Figure 2.0: City of Cold Lake Projected Population Growth 2007 – 2037

Based on 2006 census figures, for the purposes of this Municipal Development Plan the City of Cold Lake has established a target population of 30,000 by the year 2037.

2022

14,060

16,461

19,242

2027

14,778

18,174

22,307

2032

15,531

20,066

25.860

2037 16,324

22,154

29,978

14,000 12,000 10,000

-1.00%

2.00%

3.00%

2007

12,111

12,231

12,351

2012

12,729

13,504

14,318

#### 2.3.5 Historic Sites

Alberta Culture and Community Spirit (ACCS) classifies Heritage Resource Sites into one of six categories: archaeological, cultural, geological, historic period, natural and palaeontological. Within these classifications, the sites are given a Historical Resource Value (HRV). The highest level of protection (HRV 1) is afforded to lands that have been designated under the *Historical Resources Act* as Provincial Historic Resources. An HRV of 1 is also used to identify lands owned by ACCS for historic resource protection and promotion purposes.

Based on the inventory provided by the Historic Resource Management Branch at Alberta Community Development, there are four known archaeological sites within the in the IDP area and many areas of high archaeological resource sensitivity. The Historic Resource Management Branch maintain a listing of historic resources that is updated twice a year on our website:

http://culture.alberta.ca/heritage/resourcemanagement/landuseplanning/default.aspx.

### 2.4 Population and Growth Forecasts

In order to forecast the demand for land for expansion of existing usages and to accommodate proposed usages within the IDP area, a population projection was prepared. A high, moderate and low growth rates were assumed as shown in Figure 1 - M.D. of Bonnyville #87 Population Projection Chart based on 2006 Population Census and Figure 2 - City of Cold Lake Population Projection Chart based on 2006 Population Census. Current population contained within the M.D. of Bonnyville (excluding the Town of Bonnyville) is approximately 22,200 people.

#### 2.4.1 M.D. of Bonnyville #87

The M.D. of Bonnyville grew from 8,977 to 9,473 between 1996 and 2001, a growth rate of 5.5% in five years. According to the Federal Census, in 2006 the M.D. of Bonnyville had a population of 10,194. Three population projections have been prepared for the M.D. of Bonnyville. The high projection assumes a 2.5% annual growth rate, envisions the population rising to 17,122 by 2027. The moderate projection assumes a 1.5% annual growth rate resulting in a 2027 population of 13,936. The low projection for the IDP area assumes a 0.5% annual growth rate. This projection would result in a 2027 population of 11,320.

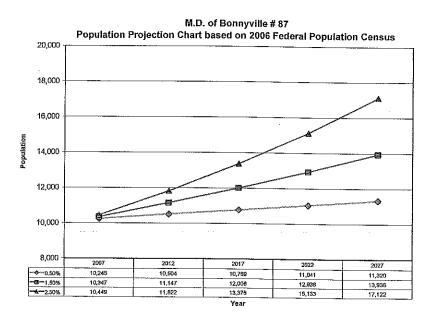


Figure 1 - M.D. of Bonnyville #87 Population Projection Chart based on 2006 Federal Population Census

Based on the 2006 Federal Community Profile, the IDP area had an estimated population of 1% of the M.D. of Bonnyville total population. Without urban expansion of the City of Cold Lake, the population within the IDP area is likely to experience no significant increase in the short-term.

#### 2.4.2 City of Cold Lake

The City of Cold Lake has experienced considerable growth in the past fifteen years, the average growth being 1.5 %. Three population projections have been prepared for the 20-year period ending in 2037. The high projection of 3% annual growth is consistent with the Cold Lake MDP. In this instance, the population is projected to rise to 22,307 by 2027, an increase of 9,956 people. The moderate projection assumes a growth rate of 2%. With this assumption the City's population is projected to increase to 18,174, an increase of 5,943. The low projection assumes an annual growth rate of 1%. Based on this assumption the City's population is projected to be 14,778 in 2027, an increase of 2,667.

It is likely that the IDP area growth rate would tend to be close to the City of Cold Lake's projections given its proximity to the City of Cold Lake.

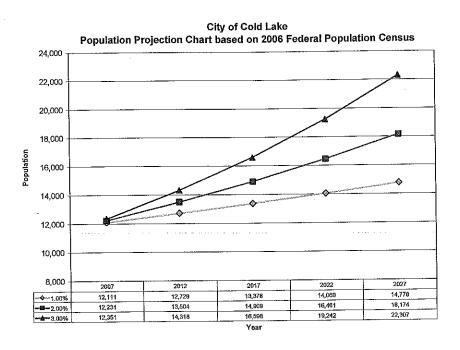


Figure 2 - City of Cold Lake Population Projection Chart based on 2006 Federal Population Census.

#### 2.4.3 Demand for Land to Accommodate Residential Growth

Land requirements need to assume a factor for grossing up the lot size to accommodate the need for roadways and municipal and environmental reserve requirements. Lots in the M.D. of Bonnyville will tend to be larger than in the City of Cold Lake. The resulting additional land required to accommodate residential growth within the IDP area will depend on urban expansion of the City of Cold Lake.

### 2.5 Existing Zoning

Map 4 - Existing Land Use shows land use within the IDP area as of June, 2006. They are summarized as follows.

### 2.5.1 City of Cold Lake Land Use Bylaw 301-LU-07

There are several land uses within the City of Cold Lake bordering the IDP boundary. *Map 4 - Existing Land Use* shows current land uses within the IDP area. The most important one is the Arterial Commercial (C2) for its location, primarily along Highway 28.



# **Appendix B - Design Criteria for Capacity Analysis**

City of Cold Lake Transportation Study Project No: 2010-3050 Date: July 5, 2010

#### TRAFFIC ANALYSIS CRITERIA

A micro model using Synchro/SimTraffic 7.0 will be developed to identify and review intersection capacity needs. Level of service will be used as the common reference in terms of average delay times categorized into six general grades. Table 1.1 defines the LOS criteria for signalized intersections and unsignalized intersections.

**Table 1.1 Level of Service Definitions** 

Level of Service (LOS)	Overall Average Delay at Unsignalized Intersection	Overall Average Delay at Signalized Intersection
А	≤ 10 seconds	≤ 10 seconds
В	> 10 and ≤ 15 seconds	> 10 and ≤ 20 seconds
С	> 15 and ≤ 25 seconds	> 20 and ≤ 35 seconds
D	> 25 and ≤ 35 seconds	> 35 and ≤ 55 seconds
E	> 35 and ≤ 50 seconds	> 55 and ≤ 80 seconds
F	> 50 seconds	> 80 seconds

The minimum LOS criteria recommended by Associated Engineering (AE) is LOS C for the overall intersection. Additionally, each specific movement is targeted to achieve a LOS C or better in all cases. To achieve improved levels of service, the following criteria are proposed where applicable in the traffic network model:

- Right turn channelization (yield condition) provided when turning movements exceed 60 vehicles per hour.
- Right turn bays provided to satisfy LOS E or queuing issues in right or through movements.
- Left and right turn bay lengths provided based on 95<sup>th</sup> queue lengths from Synchro with a minimum storage length of 60 meters.
- Double left turn lanes provided when turning volumes significantly exceed 300 vehicles per hour and LOS or v/c ratios are above the stated minimums.

Table 2.2 presents the recommended traffic analysis assumptions that will be used in the Synchro model. The table also presents assumptions used by four different municipalities within Alberta including the Regional Municipality of Wood Buffalo (RMWB), the City of Calgary, the City of Lethbridge and the City of Medicine Hat. The assumptions used by the RMWB were developed by AE for a specific project.

**Table 2.2 Traffic Analysis Assumptions for Synchro** 

	Traffic Analysis Parameters								
Parameter	RMWB*	City of Calgary	City of Lethbridge	City of Medicine Hat	Recommended				
Link Speed	peed Existing posted speed limits								
Lane Widths		3.7	7m		3.7m				
Storage Length		Minimu	ım 60m		Minimum 60m				
Adjacent Parking Lanes		Apply data where available							
Lane Window									
Ideal Saturation Flow (vphpl)	1900	1850	1850 1750 1850 (through) 1650 (turning)		1850				
Lost Time	-	Default	Default	Default	Default				
Leading Detector	2m (turning) 10m (through)	8m (left turn) 4m (through)	Default	-	Default				
Trailing Detector	0	2m	Default	-	Default				
Turning Speed	-	Default	Default	Default	Default				
Lane Utilization	-	Default	Default	Default	Default				
Right Turn Factor	-	Default	Default	Default	Default				
Left Turn Factor (protected)	-	Default	Default	Default	Default				
Saturated Flow Rate (protected)	-	Default	Default	Default	Default				

Left Turn Factor (permitted)	-	Default	Default	Default	Default			
Saturated Flow Rate (permitted)	-	Default	Default	Default	Default			
Saturated Flow Rate (RTOR)	-	Default	Default	Default	Default			
Headway Factor	-	Default	Default	Default	Default			
Volume Window								
Conflicting Pedestrian #	-	Apply data where available	Apply data where available	Apply data where available. Minimum = 5.	Apply data where available. Minimum = 5.			
Conflicting Bikes #	-	Apply data where available	Apply data where available	Apply data where available. Minimum = 5.	Apply data where available. Minimum = 5.			
Peak Hour Factor	1.00	1.00	0.88 1.00 (15 min data used)	0.95 – Congested Urban Conditions 0.92 – Current / Base Case Urban Conditions 0.88 – Current / Base Case Undeveloped areas 0.85 – Forecast Case, Local and Collector Roads 0.93 – Forecast Case, Congested Collectors and Minor Arterial Roads 0.95 – Forecast Case, Principal Arterials	0.86			
Growth Factor	1.0	1.0	1.0	1.0	1.0			

	1	1	ı		
Heavy Vehicle (%)	5	Apply data where available. Default 5% (main street), 2% (side street) and 7.5% or greater in industrial areas.	Apply data where available. Default 5% (main street), 2% (side street) and 10% in industrial areas.	Apply data where available. Default 7.5% or greater in industrial areas.	Apply data where available. Default 5% (main street), 2% (side street) and 7.5% or greater in industrial areas.
Bus Blockage (#/hour)	0	Apply data where available	Apply data where available	Apply data where available	Apply data where available
Traffic from Mid- Block (%)	None	Apply data where available	Apply data where available	Apply data where available	Apply data where available
Link OD Volumes	-	Alterations must be documented in detail	Alterations must be documented in detail	Alterations must be documented in detail	Default
Lane Group Flow	-	Default	Default	Default	Default
Vehicle Clearance / Existing Timings	-	Contact City of Calgary - Traffic Signals	Contact City of Lethbridge – Traffic Operations	Minimum Green = 7 seconds on left turns, 10 seconds for through Maximum Time = 20 - 30 seconds on main road	Use existing signal timing where available
		Timing '	Window		
Main Street Minimum Initial	-	20 seconds or pedestrian time, whichever is greater	20 seconds or pedestrian interval, whichever is greater	10 seconds or pedestrian time, whichever is greater	15 seconds or pedestrian interval, whichever is greater
Side Street Minimum Initial	-	10 seconds	10 seconds or minimum pedestrian interval, whichever is greater	10 seconds	12 seconds
Minimum Initial Arrows	-	5 seconds	5 seconds	7 seconds	7 seconds
Minimum Initial Split	-	Default	-	Default	Default

Recall	-	Main Street – Ped. / min. unless on fixed (pretimed) mode. Fixed mode generally used in Downtown / Beltline areas. Minor Street or Turns – No recall.	Main Street – Ped. / min. unless on fixed (pretimed) mode. Minor Street or Turns – No recall.	Main Street – Ped. / min. unless on fixed (pretimed) mode. Fixed mode generally used in Downtown area. Minor Street or Turns – No recall.	Main Street – Ped. / min. unless on fixed (pretimed) mode. Minor Street or Turns – No recall.
		Phasing	Window		
Pedestrian Walk Time	8 seconds	Minimum 8 seconds	Minimum 6 seconds	20 seconds	7 seconds
Pedestrian Clearance Time (Don't Walk)	11 seconds	Contact City of Calgary – Traffic Signals	Minimum value derived from actual crossing distance (m) divided by walking speed of 1.2 m/s. In areas with high senior citizens, walking speed of 1.0 m/s should be used.	Pedestrian walk time plus 7 seconds (27 seconds)	17 seconds
Pedestrian Calls (#/hr)	5	Apply data where available	Apply data where available	Apply data where available. Minimum = 5.	Apply data where available. Minimum = 5.
Minimum Splits for Arrows	-	10 seconds plus clearance. In extreme cases 8 seconds plus clearance for prot / perm arrows, 9 seconds plus clearance for prot only arrows.	10 seconds plus clearance. In extreme cases 8 seconds plus clearance for prot / perm arrows, 9 seconds plus clearance for prot only arrows.	10 seconds plus vehicle clearance	10 seconds plus vehicle clearance
Dual Entry	Yes	Yes	Yes	Yes	Yes

Inhibit Max	Yes	Contact City of Calgary – Traffic Signals	Default	No. Contact City of Medicine Hat – Municipal Engineering.	Yes
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<sup>\*</sup>Project specific to the West Loop Road Project

All other factors will be set at the default or calculated values.

#### General comments:

- If an arrow (protected) phase is found to be needed in one peak period, it will be included in the signal phasing in the analysis of all peak hours.
- Summary sheets will include v/c ratios, level of service values and 95<sup>th</sup> queue lengths.

### **TECHNICAL MEMORANDUM**



# Appendix C - Synchro Results

Project: Cold Lake Transportation Study Project No: 2010-3050

Date Revised: April 5, 2011

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
		-		Overall Intersed	ction		0.13	5.1	A	
			ED	Left	LTD	11	0.09	11.8	В	2.3
			EB	Through	LTR	27	0.09	11.8	В	2.3
				Right Left		5 11	0.09 0.13	11.8 10.5	B B	2.3
	1 Avenue & 28	Unsignalized	WB	Through	LTR	21	0.13	10.5	В	3.7 3.7
101	Street / English	Stop Control - EB/WB	VVD	Right	LIK	55	0.13	10.5	В	3.7
101	Bay Road	Approaches		Left		6	0.00	0.0	A	0.1
	Day Hoad	7.551.0001.00	NB	Through	LTR	79	0.00	0.4	A	0.1
				Right		30	0.00	0.4	A	0.1
				Left		42	0.03	0.3	А	0.8
			SB	Through	LTR	62	0.03	3.1	Α	0.8
				Right		5	0.03	3.1	Α	0.8
				Overall Intersed	ction		0.10	4.1	Α	
				Left						
			EB	Through	TR	81	0.06	0.0	Α	0.0
		<u> </u>		Right		11	0.06	0.0	Α	0.0
				Left		66	0.05	0.4	Α	1.3
	1 Avenue & 25	Unsignalized	WB	Through	LT	81	0.05	3.6	A	1.3
102	Street Stop Control - NB Approach		Right							
		ND	Left		1	0.10	9.3	A	2.8	
			NB	Through	LR		0.10			
				Right		83	0.10	9.3	Α	2.8
			SB	Left						
		SB	Through							
				Right Overall Intersec	otion		0.11	1.3	۸	
			Left	T		0.11	1.3	Α		
			EB	Through	TR	116	0.11	0.0	A	0.0
		LD	Right	111	48	0.11	0.0	A	0.0	
			Left		9	0.01	0.0	A	0.2	
	Unsignalized	WB	Through	LT	127	0.01	0.6	A	0.2	
103	103 1 Avenue &	Stop Control - NB		Right						
	Nelson Street	Approach		Left		20	0.05	10.3	В	1.4
			NB	Through	LR					
				Right		13	0.05	10.3	В	1.4
				Left						
			SB	Through						
				Right						
		<u> </u>		Overall Intersed	ction		0.09	1.9	Α	
				Left						
			EB	Through	TR	91	0.09	0.0	A	0.0
				Right		38	0.09	0.0	A	0.0
		Linaina -!:i	WD	Left		7	0.01	0.0	A	0.1
104	1 Avenue & 16	Unsignalized Stop Control - NB	WB	Through	LT	100	0.01	0.5	A	0.1
104	Street	Approach		Right Left		36	0.07	10.2	В	1.9
		дриоасн	NB	Through	LR	טט	0.07	10.2	Ð	1.5
			140	Right	LIX	12	0.07	10.2	В	1.9
				Left		12	0.07	10.2		1.5
			SB	Through						
			- <del>-</del>	Right						
				Overall Intersed	ction		0.05	2.1	Α	
				Left		12	0.01	0.1	Α	0.2
			EB	Through	LT	95	0.01	0.9	A	0.2
				Right						
				Left						
	1 Avenue / 2	Unsignalized	WB	Through						
105	Avenue & 10	Yield Control - SB		Right						
	Street	Approach	<u> </u>	Left						
			NB	Through	TR	75	0.05	0.0	Α	0.0
				Right		4	0.05	0.0	Α	0.0
			_	Left		0	-	-	-	-
I I		S	SB	Through	LR					
				Right		41	0.05	9.2	Α	1.3

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)	
				Overall Intersec	ction		0.07	4.3	Α		
				Left		14	0.07	9.1	Α	1.7	
			EB	Through	LR						
		-		Right		40	0.07	9.1	Α	1.7	
		Unsignalized	WB	Left							
106	8 Avenue &	Stop Control - EB	WD	Through Right						+	
100	Lakeshore Drive	Approach		Left		27	0.02	0.2	Α	0.5	
		7.557.00011	NB	Through	LT	39	0.02	3.1	A	0.5	
				Right				-			
				Left							
			SB	Through	TR	25	0.03	0.0	Α	0.0	
				Right		17	0.03	0.0	Α	0.0	
				Overall Intersec	ction	1	0.21	6.6	Α		
			ED	Left	LTD	7	0.01	0.0	A	0.1	
			EB	Through	LTR	61 29	0.01	0.6	A	0.1	
		Unsignalized		Right Left		37	0.01 0.03	0.6	A A	0.1	
			WB	Through	LTR	55	0.03	2.7	A	0.7	
107	8 Avenue & 10   Stop Contro	Stop Control - NB/SB	***	Right	LIIX	17	0.03	2.7	A	0.7	
-	Street	Street Approaches		Left		17	0.21	11.4	В	6.3	
			NB	Through	LTR	52	0.21	11.4	В	6.3	
				Right		60	0.21	11.4	В	6.3	
				Left		15	0.14	12.2	В	4.0	
			SB	Through	LTR	51	0.14	12.2	В	4.0	
				Right		5	0.14	12.2	В	4.0	
	108 8 Avenue & 16			Overall Intersec	ction	1	0.59	12.0	В		
			EB	Left	LTD	319	0.27	2.9	A	8.7	
			ED	Through Right	LTR	247 53	0.27 0.27	5.8 5.8	A A	8.7 8.7	
				Left		4	0.00	0.0	A	0.1	
		Unsignalized	WB	Through	LTR	110	0.00	0.3	A	0.1	
108		Stop Control - NB/SB		Right		28	0.00	0.3	A	0.1	
	Street	Approaches NB			Left		32	0.59	77.7	F	22.5
			Through	LTR	18	0.59	77.7	F	22.5		
			Right		4	0.59	77.7	F	22.5		
				Left		27	0.47	30.1	D	19.1	
				SB	Through	LTR	8	0.47	30.1	D	19.1
				Right Overall Intersed	ntio n	74	0.47	30.1	D	19.1	
		-		Left	l	119	0.31 0.12	3.8 1.4	A A	3.2	
			EB	Through	LT	422	0.12	2.9	A	3.2	
			LD	Right	L	722	0.12	2.3	^	3.2	
		ļ		Left							
	Highway 20 9 25	Unsignalized	WB	Through	TR	273	0.21	0.0	Α	0.0	
109	Highway 28 & 25 Street	Stop Control - SB		Right		30	0.21	0.0	Α	0.0	
	Gueet	Approach		Left							
			NB	Through							
				Right		40	0.04	40.7	,	400	
			SB	Left Through	LR	10	0.31	13.7	В	10.6	
			SD	Right	LK	151	0.31	13.7	В	10.6	
				Overall Intersed	ction	131	0.31	1.8	A	10.0	
				Left		43	0.12	0.4	A	0.9	
			EB	Through	LT	293	0.04	1.3	A	0.9	
				Right			-	-			
				Left							
	Highway 55 & 28	Unsignalized	WB	Through	TR	150	0.12	0.0	Α	0.0	
110	110 Street/ English	Stop Control - SB		Right		23	0.12	0.0	Α	0.0	
	Bay Road	Approach		Left							
			NB	Through							
				Right		20	0.40	12.6	L.	2.6	
			SB	Left Through	LR	32	0.10	13.6	В	2.6	
		SD		LK	7	0.10	13.6	В	2.6		
					Right	l		U. 1U	13.0		2.0

Highway 28 & Right   Figure	Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
Highway 28 & Highway 28 & Agrovaches   EB					Overall Interse	ction		0.56	12.0	В	
Highway 28 & Highway 56 / 16   Avenue   Highway 56 / 16   Avenue   Highway 56 / 16   Avenue   Highway 28 & Highway 28 & Highway 56 / 16   Avenue   Highway 28 / 15   Avenue   A to 2   Avenue   Highway 28 / 15					Left		145	-	-	-	-
Highway 28 & Highway 55 / 16   Avenue   Highway 55 / 16   Avenue   Highway 56 / 16   Avenue   Highway 56 / 16   Avenue   NB   Signalized   NB   Right   Si   Si   Si   Si   Si   Si   Si   S				EB	Through	LT	31	0.43	15.0	В	29.8
Highway 28 / 5   Approaches   WB					Right	R	149	0.24	3.3	Α	8.5
Highway 55 / 16   Avenue   A					Left		133	-	ı	-	-
Avenue				WB	Through	LTR		0.47	14.2	В	30.6
NB	111	Highway 55 / 16	Signalized 1		Right		51	-	-	-	-
Right   R   118   0.20   3.2   A   7.3		Avenue			Left	L	88	0.29	13.3		15.7
112   16 Avenue & 16   Street     SB				NB	Through	T					
112   16 Avenue & 10   SB					Right	R					
Right R 75					Left	L		0.17	11.6		9.0
112				SB	Through			0.49		В	43.2
112							75	0.13	3.5	Α	6.0
112					Overall Interse	ction			2.0		
112								0.01	0.1	Α	0.3
16 Avenue & 16   Street   Unsignalized Stop Control - NB/SB   Approaches   NB				EB	Through	LTR	137	0.01	0.8	Α	0.3
16 Avenue & 16   Street   St					Right			0.01	8.0	Α	0.3
112   16 Avenue & 10   Sizinet   Stop Control - NB/SB   Approaches   NB					Left						
Street		16 Avenue & 16		WB	Through	LTR	63	0.00	0.0	Α	0.0
Approaches	112		Stop Control - NB/SB		Right			0.00	0.0		0.0
Right		Olicot	Approaches		Left			0.03	11.1	В	0.7
SB				NB	Through	LTR	5	0.03	11.1	В	0.7
SB					Right		0	-	ı	-	-
Right					Left		12	0.04	10.1	В	1.0
113				SB	Through	LTR	2	0.04	10.1	В	1.0
113					Right		11	0.04	10.1	В	1.0
113   16 Avenue & 10   Street					Overall Interse	ction		0.11	3.7	Α	
16   Avenue & 10   Street   Step Control - NB/SB   Approaches   WB					Left		8	0.01	0.1	Α	0.2
113				EB	Through	LTR	86	0.01	0.6	Α	0.2
113					Right		10	0.01	0.6	Α	0.2
Street					Left		13	0.01	0.1	Α	0.2
Street		16 Avenue 9 10	Unsignalized	WB	Through	LTR	64	0.01	0.9	Α	0.2
Approaches	113		Stop Control - NB/SB		Right		34	0.01	0.9	Α	0.2
Right   B		Sireet	Approaches				6	0.04	10.3	В	29.8 8.5 - 30.6 - 15.7 50.0 7.3 9.0 43.2 6.0 - 0.3 0.3 0.3 - 0.0 0.0 0.7 0.7 - 1.0 1.0 1.0 1.0 - 1.0 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0
SB				NB	Through	LTR	9	0.04	10.3	В	0.9
SB					Right		8	0.04	10.3	В	0.9
Right					Left		38	0.11	10.8	В	2.9
Highway 28 / 55   Highway 28 / 55   Energy Centre Access   Energy Centre Access   Highway 28 / 55   Energy Centre Acc				SB	Through	LTR	11	0.11	10.8	В	2.9
Highway 28 / 55   Energy Centre Access   Highway 28 / 55   Energy Centre Access   Signalized   Signalized   Signalized   EB							16	0.11	10.8	В	2.9
Highway 28 / 55 & Energy Centre Access    Unsignalized Stop Control - EB/WB Approaches    Highway 28 / 55 & Energy Centre Access    Unsignalized Stop Control - EB/WB Approaches    WB Through LTR 10					Overall Interse	ction		0.70	12.6	В	
Highway 28 / 55   Signalized 2   Signalized 2   Signalized 2   WB					Left		10	-	-	-	-
Highway 28 / 55   Signalized				EB	Through	LTR	10	0.09	12.5	В	1.0  0.2  0.2  0.2  0.2  0.2  0.2  0.9  0.9
Highway 28 / 55 & Energy Centre Access   Signalized 2   WB					Right		10	-	'n	-	-
Signalized   Signalized   Signalized   Right   1111					Left	L	131	0.46	22.3	С	26.6
Access    NB				WB		TR		0.30	6.9	A	11.5
NB	201	& Energy Centre	Signalized <sup>2</sup>		Right			-	-	-	-
Right R 72 0.09 2.7 A 5.8		Access	[		Left		10	-	-		-
Left   61   0.26   10.7   B   13.7     Through   L   526   0.57   10.8   B   88.4     Right   TR   10   -   -   -   -     Right   TR   10   -   -   -   -     Right   TR   10   -   -   -     SB				NB	Through	LT	645	0.70	14.5	В	#132.6
SB					Right	R					
Right TR   10   -   -   -   -   -   -   -   -   -					Left			0.26	10.7		13.7
Highway 28 / 55 & 75 Avenue   Highway 28 / 55 Avenue   High				SB	Through	L	526	0.57	10.8	В	88.4
Highway 28 / 55 & 75 Avenue   Highway 28 / 55 Ave							10	-	-	-	-
Highway 28 / 55 & 75 Avenue   Highway 28 / 55 Avenue   High					Overall Interse	ction		1.61	26.8	D	
Highway 28 / 55   8 / 75 Avenue   Highway 28 / 55   WB   Highway 28 / 55   WB   Highway 28 / 55   Control - EB/WB   Approaches   WB   Highway 28 / 55   Control - EB/WB   Approaches   Highway 20 / 55   Control - EB/WB   Approach					Left		0		-	-	-
Highway 28 / 55   WB   Stop Control - EB/WB Approaches   WB   Control - EB/WB Approaches   Control - EB/WB Approach				EB	Through	LTR	10	0.24	24.6	С	7.3
202 Highway 28 / 55 & 75 Avenue Highway 28 / 5 Avenue Highway 28					Right		40	0.24	24.6	С	7.3
202 Highway 28 / 55 & 75 Avenue Highway 28 / 55 & 75 Avenue Approaches  Unsignalized Stop Control - EB/WB Approaches  WB Through LTR 10 1.61 440.6 F 74.4  Right 10 0.01 0.3 A 0.3  Through LTR 717 0.01 0.4 A 0.3  Right 20 0.01 0.4 A 0.3  Left 0  SB Through LTR 657 0.00 0.0 A 0.0							71	1.61	440.6		74.4
8 75 Avenue		Highway 29 / FF			Through	LTR		1.61		F	74.4
Approaches  NB  Left 10 0.01 0.3 A 0.3  Through LTR 717 0.01 0.4 A 0.3  Right 20 0.01 0.4 A 0.3  Left 0 SB Through LTR 657 0.00 0.0 A 0.0	202	2 75 Avanua	Stop Control - EB/WB		Right		10	1.61	440.6	F	74.4
NB Through LTR 717 0.01 0.4 A 0.3  Right 20 0.01 0.4 A 0.3  Left 0  SB Through LTR 657 0.00 0.0 A 0.0		a 15 Avenue	Approaches		Left			0.01	0.3	Α	0.3
Right         20         0.01         0.4         A         0.3           Left         0         -				NB	Through	LTR	717				
Left         0         -											
SB Through LTR 657 0.00 0.0 A 0.0											
				SB T		LTR		0.00	0.0	Α	0.0
					Right		10	0.00	0.0	A	0.0

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Interse	ction		0.69	14.3	В	
				Left						
			EB	Through						
				Right		_			-	
			WD	Left	L	7	0.02	18.1	В	3.5
203	Highway 28 / 55	Signalized <sup>1</sup>	WB	Through	R	104	0.05	6.2	۸	10.7
203	& 69 Avenue	Signalized		Right Left	K	104	0.25	0.2	A	10.7
			NB	Through	Т	644	0.69	14.8	В	102 4
				Right	R	26	0.03	3.4	A	
				Left	L	118	0.50	17.3	В	27.1
			SB	Through	T	650	0.69	15.0	В	103.8
				Right						
				Overall Interse	ction		0.58	9.7	В	
				Left						
			EB	Through						
				Right		00	0.00	40.0		47.0
	Highway 28 / 55		WB	Left	L	83	0.30	18.9	В	17.6
204	& Tri-City Mall	Signalized <sup>2</sup>	WD	Through Right	R	10	0.04	8.8	A	(m)  3.5  10.7  102.4  3.2  27.1  103.8  17.6  3.0  #126.2  5.8  11.4  105.7  20.2  0.7  0.0  0.0  0.4  0.4  14.0  13.6  5.7  18.2  15.5  16.9  6.3  108.4  -  #57.3  102.3  -  72.3  72.3  72.3  72.3  Fror  Error  Error
204	Access	Signalized		Left	- 1	10	0.04	0.0	Λ	5.0
	7100000		NB	Through	TR	659	0.58	10.3	В	#126.2
				Right		72	0.08	2.3	A	
				Left		55	0.16	7.2	A	
			SB	Through	LT	602	0.53	9.0	Α	
				Right						
				Overall Interse	ction		0.58	2.9	Α	
				Left						
			EB	Through						
				Right					_	
	11:	l la si sa sila s d	WD	Left	L	37	0.58	106.5	F	20.2
205	Highway 28 / 55 & 62 Avenue / 61	Unsignalized Stop Control - WB	WB	Through	R	8	0.02	45.0		0.7
205	Avenue	Approach		Right Left	ĸ	ō	0.03	15.9	С	0.7
	Avenue	Арргоаст	NB	Through	TR	723	0.54	0.0	A	0.0
			110	Right	111	70	0.54	0.0	A	
				Left		9	0.01	0.3	A	
			SB	Through	LT	676	0.01	0.4	A	
				Right						
				Overall Interse	ction		0.86	19.5	В	
				Left	L	51	0.16	20.2	С	14.0
			EB	Through	T	51	0.11	19.3	В	
				Right	R	31	0.08	7.5	Α	
			WD	Left	L	71	0.22	21.1	С	
301	Highway 28 / 55	Cianali 1	WB	Through	T	61 153	0.14	19.5	В	
301	& 54 Avenue	Signalized <sup>1</sup>		Right Left	R L	31	0.34 0.15	8.5 9.1	A A	
			NB	Through	TR	602	0.15	17.6	В	
			.10	Right	111	61	-	-	-	
				Left	L	163	0.86	52.5	D	
			SB	Through	TR	581	0.74	16.5	В	10.7  102.4 3.2 27.1 103.8  17.6  3.0  #126.2 5.8 11.4 105.7  20.2  0.7  0.0 0.0 0.4 0.4 0.4  14.0 13.6 5.7 18.2 15.5 16.9 6.3 108.4 - #57.3 102.3 - 72.3 72.3 72.3 Error Error Error Error Error Error Error 2.3 2.3
				Right		61	-	-	-	
				Overall Interse	ction		3.35	849.3	F	
				Left		30	2.25	812.5	F	72.3
			EB	Through	LTR	30	2.25	812.5	F	
				Right		10	2.25	812.5	F	
				Left	L	50	3.35	Error	F	
000	Highway 28 / 55	Unsignalized	WB	Through	LTR	30	3.35	Error	F	
302	& 52 Avenue	Stop Control - EB/WB		Right		55	3.35	Error	F	
		Approaches	NID	Left	LTTD	70	0.09	1.1	A	
			NB	Through	LTTR	615	0.27	1.3	A	
				Right Left	1	80 140	0.27 0.21	0.0 2.7	A A	
			SB	Through	LTTR	510	0.21	2.7	A	
			35	Right	LIII	40	0.21	0.0	A	
				ragiit	I	_+∪	0.20	0.0		0.0

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Interse	ction		0.76	14.3	В	
				Left	L	375	0.76	22.5	С	76.5
			EB	Through	Т	329	0.42	11.4	В	48.0
				Right	R [C]	135	0.19	2.3	Α	7.1
			WD	Left		9	-	-	-	-
303	Highway 28 / 55	Signalized <sup>2</sup>	WB	Through	LT R [C]	161 155	0.22 0.21	9.3 2.2	A	
303	& 50 Avenue	Signalized		Right Left	K [C]	133	0.40	22.9	A C	
			NB	Through	2T	275	0.40	17.3	В	
			110	Right	R [C]	162	0.01	10.8	В	
				Left	L	116	0.44	23.5	C	
			SB	Through	2T	232	0.28	17.6	В	
				Right	R [C]	5	0.30	4.7	А	11.4
				Overall Interse	ction		0.47	5.1	Α	
				Left		16	0.02	0.2	Α	0.5
			EB	Through	LTTR	486	0.20	0.4	Α	0.5
		<u> </u>		Right		50	0.20	0.0	Α	0.0
				Left		98	0.12	1.4	Α	
004	Highway 28 / 55	Unsignalized	WB	Through	LTTR	523	0.21	1.8	A	
304	& 52 Street	Stop Control - NB/SB		Right		42	0.21	0.0	A	
		Approaches	NB	Left Through	LTD	23 10	0.47 0.47	32.2 32.2	D D	
			IND	Right	LTR	66	0.47	32.2	D	_
		<del> </del>		Left		21	0.41	42.6	E	
			SB	Through	LTR	8	0.41	42.6	Ē	
			OD	Right	LIIX	27	0.41	42.6	E	
				Overall Interse	ction	2,	0.22	0.2	A	11.0
				Left			5.22	0.2		
			EB	Through	2T	634	0.22	0.0	Α	0.0
				Right						
		T		Left						
	Highway 28 / 55	Unsignalized	WB	Through	2T	504	0.17	0.0	Α	0.0
305	& 51 Street	Stop Control - SB		Right	R [C]	23	0.02	0.0	Α	0.0
	G 0 1 0 11 0 0 1	Approach		Left						
			NB	Through						24.4 7.5 29.3 22.8 2.3 33.1 26.7 11.4  0.5 0.5 0.0 3.2 3.2 0.0 18.7 18.7 14.5 14.5 14.5 14.5 0.0  0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
				Right						
			SB	Left Through					-	
			35	Right	R [C]	19	0.03	10.3	В	0.8
				Overall Interse		19	0.03	12.3	В	0.8
		<del> </del>		Left			0.70	12.0		
			EB	Through						
				Right						
				Left	L	239	0.43	16.3	В	44.6
	Highway 28 / 55		WB	Through						
306	& 50 Street	Signalized <sup>1</sup>		Right	R	144	0.25	4.0	А	9.9
	Q 00 011001			Left						
			NB	Through	2T	383	0.29	10.4	В	
				Right	R	188	0.28	2.6	Α	8.2
			SB	Left	1	215	- 0.70	- 10.7	- D	- 47.5
			SB	Through Right	LTT	419	0.70	16.7	В	47.5
-				Overall Interse	ction		0.22	1.0	٨	
		 		Left	L	11	0.22	33.2	A D	2.4
			EB	Through		1 1	0.00	JU.2	<u> </u>	4.7
				Right	R	34	0.07	11.4	В	17
				Left		٠.			_	
	Llighway: 00 / 55	Unsignalized	WB	Through						
307	Highway 28 / 55	Stop Control - EB/WB		Right	R [C]	10	0.02	10.7	В	0.4
	& 46 Avenue	Approaches		Left		33	0.05	0.6	А	
			NB	Through	LTTR [C]	550	0.21	0.8	Α	1.2
		[		Right		32	0.21	0.0	Α	0.0
		[		Left						
Ī			SB	Through	2T	649	0.22	0.0	Α	0.0
				Right	R	10	0.01	0.0	Α	0.0

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay	LOS	95th Queue
			••	Overall Interse	ction		0.30	(s) 9.9	Λ	(m)
		-		Left		78	0.38 0.19		A B	15.0
			EB	Through	L TR	29	0.19	13.1 7.5		
			ED	Right	IK	35	0.12	- 7.5	A -	
		-		Left	-	114	0.28	14.0	В	
			WB	Through	T	49	0.28	11.9	В	
308	Highway 28 / 55	Signalized <sup>1</sup>	WD	Right	R	73	0.08	4.4	A	
000	& 43 Avenue	Signalized		Left	L	19	0.05	8.9	A	
			NB	Through	TTR	464	0.33	9.6	A	
			115	Right	1111	67	-		-	
				Left	-	137	0.38	14.2	В	
			SB	Through	2T	474	0.29	9.7	A	
				Right	R	72	0.10	2.8	A	
				Overall Interse			0.13	4.2	A	0.0
				Left	1		0.10	1.2	,	
			EB	Through						
				Right						
				Left		82	0.13	9.8	Α	3.5
		Unsignalized	WB	Through	LR					
309	52 Avenue & 57	Stop Control - WB		Right		13	0.13	9.8	Α	3.5
	Street (North)	Approach		Left						
		' '	NB	Through	TR	31	0.08	0.0	Α	0.0
				Right	İ	82	0.08	0.0	A	0.0
				Left		9	0.01	0.1	Α	0.2
			SB	Through	LT	20	0.01	2.4	Α	0.2
				Right						
				Overall Interse	ction	•	0.07	2.5	Α	
				Left		46	0.07	10.0	Α	1.8
			EB	Through	LR					
				Right		1	0.07	10.0	Α	1.8
				Left						
	52 Avenue & 57	Unsignalized	WB	Through						
310	Street (South)	Stop Control - EB		Right						
	Sileer (Souri)	Approach		Left		11	0.01	0.1	Α	0.2
			NB	Through	LT	66	0.01	1.1	Α	0.2
				Right						0.0 0.2 0.2 0.2 1.8 1.8 1.8 1.8 0.2 0.2 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
				Left						
			SB	Through	TR	46	0.07	0.0	Α	0.0
				Right		55	0.07	0.0	Α	0.0
				Overall Interse	ction		0.08	6.0	Α	
				Left		2	0.00	0.0	Α	0.0
			EB	Through	LTR	10	0.00	0.7	A	9.5 - 2.2 10.6 7.1 4.2 28.4 - 23.0 26.0 5.0  3.5  3.5  0.0 0.0 0.2 0.2 0.2  1.8  1.8  1.8  1.8  1.8  1.8  1.8  1
				Right		10	0.00	0.7	A	
				Left		12	0.01	0.1	Α	
	50 Avenue & 59	Unsignalized	WB	Through	LTR	14	0.01	2.2	Α	
311	Street	Stop Control - NB/SB		Right		14	0.01	2.2	Α	
	0	Approaches		Left	L	22	0.08	9.5	Α	
			NB	Through	LTR	22	0.08	9.5	Α	
		<u> </u>		Right		19	0.08	9.5	Α	
			-	Left		6	0.02	9.6	A	
			SB	Through	LTR	6	0.02	9.6	A	
				Right	<u>.</u>	1	0.02	9.6	Α	0.5
		<u> </u>		Overall Interse	ction	1 -	0.12	7.2	A	
				Left		22	0.02	0.1	A	
			EB	Through	LTR	5	0.02	3.4	A	
		<u> </u>		Right	<b></b>	22	0.02	3.4	A	
			14.5	Left		8	0.01	0.0	A	
040	50 Avenue & 57	Unsignalized	WB	Through	LTR	8	0.01	1.8	A	
312	Street	Stop Control - NB/SB		Right	<del>                                     </del>	18	0.01	1.8	A	
		Approaches		Left		37	0.12	10.4	В	
			NB	Through	LTR	37	0.12	10.4	В	
				Right	<b></b>	8	0.12	10.4	В	
			65	Left		5	0.06	9.5	A	
	İ		SB	Through Right	LTR	21	0.06	9.5	A A	
					1	21	0.06	9.5		

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Intersed	ction	•	0.26	2.3	Α	` '
				Left		45	0.05	0.9	Α	1.2
			EB	Through	LTR	775	0.05	1.3	A	1.2
		-		Right		17	0.05	1.3	Α	
			W.D.	Left		1	0.00	0.0	A	
040	Centre Avenue &	Unsignalized	WB	Through	LTR	353	0.00	0.0	A	
313	59 Street	Stop Control - NB/SB Approaches		Right Left		13 5	0.00 0.13	0.0 32.7	A D	1.2 1.2 0.0 0.0 0.0 0.0 3.6 3.6 3.6 3.6 7.9 7.9 7.9 1.2 1.2 1.2 1.6 1.6 0.0 7.8 7.8 22.5 22.5 22.5 22.5 22.5 22.5 22.7 2.7 2.7 2.7 2.7 2.7 2.7 2.7 2.7 2.
		Apploacties	NB	Through	LTR	5	0.13	32.7	D	
			ND	Right	LIIX	7	0.13	32.7	D	
		-		Left		17	0.26	44.5	E	
			SB	Through	LTR	5	0.26	44.5	Ē	
				Right		6	0.26	44.5	Е	
				Overall Intersed	ction	•	0.59	5.3	Α	
				Left		44	0.05	0.9	Α	1.2
			EB	Through	LTR	808	0.05	1.3	Α	1.2
		<u> </u>		Right		17	0.05	1.3	Α	1.2
				Left		37	0.06	0.8	Α	
	Centre Avenue &	Unsignalized	WB	Through	LT	351	0.06	1.8	A	(m)  1.2 1.2 1.2 0.0 0.0 0.0 0.0 3.6 3.6 3.6 7.9 7.9 7.9 7.9 1.2 1.2 1.2 1.2 1.6 1.6 0.0 7.8 7.8 22.5 22.5 22.5 22.5 22.5 22.5 22.7  1.8 1.8 1.8 1.8 1.8 1.8 1.8 1.8 1.8 1.
314	57 Street	Stop Control - NB/SB		Right	R	32	0.02	0.0	A	
		Approaches	ND	Left	1.70	5	0.26	33.0	D	
			NB	Through	LTR	6	0.26	33.0	D	
		-		Right		27	0.26	33.0	D	
			SB	Left Through	LTR	26 9	0.59 0.59	81.7 81.7	F F	
			35	Right	LIK	17	0.59	81.7	F	
				Overall Interse	ction	17	0.39	1.8	A	22.3
		<u> </u>		Left			0.13	1.0	Α	
			EB	Through	TR	143	0.13	0.0	Α	0.0
				Right	111	46	0.13	0.0	A	
				Left		3	0.00	0.0	A	
	54.4	Unsignalized	WB	Through	LT	114	0.00	0.2	A	
315	54 Avenue & 51 Street	Stop Control - NB		Right						
	Sireei	Approach		Left		56	0.10	11.1	В	1.2 1.2 0.0 0.0 0.0 0.0 3.6 3.6 3.6 3.6 7.9 7.9 7.9 1.2 1.2 1.2 1.6 1.6 0.0 7.8 7.8 22.5 22.5 22.5 22.5 22.5 22.7 2.7 2.7 2.7 2.7 2.7 2.7 2.7 2.7 2.
			NB	Through	LR					
				Right		1	0.10	11.1	В	2.7
				Left						
			SB	Through						
				Right						
		-		Overall Intersed	ction		0.16	3.1	A	4.0
			EB	Left		71	0.07	0.7	A	
			ED	Through Right	LTR	270 15	0.07 0.07	2.2	Α	1.2 1.2 0.0 0.0 0.0 0.0 3.6 3.6 3.6 3.6 7.9 7.9 7.9 1.2 1.2 1.2 1.2 1.6 0.0 0.0 0.0 0.1 0.1 0.1 0.1 0.1 0.1 0.1
				Left		2	0.07	0.0	A A	
		Unsignalized	WB	Through	LTR	231	0.00	0.0	A	
316	50 Avenue & 53	Stop Control - NB/SB	****	Right	LIIX	19	0.00	0.1	A	
	Street	Approach		Left		14	0.09	18.2	C	
		11	NB	Through	LTR	3	0.09	18.2	Č	
				Right		6	0.09	18.2	C	22.5  0.0  0.0  0.1  0.1  2.7  2.7  1.8  1.8  1.8  0.0  0.0  0.0  2.3  2.3  2.3  4.7
				Left		21	0.16	15.9	С	
			SB	Through	LTR	3	0.16	15.9	С	4.7
				Right		32	0.16	15.9	С	4.7
				Overall Intersed	ction		0.45	11.1	В	
			_	Left		48	0.45	12.0	В	
			EB	Through	LTR	208	0.45	12.0	В	
				Right		18	0.45	12.0	В	
		Uneter-ti-	WP	Left	1.75	20	0.38	11.1	В	
317	50 Avenue & 52	Unsignalized Stop Control - All	WB	Through	LTR	178	0.38	11.1	В	
317	Street	Approaches		Right		39	0.38	11.1	В	1
		Approacties	NB	Left Through	LTR	26 31	0.15 0.15	9.6 9.6	A	
			IND	Right	LIK	24	0.15	9.6	A A	
				Left		47	0.15	10.1	В	
			SB	Through	LTR	35	0.23	10.1	В	
			35	Right	LIIX	48	0.23	10.1	В	
	l .	1		ragin	1	ro .	0.20	10.1		

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Interse	ction		0.47	11.2	В	
				Left		62	0.47	12.5	В	-
			EB	Through	LTR	211	0.47	12.5	В	-
				Right		19	0.47	12.5	В	-
				Left		8	0.33	10.6	В	-
	50 Avenue & 51	Unsignalized	WB	Through	LTR	164	0.33	10.6	В	-
318	Street	Stop Control - All		Right		26	0.33	10.6	В	-
	0001	Approaches		Left		28	0.14	9.5	Α	-
			NB	Through	LTR	28	0.14	9.5	Α	-
				Right		23	0.14	9.5	Α	-
				Left		74	0.23	10.3	В	-
			SB	Through	LTR	32	0.23	10.3	В	-
				Right		24	0.23	10.3	В	-
				Overall Interse	ction	1	0.46	10.9	В	
				Left		0	-	-	-	-
			EB	Through	LTR	193	0.46	11.4	В	
		Unsignalized		Right		114	0.46	11.4	В	-
		Stop Control -		Left		83	0.32	10.4	В	-
	50 Avenue & 50	EB/WB/NB	WB	Through	LTR	112	0.32	10.4	В	
319	Street	Approaches		Right		0	-		-	1
		Assumed Yield Control		Left		83	0.31	10.4	В	-
		- SB Approach	NB	Through	LTR	0	-	-	-	-
		5- 1 <b>- 1</b> - 1		Right		109	0.31	10.4	В	-
				Left		0	-	-	-	-
			SB	Through	LTR	0	-	-	-	-
				Right		0	-	-	-	-
				Overall Interse	ction		0.32	5.5	Α	
				Left		50	0.04	0.4	Α	1.1
			EB	Through	LTR	253	0.04	1.5	Α	1.1
				Right		25	0.04	1.5	Α	1.1
				Left		23	0.02	0.2	Α	0.5
	50 Avenue & 49	Unsignalized	WB	Through	LTR	135	0.02	1.2	Α	0.5
320	Street	Stop Control - NB/SB		Right		29	0.02	1.2	Α	0.5
	Olicci	Approaches		Left		10	0.19	15.5	С	
			NB	Through	LTR	28	0.19	15.5	С	
				Right		31	0.19	15.5	С	5.5
				Left		40	0.32	17.6	С	
			SB	Through	LTR	23	0.32	17.6	С	10.9
				Right		53	0.32	17.6	С	10.9
				Overall Interse	ction		0.08	2.1	Α	
				Left		34	0.03	0.3	Α	
			EB	Through	LTR	261	0.03	1.0	Α	
				Right		35	0.03	1.0	Α	0.7
				Left		7	0.01	0.1	Α	0.2
	50 Avenue & 45	Unsignalized	WB	Through	LTR	146	0.01	0.4	A	
321	Street	Stop Control - NB/SB		Right		4	0.01	0.4	Α	
	Olicei	Approaches		Left		14	0.08	13.6	В	
			NB	Through	LTR	3	0.08	13.6	В	
				Right		14	0.08	13.6	В	
		[		Left		9	0.06	12.3	В	1.6
			SB	Through	LTR	4	0.06	12.3	В	1.6
				Right		15	0.06	12.3	В	1.6
				Overall Interse	ction		0.08	2.9	Α	
		[		Left		72	0.06	0.5	А	
			EB	Through	LT	168	0.06	2.7	Α	1.5
				Right						
		[	WB -	Left						
	50 Avenue & 41	Unsignalized		Through	TR	106	0.08	0.0	Α	0.0
322	Street	Stop Control - SB		Right		8	0.08	0.0	Α	0.0
	Sueet	Approaches	_	Left						
			NB	Through						
				Right						
				Left		14	0.08	10.4	В	2.2
			SB	Through	LR					
				Right		38	0.08	10.4	В	2.2
				. agair	1	- 50	0.00			

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Interse	ction		0.11	5.3	Α	
				Left		82	0.06	0.5	А	1.5
			EB	Through	LT	69	0.06	4.3	A	1.5
		_		Right						
	50 Avenue / Twp	Unaignalizad	WB	Left		00	0.00	0.0		0.0
323	Rd 630 &	Unsignalized Stop Control - SB	VVD	Through Right	TR	26 11	0.03	0.0	A A	
020	"Baywood Road"	Approaches		Left		- 11	0.03	0.0	Α	0.0
	/ RR 20	7.551.0001.00	NB	Through						
				Right						
				Left		18	0.11	9.4	Α	3.0
			SB	Through	LR					
				Right		70	0.11	9.4	Α	3.0
				Overall Interse	ction		0.54	8.4	Α	
			ED.	Left		057	0.54	0.4		400.0
			EB	Through Right	TR	657 6	0.54	9.4	A	
		-		Left	1	5	0.01	6.4	A	
			WB	Through	T	268	0.22	5.8	A	(m) 1.5 1.5 0.0 0.0
401	Kingsway &	Signalized 1		Right			0.22	0.0		00
	Medley Road	3		Left		16	0.11	10.9	В	8.0
			NB	Through	LR					
				Right		20	-	-	-	-
			0.5	Left						
			SB	Through					<b>!</b>	
				Right Overall Interse	otion		0.74	12.6	Р	
		-		Left	i i	153	0.74 0.33	13.6 10.5	B B	21.7
			EB	Through	T	608	0.33	17.1	В	
				Right		000	0.7 1	17.1		02.0
				Left						
	Kingsway &		WB	Through	Т	261	0.32	9.4	Α	30.9
402	Glenwood Drive	Signalized <sup>1</sup>		Right	R [C]	107	0.15	2.0	А	5.4
	(East)			Left						
			NB	Through						1.5  0.0 0.0 0.0 3.0 3.0 3.0 102.9 1.7 31.5 8.0 21.7 92.6 30.9 5.4 29.5 0.0 0.0 0.0 0.2 0.0 0.0 0.2 0.0 1.1 1.1 1.1 4.4 37.6 3.0 8.1 9.3
		-		Right		404	0.00	40.0	-	
			SB	Left Through	LR	131	0.29	18.9	В	29.5
			35	Right	LIX	1	-	-	-	_
				Overall Interse	ction		0.51	0.4	А	
				Left						
			EB	Through	TR	749	0.51	0.0	Α	0.0
				Right		1	0.51	0.0	Α	1.5  0.0  0.0  3.0  3.0  3.0  102.9  - 1.7  31.5  8.0  - 21.7  92.6  30.9  5.4  29.5  - 0.0  0.0  0.0  0.2  0.0  1.1  1.1  1.1
				Left	L	5	0.01	9.8	Α	
400	Kingsway &	Unsignalized	WB	Through	Т	257	0.18	0.0	Α	0.0
403	Glenwood Drive (West)	Stop Control - NB/SB Approaches		Right Left	-		0.04	20.6	С	1.4
	(vvest)	Approacties	NB	Through	L	6	0.04	20.6	U	1.1
			ואט	Right	R	11	0.04	20.6	С	11
				Left				_5.0		
			SB	Through						
				Right	R	0	-	-	-	-
			· ·	Overall Interse	ction		0.45	10.2	В	
				Left	L	20	0.05	9.7	Α	
			EB	Through	TR	277	0.45	13.4	В	
				Right		10	- 0.24	- 44.7	- D	
			WB	Left Through	TR	66 161	0.21 0.34	11.7 11.1	B B	
404	Kingsway &	Signalized <sup>1</sup>	VVD	Right	I K	49	- 0.34	- 11.1	- В	
107	Timberline Drive	Olgilalized		Left		6	-	-	-	
			NB	Through	LT	5	0.02	9.3	А	
				Right	R	161	0.27	3.1	A	
				Left		51	-	-	-	-
			SB	Through	LTR	4	0.12	9.6	А	
				Right		5	-	-	-	-

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Interse	ction		0.26	8.7	Α	
				Left		3	0.08	8.6	Α	-
			EB	Through	LTR	45	0.08	8.6	Α	-
				Right		0	-	-	-	-
				Left		24	0.23	8.8	Α	-
	Kingsway &	Unsignalized	WB	Through	LTR	61	0.23	8.8	Α	-
405	Queensway	Stop Control - All		Right		70	0.23	8.8	Α	-
	Queensway	Approaches		Left		3	0.26	8.7	Α	-
			NB	Through	LTR	57	0.26	8.7	Α	-
				Right		127	0.26	8.7	Α	-
				Left		89	0.16	8.8	Α	-
			SB	Through	LTR	9	0.16	8.8	Α	-
				Right		2	0.16	8.8	Α	-
				Overall Interse	ction		0.10	8.3	Α	
				Left		14	0.10	9.9	Α	2.8
			EB	Through	LTR	46	0.10	9.9	Α	2.8
				Right		14	0.10	9.9	Α	2.8
				Left		9	0.09	10.0	В	2.3
	Kingsway &	Unsignalized	WB	Through	LTR	49	0.09	10.0	В	2.3
406	Tennis Court	Stop Control - EB/WB		Right		1	0.09	10.0	В	
	Road	Approaches		Left		24	0.02	0.1	Α	0.4
			NB	Through	LTR	7	0.02	4.0	Α	0.4
				Right		13	0.02	4.0	Α	0.4
		Ī		Left		0	-	-	-	2.8 2.8 2.3 2.3 2.3 0.4 0.4 0.4 - - 0.0
			SB	Through	LTR	0	-	-	-	-
				Right		5	0.00	0.0	Α	0.0
				Overall Interse	ction		0.02	1.4	Α	
				Left		1	0.00	0.0	Α	0.0
			EB	Through	LTR	46	0.00	0.2	Α	0.0
				Right		1	0.00	0.2	Α	0.0
		[		Left		1	0.00	0.0	Α	0.0
	Queensway &	Unsignalized	WB	Through	LTR	68	0.00	0.1	Α	0.0
407	Tennis Court	Stop Control - EB/WB		Right		5	0.00	0.1	Α	0.0
	Road	Approaches		Left		7	0.02	9.4	Α	0.5
			NB	Through	LTR	3	0.02	9.4	Α	0.5
				Right		5	0.02	9.4	Α	0.5
		[		Left		2	0.01	9.1	Α	0.2
			SB	Through	LTR	0	-	-	-	-
				Right		3	0.01	9.1	Α	0.2

Assumed same timing plan as Highway 28 & 50 Avenue Timing Plan sent from City - August 31, 2010
 Assume timing plan as per Timing Plan sent from City - August 31, 2010

Project: Cold Lake Transportation Study Project No: 2010-3050 Date Revised: April 5, 2011

### Synchro Results - 2010 Horizon with Highway 28 Upgrades PHF = 0.86

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	Los	95th Queue (m)
				Overall Inte	rsection		0.13	5.1	Α	
				Left		11	0.09	11.8	В	2.3
			EB	Through	LTR	27	0.09	11.8	В	(m)   A   B   2.3   B   2.3   B   2.3   B   2.3   B   2.3   B   3.7   B   3.7   B   3.7   A   0.1   A   0.1   A   0.1   A   0.1   A   0.8   A   0.0   A
				Right		5	0.09	11.8		
				Left		11	0.13	10.5		
	1 Avenue & 28 Street /	Unsignalized	WB	Through	LTR	21	0.13	10.5		
101	English Bay Road	Stop Control - EB/WB		Right		55	0.13	10.5		
		Approaches		Left		6	0.00	0.0		(m)  2.3 2.3 2.3 2.3 3.7 3.7 3.7 0.1 0.1 0.1 0.8 0.8 0.8 0.8 0.0 0.0 1.3 1.3 1.3 2.8 2.8  0.0 0.0 0.0 0.0 1.4 1.4 1.4 1.4 1.9 1.9 1.9
			NB	Through	LTR	79	0.00	0.4		
				Right		30	0.00	0.4		
			0.5	Left		42	0.03	0.3		
			SB	Through	LTR	62	0.03	3.1		
				Right		5	0.03	3.1		0.8
				Overall Inte	rsection	<b>.</b>	0.10	4.1	A	
				Left						
			EB	Through	TR	81	0.06	0.0	B 3.7 B 3.7 A 0.1 A 0.1 A 0.1 A 0.8 A 0.8 A 0.8 A 0.8 A 0.0 A 0.0 A 1.3 A 1.3 A 1.3 A 2.8  A 2.8  A 2.8  A 2.8  A 0.0 A 0.0 A 0.0 A 0.0 A 0.0 A 1.3 A	
				Right		11	0.06	0.0		
			14/5	Left		66	0.05	0.4		
400		Unsignalized	WB	Through	LT	81	0.05	3.6	A	1.3
102	1 Avenue & 25 Street	Stop Control - NB		Right						
		Approach		Left		1	0.10	9.3	A	2.8
			NB	Through	LR					
				Right		83	0.10	9.3	Α	2.8
				Left						
			SB	Through						
				Right						
				Overall Inte	rsection		0.11	1.3	Α	
				Left						
			EB	Through	TR	116	0.11	0.0	Α	0.0
				Right		48	0.11	0.0	Α	0.0
				Left		9	0.01	0.1		0.2
	4 A O Nieles	Unsignalized	WB	Through	LT	127	0.01	0.6		
103	1 Avenue & Nelson	Stop Control - NB		Right						
	Street	Approach		Left		20	0.05	0.1         A         0.2           0.6         A         0.2           10.3         B         1.4		
		11	NB	Through	LR					
				Right		13	0.05	10.3	В	1.4
				Left						
			SB	Through						
			-	Right						
				Overall Inte	rsection		0.09	1.9	Δ	
				Left	locolion		0.00	1.0	, ,	
			EB	Through	TR	91	0.09	0.0	Δ	0.0
	1			Right	- 11	38	0.09	0.0		
	1			Left	1	7	0.09	0.0		
	1	Unsignalized	WB	Through	LT	100	0.01	0.5		
104	1 Avenue & 16 Street	Stop Control - NB	,,,,	Right		100	0.01	0.0	^	U. I
104	. Avenue & 10 Stieet	Approach		Left		36	0.07	10.2	R	10
	1	Αρρισασιί	NB	Through	LR	30	0.07	10.2		1.3
			ND	Right	LN	12	0.07	10.2	D	10
	1		-	Left		14	0.07	10.2	P	1.8
	1		SB							
	1		SD	Through						
				Right	l roooti	I	0.05	0.1		
	1			Overall Inte	section	1 40	0.05	2.1		0.0
	1			Left		12	0.01	0.1		2.3 2.3 3.7 3.7 3.7 3.7 0.1 0.1 0.1 0.8 0.8 0.8 0.8 0.8 0.0 0.0 0.0 1.3 1.3 2.8 2.8 2.8 2.8 4 1.4 1.4 1.4 1.9 1.9 1.9 1.9
			EB	Through	LT	95	0.01	0.9	A	0.2
	1			Right						
	1	11	14/5	Left						2.3 2.3 2.3 3.7 3.7 3.7 0.1 0.1 0.1 0.8 0.8 0.8 0.8 0.0 0.0 0.0 0.0 1.3 1.3 2.8 2.8 2.8 2.8 2.8 3.7 0.1 0.1 0.1 0.1 0.1 0.1 0.0 0.0 0.0 0.0
4.5-	1 Avenue / 2 Avenue &	Unsignalized	WB	Through						
105	10 Street	Yield Control - SB		Right						
	511001	Approach		Left						
			NB	Through	TR	75	0.05	0.0	Α	
	1			Right		4	0.05	0.0	Α	
	1			Left		0	-	-	-	-
			SB	Theresis	LR					
			SD	Through	LIN					

### Synchro Results - 2010 Horizon with Highway 28 Upgrades PHF = 0.86

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inte	rsection		0.07	4.3	Α	
				Left		14	0.07	9.1	A	1.7
			EB	Through	LR	- 40			•	
				Right		40	0.07	9.1	Α	1.7
		Unsignalized	WB	Left						
106	8 Avenue & Lakeshore	Stop Control - EB	VV D	Through Right						
100	Drive	Approach		Left		27	0.02	0.2	Α	0.5
		7.pp.rodon	NB	Through	LT	39	0.02	3.1	A	0.5
				Right						-
				Left						
			SB	Through	TR	25	0.03	0.0	Α	0.0
				Right		17	0.03	0.0	Α	0.0
				Overall Inte	rsection		0.20	6.5	Α	
				Left		7	0.01	0.0	A	
			EB	Through	LT	61	0.01	0.8	A	
				Right Left	R	29 37	0.02	0.0	A A	
		Unsignalized	WB	Through	LTR	55	0.03	2.7	A	
107	8 Avenue & 10 Street	Stop Control - NB/SB	""	Right	LIIX	17	0.03	2.7	A	
		Approaches		Left		17	0.20	11.2	В	6.1
			NB	Through	LTR	52	0.20	11.2	В	6.1
				Right		60	0.20	11.2	В	6.1
				Left		15	0.14	12.2	В	3.9
			SB	Through	LTR	51	0.14	12.2	В	3.9
				Right		5	0.14	12.2	В	3.9
				Overall Inte			0.79	14.9	В	
				Left	L	319	0.79	27.5	C	
			EB	Through	TTR	247	0.25	8.5	Α	
				Right		53	-	-	-	
			WB	Left Through	L TTR	4 110	0.01 0.12	9.2 7.6	A A	
108	8 Avenue & 16 Street	Signalized <sup>1</sup>	***	Right	IIK	28	- 0.12	7.0	-	
100	o / Worldo de 10 Olifoot	Olgridiized		Left		32	-	-	-	1.9 9.1 - - 10.0
			NB	Through	LTR	18	0.12	12.0	В	10.0
				Right		4	-	-	-	-
				Left		27	-	-	-	-
			SB	Through	LTR	8	0.22	6.6	Α	10.7
				Right		74	-	-	-	-
				Overall Inte			0.25	2.9	Α	
				Left	L	119	0.12	8.5	A	
			EB	Through	2T	422	0.14	0.0	Α	0.0
				Right Left			1			
		Unsignalized	WB	Through	2T	273	0.09	0.0	A	0.0
109	Highway 28 & 25 Street	Stop Control - SB		Right	R	30	0.09	0.0	A	
	5 .,	Approach		Left						
		11	NB	Through						
				Right						
				Left		10	0.25	11.5	В	8.0
			SB	Through	LR		ļ			_
				Right		151	0.25	11.5	В	8.0
				Overall Inte	rsection	1 40	0.12	1.8	A	0.0
			EB	Left Through	LT	43 293	0.04 0.04	0.4 1.3	A A	6.1 6.1 3.9 3.9 3.9 473.5 18.1 - 1.9 9.1 - 10.0 - 10.7 - 2 0.0
			EB	Right	L	293	0.04	1.3	А	0.9
				Left						
	11. h	Unsignalized	WB	Through	TR	150	0.12	0.0	Α	0.0
110	Highway 55 & 28 Street/ English Bay Road	Stop Control - SB		Right		23	0.12	0.0	A	
	English Bay Road	Approach		Left						
			NB	Through						
				Right						
				Left		32	0.10	13.6	В	2.6
			SB	Through	LR					_
				Right		7	0.10	13.6	В	2.6

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inte	rsection		0.42	18.1	В	-
				Left	L	145	0.32	15.6	В	28.3
			EB	Through	T	31	0.10	25.5	С	11.6
				Right	R	149	0.40	8.2	A	14.1
			WD	Left	L	133	0.31	17.5	В	26.3
111	Highway 28 & Highway	Signalized <sup>2</sup>	WB	Through	T R	10 51	0.03 0.15	24.8 9.1	C A	5.7 8.7
1111	55 / 16 Avenue	Signalized		Right Left	2L	88	0.15	29.7	C	13.1
			NB	Through	2T	345	0.42	21.9	C	36.0
			112	Right	R	118	0.42	6.0	A	11.0
				Left	L	45	0.27	31.8	C	15.8
			SB	Through	2T	304	0.41	22.9	C	31.4
				Right	R	75	0.20	6.7	Α	9.0
				Overall Inte	rsection		0.04	2.0	Α	
				Left		17	0.01	0.1	Α	0.3
			EB	Through	LTR	137	0.01	8.0	Α	0.3
				Right		15	0.01	8.0	Α	0.3
				Left		0	-	-	-	-
440		Unsignalized	WB	Through	LTR	63	0.00	0.0	A	0.0
112	16 Avenue & 16 Street	Stop Control - NB/SB		Right		7	0.00	0.0	A	0.0
		Approaches	ND	Left	LTD	9	0.03	11.1	В	0.7
			NB	Through Right	LTR	5 0	0.03	11.1	B -	0.7
				Left		12	0.04	10.1	В	1.0
			SB	Through	LTR	2	0.04	10.1	В	1.0
			OB	Right	LIIX	11	0.04	10.1	В	1.0
				Overall Inte	rsection		0.04	3.7	A	1.0
				Left	100001011	8	0.01	0.1	A	0.2
			EB	Through	LTR	86	0.01	0.6	A	0.2
				Right		10	0.01	0.6	A	0.2
				Left		13	0.01	0.1	Α	0.2
		Unsignalized	WB	Through	LTR	64	0.01	0.9	Α	0.2
113	16 Avenue & 10 Street	Stop Control - NB/SB		Right		34	0.01	0.9	A	0.2
		Approaches		Left		6	0.04	10.3	В	0.9
			NB	Through	LTR	9	0.04	10.3	В	0.9
				Right		8	0.04	10.3	В	0.9
			0.0	Left	LTD	38	0.11	10.8	В	2.9
			SB	Through	LTR	11	0.11	10.8	В	2.9
				Right Overall Inte	reaction	16	0.11 0.59	10.8	В	2.9
				Left	ISECTION	10	0.59	14.2	B -	-
			EB	Through	LTR	10	0.09	14.0	В	7.7
				Right	LIIX	10	- 0.03	-	-	-
				Left	LT	131	0.49	24.1	С	30.7
			WB	Through	R	10	0.26	5.7	Ā	9.6
201	Highway 28 / 55 &	Signalized <sup>2</sup>		Right		111	-	-	-	-
	Energy Centre Access	=		Left	L	10	0.03	7.8	Α	2.5
			NB	Through	2T	645	0.59	17.4	В	57.1
				Right	R	72	0.14	5.1	Α	7.6
			-	Left	L	61	0.17	7.3	A	8.3
			SB	Through	2T	526	0.39	11.6	В	43.7
			ļ	Right	R	10	0.02	6.9	A	2.9
				Overall Inte	rsection	0	1.00	10.2	В	
			EB	Left	LTR	0 10	0.20	20.4	- C	- 5 0
			ED	Through Right	LIK	40	0.20	20.4	C	5.8 5.8
				Left		71	1.00	161.2	F	50.2
		Unsignalized	WB	Through	LTR	10	1.00	161.2	F	50.2
202	Highway 28 / 55 & 75	Stop Control - EB/WB	'''	Right	LIIX	10	1.00	161.2	F	50.2
	Avenue	Approaches		Left	L	10	0.01	9.5	A	0.3
			NB	Through	2T	717	0.25	0.0	A	0.0
				Right	R	20	0.01	0.0	A	0.0
				Left	L	0	-	-	-	-
			SB	Through	2T	657	0.22	0.0	Α	0.0
				Right	R	10	0.01	0.0	Α	0.0

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inte	rsection		0.47	10.1	В	-
				Left						
			EB	Through						
				Right						
				Left	L	7	0.02	18.6	В	3.6
	Highway 28 / 55 & 69	Signalized <sup>2</sup>	WB	Through						
203	Avenue	Olgridiized		Right	R	104	0.30	6.8	Α	10.1
	Avondo			Left						
			NB	Through	2T	644	0.47	15.3	В	49.4
				Right	R	26	0.04	5.9	Α	4.2
				Left	L	118	0.28	6.0	Α	10.4
			SB	Through	2T	650	0.34	6.4	Α	28.1
				Right						
				Overall Inte	rsection		0.29	8.3	Α	
				Left						
			EB	Through						
				Right						
				Left	L	83	0.28	19.8	В	20.5
	Highway 29 / EE & Tri	Signalized <sup>2</sup>	WB	Through						
204	Highway 28 / 55 & Tri- City Mall Access	Signalized		Right	R	10	0.04	11.2	В	3.5
	City Iviail Access			Left						
			NB	Through	TTTR	659	0.29	9.9	Α	32.6
				Right		72	-	-	-	-
				Left	L	55	0.13	4.9	Α	5.6
			SB	Through	2T	602	0.28	5.0	A	25.4
				Right						
				Overall Inte	rsection		0.27	1.0	Α	
				Left	I		0.21	1.0		
			EB	Through						
				Right						
						07	0.07	20.0	-	0.4
		Unoignolizad	WB	Left	L	37	0.27	36.2	E	8.4
205	Highway 28 / 55 & 62	Unsignalized Stop Control - WB	WD	Through	_		0.00	44.4		0.4
205	Avenue / 61 Avenue	Approach		Right	R	8	0.02	11.4	В	0.4
		Approach	ND	Left	οT	700	0.05	0.0		0.0
			NB	Through	2T	723	0.25	0.0	A	0.0
				Right	R	70	0.05	0.0	A	0.0
			0.5	Left	L	9	0.01	10.1	В	0.4
			SB	Through	2T	676	0.23	0.0	Α	0.0
				Right						
				Overall Inte	rsection	,	0.67	20.6	С	
				Left	L	51	0.14	15.9	В	12.7
			EB	Through	T	51	0.15	25.3	С	16.8
				Right	R	31	0.10	9.4	A	6.5
				Left	L	71	0.18	15.7	В	16.5
	Highway 28 / 55 & 54	Signalized <sup>2</sup>	WB	Through	Т	61	0.15	23.7	С	19.0
301	Avenue	Oignailzeu		Right	R	153	0.34	6.2	A	12.7
	Avenue			Left	L	31	0.15	21.3	С	11.7
			NB	Through	2T	602	0.67	26.1	С	80.0
				Right	R	61	0.14	6.8	Α	9.1
				Left	L	163	0.46	25.1	С	44.7
			SB	Through	2T	581	0.53	19.8	В	84.1
				Right	R	61	-	-	-	-
				Overall Inte	rsection		3.35	849.3	F	
				Left		30	2.25	812.5	F	72.3
			EB	Through	LTR	30	2.25	812.5	F	72.3
				Right		10	2.25	812.5	F	72.3
				Left		50	3.35	Error	F	Error
		Unsignalized	WB	Through	LTR	30	3.35	Error	F	Error
302	Highway 28 / 55 & 52	Stop Control - EB/WB		Right		55	3.35	Error	F	Error
552	Avenue	Approaches		Left		70	0.09	1.1	A	2.3
		7 10000103	NB	Through	LTTR	615	0.09	1.3	A	2.3
			IND	Right	LIIK	80	0.27	0.0	A	0.0
1						140	0.27	2.7	A	6.2
			SB	Left	LTTD					
			SB	Through	LTTR	510	0.21	2.8	A	6.2
				Right	<u> </u>	40	0.20	0.0	Α	0.0

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inte	rsection		0.76	14.3	В	
				Left	L	375	0.76	22.5	С	76.5
			EB	Through	T	329	0.42	11.4	В	48.0
				Right	R [C]	135	0.19	2.3	Α	7.1
			WD	Left		9	-	-	-	-
303	Highway 28 / 55 & 50	Signalized <sup>3</sup>	WB	Through	LT R [C]	161 155	0.22 0.21	9.3 2.2	A A	24.4 7.5
303	Avenue	Signalized		Right Left	K [C]	133	0.40	22.9	C	29.3
			NB	Through	2T	275	0.40	17.3	В	22.8
			110	Right	R [C]	162	0.01	10.8	В	2.3
				Left	L	116	0.44	23.5	C	33.1
			SB	Through	2T	232	0.28	17.6	В	26.7
				Right	R [C]	5	0.30	4.7	Α	11.4
				Overall Inte			0.47	5.1	A	
				Left		16	0.02	0.2	A	0.5
			EB	Through	LTTR	486	0.20	0.4	Α	0.5
				Right		50	0.20	0.0	Α	0.0
				Left		98	0.12	1.4	Α	3.2
	Highway 28 / 55 & 52	Unsignalized	WB	Through	LTTR	523	0.21	1.8	Α	3.2
304	Street	Stop Control - NB/SB		Right		42	0.21	0.0	Α	0.0
	Girect	Approaches		Left		23	0.47	32.2	D	18.7
			NB	Through	LTR	10	0.47	32.2	D	18.7
				Right		66	0.47	32.2	D	18.7
				Left		21	0.41	42.6	E	14.5
			SB	Through	LTR	8	0.41	42.6	E	14.5
				Right		27	0.41	42.6	E	14.5
				Overall Inte	rsection	•	0.22	0.2	Α	
				Left						
			EB	Through	2T	634	0.22	0.0	A	0.0
				Right						
		Hardwar Parad	WD	Left		=0.	<del>.</del>			
205	Highway 28 / 55 & 51	Unsignalized	WB	Through	2T	504	0.17	0.0	A	0.0
305	Street	Stop Control - SB Approach		Right	R [C]	23	0.02	0.0	A	0.0
		Арргоасп	NB	Left						
			IND	Through Right						
				Left						
			SB	Through						
			OB	Right	R [C]	19	0.03	10.3	В	0.8
				Overall Inte		1.5	0.70	12.3	В	0.0
				Left	locotion	1	0.70	12.0		
			EB	Through						
				Right						
				Left	L	239	0.43	16.3	В	44.6
	11"-1 00 / FF 0 F0		WB	Through						
306	Highway 28 / 55 & 50	Signalized 3		Right	R	144	0.25	4.0	Α	9.9
	Street			Left						
			NB	Through	2T	383	0.29	10.4	В	23.3
				Right	R	188	0.28	2.6	Α	8.2
				Left		215	-	-	-	-
			SB	Through	LTT	419	0.70	16.7	В	47.5
				Right						
				Overall Inte		,	0.22	1.0	Α	
				Left	L	11	0.09	33.2	D	2.4
			EB	Through						
				Right	R	34	0.07	11.4	В	1.7
		Unainnalinad	WD	Left						
207	Highway 28 / 55 & 46	Unsignalized	WB	Through	D (C)	40	0.00	40.7	_ [	0.4
307	Avenue	Stop Control - EB/WB		Right	R [C]	10	0.02	10.7	В	0.4
		Approaches	NID	Left	LTTD IO	33	0.05	0.6	A	1.2
			NB	Through Right	LTTR [C]		0.21	0.8	A	1.2 0.0
				Left		32	0.21	0.0	Α	U.U
			SB	Through	2T	649	0.22	0.0	Α	0.0
			36	Right	R	10	0.22	0.0	A	0.0
	1	l		rigni	, r	10	U.U I	0.0	А	0.0

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inte	rsection		0.38	9.9	А	
				Left	L	78	0.19	13.1	В	15.8
			EB	Through	TR	29	0.12	7.5	A	9.5
				Right		35	-	-	-	-
			WB	Left	L T	114	0.28	14.0 11.9	B B	2.2
308	Highway 28 / 55 & 43	Signalized <sup>3</sup>	WD	Through Right	R	49 73	0.08 0.14	4.4	A	10.6 7.1
300	Avenue	Signalized		Left	L	19	0.14	8.9	A	4.2
			NB	Through	TTR	464	0.33	9.6	A	28.4
				Right		67	-	-	-	-
				Left	L	137	0.38	14.2	В	23.0
			SB	Through	2T	474	0.29	9.7	Α	26.0
				Right	R	72	0.10	2.8	Α	5.0
				Overall Inte	rsection		0.13	4.2	Α	
				Left						
			EB	Through						
				Right		00	0.40	0.0		0.5
		Unsignalized	WB	Left		82	0.13	9.8	Α	3.5
309	52 Avenue & 57 Street	Stop Control - WB	WD	Through Right	LR	13	0.13	9.8	A	3.5
309	(North)	Approach		Left		13	0.13	9.0	A	3.3
		присаси	NB	Through	TR	31	0.08	0.0	Α	0.0
				Right	111	82	0.08	0.0	A	0.0
				Left		9	0.01	0.1	A	0.2
			SB	Through	LT	20	0.01	2.4	A	0.2
				Right						
				Overall Inte	rsection		0.07	2.5	Α	
				Left		46	0.07	10.0	А	1.8
			EB	Through	LR					
				Right		1	0.07	10.0	Α	1.8
			14/5	Left						
040	52 Avenue & 57 Street	Unsignalized	WB	Through						
310	(South)	Stop Control - EB Approach		Right Left		11	0.01	0.1	Δ	0.2
		Арргоасп	NB	Through	LT	11 66	0.01	1.1	A A	0.2
			145	Right	LI	- 00	0.01	1-1	Α	0.2
				Left						
			SB	Through	TR	46	0.07	0.0	А	0.0
				Right		55	0.07	0.0	А	0.0
				Overall Inte	rsection		0.08	6.0	Α	
				Left		2	0.00	0.0	А	0.0
			EB	Through	LTR	10	0.00	0.7	Α	0.0
				Right		10	0.00	0.7	Α	0.0
			14/5	Left		12	0.01	0.1	A	0.2
244	EO Avenue 9 EO China	Unsignalized	WB	Through	LTR	14	0.01	2.2	A	0.2
311	50 Avenue & 59 Street	Stop Control - NB/SB Approaches	<u> </u>	Right		14 22	0.01	2.2	A A	0.2
		Approacties	NB	Left Through	LTR	22	0.08	9.5 9.5	A	2.2
			.,0	Right	LIIX	19	0.08	9.5	A	2.2
				Left		6	0.00	9.6	A	0.5
			SB	Through	LTR	6	0.02	9.6	A	0.5
			1	Right		1	0.02	9.6	A	0.5
				Overall Inte	rsection	•	0.12	7.2	Α	
				Left		22	0.02	0.1	Α	0.4
			EB	Through	LTR	5	0.02	3.4	Α	0.4
				Right		22	0.02	3.4	Α	0.4
			l	Left		8	0.01	0.0	Α	0.1
		Unsignalized	WB	Through	LTR	8	0.01	1.8	A	0.1
312	50 Avenue & 57 Street	Stop Control - NB/SB	ļ	Right		18	0.01	1.8	A	0.1
		Approaches		Left	L	37	0.12	10.4	В	3.4
			NB	Through	LTR	37	0.12	10.4	В	3.4
			<b>-</b>	Right	-	8 5	0.12 0.06	10.4 9.5	B A	3.4 1.7
			SB	Left Through	LTR	21	0.06	9.5	A	1.7
				Right	LIN	21	0.06	9.5	A	1.7
	l .	l .	L	raynt	1		0.00	ჟ.ე	^	1.7

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inte	rsection		0.26	2.3	Α	
				Left		45	0.05	0.9	Α	1.2
			EB	Through	LTR	775	0.05	1.3	Α	1.2
				Right		17	0.05	1.3	Α	1.2
				Left		1	0.00	0.0	Α	0.0
0.40	Centre Avenue & 59	Unsignalized	WB	Through	LTR	353	0.00	0.0	A	0.0
313	Street	Stop Control - NB/SB		Right		13	0.00	0.0	A	0.0
		Approaches	ND	Left		5	0.13	32.7	D	3.6
			NB	Through	LTR	5	0.13	32.7	D	3.6
				Right		7	0.13	32.7	D	3.6
			SB	Left	LTD	17	0.26	44.5	E	7.9
			36	Through	LTR	5	0.26	44.5	E	7.9
				Right Overall Inte	reaction	6	0.26 0.59	44.5	E	7.9
					Section	44		5.3	A	4.0
			EB	Left Through	LTR	44 808	0.05 0.05	0.9 1.3	A A	1.2 1.2
			ED		LIK	17		1.3	A	1.2
				Right Left		37	0.05 0.06	0.8	A	1.6
		Unsignalized	WB		LT		0.06	1.8	A	
314	Centre Avenue & 57	Stop Control - NB/SB	VVD	Through Right	LT R	351 32	0.06	0.0	A	1.6 0.0
314	Street	Approaches		Left	K	5	0.02	33.0	D	7.8
		Approacties	NB	Through	LTR	6	0.26	33.0	D	7.8
			IND	Right	LIK	27	0.26	33.0	D	7.8
				Left				81.7	F	
			SB	Through	LTR	26 9	0.59 0.59	81.7	F	22.5 22.5
			35		LIK	17			F	
				Right Overall Inte	reaction	17	0.59	81.7		22.5
				Left	SECTION		0.13	1.8	Α	
			EB	Through	TR	143	0.13	0.0	A	0.0
			EB	Right	IK	46	0.13	0.0	A	0.0
				Left		3	0.13	0.0	A	0.0
		Unsignalized	WB	Through	LT	114	0.00	0.0	A	0.1
315	54 Avenue & 51 Street	Stop Control - NB	WB	Right	LI	114	0.00	0.2		0.1
313	34 Avenue a 31 Olicet	Approach		Left		56	0.10	11.1	В	2.7
		украгодон	NB	Through	LR	30	0.10			£.1
			110	Right	LIX	1	0.10	11.1	В	2.7
				Left			0.10			£.1
			SB	Through						
			OB	Right						
				Overall Inte	rsection		0.16	3.1	Α	
				Left	000000	71	0.07	0.7	A	1.8
			EB	Through	LTR	270	0.07	2.2	A	1.8
				Right		15	0.07	2.2	A	1.8
				Left		2	0.00	0.0	A	0.0
		Unsignalized	WB	Through	LTR	231	0.00	0.1	A	0.0
316	50 Avenue & 53 Street	Stop Control - NB/SB		Right		19	0.00	0.1	A	0.0
		Approach		Left		14	0.09	18.2	C	2.3
		1,	NB	Through	LTR	3	0.09	18.2	C	2.3
				Right		6	0.09	18.2	C	2.3
				Left		21	0.16	15.9	С	4.7
			SB	Through	LTR	3	0.16	15.9	С	4.7
				Right		32	0.16	15.9	C	4.7
				Overall Inte	rsection		0.45	11.1	В	
				Left		48	0.45	12.0	В	-
			EB	Through	LTR	208	0.45	12.0	В	-
				Right		18	0.45	12.0	В	-
				Left		20	0.38	11.1	В	-
		Unsignalized	WB	Through	LTR	178	0.38	11.1	В	-
	i	Stop Control - All		Right		39	0.38	11.1	В	-
317	50 Avenue & 52 Street	Otop Control All				26	0.15	9.6	A	-
317	50 Avenue & 52 Street	Approaches		Left		20	0.15	9.0		
317	50 Avenue & 52 Street		NB	Left Through	LTR			9.6		-
317	50 Avenue & 52 Street		NB	Through	LTR	31 24	0.15 0.15		A	-
317	50 Avenue & 52 Street		NB		LTR	31	0.15	9.6	Α	
317	50 Avenue & 52 Street		NB SB	Through Right	LTR	31 24	0.15 0.15	9.6 9.6	A A	-

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inte	rsection		0.47	11.2	В	
				Left		62	0.47	12.5	В	-
			EB	Through	LTR	211	0.47	12.5	В	-
				Right		19	0.47	12.5	В	-
				Left		8	0.33	10.6	В	-
		Unsignalized	WB	Through	LTR	164	0.33	10.6	В	-
318	50 Avenue & 51 Street	Stop Control - All		Right		26	0.33	10.6	В	-
		Approaches		Left		28	0.14	9.5	A	-
			NB	Through	LTR	28	0.14	9.5	A	-
				Right		23	0.14	9.5	A	-
			CD	Left	LTD	74	0.23	10.3	В	-
			SB	Through	LTR	32	0.23	10.3	В	-
				Right		24	0.23	10.3	В	-
				Overall Inte	rsection		0.46	10.9	В	
				Left	1.70	0	-		-	-
			EB	Through	LTR	193	0.46	11.4	В	-
		I I and a superficient		Right		114	0.46	11.4	В	-
		Unsignalized	MD	Left	1.70	83	0.32	10.4	В	-
040	50 4 0 50 0	Stop Control -	WB	Through	LTR	112	0.32	10.4	В	-
319	50 Avenue & 50 Street	EB/WB/NB Approaches		Right		0	-	- 40.4	-	-
		Assumed Yield Control -	ND	Left	1.70	83	0.31	10.4	В	-
		SB Approach	NB	Through	LTR	0	-	-	-	-
				Right		109	0.31	10.4	В	-
			CD	Left	LTD	0	-	-	-	-
			SB	Through	LTR	0	-	-	-	-
				Right		0	-		-	-
				Overall Inte	rsection	=0	0.32	5.5	A	
				Left	1.70	50	0.04	0.4	A	1.1
			EB	Through	LTR	253	0.04	1.5	A	1.1
				Right		25	0.04	1.5	A	1.1
		l lasianalias d	WD	Left	LTD	23	0.02	0.2	A	0.5
200	50 A	Unsignalized	WB	Through	LTR	135	0.02	1.2	A	0.5
320	50 Avenue & 49 Street	Stop Control - NB/SB		Right		29 10	0.02	1.2	A	0.5
		Approaches	ND	Left	LTD		0.19	15.5	С	5.5
			NB	Through	LTR	28	0.19	15.5	C	5.5
				Right		31	0.19	15.5		5.5
			CD	Left	LTD	40	0.32	17.6	C	10.9
			SB	Through	LTR	23	0.32	17.6	С	10.9
				Right		53	0.32	17.6	C	10.9
				Overall Inte	rsection	0.4	0.08	2.1	A	0.7
			ED	Left	LTD	34	0.03	0.3	A	0.7
		1	EB	Through	LTR	261	0.03	1.0	A	0.7
		1		Right		35	0.03	1.0	A	0.7
		Ungignalizad	WB	Left	LTD	7	0.01	0.1	A	0.2
204	50 Avonuo 9 45 Ctr	Unsignalized	WB	Through	LTR	146	0.01	0.4	A	0.2
321	50 Avenue & 45 Street	Stop Control - NB/SB		Right	-	4	0.01	0.4	A B	0.2
		Approaches	NB	Left	LTD	14 3	0.08	13.6	В	2.1
		1	IND	Through	LTR	14	0.08	13.6 13.6	В	2.1
		1		Right Left	-	9		13.6	В	
		1	SB		LTR	4	0.06 0.06	12.3	В	1.6
		1	SD	Through	LIK	15			В	1.6
				Right	rootion	15	0.06	12.3		1.6
		1		Overall Inte	section	70	0.08	2.9	A	4.5
		1	E5	Left	1	72	0.06	0.5	A	1.5
			EB	Through	LT	168	0.06	2.7	Α	1.5
		1		Right						
		Linoigra eli-e el	WD	Left		400	0.00	0.0		
200	50 August 9 44 Ct	Unsignalized	WB	Through	TR	106	0.08	0.0	A	0.0
322	50 Avenue & 41 Street	Stop Control - SB		Right		8	0.08	0.0	Α	0.0
		Approaches	ND	Left						
		1	NB	Through						
				Right		4.4	0.00	40.4	,	
		1	CD.	Left		14	0.08	10.4	В	2.2
			SB	Through	LR	62	0.00	40.	-	
1	1	I .		Right	l	38	0.08	10.4	В	2.2

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inte	rsection		0.11	5.3	А	
			- FD	Left		82	0.06	0.5	A	1.5
			EB	Through Right	LT	69	0.06	4.3	Α	1.5
				Left					-	
	50 Avenue / Twp Rd 630	Unsignalized	WB	Through	TR	26	0.03	0.0	Α	0.0
323	& "Baywood Road" / RR	Stop Control - SB		Right		11	0.03	0.0	Α	0.0
	20	Approaches		Left						
			NB	Through						
				Right			0.11			
			SB	Left	LR	18	0.11	9.4	Α	3.0
			36	Through Right	LK	70	0.11	9.4	А	3.0
				Overall Inte	rsection	70	0.54	8.4	A	3.0
				Left			0.01	0.1	1	
			EB	Through	TR	657	0.54	9.4	Α	102.9
				Right		6	-	-	-	-
				Left	L	5	0.01	6.4	Α	1.7
404	Kingsway & Medley	a3	WB	Through	Т	268	0.22	5.8	Α	31.5
401	Road	Signalized <sup>3</sup>		Right Left		16	0.11	10.9	В	8.0
			NB	Through	LR	10	0.11	10.9	D	6.0
			l IND	Right	LIX	20	-	-	-	-
				Left						
			SB	Through						
				Right						
				Overall Inte	rsection		0.74	13.6	В	
				Left	L	153	0.33	10.5	В	21.7
			EB	Through	Т	608	0.74	17.1	В	92.6
				Right Left						
			WB	Through	Т	261	0.32	9.4	Α	30.9
402	Kingsway & Glenwood	Signalized <sup>3</sup>		Right	R [C]	107	0.15	2.0	A	5.4
	Drive (East)	3		Left						
			NB	Through						
				Right						
			CD.	Left		131	0.29	18.9	В	29.5
			SB	Through Right	LR	1	-	-	-	-
				Overall Inte	rsection	l	0.51	0.4	A	-
				Left			0.01	0.1	1	
			EB	Through	TR	749	0.51	0.0	А	0.0
				Right		1	0.51	0.0	Α	0.0
				Left	L	5	0.01	9.8	Α	0.2
400	Kingsway & Glenwood	Unsignalized	WB	Through	Т	257	0.18	0.0	Α	0.0
403	Drive (West)	Stop Control - NB/SB Approaches		Right Left	L	6	0.04	20.6	С	1.1
		Approacties	NB	Through	L	0	0.04	∠∪.0	U	1.1
				Right	R	11	0.04	20.6	С	1.1
				Left						
			SB	Through						
				Right	R	0	-	-	-	-
				Overall Inte			0.45	10.2	В	
			EB	Left	L TR	20 277	0.05	9.7	A	4.4
			ED	Through Right	IK	10	0.45	13.4	B -	37.6
				Left	L	66	0.21	11.7	В	11.2
	Kingoway 9 Timber		WB	Through	TR	161	0.34	11.1	В	25.7
404	Kingsway & Timberline Drive	Signalized <sup>3</sup>		Right		49	-	-	-	-
	Dilve	=		Left		6	-	-	-	-
			NB	Through	LT	5	0.02	9.3	Α	3.0
				Right	R	161	0.27	3.1	Α	8.1
			SB	Left	LTR	51 4	0.12	9.6	- A	9.3
			JD.	Through Right	LIK	5	0.12	9.6	- A	9.3
1	I		l	rignt	1	j j	-			_

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inte	rsection		0.26	8.7	Α	
				Left		3	0.08	8.6	Α	-
			EB	Through	LTR	45	0.08	8.6	Α	-
				Right		0	-	-	-	-
				Left		24	0.23	8.8	Α	-
		Unsignalized	WB	Through	LTR	61	0.23	8.8	Α	-
405	Kingsway & Queensway	Stop Control - All		Right		70	0.23	8.8	Α	-
		Approaches		Left		3	0.26	8.7	Α	-
			NB	Through	LTR	57	0.26	8.7	Α	-
				Right		127	0.26	8.7	Α	-
				Left		89	0.16	8.8	Α	-
			SB	Through	LTR	9	0.16	8.8	Α	-
				Right		2	0.16	8.8	Α	-
				Overall Inte	rsection		0.10	8.3	Α	
				Left		14	0.10	9.9	Α	2.8
			EB	Through	LTR	46	0.10	9.9	Α	2.8
				Right		14	0.10	9.9	Α	2.8
				Left		9	0.09	10.0	В	2.3
	16' 0 T '- 0	Unsignalized	WB	Through	LTR	49	0.09	10.0	В	2.3
406	Kingsway & Tennis Court	Stop Control - EB/WB		Right		1	0.09	10.0	В	2.3
	Road	Approaches		Left		24	0.02	0.1	Α	0.4
			NB	Through	LTR	7	0.02	4.0	Α	0.4
				Right		13	0.02	4.0	Α	0.4
				Left		0	-	-	-	-
			SB	Through	LTR	0	-	-	-	-
				Right		5	0.00	0.0	Α	0.0
				Overall Inte	rsection		0.02	1.4	Α	
				Left		1	0.00	0.0	Α	0.0
			EB	Through	LTR	46	0.00	0.2	A	0.0
				Right		1	0.00	0.2	A	0.0
				Left		1	0.00	0.0	Α	0.0
	0	Unsignalized	WB	Through	LTR	68	0.00	0.1	A	0.0
407	Queensway & Tennis	Stop Control - EB/WB		Right		5	0.00	0.1	A	0.0
-	Court Road	Approaches		Left		7	0.02	9.4	A	0.5
		'''	NB	Through	LTR	3	0.02	9.4	A	0.5
				Right		5	0.02	9.4	A	0.5
				Left		2	0.01	9.1	A	0.2
			SB	Through	LTR	0	-	-	-	-
				Right		3	0.01	9.1	Α	0.2

<sup>1.</sup> Assumed same timing plan as Highway 28 & 54 Avenue Timing Plan sent from City - May 13, 2010
2. Assumed timing plan as per Timing Plan sent from City - May 13, 2010
3. Assumed timing plan from existing 2010 horizon - same timing plan as Highway 28 & 50 Avenue Timing Plan sent from City - August 31, 2010

Project: Cold Lake Transportation Study Project No: 2010-3050

Date Revised: April 5, 2011

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay	LOS	95th Queue
			10	Overall Inte			0.13	(s) 5.1	A	(m)
				Left	36011011	11	0.13	11.8	В	2.3
			EB	Through	LTR	27	0.09	11.8	В	2.3
				Right		5	0.09	11.8	В	2.3
				Left		11	0.13	10.5	В	3.7
	1 Avenue & 28 Street /	Unsignalized	WB	Through	LTR	21	0.13	10.5	В	3.7
101	English Bay Road	Stop Control - EB/WB		Right		55	0.13	10.5	В	3.7
	English Day Road	Approaches		Left		6	0.00	0.0	Α	0.1
			NB	Through	LTR	79	0.00	0.4	Α	0.1
				Right		30	0.00	0.4	A	0.1
			0.0	Left		42	0.03	0.3	A	0.8
			SB	Through	LTR	62	0.03	3.1	A	0.8
				Right Overall Inter	racetion	5	0.03	3.1	A	0.8
				Left	Section		0.10	4.1	Α	
			EB	Through	TR	81	0.06	0.0	A	0.0
			LD	Right	IN	11	0.06	0.0	A	0.0
				Left		66	0.05	0.4	A	1.3
		Unsignalized	WB	Through	LT	81	0.05	3.6	A	1.3
102	1 Avenue & 25 Street	Stop Control - NB		Right		J.	5.55	0.0		
		Approach		Left		1	0.10	9.3	А	2.8
		,,	NB	Through	LR					
				Right		83	0.10	9.3	А	2.8
				Left						
			SB	Through						
				Right						
				Overall Inte	rsection		0.11	1.3	Α	
				Left						
			EB	Through	TR	116	0.11	0.0	Α	0.0
				Right		48	0.11	0.0	Α	0.0
				Left		9	0.01	0.1	Α	0.2
400	1 Avenue & Nelson	Unsignalized	WB	Through	LT	127	0.01	0.6	A	0.2
103	Street	Stop Control - NB		Right		00	0.05	40.0		4.4
		Approach	NB	Left Through	LR	20	0.05	10.3	В	1.4
			IND	Right	LK	13	0.05	10.3	В	1.4
				Left		13	0.03	10.3	В	1.4
			SB	Through						
			0.5	Right						
				Overall Inte	rsection		0.09	1.9	Α	
				Left						
			EB	Through	TR	91	0.09	0.0	А	0.0
				Right		38	0.09	0.0	Α	0.0
				Left		7	0.01	0.0	Α	0.1
		Unsignalized	WB	Through	LT	100	0.01	0.5	A	0.1
104	1 Avenue & 16 Street	Stop Control - NB		Right						
		Approach		Left		36	0.07	10.2	В	1.9
			NB	Through	LR	4.	o -=	46.5		
				Right		12	0.07	10.2	В	1.9
			SB	Left Through						
1			36	Right						
				Overall Inter	rsection	1	0.05	2.1	A	
				Left	000001	12	0.03	0.1	A	0.2
			EB	Through	LT	95	0.01	0.9	A	0.2
				Right		30	5.51	5.0	, i	
				Left						
	1 Avenue / 2 Avenue 9	Unsignalized	WB	Through						
105	1 Avenue / 2 Avenue & 10 Street	Yield Control - SB		Right						
	10 Street	Approach		Left						
			NB	Through	TR	75	0.05	0.0	А	0.0
				Right		4	0.05	0.0	A	0.0
				Left		0	-	-	-	-
			SB	Through	LR		_			
				Right		41	0.05	9.2	Α	1.3

				Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	(m)
				Overall Inte	rsection		0.07	4.3	Α	, , ,
				Left		14	0.07	9.1	Α	1.7
			EB	Through	LR					
				Right		40	0.07	9.1	A	1.7
		Unadana Basad	WD	Left						
106	8 Avenue & Lakeshore	Unsignalized Stop Control - EB	WB	Through			<del>                                     </del>		1	
106	Drive	Approach		Right Left		27	0.02	0.2	A	0.5
		Арргоасп	NB	Through	LT	39	0.02	3.1	A	0.5
			110	Right		- 55	0.02	0.1	Λ	0.5
				Left						
			SB	Through	TR	25	0.03	0.0	Α	0.0
				Right		17	0.03	0.0	Α	0.0
				Overall Inte	rsection		0.20	6.5	Α	
				Left		7	0.01	0.0	Α	0.1
			EB	Through	LT	61	0.01	0.8	Α	0.1
				Right	R	29	0.02	0.0	Α	0.0
		Unationally at	WD	Left	LTD	37	0.03	0.2	A	0.7
107	8 Avenue & 10 Street	Unsignalized Stop Control - NB/SB	WB	Through Right	LTR	55 17	0.03	2.7 2.7	A A	0.7 0.7
107	o Avenue & 10 Sheet	Approaches		Left		17	0.03	11.2	В	6.1
		Approacties	NB	Through	LTR	52	0.20	11.2	В	6.1
			ND	Right	LIIX	60	0.20	11.2	В	6.1
				Left		15	0.14	12.2	В	3.9
			SB	Through	LTR	51	0.14	12.2	В	3.9
				Right		5	0.14	12.2	В	3.9
				Overall Inte	rsection		0.79	14.9	В	
				Left	L	319	0.79	27.5	С	#73.5
			EB	Through	TTR	247	0.25	8.5	Α	18.1
				Right		53	-	-	-	-
			5	Left	L	4	0.01	9.2	Α	1.9
100	O Avenue 9 40 Otroct	0:	WB	Through	TTR	110	0.12	7.6	Α	9.1
108	8 Avenue & 16 Street	Signalized <sup>1</sup>		Right Left		28 32	-	-	-	-
			NB	Through	LTR	18	0.12	12.0	В	10.0
			ND	Right	LIIX	4	- 0.12	-	-	-
				Left		27	-	-	-	-
			SB	Through	LTR	8	0.22	6.6	Α	10.7
				Right		74	-	-	-	-
				Overall Inte	rsection	•	0.25	2.9	Α	
				Left	L	119	0.12	8.5	Α	3.2
			EB	Through	2T	422	0.14	0.0	Α	0.0
				Right						
		11 2 2 1	W.D.	Left						
100	Highwoy 20 9 OF Cturt	Unsignalized	WB	Through	2T	273	0.09	0.0	A	0.0
109	Highway 28 & 25 Street	Stop Control - SB Approach		Right Left	R	30	0.02	0.0	Α	0.0
		Approach	NB							
			IAD	Through Right						
				Left		10	0.25	11.5	В	8.0
			SB	Through	LR	-10	5.25	11.0		5.0
				Right		151	0.25	11.5	В	8.0
				Overall Inte	rsection		0.12	1.8	A	
				Left		43	0.04	0.4	Α	0.9
			EB	Through	LT	293	0.04	1.3	Α	0.9
				Right						
				Left				_		
440	Highway 55 & 28 Street/	Unsignalized	WB	Through	TR	150	0.12	0.0	A	0.0
110	English Bay Road	Stop Control - SB		Right		23	0.12	0.0	Α	0.0
	- 1	Approach	NB	Left						
			IND	Through Right						
				Left		32	0.10	13.6	В	2.6
			SB	Through	LR	32	5.10	10.0		2.0
				Right		7	0.10	13.6	В	2.6

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inter	rsection		0.42	18.1	В	-
				Left	L	145	0.32	15.6	В	28.3
			EB	Through	Т	31	0.10	25.5	С	11.6
				Right	R	149	0.40	8.2	Α	14.1
			14/5	Left	L -	133	0.31	17.5	В	26.3
111	Highway 28 & Highway	0: 1: 12	WB	Through	T	10	0.03	24.8	C	5.7
111	55 / 16 Avenue	Signalized <sup>2</sup>		Right	R 2L	51 88	0.15 0.27	9.1 29.7	A C	8.7 13.1
			NB	Left Through	2T	345	0.42	21.9	C	36.0
			ND	Right	R	118	0.42	6.0	A	11.0
				Left	L	45	0.27	31.8	C	15.8
			SB	Through	2T	304	0.41	22.9	C	31.4
				Right	R	75	0.20	6.7	A	9.0
				Overall Inter	rsection		0.04	2.0	А	
				Left		17	0.01	0.1	Α	0.3
			EB	Through	LTR	137	0.01	0.8	Α	0.3
				Right		15	0.01	0.8	А	0.3
				Left		0	-	-	-	-
		Unsignalized	WB	Through	LTR	63	0.00	0.0	A	0.0
112	16 Avenue & 16 Street	Stop Control - NB/SB		Right		7	0.00	0.0	A	0.0
		Approaches	ND	Left	1.70	9	0.03	11.1	В	0.7
			NB	Through	LTR	5 0	0.03	11.1	B -	0.7
				Right Left		12	0.04	10.1	В	1.0
			SB	Through	LTR	2	0.04	10.1	В	1.0
			SB	Right	LIN	11	0.04	10.1	В	1.0
				Overall Inter	rsection		0.04	3.7	A	1.0
				Left	000001	8	0.01	0.1	A	0.2
			EB	Through	LTR	86	0.01	0.6	A	0.2
				Right		10	0.01	0.6	Α	0.2
				Left		13	0.01	0.1	А	0.2
		Unsignalized	WB	Through	LTR	64	0.01	0.9	Α	0.2
113	16 Avenue & 10 Street	Stop Control - NB/SB		Right		34	0.01	0.9	А	0.2
		Approaches		Left		6	0.04	10.3	В	0.9
			NB	Through	LTR	9	0.04	10.3	В	0.9
				Right		8	0.04	10.3	В	0.9
			0.0	Left		38	0.11	10.8	В	2.9
			SB	Through	LTR	11	0.11	10.8	В	2.9
				Right Overall Inter	reaction	16	0.11	10.8	B B	2.9
				Left	Section	10	0.59	14.2	- B	-
			EB	Through	LTR	10	0.09	14.0	В	7.7
			25	Right	LIIX	10	-	-	-	-
				Left	LT	131	0.49	24.1	С	30.7
			WB	Through	R	10	0.26	5.7	A	9.6
201	Highway 28 / 55 &	Signalized <sup>2</sup>		Right		111	-	-	-	-
	Energy Centre Access			Left	L	10	0.03	7.8	Α	2.5
			NB	Through	2T	645	0.59	17.4	В	57.1
				Right	R	72	0.14	5.1	Α	7.6
			0-	Left	L	61	0.17	7.3	Α	8.3
			SB	Through	2T	526	0.39	11.6	В	43.7
				Right	R	10	0.02	6.9	A	2.9
				Overall Inter	section		0.37	7.6	Α	
			EB	Left	LTD	0 10	- 0.15	- 7.0	-	6.9
			LD	Through Right	LTR	40	0.15	7.2	A -	6.9
				Left		71		-	-	
			WB	Through	LTR	10	0.35	15.4	В	16.3
202	Highway 28 / 55 & 75	Only Option: Signalized	5	Right	LIII	10	-	-	-	-
	Avenue	Intersection		Left	L	10	0.03	7.4	Α	2.4
			NB	Through	2T	717	0.37	7.3	A	35.3
				Right	R	20	0.02	3.6	A	2.5
				Left	L	0	-	-	-	-
			SB	Through	2T	657	0.34	7.0	Α	31.8
				Right	R	10	0.01	4.1	Α	1.8

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inte	rsection		0.47	10.1	В	-
				Left						
			EB	Through						
				Right						
				Left	L	7	0.02	18.6	В	3.6
	Highway 28 / 55 & 69	Signalized <sup>2</sup>	WB	Through						
203	Avenue	Olgridiized		Right	R	104	0.30	6.8	Α	10.1
	rtvondo			Left						
			NB	Through	2T	644	0.47	15.3	В	49.4
				Right	R	26	0.04	5.9	Α	4.2
				Left	L	118	0.28	6.0	Α	10.4
			SB	Through	2T	650	0.34	6.4	Α	28.1
				Right						
				Overall Inte	rsection		0.29	8.3	Α	
				Left						
			EB	Through						
				Right						
				Left	L	83	0.28	19.8	В	20.5
	Highway 28 / 55 & Tri-	Signalized <sup>2</sup>	WB	Through						
204	City Mall Access	Olgrializeu		Right	R	10	0.04	11.2	В	3.5
	Oity Mail ACCESS			Left						
			NB	Through	TTTR	659	0.29	9.9	Α	32.6
				Right		72	-	-	-	-
				Left	L	55	0.13	4.9	Α	5.6
			SB	Through	2T	602	0.28	5.0	Α	25.4
				Right						
				Overall Inte	rsection		0.31	4.6	Α	
				Left			0.01	1.0	7.	
			EB	Through						
			LD	Right						
				Left	L	37	0.15	13.7	В	8.4
		Only Option: Signalize	WB	Through		31	0.13	13.7	Б	0.4
205	Highway 28 / 55 & 62	Intersection.	VVD	Right	R	8	0.03	8.5	A	2.5
203	Avenue / 61 Avenue	Channelize NB R		Left	N.	0	0.03	0.0	A	2.5
		Charmenze NB K	NB		2T	723	0.31	4.5	۸	33.3
			IND	Through	R [C]		0.31	2.0	A	
				Right		70	0.07		A	4.3 2.0
			CD	Left	L OT	9		5.6	A	
			SB	Through	2T	676	0.29	4.4	Α	30.7
				Right			0.07	00.0		
				Overall Inte		F.4	0.67	20.6	С	40.7
			ED	Left	L	51	0.14	15.9	В	12.7
			EB	Through	T	51	0.15	25.3	C	16.8
				Right	R	31	0.10	9.4	A	6.5
			14/5	Left	L T	71	0.18	15.7	В	16.5
00.1	Highway 28 / 55 & 54	Signalized <sup>2</sup>	WB	Through	T	61	0.15	23.7	С	19.0
301	Avenue	- 5		Right	R	153	0.34	6.2	A	12.7
				Left	L	31	0.15	21.3	С	11.7
			NB	Through	2T	602	0.67	26.1	С	80.0
				Right	R	61	0.14	6.8	Α	9.1
				Left	L	163	0.46	25.1	С	44.7
			SB	Through	2T	581	0.53	19.8	В	84.1
				Right	R	61	-	-	-	-
				Overall Inte	rsection		0.70	13.3	В	
				Left		30	-	-	-	-
			EB	Through	LTR	30	0.22	15.0	В	13.4
				Right		10	-	-	-	-
			_	Left		50	-	-	-	-
	Highway 20 / FF 9 FO	Only Option: Signalize	WB	Through	LTR	30	0.39	13.1	В	18.7
302	Highway 28 / 55 & 52	Intersection		Right		55	-	-	-	-
	Avenue	Channelize NB R		Left		70	-	-	-	-
			NB	Through	LTTR [C]	615	0.59	11.4	В	60.7
				Right		80	-	-	-	-
				Left		140	-	-	-	-
			SB	Through	LTTR	510	0.70	15.3	В	#73.7
				Right		40	-	-	-	-
				Nigrit		+∪	_	-	_	_

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inte	rsection		0.76	14.3	В	
				Left	L	375	0.76	22.5	С	76.5
			EB	Through	T	329	0.42	11.4	В	48.0
				Right	R [C]	135	0.19	2.3	Α	7.1
			WD	Left		9	-	-	-	-
000	Highway 28 / 55 & 50	3	WB	Through	LT	161	0.22	9.3	A	24.4
303	Avenue	Signalized <sup>3</sup>		Right	R [C]	155	0.21	2.2	A	7.5
			ND	Left	L	133	0.40	22.9	С	29.3
			NB	Through	2T	275	0.23	17.3	В	22.8
				Right	R [C]	162	0.01	10.8	В	2.3
			0.0	Left	L	116	0.44	23.5	С	33.1
			SB	Through	2T	232	0.28	17.6	В	26.7
				Right	R [C]	5	0.30	4.7	A	11.4
				Overall Inte	rsection		0.71	16.5	С	
			- F-D	Left		16	0.52	14.5	В	-
			EB	Through	LTTR	486	0.57	15.1	С	-
				Right		50	0.57	15.7	C	-
		0-44-04-4-4	WD	Left		98	0.71	21.8	С	-
204	Highway 28 / 55 & 52	Option 1: Convert to 4-	WB	Through	LTTR	523	0.71	18.7	C	-
304	Street	way stop.		Right		42	0.58	15.5	С	-
		Channelize NB R	ND	Left	1 TD (O)	23	0.21	11.1	В	-
			NB	Through	LTR [C]	10	0.21	11.1	В	-
				Right		66	0.21	11.1	В	-
			CD	Left	1.70	21	0.12	10.7	В	-
			SB	Through	LTR	8	0.12	10.7	В	-
				Right		27	0.12	10.7	В	-
				Overall Inte	rsection	40	0.66	11.8	В	
			- F-D	Left		16	-	-	-	- 07.0
			EB	Through	LTTR	486	0.46	10.5	В	37.6
		Ontion 2: Signalize		Right		50	-	-	-	-
	Highway 28 / 55 & 52 Street		WD	Left		98	-		-	-
004		Option 2: Signalize	WB	Through	LTTR	523	0.66	14.0	В	51.8
304		intersection. Channelize NB R		Right		42	-	-	-	-
			ND	Left	1 TD (O)	23	-	- 7.0	-	-
			NB	Through	LTR [C]	10 66	0.22	7.0	Α	10.3
				Right		21	-	-	-	-
			SB	Left	LTD	8	- 0.42	- 0.5	-	- 0.4
			SB	Through	LTR		0.13	8.5	Α	8.1
				Right Overall Inte	roootion	27		-	-	-
				Left	ISECTION		0.22	0.2	Α	
			EB		OΤ	634	0.00	0.0	^	0.0
			ED	Through	2T	034	0.22	0.0	Α	0.0
				Right						
		Unsignalized	WB	Left	эт	E04	0.17	0.0		0.0
305	Highway 28 / 55 & 51	Stop Control - SB	VVD	Through Right	2T R [C]	504 23	0.17 0.02	0.0	A A	0.0
303	Street	Approach		Left	r [U]	۷۵	0.02	0.0	Α	0.0
		Арргоасті	NB	Through						
			140	Right						
			SB	Left Through						
			JD.	Right	R [C]	19	0.03	10.3	В	0.8
<b>-</b>				Overall Inte		19			В	U.O
ĺ				Left	SECTION		0.70	12.3	D	
			EB	Through						
			LD	Right						
				Left	L	239	0.43	16.3	В	44.6
			WB	Through		233	0.43	10.3	ь	74.0
306	Highway 28 / 55 & 50	Signalized <sup>3</sup>	440	Right	R	144	0.25	4.0	A	9.9
300	Street	Signalized			ĸ	144	0.25	4.0	A	9.9
			NID	Left	οT	202	0.20	10.4	В	22.2
			NB	Through	2T	383	0.29	10.4	В	23.3
				Right	R	188	0.28	2.6	Α	8.2
			CD.	Left	177	215	- 0.70	- 40.7	- D	- 47.5
			SB	Through	LTT	419	0.70	16.7	В	47.5
l	1			Right					1	1

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inte	rsection		0.22	1.0	Α	
				Left	L	11	0.09	33.2	D	2.4
			EB	Through						
				Right	R	34	0.07	11.4	В	1.7
		l la cian eliza d	WB	Left						1
307	Highway 28 / 55 & 46	Unsignalized Stop Control - EB/WB	WD	Through Right	R [C]	10	0.02	10.7	В	0.4
307	Avenue	Approaches		Left	IV [O]	33	0.02	0.6	A	1.2
		7.155.000.100	NB	Through	LTTR [C]	550	0.21	0.8	A	1.2
				Right		32	0.21	0.0	A	0.0
				Left						
			SB	Through	2T	649	0.22	0.0	Α	0.0
				Right	R	10	0.01	0.0	Α	0.0
				Overall Inte	rsection		0.38	9.9	Α	
				Left	L	78	0.19	13.1	В	15.8
			EB	Through	TR	29	0.12	7.5	Α	9.5
				Right		35	-	-	-	-
				Left	L	114	0.28	14.0	В	2.2
000	Highway 28 / 55 & 43	03	WB	Through	T	49	0.08	11.9	В	10.6
308	Avenue	Signalized <sup>3</sup>		Right	R	73	0.14	4.4	A	7.1
			ND	Left	L	19	0.05	8.9	A	4.2
			NB	Through	TTR	464 67	0.33	9.6	A -	28.4
				Right Left	L	137	0.38	14.2	В	23.0
			SB	Through	2T	474	0.36	9.7	A	26.0
			OD.	Right	R	72	0.29	2.8	A	5.0
				Overall Inte		12	0.10	4.2	A	3.0
				Left			0.13	4.2	Α	
			EB	Through						
				Right						
		Unsignalized Stop Control - WB Approach		Left		82	0.13	9.8	А	3.5
	50.4 0.57.0		WB	Through	LR					
309	52 Avenue & 57 Street			Right		13	0.13	9.8	Α	3.5
	(North)			Left						
			NB	Through	TR	31	0.08	0.0	Α	0.0
				Right		82	0.08	0.0	Α	0.0
				Left		9	0.01	0.1	Α	0.2
			SB	Through	LT	20	0.01	2.4	Α	0.2
				Right						
				Overall Inte	rsection	1	0.07	2.5	Α	
				Left		46	0.07	10.0	A	1.8
			EB	Through	LR	4	0.07	40.0		4.0
				Right		1	0.07	10.0	A	1.8
		Unsignalized	WB	Left						
310	52 Avenue & 57 Street	Stop Control - EB	44.0	Through Right						
010	(South)	Approach		Left		11	0.01	0.1	A	0.2
		Αρρισασί	NB	Through	LT	66	0.01	1.1	A	0.2
				Right		30	5.51		, , , , , , , , , , , , , , , , , , ,	J.E.
				Left					İ	
			SB	Through	TR	46	0.07	0.0	Α	0.0
				Right		55	0.07	0.0	Α	0.0
				Overall Inte	rsection		0.08	6.0	Α	
				Left		2	0.00	0.0	Α	0.0
			EB	Through	LTR	10	0.00	0.7	Α	0.0
				Right		10	0.00	0.7	Α	0.0
				Left		12	0.01	0.1	Α	0.2
		Unsignalized	WB	Through	LTR	14	0.01	2.2	Α	0.2
311	50 Avenue & 59 Street	Stop Control - NB/SB		Right		14	0.01	2.2	Α	0.2
		Approaches		Left	L	22	0.08	9.5	A	2.2
			NB	Through	LTR	22	0.08	9.5	A	2.2
				Right		19	0.08	9.5	A	2.2
			SB	Left	LTD	6	0.02	9.6	A	0.5 0.5
			SB	Through	LTR	6	0.02	9.6	A	
				Right	<u> </u>	1	0.02	9.6	Α	0.5

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inter	rsection		0.12	7.2	Α	
				Left		22	0.02	0.1	Α	0.4
			EB	Through	LTR	5	0.02	3.4	A	0.4
		Unsignalized Stop Control - NB/SB		Right		22	0.02	3.4	A	0.4
			WB	Left	LTR	<u>8</u> 8	0.01 0.01	0.0	A	0.1
312	50 Avenue & 57 Street		VVD	Through Right	LIK	18	0.01	1.8 1.8	A A	0.1 0.1
312	30 Avenue & 37 Street	Approaches		Left		37	0.01	10.4	В	3.4
		присаснос	NB	Through	LTR	37	0.12	10.4	В	3.4
				Right		8	0.12	10.4	В	3.4
				Left		5	0.06	9.5	Α	1.7
			SB	Through	LTR	21	0.06	9.5	Α	1.7
				Right		21	0.06	9.5	Α	1.7
				Overall Inter	rsection		0.73	16.0	С	
				Left		45	0.73	20.2	С	-
			EB	Through	LTTR	775	0.73	18.6	С	-
				Right		17	0.68	17.1	С	-
		Option 1: Convert to 4-	WD	Left	LTTD	1	0.33	10.5	В	-
313	Centre Avenue & 59	way stop. Provide	WB	Through Right	LTTR	353 13	0.35 0.35	10.6 10.7	B B	-
313	Street	additional lane on		Left		5	0.35	9.5	A	-
		EB/WB approaches.	NB	Through	LTR	5	0.03	9.5	A	-
			110	Right	LIIX	7	0.03	9.5	A	-
				Left		17	0.06	9.8	A	-
			SB	Through	LTR	5	0.06	9.8	A	-
				Right		6	0.06	9.8	Α	-
				Overall Inter	rsection		0.87	17.6	В	
				Left		45	-	-	-	-
			EB	Through	LTR	775	0.87	21.9	С	#229.6
				Right		17	-	-	-	-
		\$ 59 <b>Option 2:</b> Signalize intersection –		Left		1		-	-	
040	Centre Avenue & 59		WB	Through	LTR	353	0.37	7.3	Α	52.0
313	Street			Right		13	-	-	-	-
			NB	Left Through	LTR	5 5	0.06	20.5	- C	6.8
			ND	Right	LIK	7	-	-	-	-
				Left		17	-	-	-	_
			SB	Through	LTR	5	0.11	23.4	С	10.2
				Right		6	-	-	-	-
				Overall Inter	rsection		0.81	19.8	С	
				Left		44	0.81	26.8	D	-
			EB	Through	LTTR	808	0.81	24.4	С	-
				Right		17	0.75	22.0	С	-
		Option 1: Convert to 4-		Left		37	0.43	12.7	В	-
0.1.1	Centre Avenue & 57	way stop. Provide	WB	Through	LTTR	351	0.43	12.3	В	-
314	Street	additional lane on		Right		32	0.41	11.9	В	-
		EB/WB approaches.	ND	Left	LTD	5	0.08	9.9	A	-
			NB	Through	LTR	6 27	0.08	9.9 9.9	A A	-
				Right Left		26	0.08	10.4	В	-
			SB	Through	LTR	9	0.11	10.4	В	-
			SB	Right	LIIX	17	0.11	10.4	В	-
				Overall Inter	rsection	.,	0.88	17.9	В	-
				Left		44	-	-	-	-
			EB	Through	LTR	808	0.88	22.6	С	#244.7
				Right		17	-	-	=	-
				Left		37	-	-	-	-
	Centre Avenue & 57	Option 2: Signalize	WB	Through	LT	351	0.44	8.3	Α	61.2
314	Street	intersection		Right	R	32	0.04	2.3	Α	3.3
	200.		ND	Left		5	-	-	-	-
			NB	Through	LTR	6	0.14	14.4	В	9.1
				Right		27	-	-	-	-
			SB	Left	LTR	26	- 0.22	- 21.0	- C	14.4
			SB	Through	LIK	9 17	0.22	21.9	-	
				Right		17	-	-		-

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inte	rsection		0.13	1.8	А	,,
				Left						
			EB	Through	TR	143	0.13	0.0	Α	0.0
				Right		46	0.13	0.0	Α	0.0
				Left		3	0.00	0.0	Α	0.1
		Unsignalized	WB	Through	LT	114	0.00	0.2	Α	0.1
315	54 Avenue & 51 Street	Stop Control - NB		Right			0.40	444	1	0.7
		Approach	NB	Left	LR	56	0.10	11.1	В	2.7
			IND	Through Right	LK	1	0.10	11.1	В	2.7
				Left		ı	0.10	11.1	В	2.1
			SB	Through						
			02	Right						
				Overall Inte	rsection		0.16	3.1	А	
				Left		71	0.07	0.7	A	1.8
			EB	Through	LTR	270	0.07	2.2	A	1.8
				Right		15	0.07	2.2	Α	1.8
				Left		2	0.00	0.0	Α	0.0
		Unsignalized	WB	Through	LTR	231	0.00	0.1	Α	0.0
316	50 Avenue & 53 Street	Stop Control - NB/SB		Right		19	0.00	0.1	Α	0.0
		Approach		Left		14	0.09	18.2	С	2.3
			NB	Through	LTR	3	0.09	18.2	С	2.3
				Right		6	0.09	18.2	С	2.3
				Left		21	0.16	15.9	С	4.7
			SB	Through	LTR	3	0.16	15.9	С	4.7
				Right		32	0.16	15.9	С	4.7
				Overall Inte	rsection	40	0.45	11.1	В	
				Left		48	0.45	12.0	В	-
			EB	Through	LTR	208	0.45	12.0	В	-
				Right		18	0.45	12.0	В	-
		Unsignalized et Stop Control - All Approaches	WB	Left	LTR	20 178	0.38 0.38	11.1 11.1	B B	-
317	50 Avenue & 52 Street		WD	Through Right	LIK	39	0.38	11.1	В	-
317	30 Avenue & 32 Street			Left		26	0.36	9.6	A	-
			NB	Through	LTR	31	0.15	9.6	A	_
				Right	LIIV	24	0.15	9.6	A	-
				Left		47	0.23	10.1	В	-
			SB	Through	LTR	35	0.23	10.1	В	-
				Right		48	0.23	10.1	В	-
				Overall Inte	rsection		0.47	11.2	В	
				Left		62	0.47	12.5	В	-
			EB	Through	LTR	211	0.47	12.5	В	-
				Right		19	0.47	12.5	В	-
				Left		8	0.33	10.6	В	-
		Unsignalized	WB	Through	LTR	164	0.33	10.6	В	-
318	50 Avenue & 51 Street	Stop Control - All		Right		26	0.33	10.6	В	-
		Approaches	NE	Left	1.75	28	0.14	9.5	A	-
			NB	Through	LTR	28	0.14	9.5	A	-
				Right		23 74	0.14 0.23	9.5	A	-
			SB	Left Through	LTR	32	0.23	10.3 10.3	B B	-
			SD	Right	LIK	32 24	0.23	10.3	В	-
				Overall Inte	rsection	<b>4</b> 4	0.23	10.3	В	-
				Left	55561011	0	-	-	- B	-
			EB	Through	LTR	193	0.46	11.4	В	-
				Right		114	0.46	11.4	В	-
		Unsignalized		Left		83	0.32	10.4	В	-
		Stop Control -	WB	Through	LTR	112	0.32	10.4	В	-
319	50 Avenue & 50 Street	EB/WB/NB Approaches		Right		0	-	-	-	-
		Assumed Yield Control -		Left		83	0.31	10.4	В	-
		SB Approach	NB	Through	LTR	0	-	-	-	-
				Right		109	0.31	10.4	В	-
				Left		0	-	-	-	-
		i	SB	Through	LTR	0	-	-	-	-
			SD	Right	LIIV	0				

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inte	rsection		0.32	5.5	Α	
				Left		50	0.04	0.4	Α	1.1
			EB	Through	LTR	253	0.04	1.5	Α	1.1
				Right		25	0.04	1.5	A	1.1
		11 1 1 1	W.D	Left		23	0.02	0.2	A	0.5
200	FO Avenue 8 40 Ctreet	Unsignalized	WB	Through	LTR	135	0.02	1.2	A	0.5
320	50 Avenue & 49 Street	Stop Control - NB/SB		Right		29	0.02	1.2	A	0.5
		Approaches	NB	Left	LTD	10 28	0.19 0.19	15.5	C	5.5 5.5
			IND	Through Right	LTR	31	0.19	15.5 15.5	C	5.5
				Left		40	0.19	17.6	C	10.9
			SB	Through	LTR	23	0.32	17.6	C	10.9
			0.5	Right	LIIX	53	0.32	17.6	C	10.9
				Overall Inte	rsection	55	0.08	2.1	A	10.5
				Left		34	0.03	0.3	A	0.7
			EB	Through	LTR	261	0.03	1.0	A	0.7
				Right		35	0.03	1.0	A	0.7
				Left		7	0.01	0.1	A	0.2
		Unsignalized	WB	Through	LTR	146	0.01	0.4	Α	0.2
321	50 Avenue & 45 Street	Stop Control - NB/SB		Right		4	0.01	0.4	Α	0.2
		Approaches		Left		14	0.08	13.6	В	2.1
			NB	Through	LTR	3	0.08	13.6	В	2.1
				Right		14	0.08	13.6	В	2.1
				Left		9	0.06	12.3	В	1.6
			SB	Through	LTR	4	0.06	12.3	В	1.6
				Right		15	0.06	12.3	В	1.6
				Overall Inte	rsection		0.08	2.9	Α	
				Left		72	0.06	0.5	Α	1.5
			EB	Through	LT	168	0.06	2.7	Α	1.5
				Right						
	50 Avenue & 41 Street	Unsignalized Stop Control - SB Approaches		Left						
			WB	Through	TR	106	0.08	0.0	Α	0.0
322				Right		8	0.08	0.0	Α	0.0
			ND	Left						
			NB	Through						-
				Right		4.4	0.00	40.4		0.0
			SB	Left	LR	14	0.08	10.4	В	2.2
			SD	Through Right	LK	38	0.08	10.4	В	2.2
-				Overall Inte	reaction	30	0.08	5.3	A	2.2
				Left	136011011	82	0.11	0.5	A	1.5
			EB	Through	LT	69	0.06	4.3	A	1.5
				Right		- 00	0.00	7.5	7.	1.0
				Left						
	50 Avenue / Twp Rd 630	Unsignalized	WB	Through	TR	26	0.03	0.0	Α	0.0
323	& "Baywood Road" / RR	Stop Control - SB	. –	Right	<u> </u>	11	0.03	0.0	A	0.0
	20	Approaches		Left				-		-
		• • •	NB	Through						
				Right						
				Left		18	0.11	9.4	Α	3.0
			SB	Through	LR					
				Right		70	0.11	9.4	А	3.0
				Overall Inte	rsection		0.54	8.4	A	
				Left						
			EB	Through	TR	657	0.54	9.4	Α	102.9
				Right		6	-	-	-	-
				Left	L	5	0.01	6.4	Α	1.7
40.	Kingsway & Medley		WB	Through	Т	268	0.22	5.8	Α	31.5
401	Road	Signalized <sup>3</sup>		Right					_	
			NO	Left		16	0.11	10.9	В	8.0
			NB	Through	LR					
				Right		20	-	-	-	-
			SB	Left			l		<b>l</b>	
			SB	Through						
	i l		Ì	Right			l	1	1	

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inte	rsection		0.74	13.6	В	
				Left	L	153	0.33	10.5	В	21.7
			EB	Through	Т	608	0.74	17.1	В	92.6
				Right						
			WD	Left	-		0.00	0.4		00.0
402	Kingsway & Glenwood	0: 1: 13	WB	Through	T	261	0.32	9.4	A	30.9
402	Drive (East)	Signalized <sup>3</sup>		Right	R [C]	107	0.15	2.0	Α	5.4
			NB	Left Through						
			ND	Right						
				Left		131	0.29	18.9	В	29.5
			SB	Through	LR	.0.	0.20	10.0	_	25.0
				Right		1	-	-	-	-
				Overall Inte	rsection		0.51	0.4	Α	
				Left						
			EB	Through	TR	749	0.51	0.0	Α	0.0
				Right		1	0.51	0.0	Α	0.0
				Left	L	5	0.01	9.8	Α	0.2
	Kingsway & Glenwood	Unsignalized	WB	Through	Т	257	0.18	0.0	Α	0.0
403	Drive (West)	Stop Control - NB/SB		Right						
	Dilve (vvest)	Approaches		Left	L	6	0.04	20.6	С	1.1
			NB	Through						
				Right	R	11	0.04	20.6	С	1.1
				Left						
			SB	Through						
				Right	R	0	-	-	-	-
				Overall Inte	rsection		0.45	10.2	В	
				Left	L	20	0.05	9.7	A	4.4
			EB	Through	TR	277	0.45	13.4	В	37.6
				Right		10	-	- 44.7	-	-
		Signalized <sup>3</sup>	WB	Left	L	66	0.21	11.7	В	11.2
404	Kingsway & Timberline		VVD	Through	TR	161 49	0.34	11.1	В	25.7
404	Drive			Right Left		6	-	-	-	-
			NB	Through	LT	5	0.02	9.3	A	3.0
			ND	Right	R	161	0.02	3.1	A	8.1
				Left	- 1	51	-	-	-	-
			SB	Through	LTR	4	0.12	9.6	Α	9.3
			0.5	Right	LIII	5	-	-	-	-
				Overall Inte	rsection	ŭ	0.26	8.7	Α	
				Left		3	0.08	8.6	A	-
			EB	Through	LTR	45	0.08	8.6	Α	-
				Right		0	-	-	-	-
				Left		24	0.23	8.8	Α	-
		Unsignalized	WB	Through	LTR	61	0.23	8.8	Α	-
405	Kingsway & Queensway	Stop Control - All		Right		70	0.23	8.8	А	-
		Approaches		Left		3	0.26	8.7	Α	-
			NB	Through	LTR	57	0.26	8.7	Α	-
				Right		127	0.26	8.7	Α	-
				Left		89	0.16	8.8	Α	-
			SB	Through	LTR	9	0.16	8.8	A	-
				Right	<u>.</u>	2	0.16	8.8	A	-
				Overall Inte	rsection		0.10	8.3	A	6.5
1			F5	Left	1.70	14	0.10	9.9	A	2.8
1			EB	Through	LTR	46	0.10	9.9	A	2.8
			-	Right Left		14 9	0.10	9.9	A B	2.8
		Unsignalized	WB	Through	LTR	49	0.09	10.0 10.0	В	2.3 2.3
406	Kingsway & Tennis Court	Stop Control - EB/WB	W	Right	LIK	1	0.09	10.0	В	2.3
400	Road	Approaches		Left		24	0.09	0.1	A	0.4
		πρρισασίες	NB	Through	LTR	7	0.02	4.0	A	0.4
				Right	LIIN	13	0.02	4.0	A	0.4
				Left		0	-	-	-	-
			SB	Through	LTR	0	-	-	-	-
				Right		5	0.00	0.0	A	0.0
	1		L	ragiit		J	0.00	0.0		0.0

Node #	Intersection	Traffic Control	Approach	Movement	Laning	Volume	V/C Ratio	Delay (s)	LOS	95th Queue (m)
				Overall Inte	rsection		0.02	1.4	Α	
				Left		1	0.00	0.0	Α	0.0
			EB	Through	LTR	46	0.00	0.2	Α	0.0
				Right		1	0.00	0.2	Α	0.0
				Left		1	0.00	0.0	Α	0.0
	Queensway & Tennis	Unsignalized Stop Control - EB/WB	WB	Through	LTR	68	0.00	0.1	Α	0.0
407	Court Road			Right		5	0.00	0.1	Α	0.0
	Court Road	Approaches		Left		7	0.02	9.4	Α	0.5
			NB	Through	LTR	3	0.02	9.4	Α	0.5
				Right		5	0.02	9.4	Α	0.5
				Left		2	0.01	9.1	Α	0.2
			SB	Through	LTR	0	-	-	-	-
				Right		3	0.01	9.1	Α	0.2

Assumed same timing plan as Highway 28 & 54 Avenue Timing Plan sent from City - May 13, 2010
 Assume timing plan as per Timing Plan sent from City - May 13, 2010

<sup>3.</sup> Assumed timing plan from existing 2010 horizon - same timing plan as Highway 28 & 50 Avenue Timing Plan sent from City - August 31, 2010



**Appendix B - Transportation Study Collision History Review and Analysis** 

# **Technical Memorandum**

## **City of Cold Lake**

**Transportation Study Collision History Review and Analysis** 

### **April 2011**



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## **TECHNICAL MEMORANDUM**

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## Introduction

The City of Cold Lake (City) is located 287 km northeast of the City of Edmonton in Alberta and was formed in 1996 by merging three municipalities, namely Grand Centre, Medley (Canadian Forces Base W4) and Cold Lake. Grand Centre was subsequently renamed Cold Lake South (CLS) and the original Cold Lake is now known as Cold Lake North (CLN).

The City has experienced noticeable growth in recent years. According to municipal census the City had a population of 11,991 in 2006 and 13,924 in 2009. This corresponds to a 5.4% linear growth annually. Current transportation improvements within the City have been based on the previous transportation study completed in 2000 and is no longer considered representative of the actual transportation network required to address current and future transportation needs.

In light of the continuing accelerated pace of development in the region and the need to rationalize and identify the transportation network requirements for the City, including surrounding rural municipalities and counties, the existing transportation plan requires a comprehensive update.

#### 1.1 STUDY BACKGROUND

Associated Engineering (AE) was retained by the City to update the existing transportation study. The purpose of the transportation study is to provide a comprehensive long-range plan that integrates the transportation infrastructure with requirements of the existing and future land uses. The transportation study will provide the City with a blueprint on which to plan and implement specific transportation network improvement projects over the next 20 years in 5-year, 10-year, 15-year, and 20-year planning horizons.

One component of the transportation study was to review the City's collision history and identify high collision locations. The collision data was also analyzed at the high collision locations to determine potential safety concerns. This technical memorandum documents the review of the collision history and the collision analysis completed for the locations with high collision rates.

#### 1.2 STUDY METHODOLOGY

The following tasks were completed for the collision history review and analysis:

- Collect and review collision data
- Identify high collision locations
- Determine collision distribution patterns at high collision locations
- Determine potential safety concerns at high collision locations
- Recommend improvement options at the Chrysler intersection
- Produce draft and final report.



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## **Collision Data**

Collision data was obtained from Alberta Transportation (AT) for collisions which occurred in the City between 2005 and 2009. A total of 2,079 collisions were identified within the 5-year timeframe.

The collision data was analyzed to determine the number of collisions which occurred at each intersection within the City limits, over the 5-year timeframe. Following this exercise, it was discovered that eight (8) of the 2,079 collisions occurred outside the City limits; therefore, a total of 2,071 collisions occurred in Cold Lake between 2005 and 2009. Table 2.1 presents a breakdown of the collisions by year, over the 5-year timeframe.

Table 2.1
Breakdown of Collisions by Year (2005 – 2009)

YEAR	NUMBER OF COLLISIONS	NUMBER OF COLLISIONS			
TEAR		WITHIN CITY	OUTSIDE CITY		
2005	326	326	0		
2006	368	363	5		
2007	436	434	2		
2008	515	514	1		
2009	434	434	0		
TOTAL	2,079	2,071	8		

Intersections with five or more collisions within the 5-year timeframe have been summarized in Table 2.2. The intersections listed in the table have been organized in descending order to present intersections with higher collision frequency first.

## **TECHNICAL MEMORANDUM**

Table 2.2 Intersections with Five or More Collision (2005 - 2009)

RANK	INTERSECTION	5 YEAR TOTAL	2005	2006	2007	2008	2009
1	HWY 28 & 54 AVENUE	42	10	11	9	11	1
-	HWY 28 & UNKNOWN		5	11	5	12	5
2	HWY 28 & TRI CITY MALL INTERSECTION	29	6	6	7	9	1
3	HWY 28 & 50 AVENUE	25	14	3	4	4	0
4	55 STREET & 54 AVENUE	21	0	5	5	4	7
	55A STREET & 54 AVENUE	2	0	0	0	1	1
	COMBINED 55/55A STREET & 54 AVENUE	23	0	5	5	5	8
5	50 STREET & 50 AVENUE	13	2	1	5	3	2
6	50 STREET & 46 AVENUE	13	4	3	2	3	1
7	50 STREET & HWY 28	13	5	2	2	4	0
8	51 STREET & 50 AVENUE	13	1	4	2	5	1
9	50 STREET & 43 AVENUE	11	3	2	2	3	1
10	52 STREET & 50 AVENUE	11	4	2	2	1	2
11	49 STREET & 51 AVENUE	10	2	1	2	0	5
12	HWY 28 & 43 AVENUE	10	1	5	0	3	1
13	HWY 28 & 55 AVENUE	10	1	2	3	3	1
14	HWY 28 & 52 AVENUE	9	1	2	2	4	0
15	HWY 28 & 40 AVENUE	9	0	3	2	2	2
16	16 STREET & 8 AVENUE	8	0	2	1	2	3
17	HWY 28 & 16 AVENUE	8	0	3	3	1	1
18	55 STREET & 50 AVENUE	7	1	0	3	2	1
19	HWY 28 & 61 AVENUE	7	1	3	1	1	1
20	HWY 28 & 75 AVENUE (IMPERIAL PARK ROAD)	7	1	1	5	0	0
21	50 STREET & WAL-MART ENTRANCE	7	0	3	1	3	0
22	50 STREET & 48 AVENUE	7	0	0	3	2	2
23	10 STREET & 8 AVENUE	6	2	0	3	1	0
24	16 STREET & 5 AVENUE	6	2	0	0	2	2
25	HWY 28 & 47 AVENUE	6	1	2	0	3	0
26	HWY 28 & 57 AVENUE	6	2	0	1	3	0
27	12 STREET & 8 AVENUE	5	1	1	1	1	1
28	25 STREET & HWY 28		1	0	1	1	2
29	45 STREET & 50 AVENUE	5	1	0	1	1	2



RANK	INTERSECTION	5 YEAR TOTAL	2005	2006	2007	2008	2009
30	52 STREET & HWY 28	5	1	2	2	0	0
31	HWY 28 & 46 AVENUE	5	1	0	2	1	1
32	16 STREET & 16 AVENUE	5	0	2	2	1	0
33	49 STREET & 50 AVENUE	5	0	1	0	2	2
34	53 STREET & 50 AVENUE	5	0	2	0	1	2
35	HWY 28 & ENERGY CENTRE ROAD (78 AVENUE)	5	0	0	1	0	4

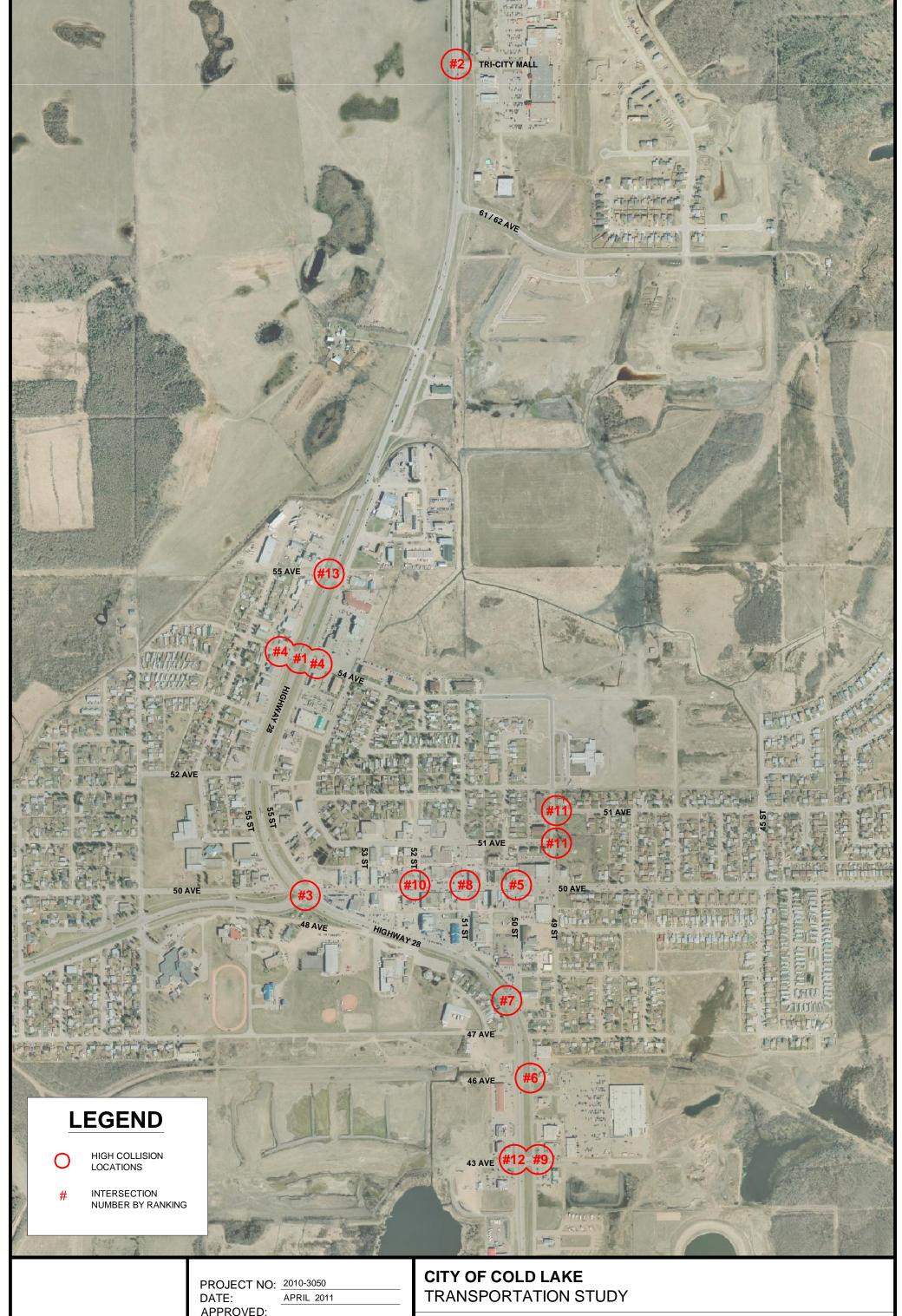
The 'Highway 28 & Unknown' intersection represents collisions which were known to have occurred at an intersection along Highway 28 but where the cross street was unknown. After sorting the data it is unclear if the 38 collisions occurred at one or more intersections. For this reason, the Highway 28 & unknown intersection was excluded from the list of high collision locations.

The intersections of 55 Street/54 Avenue and 55A Street/54 Avenue were considered to be one intersection. Both 55 Street and 55A Street were used interchangeably to identify the service roads located on either side of Highway 28, between 57 Avenue and 50 Avenue. It was unclear whether the intersection of 55 Street/54 Avenue applied to the intersection west of Highway 28, and 55A Street/54 Avenue applied to the intersection east of Highway 28, or vice versa.

49 Street and 51 Avenue is an offset intersection; the westbound approach is located approximately 80 m north of the eastbound approach. The collision data did not differentiate between collisions which occurred on 49 Street and the westbound approach of 51 Avenue and 49 Street and the eastbound approach of 51 Avenue. For this reason, the intersections were treated as one.

Intersections with 10 or more collisions within the 5-year timeframe were identified as high collision locations within the City and selected for further collision analysis. Thirteen intersections met the criteria and have been highlighted in Table 2.2 above and presented in Figure 2.1.





APPROVED: NTS SCALE:

DWG NO:

FIGURE 2.1 HIGH COLLISION LOCATIONS 3

## **Collision Analysis**

The collision data for each high collision location was extracted and provided in Appendix A. The data was further analyzed to determine collision distribution patterns in accordance with the Transportation Association of Canada's The Canadian Guide to In-Service Road Safety Reviews (TAC Guide). The collision distribution patterns analyzed include:

- Temporal collision distributions By year, by month, by day, and by hour
- Type and cause distributions By type, by cause, and by severity
- Environmental distributions By weather conditions, by road surface condition, and by light condition.

The detailed collision analysis completed for each location has been included in Appendix B and summarized in the following sections. The 5-year collision data was analyzed collectively for each location, aside from the collision distribution pattern by year.

#### 3.1 HIGHWAY 28 AND 54 AVENUE

Highway 28 and 54 Avenue had a total of 42 collisions between 2005 and 2009. The key findings from the collision analysis were:

- By Year: Between 2005 and 2008, the number of collisions remained relatively constant with an average of 10 collisions each year. In 2009, there was one collision.
- By Month: More collisions occurred during the winter months (November to February) than the summer months (May to August); 45% in winter versus 33% in summer.
- By Day: The majority of collisions (86%) occurred during the weekday. The highest collision frequency occurred on Friday (21%).
- By Hour: The highest number of collisions occurred between 5:00 p.m. and 6:00 p.m. (17%). The
  majority of collisions (64%) occurred during the afternoon period (between 1:00 p.m. and
  7:00 p.m.).
- By Type: The predominant collision types were 'Left Turn Across Path' (33%) and 'Rear End' (33%).
- By Cause: The predominant collision cause was 'Left Turn Across Path' (21%).
- By Severity: The majority of collisions (79%) were property damage only. There were no fatalities.
- By Weather: The majority of collisions (71%) occurred when the weather was clear.



- By Road Surface Condition: Most collisions (45%) occurred when the road surface was dry.
- By Light Condition: The majority of collisions (81%) occurred during daylight.

#### 3.2 HIGHWAY 28 AND TRI CITY MALL

Highway 28 and Tri City Mall had a total of 29 collisions between 2005 and 2009. The key findings from the collision analysis were:

- By Year: Between 2005 and 2008, the number of collisions remained relatively constant with an average of 7 collisions each year. In 2009, there was only one collision.
- By Month: More collisions occurred during the winter months (November to February) than the summer months (May to August); 59% in winter versus 21% in summer.
- By Day: The majority of collisions (83%) occurred during the weekday. The highest collision frequency occurred on Friday (31%).
- By Hour: The highest number of collisions occurred between 12:00 p.m. and 1:00 p.m. (21%). The majority of collisions (59%) occurred during the afternoon period (between 1:00 p.m. and before 7:00 p.m.).
- By Type: The predominant collision type was 'Rear-End' (69%).
- By Cause: The predominant collision cause was 'Followed Too Closely' (34%).
- By Severity: The majority of collisions (90%) were property damage only. There were no fatalities.
- By Weather: The majority of collisions (76%) occurred when the weather was clear.
- By Road Surface Condition: Most collisions (45%) occurred when there was slush/snow/ice on the road surface.
- By Light Condition: The majority of collisions (83%) occurred during daylight.

#### 3.3 HIGHWAY 28 AND 50 AVENUE

Highway 28 and 50 Avenue had a total of 25 collisions between 2005 and 2009. The key findings from the collision analysis were:

 By Year: Between 2006 and 2008, the number of collisions remained relatively constant with an average of 4 collisions each year. In 2005, there were 14 collisions and in 2009, there were no collisions.

- By Month: More collisions occurred during the winter months (November to February) than the summer months (May to August); 40% in winter versus 28% in summer.
- By Day: The majority of collisions (92%) occurred during the weekday. The highest collision frequency occurred on Thursday (32%).
- By Hour: The highest number of collisions occurred between 7:00 a.m. and 8:00 a.m., between 12:00 p.m. and 1:00 p.m., and between 2:00 p.m. and 3:00 p.m. (12% each). The morning period (between 6:00 a.m. and 12:00 p.m.) and the afternoon period (between 1:00 p.m. and 7:00 p.m.) experienced the same number of collisions (36%).
- By Type: The predominant collision type was 'Rear End' (36%).
- By Cause: The predominant collision cause was 'Left Turn Across Path' (16%).
- By Severity: The majority of collisions (80%) were property damage only. There were no fatalities.
- By Weather: The majority of collisions (68%) occurred when the weather was clear.
- By Road Surface Condition: Most collisions (44%) occurred when the road surface was dry.
- By Light Condition: The majority of collisions (80%) occurred during daylight.

#### 3.4 55/55A STREET AND 54 AVENUE

55/55A Street and 54 Avenue had a total of 23 collisions between 2005 and 2009. The key findings from the collision analysis were:

- By Year: Between 2006 and 2008, the number of collisions remained constant at 5 collisions each year. In 2005, there were zero collisions and in 2009, there were 8 collisions.
- By Month: More collisions occurred during the winter months (November to February) than the summer months (May to August); 52% in winter versus 26% in summer.
- By Day: The majority of collisions (91%) occurred during the weekday. The highest collision frequency occurred on Monday (26%).
- By Hour: The highest number of collisions occurred between 5:00 p.m. and 6:00 p.m. (17%). The
  majority of collisions (57%) occurred during the afternoon period (between 1:00 p.m. and 7:00
  p.m.).
- By Type: The predominant collision type was 'Rear End' (26%).

- By Cause: The predominant collision cause was 'Improper Turn' (13%).
- By Severity: The majority of collisions (83%) were property damage only. There were no fatalities.
- By Weather: The majority of collisions (57%) occurred when the weather was clear.
- By Road Surface Condition: The majority of collisions (52%) occurred when there was slush/snow/ice on the road surface.
- By Light Condition: The majority of collisions (65%) occurred during daylight.

#### 3.5 50 STREET AND 50 AVENUE

50 Street and 50 Avenue had a total of 13 collisions between 2005 and 2009. The key findings from the collision analysis were:

- By Year: There were 2 collisions in 2005, 1 collision in 2006, 5 collisions in 2007, 3 collisions in 2008, and 2 collisions in 2009.
- By Month: More collisions occurred during the winter months (November to February) than the summer months (May to August); 46% in winter versus 31% in summer.
- By Day: The majority of collisions (77%) occurred during the weekday. The highest collision frequency occurred on Friday (46%).
- By Hour: The highest number of collisions occurred between 6:00 p.m. and 7:00 p.m. (23%). The
  majority of collisions (62%) occurred during the afternoon period (between 1:00 p.m. and 7:00
  p.m.).
- By Type: The predominant collision type was 'Rear End' (38%).
- By Cause: The predominant collision cause was 'Backed Unsafely' (31%).
- By Severity: The majority of collisions (85%) were property damage only. There were no fatalities.
- By Weather: The majority of collisions (85%) occurred when the weather was clear.
- By Road Surface Condition: The majority of collisions (62%) occurred when the road surface was dry.
- By Light Condition: The majority of collisions (77%) occurred during daylight.

#### 3.6 50 STREET AND 46 AVENUE

50 Street and 46 Avenue had a total of 13 collisions between 2005 and 2009. The key findings from the collision analysis were:

- By Year: There were 4 collisions in 2005, 3 collisions in 2006, 2 collisions in 2007, 3 collisions in 2008, and 1 collision in 2009.
- By Month: More collisions occurred during the winter months (November to February) than the summer months (May to August); 38% in winter versus 15% in summer.
- By Day: The majority of collisions (69%) occurred during the weekday; however, the highest collision frequency occurred on Saturday (23%).
- By Hour: The highest number of collisions occurred between 5:00 p.m. and 6:00 p.m. (23%). The
  majority of collisions (69%) occurred during the afternoon period (between 1:00 p.m. and 7:00
  p.m.).
- By Type: The predominant collision type was 'Rear End' (38%).
- By Cause: The predominant collision cause was 'Followed Too Closely' (15%).
- By Severity: All the collisions were property damage only.
- By Weather: The majority of collisions (62%) occurred when the weather was clear.
- By Road Surface Condition: Most of the collisions (46%) occurred when the road surface was dry.
- By Light Condition: The majority of collisions (62%) occurred during daylight.

#### 3.7 HIGHWAY 28 AND 50 STREET

Highway 28 and 50 Street had a total of 13 collisions between 2005 and 2009. The key findings from the collision analysis were:

- By Year: There were 5 collisions in 2005, 2 collisions in 2006, 2 collisions in 2007, 4 collisions in 2008, and no collisions in 2009.
- By Month: More collisions occurred during the winter months (November to February) than the summer months (May to August); 54% in winter versus 8% in summer.
- By Day: The majority of collisions (85%) occurred during the weekday.

- By Hour: The highest number of collisions occurred between 8:00 a.m. and 9:00 a.m., and between 11:00 a.m. and 12:00 p.m. (15% each). The majority of collisions (54%) occurred during the morning period (between 6:00 a.m. and 12:00 p.m.).
- By Type: The predominant collision type was 'Rear End' (46%).
- By Cause: The predominant collision causes were 'Stop Sign Violation', 'Followed Too Closely' and "Improper Lane Change' (8% each).
- By Severity: The majority of collisions (85%) were property damage only. There were no fatalities.
- By Weather: The majority of collisions (62%) occurred when the weather was clear.
- By Road Surface Condition: Most of the collisions (46%) occurred when there was slush/snow/ice
  on the road surface.
- By Light Condition: The majority of collisions (69%) occurred during daylight.

#### 3.8 51 STREET AND 50 AVENUE

51 Street and 50 Avenue had a total of 13 collisions between 2005 and 2009. The key findings from the collision analysis were:

- By Year: There was 1 collision in 2005, 4 collisions in 2006, 2 collisions in 2007, 5 collisions in 2008, and 1 collision in 2009.
- By Month: More collisions occurred during the summer months (May to August) than the winter months (November to February); 38% in summer versus 15% in winter.
- By Day: The majority of collisions (85%) occurred during the weekday. The highest collision frequency occurred on Friday (23%).
- By Hour: The highest number of collisions occurred between 4:00 p.m. and 5:00 p.m. (31%). The
  majority of collisions (77%) occurred during the afternoon period (between 1:00 p.m. and 7:00
  p.m.).
- By Type: The predominant collision type was 'Backing' (62%).
- By Cause: The predominant collision cause was 'Backed Unsafely' (54%).
- By Severity: All the collisions were property damage only.
- By Weather: All the collisions occurred when the weather was clear.



- By Road Surface Condition: The majority of collisions (85%) occurred when the road surface was dry.
- By Light Condition: The majority of collisions (77%) occurred during daylight.

#### 3.9 50 STREET AND 43 AVENUE

50 Street and 43 Avenue had a total of 11 collisions between 2005 and 2009. The key findings from the collision analysis were:

- By Year: There were 3 collisions in 2005, 2 collisions in 2006, 2 collisions in 2007, 3 collisions in 2008, and 1 collision in 2009.
- By Month: More collisions occurred during the summer months (May to August) than the winter months (November to February); 36% in summer versus 27% in winter.
- By Day: The majority of collisions (55%) occurred during the weekday; however, the highest collision frequency occurred on Sunday (27%).
- By Hour: The highest number of collisions occurred between 3:00 p.m. and 4:00 p.m. (27%). The
  majority of collisions (64%) occurred during the afternoon period (between 1:00 p.m. and 7:00
  p.m.).
- By Type: The predominant collision type was 'Right Angle' (36%).
- By Cause: The predominant collision cause was 'Stop Sign Violation' (18%).
- By Severity: The majority of collisions (82%) were property damage only. There were no fatalities.
- By Weather: The majority of collisions (55%) occurred when the weather was clear.
- By Road Surface Condition: Most of the collisions (45%) occurred when the road surface was dry.
- By Light Condition: The majority of collisions (82%) occurred during daylight.

#### 3.10 52 STREET AND 50 AVENUE

52 Street and 50 Avenue had a total of 11 collisions between 2005 and 2009. The key findings from the collision analysis were:

 By Year: There were 4 collisions in 2005, 2 collisions in 2006, 2 collisions in 2007, 1 collision 2008, and 2 collisions in 2009.

- By Month: More collisions occurred during the summer months (May to August) than the winter months (November to February); 55% in summer versus 28% in winter.
- By Day: All the collisions occurred during the weekday. The highest collision frequency occurred on Monday (36%).
- By Hour: The highest number of collisions occurred between 12:00 p.m. and, 1:00 p.m. and between 1:00 p.m. and 2:00 p.m. (18% each). Most collisions (36%) occurred during the afternoon period (between 1:00 p.m. and 7:00 p.m.).
- By Type: The predominant collision type was 'Backing' (55%).
- By Cause: The predominant collision cause was 'Backed Unsafely' (45%).
- By Severity: The majority of collisions (82%) were property damage only. There were no fatalities.
- By Weather: The majority of collisions (82%) occurred when the weather was clear.
- By Road Surface Condition: The majority of collisions (64%) occurred when the road surface was dry.
- By Light Condition: The majority of collisions (82%) occurred during daylight.

#### 3.11 49 STREET AND 51 AVENUE

49 Street and 51 Avenue had a total of 10 collisions between 2005 and 2009. The key findings from the collision analysis were:

- By Year: There were 2 collisions in 2005, 1 collision in 2006, 2 collisions in 2007, no collisions in 2008, and 5 collisions in 2009.
- By Month: More collisions occurred during the winter months (November to February) than the summer months (May to August); 70% in winter versus 10% in summer.
- By Day: The majority of collisions (60%) occurred during the weekday.
- By Hour: The highest number of collisions occurred between 5:00 p.m. and 6:00 p.m. (30%). Most collisions (50%) occurred during the afternoon period (between 1:00 p.m. and 7:00 p.m.).
- By Type: The predominant collision types were 'Struck Object' (33%) and 'Right Angle' (33%).
- By Cause: The predominant collision cause was 'Followed Too Closely' (20%).



- By Severity: The majority of collisions (80%) were property damage only. There were no fatalities.
- By Weather: The majority of collisions (70%) occurred when the weather was clear.
- By Road Surface Condition: The majority of collisions (80%) occurred when there was slush/snow/ice on the road surface.
- By Light Condition: The majority of collisions (70%) occurred during daylight.

#### 3.12 HIGHWAY 28 AND 43 AVENUE

Highway 28 and 43 Avenue had a total of 10 collisions between 2005 and 2009. The key findings from the collision analysis were:

- By Year: There was 1 collision in 2005, 5 collisions in 2006, no collisions in 2007, 3 collisions in 2008, and 1 collision in 2009.
- By Month: More collisions occurred during the winter months (November to February) than the summer months (May to August); 70% in winter versus 10% in summer.
- By Day: The majority of collisions (80%) occurred during the weekday. The highest collisions frequency occurred on Thursday (30%).
- By Hour: The highest number of collisions occurred between 1:00 p.m. and 2:00 p.m., and between 6:00 p.m. and 7:00 p.m. (20% each). The majority of collisions (70%) occurred during the afternoon period (between 1:00 p.m. and 7:00 p.m.).
- By Type: The predominant collision type was 'Rear End' (60%).
- By Cause: The predominant collision cause was 'Followed Too Closely' (30%).
- By Severity: The majority of collisions (80%) were property damage only. There were no fatalities.
- By Weather: The majority of collisions (60%) occurred when the weather was clear.
- By Road Surface Condition: Half the collisions occurred when there was slush/snow/ice on the road surface.
- By Light Condition: The majority of collisions (60%) occurred during daylight.

#### 3.13 HIGHWAY 28 AND 55 AVENUE

Highway 28 and 55 Avenue had a total of 10 collisions between 2005 and 2009. The key findings from the collision analysis were:

- By Year: There was 1 collision in 2005, 2 collisions in 2006, 3 collisions in 2007, 3 collisions in 2008, and 1 collision in 2009.
- By Month: More collisions occurred during the winter months (November to February) than the summer months (May to August); 70% in winter versus 10% in summer.
- By Day: The majority of collisions (90%) occurred during the weekday.
- By Hour: The highest number of collisions occurred between 3:00 p.m. and 4:00 p.m., between 4:00 p.m. and 5:00 p.m., and 6:00 p.m. and 7:00 p.m. (20% each). The majority of collisions (80%) occurred during the afternoon period (between 1:00 p.m. and 7:00 p.m.).
- By Type: The predominant collision type was 'Rear End' (40%).
- By Cause: The predominant collision causes were 'Followed Too Closely' (30%) and 'Left Turn Across Path' (30%).
- By Severity: The majority of collisions (70%) were property damage only. There were no fatalities.
- By Weather: Half the collisions occurred when the weather was clear.
- By Road Surface Condition: Half the collisions occurred when there was slush/snow/ice on the road surface.
- By Light Condition: The majority of collisions (80%) occurred during daylight

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### **Potential Safety Concerns**

Potential safety concerns at each high collision location were identified, where possible, from the patterns identified from the collision analysis. In general, the potential safety issues identified in this section are non-conclusive since the collision data did not provide enough detail, with regards to travel direction and other factors, to identify the collision causes and the probable solutions. The high collision locations identified in this memorandum need further detailed analysis to identify the exact cause of the collisions and develop probable engineering solutions. The information presented in this section is intended to highlight potential concerns that should be considered and analyzed in further detail.

#### 4.1 HIGHWAY 28 AND 54 AVENUE

Highway 28 and 54 Avenue is currently a signalized intersection. The collision history revealed that while 42 collisions occurred in the 5-year timeframe, only one collision occurred during 2009. The collisions that occurred before 2009 were primarily 'Left Turn – Across Path' and 'Rear End' collisions. These collision types are typical at intersections where the minor road (i.e., 54 Avenue) is stop-controlled and must rely on gaps on the major road (i.e., Highway 28).

Highway 28 and 54 Avenue will be improved as part of the Highway 28 Twinning project that is currently underway. The improvements will include an additional through lane in both directions on Highway 28 and a dedicated right turn lane on the northbound approach. The intersection should be monitored after the highway twinning project to determine if the improvements will impact the collision frequency.

#### 4.2 HIGHWAY 28 AND TRI CITY MALL

Highway 28 and Tri City Mall is currently a signalized intersection. The collision history revealed that while 29 collisions occurred in the 5-year timeframe, only one collision occurred during 2009. The collisions that occurred before 2009 were primarily 'Rear End' and 'Right Angle' collisions. These collision types are typical at intersections where the minor road (i.e., Tri City Mall) is stop-controlled and must rely on gaps on the major road (i.e., Highway 28)

Highway 28 and Tri-City Mall will be improved as part of the Highway 28 Twinning project that is currently underway. The improvements will include dedicated turn lanes and an additional through lane in both directions on Highway 28. The intersection should be monitored after the highway twinning project to determine if the improvements will impact the collision frequency.

#### 4.3 HIGHWAY 28 AND 50 AVENUE

Highway 28 and 50 Avenue is currently a signalized intersection. The collision history revealed a significant drop in collisions after 2005. There were 14 collisions in 2005, approximately 4 collisions each year between 2006 and 2008, and no collisions in 2009.



The collisions that occurred between 2006 and 2009 where primarily 'Left Turn – Across Path', 'Rear End' and 'Sideswipe – Same Direction'. The 'Left Turn – Across Path' and 'Rear End' collisions could be indicative of possible sight problems associated with the geometry of the intersection and its location on horizontal curves. On the other hand, the 'Left Turn – Across Path' and 'Sideswipe – Same Direction' collisions could be indicative of problems with the intersection signal timing and driver frustration that resulted from the poor signal timing.

The Cold Lake South Arena that was previously located in the southwest corner of Highway 28 and 50 Avenue has moved. In light of the developmental changes adjacent to the study intersection, the City indicated an opportunity to review the intersection for improvements to geometry and lane configuration. The In-Service Safety Review recommended that a detailed intersection analysis be completed for Highway 28 and 50 Avenue to address the safety concerns at the study intersection. Improvements to the intersection geometry and lane configuration should reduce the collision frequency at this intersection.

#### 4.4 55/55A STREET AND 54 AVENUE

55/55A Street and 54 Avenue is currently an un-signalized intersection with stop signs provided for the northbound and southbound approaches on 55/55A Street. Both intersections east and west of Highway 28 have the same traffic control and similar intersection configurations. The collision history revealed a general increase in collisions; there was no collision in 2005, 5 collisions each year between 2006 and 2008, and 8 collisions in 2009. The collisions that occurred were primarily 'Rear End' and 'Right Angle' collisions. These collision types are typical of intersections where the minor road (i.e., 55/55A Street) is stop-controlled and must rely on gaps on the major road (i.e., 54 Avenue).

The intersection of 55/55A Street and 54 Avenue is located close to the intersection of Highway 28 and 54 Avenue. With three intersections located within 65 m, multiple conflict points are generated, turning movements become more complex, and driver workload is significantly increased. These factors could be responsible for the collisions that have occurred at this intersection.

#### 4.5 50 STREET AND 50 AVENUE

50 Street and 50 Avenue is currently an un-signalized intersection with a stop sign provided for the northbound approach on 50 Street. The southbound approach is part of a private parking lot and assumed to be yield control; traffic from driveways are expected to yield to traffic on the roadway. The 13 collisions that occurred were primarily 'Rear End' collisions. 'Rear End' collisions are typical of intersections where the minor road (i.e., 50 Street) is stop-controlled and must rely on gaps on the major road (i.e., 50 Avenue). Angle parking is provided along the south side of 50 Avenue, east and west of 50 Street. The angle parking may lead to rear end collisions with vehicles backing out from the stalls.

#### 4.6 50 STREET AND 46 AVENUE

50 Street and 46 Avenue is currently an un-signalized intersection with stop signs provided for the northbound and southbound approaches on 50 Street. The eastbound approach at 46 Avenue is a right-in-



right-out to Highway 28 and right-of-way at the intersection is provided for traffic entering from and exiting to Highway 28. The westbound approach is an entrance to the Dairy Queen and multiple driveways are located on the east side of 50 street. The 13 collisions that occurred were primarily 'Rear End' collisions. The multiple driveways near the intersection create more conflict points and thus increase driver workload; these factors could be responsible for the collisions the rear end collisions.

#### 4.7 HIGHWAY 28 AND 50 STREET

Highway 28 and 50 Street is currently a signalized intersection. Immediately east of the intersection, 50 Street diverges; one leg continues north to intersect with 50 Avenue and one leg continues south as the service road parallel to Highway 28. A stop sign is provided at the point of divergence, for northbound traffic from the service road. The 13 collisions that occurred were primarily 'Rear End' collisions. Highway 28 and 50 Street is located at the end of a horizontal curve and visibility of the intersection is poor from 50 Street. The 'Rear End' collisions are indicative of possible sight problems associated with the geometry of the intersection.

#### 4.8 51 STREET AND 50 AVENUE

51 Street and 50 Avenue is currently an un-signalized intersection with stop signs provided for all approaches. The 13 collisions that occurred were primarily 'Backing' collisions. Angle parking is provided along the south side of 50 Avenue, east and west of 51 Street, and along the east side of 51 Street, south of 50 Avenue. The 'Backing' collisions are indicative of potential problems with the angle parking provided on these roadways.

#### 4.9 50 STREET AND 43 AVENUE

50 Street and 43 Avenue is currently an un-signalized intersection with stop signs provided for the northbound and southbound approaches on 50 Street. The 11 collisions that occurred were primarily 'Right Angle' collisions. 'Right Angle' collisions are typical at intersections where the minor road (i.e., 50 Street) is stop-controlled and must rely on gaps on the major road (i.e., 43 Avenue). The 50 Street/43 Avenue intersection is located within 50 m of the Highway 28/43 Avenue intersection. The short separation distance does not provide much time for drivers stopped at 50 Street to process and respond to vehicles that have just turned off Highway 28 onto 54 Avenue. This could be a potential cause for the collisions at this intersection.

#### 4.10 52 STREET AND 50 AVENUE

52 Street and 50 Avenue is currently an un-signalized intersection with stop signs provided for all approaches. The 11 collisions that occurred were primarily 'Backing' collisions. Angle parking is provided along the north side of 50 Avenue, west of 52 Street, and along the south side of 50 Avenue, east of 52 Street. Angle parking is also provided along the east side of 52 Street, south of 50 Avenue. The 'Backing' collisions are indicative of potential problems with drivers backing out of the angle parking provided on these roadways.

#### 4.11 49 STREET AND 51 AVENUE

49 Street and 51 Avenue is an offset intersection; the westbound approach is located approximately north of the eastbound approach. The intersection is un-signalized with a yield sign provided for the westbound approach and a stop sign provided for the eastbound approach. The 10 collisions that occurred were primarily 'Struck Object' and 'Right Angle' collisions. 'Right Angle' collisions are typical at intersections where the minor road (i.e., 51 Avenue) is stop/yield-controlled and must rely on gaps on the major road (i.e., 49 Street).

#### 4.12 HIGHWAY 28 AND 43 AVENUE

Highway 28 and 43 Avenue is currently a signalized intersection. The 10 collisions that occurred were primarily 'Rear End' collisions. The 'Rear End' collisions are indicative of potential problems with the signal timing. Insufficient green time could result in driver frustration and motivate drivers to follow too closely behind the leading vehicle. Highway 28/43 Avenue is located within 50m of the 50 Street/43 Avenue intersection. The short separation distance could also be responsible for collisions. Vehicles turning off 50 Street onto westbound 54 Avenue may not have enough time to anticipate and react to traffic on 54 Avenue that have stopped for a red light at Highway 28.

#### 4.13 HIGHWAY 28 AND 55 AVENUE

Highway 28 and 55 Avenue is currently an un-signalized intersection. The 10 collisions that occurred were primarily 'Rear End' collisions. 'Rear End' collisions are typical of intersections where the minor road (i.e., 55 Avenue) is stop-controlled and must rely on gaps on the major road (i.e., Highway 28).

Similar to the intersection of Highway 28/54 Avenue, Highway 28/55 Avenue has service roads (55/55A Street) which run parallel to Highway 28 on both sides of the highway. With three intersections provided within, multiple conflict points are generated, turning movements become more complex, and driver workload is significantly increased. These factors could have led to the rear end collisions.

Highway 28 and 55 Avenue will be improved as part of the Highway 28 Twinning project that is currently underway. The improvements will include dedicated turn lanes in both directions on Highway 28. The intersection should be monitored after the highway twinning project to determine if the improvements will impact the collision frequency.

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### **Chrysler Intersection**

The Cold Lake Chrysler car dealership is located in the northeast corner of Highway 28 and 50 Street. This intersection, shown in Figure 5.1, has been identified by the City and by the collision analysis as a high collision location.

As indicated previously, Highway 28 and 50 Street is currently a signalized intersection. Immediately 30m east of the intersection, 50 Street diverges; one leg continues north to intersect with 50 Avenue and one leg continues south as the service road parallel to Highway 28. A stop sign is provided at the point of divergence, for northbound traffic from the service road. Due to the close proximity of Highway 28 with the service road, there are multiple conflict points within the area and the turning movements between the intersections result in driver confusion.

Through discussions with the City, three options were considered to improve the operation of the Chrysler intersection: provide a roundabout at the service road intersection, provide a right-in-right-out (RIRO) at the service road intersection, or provide a cul-de-sac at the service road intersection. A roundabout at the service road intersection is not feasible due to the close proximity to Highway 28 and the limited right-of-way at the intersection. Therefore, the roundabout option was eliminated.

The City should consider providing a RIRO or cul-de-sac at the service road intersection to improve safety. The RIRO would eliminate some turning movements to and from the service road, while the cul-de-sac would eliminate all turning movements to and from the service road. Both options would reduce the number of conflict points and driver confusion. After implementing the RIRO or cul-de-sac, existing traffic on 50 Street would transfer to Highway 28 or 49 Street via 47 Avenue. A detailed traffic analysis should be completed at the study intersection, and the adjacent intersections, to determine the traffic impact of the RIRO or cul-de-sac. The traffic analysis will determine if any improvements would be required on the adjacent intersections to accommodate the additional traffic volumes from 50 Street.







PROJECT NO: 2010-3050

DATE: APRIL 2011

APPROVED: SCALE: NTS

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APRIL 2011

# CITY OF COLD LAKE TRANSPORTATION STUDY

FIGURE 5.1 CHRYSLER INTERSECTION

#### TECHNICAL MEMORANDUM



### **Conclusion**

Associated Engineering was retained by the City of Cold Lake to undertake a review of the collision history within the City and identify high collision locations. AE also analyzed the collision data at the high collision locations to determine collision distribution patterns and provide the City with potential safety concerns at each location.

Between 2005 and 2009, a total of 2,071 collisions occurred within the City of Cold Lake.

Intersections with 10 collisions or more within the 5-year timeframe were identified as high collision locations. The following intersections were identified as high collision locations:

- 1. Highway 28 and 54 Avenue
- 2. Highway 28 and Tri City Mall
- 3. Highway 28 and 50 Avenue
- 4. 55/55A Street and 54 Avenue
- 5. 50 Street & 50 Avenue
- 6. 50 Street and 46 Avenue
- 7. Highway 28 and 50 Street
- 51 Street and 50 Avenue
- 9. 50 Street and 43 Avenue
- 10. 52 Street and 50 Avenue
- 11. 49 Street and 51 Avenue
- 12. Highway 28 and 43 Avenue
- 13. Highway 28 and 55 Avenue

At the intersections listed above, the following collision distribution patterns were analyzed:

- Temporal Collision Distributions By year, by month, by day, and by hour
- Type and Cause Distributions By type, by cause, and by severity
- Environmental Distributions By weather conditions, by road surface condition, and by light condition.

Detailed collision analysis at each location have been included in Appendix B and summarized in Section 3.

Potential safety concerns at each high collision location were identified, where possible, from the patterns identified from the collision analysis. In general, the potential safety issues identified in this section are non-conclusive since the collision data did not provide enough detail, with regards to travel direction and other factors, to identify the collision causes and the probable solutions.



The high collision locations identified in this memorandum need further detailed analysis to identify the exact cause of the collisions and develop probable engineering solutions. AE recommends that the City conduct in-service safety assessments at each of the high collision locations to understand the underlying causes for the collisions. As part of the in-service safety assessments, the City should obtain the full collision reports from AT to obtain a better understanding of the collision events and to facilitate the identification of the safety issues.

The Chrysler intersection, located at Highway 28 and 50 Street, was identified by the City and by the collision analysis as a high collision location. Through discussions with the City, three options were considered to improve the operation of the Chrysler intersection: provide a roundabout at the service road intersection, provide a right-in-right-out at the service road intersection, or provide a cul-de-sac at the service road intersection. The roundabout option is not feasible due to the close proximity to Highway 28 and the limited right-of-way at the intersection. AE recommends that the City consider providing a right-in-right-out or a cul-de-sac at the service road intersection. Both the right-in-right-out and cul-de-sac would eliminate some, if not all, turning movements to and from the service road, thereby removing some conflict points and reducing driver confusion. A detailed traffic analysis should be completed at the study intersection, and the adjacent intersections, to determine the traffic impact of the right-in-right-out or cul-desac.

Seven of the thirteen high collision locations occurred at intersections with Highway 28 or the service roads (55/55A Street or 50 Street) that run parallel to the highway. The separation distance that are provided between Highway 28 and the service roads are typically within 30m to 65m. Multiple intersections located within a short distance generate more conflict points and significantly increases the driver workload; thus, resulting in a higher frequency of collisions. Similar to the Chrysler intersection, the City should consider closing or providing right-in-right-out, or cul-de-sacs, at the service road and complete a traffic analysis to determine which service road intersections to close and the impact of the road closure on the surrounding road network. Intersection closures typically shift traffic to the adjacent intersections. Traffic analysis is required to determine if any improvements would be required on the adjacent intersections to accommodate the additional traffic volumes.

### **TECHNICAL MEMORANDUM**



## **Appendix A - Alberta Transportation Data**



									ON					OCCURRENCE							
COLLISION CASE NUMBER	CASE YEAR	OBJECT NUMBER	POLICE SERVICE	IN/ NEAR	CITY NAME	NORTH/SOUTH ROAD NAME	EAST/WEST ROAD NAME	ON HWY#	AT INTERSECTION WITH STREET/AVE HWY # NUE	AT INTERSECTION WITH STREET/AVENUE	IF NOT AT INTERSECTION DISTANCE	IF NOT AT INTERSECTION - DESCRIPTION	SPECIAL REFERENCE	DATE (CCYY/MM/DD)	OCCURRENCE HOUR	COLLISION SEVERITY			NUMBER FATALITIES	HIT AND RUN	PRIMARY EVENT
1315753	2005	1	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE				6/2/2005	11	2	2	1	0	2	8
1315753 1637677	2005 2005	2 1	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVE				6/2/2005 1/3/2005	11 13	2	4	3	0	2	8
1637677	2005	2	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE				1/3/2005	13	2	4	3	0	2	8
1637677 1637677	2005 2005	3 4	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVE				1/3/2005 1/3/2005	13 13	2	4	3	0	2	8
1637706	2005	1	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE			INTERSECTION BY LAKELAND INN & EXTRA FOODS	1/11/2005	16	3	2	0	0	2	5
1637706 1637722	2005 2005	2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVE			INTERSECTION BY LAKELAND INN & EXTRA FOODS	1/11/2005 7/21/2005	16 10	3 2	2	0 1	0	2	5 5
1637722	2005	2	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE				7/21/2005	10	2	2	1	0	2	5
1637738 1637738	2005 2005	1 2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVE				1/22/2005 1/22/2005	15 15	3	2	0	0	2	8
1720811	2005	1	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVENUE				8/30/2005	13	3	3	0	0	2	8
1720811	2005 2005	2	1137 1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVENUE				8/30/2005	13 13	3	3	0	0	2	8
1720811 1720963	2005	3 1	1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVENUE 54 AVE				8/30/2005 10/12/2005	16	3	2	0	0	2	8
1720963	2005	2	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE				10/12/2005	16	3	2	0	0	2	8
1720982 1720982	2005 2005	1 2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVE				9/12/2005 9/12/2005	12 12	3	2	0	0	2	11 11
1721219	2005	1	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVENUE				12/20/2005	15	3	3	Ō	0	2	5
1721219 1721219	2005 2005	2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVENUE 54 AVENUE				12/20/2005 12/20/2005	15 15	3	3	0	0	2	5
1721219	2005	1	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE				11/26/2005	17	3	2	0	0	2	5
1721287	2005	2	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE				11/26/2005	17	3	2	0	0	2	5
1721236 1721236	2006 2006	1 2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVE				1/12/2006 1/12/2006	10 10	2	2	2	0	2	3
1721296	2006	1	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE			AT INTERSECTION	6/3/2006	10	2	2	1	0	2	3
1721296 1778739	2006 2006	2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVE			AT INTERSECTION	6/3/2006 3/10/2006	10 9	2	2	1	0	2	3
1778739	2006	2	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE				3/10/2006	9	3	2	0	0	2	5
1778740	2006	1	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVENUE				2/24/2006	18	3	2	0	0	2	12
1778740 1778778	2006 2006	2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVENUE 54 AVE			NORTH	2/24/2006 7/10/2006	18 15	3	2	0	0	2	12 5
1778778	2006	2	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE			NORTH	7/10/2006	15	3	2	0	0	2	5
1778861 1778861	2006 2006	1 2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVE				5/16/2006 5/16/2006	18 18	3	2	0	0	2	8
1778913	2006	1	1137	1	COLD LAK	HWY 28	54 AVENUE	28	i	54 AVE			55 ST	6/16/2006	14	3	2	0	0	2	5
1778913	2006	2	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE			55 ST	6/16/2006	14	3	2	0	0	2	5
1873175 1873175	2006 2006	2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1				N/B IN FRONT OF ESSO-5426-55 ST N/B IN FRONT OF ESSO-5426-55 ST	8/25/2006 8/25/2006	16 16	2	2	1	0	2	1
Z524695	2006	1	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE				3/15/2006	14	3	2	0	0		8
Z524695 Z563444	2006 2006	2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVE				3/15/2006 8/8/2006	14 19	3	2	0	0		8 12
Z563444	2006	2	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE				8/8/2006	19	3	2	0	o		12
Z586822	2006	1	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE				11/9/2006	18	3	2	0	0		5
Z586822 1721140	2006 2007	1	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVENUE				11/9/2006 2/7/2007	18 14	3	2	0	0	2	5
1721140	2007	2	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVENUE				2/7/2007	14	3	2	0	0	2	5
1721160 1721160	2007 2007	1 2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVE				2/17/2007 2/17/2007	17 17	3	2	0	0	2	5
1873118	2007	1	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE				8/7/2007	10	3	2	0	ō	2	5
1873118 1873122	2007 2007	2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVE				8/7/2007 8/10/2007	10 16	3	2	0	0	2	5 97
1873122	2007	2	1137	i	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE				8/10/2007	16	3	3	0	ő	2	97
1873122 1873126	2007 2007	3	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVENUE				8/10/2007 8/24/2007	16 17	3	3	0	0	2	97 5
1873126	2007	2	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVENUE				8/24/2007	17	2	2	2	0	2	5
2040867	2007	1	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE				9/19/2007	17	3	2	0	0	2	12
2040867 2041156	2007 2007	2	1137 1137	1	COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE			54 AVENUE LIGHTS AT EXTRA FOODS	9/19/2007 11/23/2007	17 18	3	2	0	0	2	12 8
2041156	2007	2	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1				54 AVENUE LIGHTS AT EXTRA FOODS	11/23/2007	18	3	2	0	0	2	8
2041861 2041861	2007 2007	1 2	1137 1137	1	COLD LAK COLD LAK	HW Y 28 HW Y 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVE				9/17/2007 9/17/2007	12 12	3	2	0	0	2	5
Z541958	2007	1	1137	1	COLD LAK	HW Y 28	54 AVENUE	28	1	54 TH			LIGHTS @ ESTRA FOODS	11/21/2007	12	3	2	0	0		8
Z541958 2041003	2007 2008	2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1 1	54 TH			LIGHTS @ ESTRA FOODS BTW LAKELAND INN EXTRA FOODS	11/21/2007 2/7/2008	12 17	3	2	0	0	2	8 7
2041003	2008	2	1137	1	COLD LAK	HWY 28	54 AVENUE	28	i				BTW LAKELAND INN EXTRA FOODS	2/7/2008	17	3	2	0	0	2	7
2041040 2041040	2008 2008	1	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVE			(IN CITY) (IN CITY)	4/14/2008 4/14/2008	14 14	3	3	0	0	2	8
2041040	2008	3	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE			(IN CITY) (IN CITY)	4/14/2008	14	3	3	0	0	2	8
2041049	2008	1	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE			IN CITY - DO NOT PLOT	4/18/2008	17	2	3	1	0	2	8
2041049 2041049	2008 2008	2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVE			IN CITY - DO NOT PLOT IN CITY - DO NOT PLOT	4/18/2008 4/18/2008	17 17	2	3	1	0	2	8
2041252	2008	1	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54AVE			LIGHTS IN FRONT OF LAKE LAND INN	7/16/2008	15	3	2	Ô	0	2	12
2041252 2041539	2008 2008	2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54AVE 54 AVE			LIGHTS IN FRONT OF LAKE LAND INN	7/16/2008 6/10/2008	15 16	3	2	0	0	2	12
2041539	2008	2	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE				6/10/2008	16	3	3	0	0	2	5
2041539	2008	3	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE				6/10/2008	16	3	3	0	0	2	5
2041736 2041736	2008 2008	1 2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1 1	54 AVE 54 AVE			IN CITY - DO NOT PLOT IN CITY - DO NOT PLOT	1/29/2008 1/29/2008	9	3	3	0	0	2	8
2041736	2008	3	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE			IN CITY - DO NOT PLOT	1/29/2008	9	3	3	0	0	2	8
2041750 2041750	2008 2008	1 2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54AVE 54AVE			IN CITY - DO NOT PLOT IN CITY - DO NOT PLOT	1/30/2008 1/30/2008	14 14	3	2	0	0	2	13 13
2041785	2008	1	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE				2/29/2008	1	2	1	1	0	2	1
2041785 2209654	2008	2	1137 1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE 54 AVE				2/29/2008	1	2	1	1	0	2	1
2209654 2209654	2008 2008	1 2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVE				11/23/2008 11/23/2008	11 11	3 3	2	0	0	2	8
2209688	2008	1	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1	54 AVE			(IN CITY)	10/2/2008	17	3	2	0	0	2	97
2209688 2209750	2008 2008	2 1	1137 1137	1 1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1 1	54 AVE	1.5	54 AVE	(IN CITY)	10/2/2008 11/3/2008	17 10	3	2	0	0	2	97 97
2209750	2008	2	1137	1	COLD LAK	HWY 28	54 AVENUE	28	1		1.5	54 AVE		11/3/2008	10	3	2	0	0	2	97
2307453 2307453	2009 2009	1 2	1137 1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	54 AVENUE 54 AVENUE	28 28	1	54 AVE 54 AVE				2/22/2009 2/22/2009	12 12	3	2	0	0	2	8
		-				20			-	<u>-</u>					· <del>-</del>	-	-	-	-	-	-

									LI	GHT												
COLLISION CASE NUMBER	SPECIAL FACILITY	ROAD ALIGNMENT ROAD AL A		COLLISION SS LOCATION (	OBJECT TYPE OBJECT ID	DRIVER/PED AGE	DRIVER/PED POINT SEX IMPA	OF DRIVER	LIGHT CON	DITION TRAFFIC C	ONTROL TRAFFIC CON	IDITION PEDESTRIA	N DRIVER/PED C	ONTRIBUTING ROAD CONDITION	ENVIRONMENTAL CONDITION	SURFACE	LOAD DETAILS A	LOAD DETAILS B	ATTACHMENTS	TRAILER TYPE	VEHICLE CONDITION/ CONTRIBUTING FACTORS	UNSAFE SPEEDS
1315753 1315753	1	1	1 2	2	1 1	24	F M 1	10 97	1	97 2	1		1	1	1	1						
1637677	1	1	1 2 1 2	2	3 6 1 3	23 57	F 4	1	1	97 2	1		1	4	1	1 3						
1637677 1637677	1	1	1 2	2	1 1	60 36	F 8	1 1		97 2 97 2	1		1	4 1	1	3						
1637677 1637706	1	1	1 2	2	1 2	72 44	M 8	6		97 2 97 2	1		1 97	1	1	3						
1637706	1	1	1 2	2	1 2	30	М 3	9	-	97 2	1		97	1	1	3						
1637722 1637722	1	1	1 2	2	1 1	45 28	M 1 F 8	9	1 97	97 2 97 97	1 97		1	1 97	1 1	1						
1637738	1	1	1 2	2	1 3	41	F 8 M 4	6	1	97 2	1		1	1	1	3						
1637738 1720811	1	1	1 2	2	1 3	72 17	M 8	97	97	97 2 97 97			97	97	97	97						
1720811 1720811	1	1	1 2	2	1 1	17 22	M 4 M 4	97 97		97 97 97 97	97 97		97 97	97 97	97 97	97 97						
1720963	1	1	1 2	2	1 1	31	F 8	6		97 2 97 2	97 97		1	1	1	1						
1720963 1720982	1	1	1 2	2	1 5	41 36	M 1	10	1	1 2	97		1	4	2	2	1	1	1	1	1	2
1720982 1721219	1	1	1 2	2	1 1	29 45	M 6	1 97	1 97	1 2 97 97	97 97		1 97	4 97	2 97	2 97						
1721219	1	1	1 2	2	1 1	41	M 3	97	97	97 97	97		97	97	97	97						
1721219 1721287	1 97	7	1 2 1 2	2	1 3	34 30	F 7	1 97		97 2 97 2	1		97 1	1	97 1	97						
1721287 1721236	97	7	1 2	2	1 2	43 52	M 3	97	-	97 2 97 1	1		1	1	1	1						
1721236	1	1	1 2	2	1 3	31	F 8	13		97 3	97		1	i	į	2						
1721296 1721296	1 1	1 1	1 2 1 2	2	1 1 1 3	39 16	F 8 M 2	1 9	1 1	97 2	1		1 1	1 1	1	2						
1778739 1778739	1	1	1 2	2	1 1	26 41	F 8	1 9	1	1 2	1		1	1	4	3						
1778740	97	7	1 2	2	1 1	26	M 6	1	1	97 2	1		97	1	1	99						
1778740 1778778	97 1	7	1 2 1 2	2	1 2 1 2	16 41	F 1 F 8	10 1	1 1	97 2	1		97 1	1 1	1 1	99 1						
1778778 1778861	1	1	1 2	2	1 1	46 61	M 3 F 3	9		97 2 97 2	1		1	1	1	1						
1778861	1	1	1 2	2	1 3	48	M	97		97 2	1		97	1	į	1						
1778913 1778913	1	1 1	1 2 1 2	2	1 1	17 18	M 7 F 7	1 9	1	1 2	1		1 1	1	1	1						
1873175 1873175	1	1	1 2	1	1 2	47 40	F 7	1 97		97 1 97 1			1 97	1	1	1						
Z524695	1	1	1 2	2	1 1	18	F 8	31	1	1 2			31	1	1	1						
Z524695 Z563444	1	1 1	1 2 1 2	2	1 2 1 3	27 17	F 4		1	1 2				1	1	1						
Z563444 Z586822	1	1 1	1 2	2	1 1	24 46	M 5		1	1 2				1	1	1						
Z586822	1	i	1 2	2	i i	53	F 2		3	97 2				i	i	3						
1721140 1721140	97 97	1 1	1 2 1 2	2	1 1 1	54 42	M 97 F 6	1 4		97 3 97 3	97 97		1 1	1	1	3						
1721160 1721160	1	1 1	1 2	2	1 1	20 26	M 1	13		97 2 97 2	1		1 97	1	4	3						
1873118	97	1	1 4	2	1 1	57	F 3	9	1	1 2	1		1	į	1	1						
1873118 1873122	97 97	1 1	1 4 1 4	2	1 1	51 20	M 1	1 97	1 97	1 2 97 97	1 97		1 97	1 97	1 97	1 97						
1873122 1873122	97 97	1 1	1 4	2	1 3	30 49	M 3 M 97	97 97		97 97 97 97	97 97		97 97	97 97	97	97 97						
1873126	97	1	1 2	2	1 2	46	M 8	1	1	97 2	1		1	1	1	1						
1873126 2040867	97 1	1 1	1 2 1 2	2	1 1	26 64	F 2 F 5	9 1	1 1	97 2 1 2	1		1	1	1	1						
2040867 2041156	1 97	1 1	1 2 7 97	2 97	1 2	29 48	M 1	10 1	1	1 2	1		1	1 4	1	1						
2041156	97	7 7	7 97	97	1 1	16	F	6		7 2	1		1	4	4	3						
2041861 2041861	1	1 1	1 2	2	1 3	16 51	M 8 M	9 1	1	1 2	1		1	1	1	2						
Z541958 Z541958	1	1 1	1 2	2	1 2	43 18	M 4		1	1 2				1	4	3						
2041003	97 97	1 1	1 2	1	1 2	43	M 2 M 8	1			97		1	4	4	3						
2041003 2041040	1	1 1	1 2	2	1 2 1 1	24 20	M 8	1 97	1	, 1 1 2	1		1	1	1	1						
2041040 2041040	1 1	1 1	1 2 1 2	2 2	1 1 1 3	41 36	F 4 F 4	1	1	1 2 1 1	1		1	1	1 1	1						
2041049	97	1 1	1 2	2	1 1	44	F 4	1		97 2	1		97	1	1	1						
2041049 2041049	97 97	1 1	1 2	2	1 3	32 23	F 4 F 8	1 97	97	97 2 97 97	1 97		97	97	1	1						
2041252 2041252	1 1	1 1	1 2 1 2	1	1 1 1 2	23 45	F 7 M 2	1 10	1	1 1 1 1			1	1 1	1 1	1						
2041539 2041539	97 97	1 1	1 2 1 2	2 2	1 1 1 3	23 40	F 8 M 6	1	1 9	97 2 97 2	1		97 97	1 97	1	1						
2041539	97	1 1	1 2	2	1 1	48	F 8	1	1 9	7 2	1		97	1	1	1						
2041736 2041736	97 97	7 7		97 97	1 2 1 2	37 68	F 4 M 8	1	1	2 2 2 2	1 1		1	1 1	5 5	3						
2041736 2041750	97 97	7 7	7 97	97	1 2	36 25	M 5 M 4	1 8	1 9	7 2 1 2	1 97		1	1	5	3						
2041750	97	1 1	1 4	2	1 1	37	M 8	1	1	1 2	97		1	1	1	3						
2041785 2041785	97 97	1 1	l 2 l 2	2 2	1 2 2 7	51 18	M 1	99	3 9		1	2	2 97	1	1 1	1						
2209654	1	1 1	1 2	2	1 1	23	M 4	1	1 9	99 2	1		1	1	97 97	3						
2209654 2209688	1	1 1	1 2	2 1	1 1	82 31	M F 97	97 97		9 2	1		97 1	97 1	1	1						
2209688 2209750	1 97	1 1	1 2 1 2	1 2	1 3	16	M 97	97 10	1 9	9 2 9 1	1		99 1	1 1	1 2	1 2						
2209750	97	i i	1 2	2 2	1 2	49 42	M 1 F 4	1	1 9	19 1	1		1	1	2	2						
2307453 2307453	1	1 1		2	1 3	18	F 8	6		19 2	1		1	1	1	2						

	0,102	OBJECT	POLICE		AIII.	NORTH/SOUTH			AT INTERSECTION WITH		AT INTERSECTION WITH	IF NOT AT INTERSECTION	IF NOT AT INTERSECTION -		OCCURRENCE DATE	OCCURRENCE		NUMBER OF		NUMBER	
		NUMBER	SERVICE	IN/ NEAR	CITY NAME	ROAD NAME	EAST/WEST ROAD NAME	ON HWY #	HWY #	ON STREET/AVENUE	STREET/AVENUE	DISTANCE	DESCRIPTION	SPECIAL REFERENCE	(CCYY/MM/DD)	HOUR	SEVERITY	VEHICLES	INJURED	FATALITIES	IT AND RUN
	2005	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					TRI CITY MALL	1/13/2005	13	3	2	0	0	2
	2005	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					TRI CITY MALL	1/13/2005	13	3	2	0	0	2
	2005	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					INTERSECTION WITH TRI CITY MALL	2/11/2005	14	3	2	0	0	2
	2005	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					INTERSECTION WITH TRI CITY MALL	2/11/2005	14	3	2	0	0	2
	2005	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		TRI CITY MALL				11/2/2005	17	3	3	0	0	2
721007	2005	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		TRI CITY MALL				11/2/2005	17	3	3	0	0	2
721007	2005	3	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		TRI CITY MALL				11/2/2005	17	3	3	0	0	2
721193	2005	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		66 AVE			AT TRICITY MALL INTERSECTION NORTHBOUND	12/4/2005	16	2	2	1	0	2
721193	2005	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		66 AVE			AT TRICITY MALL INTERSECTION NORTHBOUND	12/4/2005	16	2	2	1	0	2
721234	2005	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					TRI CITY MALL INTERSECTION	1/12/2005	12	3	2	0	0	2
721234	2005	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					TRI CITY MALL INTERSECTION	1/12/2005	12	3	2	0	0	2
	2005	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					BY IGA(SOBEYS)	5/14/2005	16	3	1	0	0	
	2005	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					BY IGA(SOBEYS)	5/14/2005	16	3	1	0	0	
	2006	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		TRI CITY MALL			= ()	3/3/2006	15	3	2	0	0	2
	2006	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		TRI CITY MALL				3/3/2006	15	3	2	0	0	2
	2006	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		66 AVE			TRAFFIC LIGHTS @ TRI CITY MALL	3/3/2006	10	2	2	0	0	2
	2006	2	1137		COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28			66 AVE			TRAFFIC LIGHTS @ TRI CITY MALL	3/3/2006	10	2	2	0	0	2
		1		- 1				28	1					TRAFFIC LIGHTS @ TRI CITT WALL		17	3	2	0	0	2
	2006		1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION		1		65 AVE				7/10/2006		3	3	0	0	2
	2006	2	1137 1137	1	COLD LAK	HWY 28 HWY 28	TRI CITY MALL INTERSECTION	28	1		65 AVE 65 AVE				7/10/2006	17 17	3	3	U	U	2
	2006	3		1	COLD LAK		TRI CITY MALL INTERSECTION	28	1		bo AVE			AT TOUGHT AND LIGHTS	7/10/2006		3	3	U	U	2
	2006	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					AT TRI CITY MALL LIGHTS	8/10/2006	13	3	2	0	0	2
	2006	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					AT TRI CITY MALL LIGHTS	8/10/2006	13	3	2	0	0	2
	2006	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		66 AVE				9/15/2006	21	3	2	0	0	2
373196	2006	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		66 AVE				9/15/2006	21	3	2	0	0	2
373421	2006	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					TRI CITY MALL EXIT/ENTRANCE	12/15/2006	18	3	2	0	0	2
373421	2006	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					TRI CITY MALL EXIT/ENTRANCE	12/15/2006	18	3	2	0	0	2
040868	2007	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					@ TRI CITY MALL	12/14/2007	15	3	2	0	0	2
040868	2007	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					@ TRI CITY MALL	12/14/2007	15	3	2	0	0	2
041106	2007	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					@ TRI CITY MALL INTERSECTION	12/10/2007	7	3	2	0	0	2
041106	2007	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					@ TRI CITY MALL INTERSECTION	12/10/2007	7	3	2	0	0	2
	2007	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					TRI CITY MALL @ INTERSECTION	11/8/2007	6	3	2	0	0	2
	2007	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					TRI CITY MALL @ INTERSECTION	11/8/2007	6	3	2	0	0	2
	2007	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					TRI CITY MALL INTERSECTION	10/1/2007	14	3	2	0	0	2
	2007	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					TRI CITY MALL INTERSECTION	10/1/2007	1/1	3	2	0	0	2
	2007	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					LIGHTS IN FRONT OF MALL	9/17/2007	16	2	2	0	0	2
	2007	2	1137	,	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28						LIGHTS IN FRONT OF MALL	9/17/2007	15	3	3	0	0	2
	2007	3	1137	- 1	COLD LAK	HWY 28		26 28	1					LIGHTS IN FRONT OF MALL	9/17/2007	15	3	3	0	0	2
		3		1			TRI CITY MALL INTERSECTION	28	1							15	3	3	0	0	2
	2007	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION							LIGHTS AT ZELLERS	11/25/2007	12	3	2	0	0	2
	2007	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION							LIGHTS AT ZELLERS	11/25/2007	12	3	2	0	0	2
	2007	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					AT TRI-CITY MALL	12/14/2007	13	3	2	0	0	
	2007	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					AT TRI-CITY MALL	12/14/2007	13	3	2	0	0	
)40958	2008	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		65AVE			TRI CITY MALL LIGHTS	1/9/2008	18	3	2	0	0	1
040958	2008	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		65AVE			TRI CITY MALL LIGHTS	1/9/2008	18	3	2	0	0	1
041056	2008	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					TRI CITY MALL INTERSECTION	5/16/2008	15	3	2	0	0	2
041056	2008	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					TRI CITY MALL INTERSECTION	5/16/2008	15	3	2	0	0	2
041061	2008	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					THE CITY MALL INTERSECTION (IN CITY)	5/22/2008	12	3	3	0	0	2
	2008	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					THE CITY MALL INTERSECTION (IN CITY)	5/22/2008	12	3	3	0	0	2
	2008	3	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					THE CITY MALL INTERSECTION (IN CITY)	5/22/2008	12	3	3	0	0	2
	2008	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28			66 AVE			TRI-CITY MALL INTERSECTION IN CITY - DO NOT PLOT	6/25/2008	12	2	2	0	0	2
	2008	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		66 AVE			TRI-CITY MALL INTERSECTION IN CITY - DO NOT PLOT	6/25/2008	12	3	2	0	0	2
	2008	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		66 AVE			IN CITY - DO NOT PLOT	2/4/2008	13	3	1	0	0	2
	2008	1	1137	1	COLD LAK	HWY 28		28 28	1		TRI CITY MALL TURNOFF				2/4/2008	13	3	1	0	0	2
		-		1			TRI CITY MALL INTERSECTION							IN CITY - DO NOT PLOT		12	3	2	0	0	2
	2008	2	1137		COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		TRI CITY MALL TURNOFF			IN CITY - DO NOT PLOT	2/1/2008		3	2	0	0	2
	2008	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					ENTRANCE TO TRI CITY MALL - IN CITY DO NOT PLOT	2/2/2008	97	3	2	U	U	2
	2008	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					ENTRANCE TO TRI CITY MALL - IN CITY DO NOT PLOT	2/2/2008	97	3	2	0	0	2
	2008	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					TRI-CITY MALL INTERSECTION - IN CITY DO NOT PLOT	3/4/2008	16	2	2	1	0	2
041800	2008	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1					TRI-CITY MALL INTERSECTION - IN CITY DO NOT PLOT	3/4/2008	16	2	2	1	0	2
083165	2008	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		66 AVE			TRI CITY MALL	2/2/2008	12	3	2	0	0	2
83165	2008	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		66 AVE			TRI CITY MALL	2/2/2008	12	3	2	0	0	2
	2009	1	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		66 AVE				1/21/2009	19	2	3	4	0	2
	2009	2	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		66 AVE				1/21/2009	19	2	3	4	0	2
				1					1								2	3	4	0	2
	2009	3	1137	1	COLD LAK	HWY 28	TRI CITY MALL INTERSECTION	28	1		66 AVE				1/21/2009	19	2	3	4		0

JMBER	SPECIAL FACILITY	ROAD ALIGNMENT A	ROAD ALIGNMENT B		COLLISION LOCATION			AGE	DRIVER/PEI				CONDITION B		E TRAFFIC CONDITION DEVICE CONDITION PEDESTRIAN ACTION	DRIVER/PED CONDITION	CONTRIBUTING ROAD CONDITION	ENVIRONMENTAL CONDITION	SURFACE	LOAD DETAILS A	LOAD DETAILS B	ATTACHMENTS	TRAILER TYPE	VEHICLE CONDITION/ CONTRIBUTING FACTORS	UNSAFE SPEEDS
637703	1	1	1	2	2	1	3	73	М	4	1	1	97	2	1	1	1	1	3						
637703	1	1	1	2	2	1	2	28	M F	-	1 10	1	97	2	1	1	1	1	3						
637726 637726	1	1	1	2	2	1	1	63 23	M	5	10	1	97 97	2	1	1	1	1	1						
721007	1	1	1	2	2	1	2	22	M	4	1	1	97	2	1	1	1	1	1						
721007	1	1	1	2	2	1	2	18	M	8	1	1	97	2	1	1	1	1	1						
21007	1	1	1	2	2	1	2	19	M	8	97	1	97	2	1	1	1	1	1						
21193	1	1	1	2	2	1	1	50	F	4	99	99	99	2	1	99	97	1	3						
721193	1	1	1	2	2	1	4	18	M	8	99	99	99	2	1	99	97	1	3						
721234	1	1	1	2	2	1	2	31	М	8	6	1	1	2	1	1	1	1	2						
721234	1	1	1	2	2	1	3	27	М	4	1	1	1	2	1	1	1	1	2						
524820 524820	97 97	7	1	2	1	1	1 13	19	F	9		1	97	1			1 97	1	1						
778727	1	1	1	2	2	1	2	34	м	7	97	1	97	2	1	1	1	1	97						
778727	1	1	1	2	2	1	1	49	M	4	97	1	97	2	1	1	1	1	97						
778742	1	7	1	2	2	1	3	34	F	8	1	1	97	2	97	1	98	1	3						
778742	1	7	1	2	2	1	3	31	F	4	1	97	97	2	97	1	98	1	3						
78780	1	1	1	2	2	1	1	37	M	8	99	1	97	2	1	1	1	2	2						
78780	1	1	1	2	2	1	1	60	F	9	1	1	97	2	1	1	1	2	2						
78780	1	1	1	2	2	1	1	45	F	4	1	1	97	2	1	1	1	2	2						
78817	1	1	1	2	1	1	3	23	F	4	1	1	97	2	1	1	1	1	1						
778817	1	1	1	2	1	1	1	18	F	8	6	1	97	6	1	1	1	1	1						
373196 373196	1	1	1	2	2	1	2	37 17	M M	4	1 98	3 97	2 97	2	1	1	1	2	2	1 2	1				
373421	1	1	1	3	2	1	1	32	F	8	99	1	2	2	97	1	1	1	3	2					
373421	1	1	1	3	2	1	2	17	М	4	1	1	2	2	97	1	i	4	3						
040868	97	1	1	4	2	1	1	30	М	7	1	1	1	2	1	1	1	1	3						
040868	97	1	1	4	2	1	1	45	F		97	1	1	2	1	1	1	1	3						
041106	97	7	7	97	97	1	1	26	M	4	1	1	97	2	1	1	1	1	3						
041106	97	7	7	97	97	1	2	24	M	8	6	1	97	2	1	1	1	1	3						
141219	97	1	1	2	2	1	3	20	F	8	6	3	97	2	1	1	4	4	3	2					
041219	97	1	1	2	2	1	2	38	M	4	1	3	97	2	1	1	4	4	3	2					
41778	1	1	1	2	2	1	2	27	М	8	97	1	1	2	1	1	1	1	1						
141778 141864	1 97	1	1	2	2	1	2	52 19	M	8	97	1	1 97	2	1	1	1	1	1						
041864 041864	97	1	1	2	2	1	3	38	F	4	97	97	97	97	97	97	97	1	1						
)41864	97	1	1	2	2	1	3	16	М	8	97	97	97	97	97	97	97	1	1						
041888	9	•	•	-	-	1	1	41	F	4	1	1	97	2	1	97	98	1	3						
041888	9					1	3	48	F		6	1	97	2	1	97	98	1	3						
449583	97	7	7	97	97	1	3	56	M	8		1	97	1			97	4	3						
449583	97	7	7	97	97	1	2	29	M			1	97	1			97	4	3						
140958	97	1	1	2	2	1	2	68	M	4	1	3	2	2	1	1	1	1	1						
040958	97	1	1	2	2	1	2			8	6	3	2	2	1	99	1	1	1						
41056	1	1	1	2	2	1	2	48	M	4	1	1	97	2	1	99	1	1	1						
041056 041061	1 97	1	1	2	2	1	2	26	M M	8	6	1	97	1		99	1	1	1						
041061	97	1	1	4	2		3	42 32	F	4		1	1	1		1	1	1	1						
041061	97	1	1	4	2	1	2	19	М	8	6	1	97	1		1	1	1	1						
041562	1	1	1	3	2	1	1	21	F	8	1	1	97	2	1	1	1	1	1						
041562	1	1	1	3	2	1	3	46	F.	4	97	1	97	2	1	1	1	1	1						
041654	1	1	1	2	2	1	1	81	M	8	1	1	97	2	1	1	1	1	3						
041757	1	1	1	2	2	1	2	46	M	8	1	1	1	2	1	1	1	4	3						
041757	1	1	1	2	2	1	1	60	M	7	1	1	1	2	1	1	1	4	3						
041760	97	7	7	97	97	1	2	28	M	6	1	1	97	2	1	97	4	97	97						
041760	97	7	7	97	97	1	2	50	М	6	1	1	97	2	1	97	4	97	97						
041800	97	7	7	97	97	1	3	49	М	97	1	1	97	2	1	1	1	1	1						
041800	97	7	7	97	97	1	2	53	M M		6	1	97	2	1	97	1	1	1						
083165	1	1	1	2	2	1	2	41 28	M F	6	1 97	1	1	2	1	1	1	1	3						
083165	!	1	1	2	2	1	3	28 17	H M	4	97	1	1	∠ 1	T.	1	1	1	3						
1/1/03																									
041403 041403	1	1	1	2	2	1	1	42	F	7	1	3	2	1		1	1	1	3						

												IF NOT AT									
COLLISION CASE	CASE	OBJECT	POLICE			NORTH/SOUTH	EAST/WEST		AT INTERSECTION WITH		AT INTERSECTION WITH	INTERSECTION	IF NOT AT INTERSECTION		OCCURRENCE DAT		COLLISION			1	PRIMARY
NUMBER	YEAR	NUMBER	SERVICE	IN/ NEAR	CITY NAME	ROAD NAME	ROAD NAME	ON HWY #	HWY #	ON STREET/AVENUE	STREET/AVENUE	DISTANCE	- DESCRIPTION	SPECIAL REFERENCE	(CCYY/MM/DD)	HOUR	SEVERITY	VEHICLES INJU	RED FATALITI	S HIT AND RUN	EVENT
1552836	2005	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				9/10/2005	20	2	2	2 0	2	8
1552836	2005	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				9/10/2005	20	2	2	. 0	2	8
1637704	2005	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVEEET				1/13/2005	12 12	3	2 (	0	2	8
1637704	2005	2	1137 1137	1	COLD LAK	HWY 28	50 AVENUE	28 28	<u> </u>		50 AVEEET				1/13/2005	12	3	2 (	0	2	
1637727 1637727	2005 2005	2	1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	50 AVENUE 50 AVENUE	28 28	1		50 AVE 50 AVE				2/4/2005 2/4/2005	7	3	2 (		2	5
1637728	2005	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				2/4/2005	7	3	2 (		2	5
1637728	2005	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				2/4/2005	7	3	2 (		2	5
1637769	2005	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE	5	HWY 28 & 50TH AVENUE		3/7/2005	14	3	2 (	, 0	2	5
1637769	2005	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE	5	HWY 28 & 50TH AVENUE		3/7/2005	14	3	2 (	. 0	2	5
1637777	2005	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE	•	TIVI 20 G OUTTAVEROE	IN FRONT OF A & W 5002 55ST	3/17/2005	16	3	2	0	2	8
1637777	2005	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE			IN FRONT OF A & W 5002 55ST	3/17/2005	16	3	2 (	0	2	8
1637779	2005	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				3/18/2005	8	3	2 (	0	2	8
1637779	2005	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				3/18/2005	8	3	2 (	0	2	8
1637781	2005	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				3/17/2005	97	2	2	0	2	8
1637781	2005	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				3/17/2005	97	2	2	0	2	8
1637841	2005	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				5/26/2005	6	3	3 (	0	2	5
1637841	2005	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				5/26/2005	6	3	3 (	0	2	5
1637841	2005	3	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				5/26/2005	6	3	3 (	0	2	5
1720871	2005	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE			TURNING LEFT AT INTERSECTION	6/8/2005	15	3	2 (	0	2	13
1720871	2005	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE			TURNING LEFT AT INTERSECTION	6/8/2005	15	3	2 (	0	2	13
1720914	2005	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50TH AVENUE				7/27/2005	19	3	2 (	0	2	5
1720914	2005	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50TH AVENUE				7/27/2005	19	3	2 (	0	2	5
1721176	2005	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50TH AVENUE			CENTRE	11/29/2005	12	3	2 (	0	2	8
1721176	2005	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50TH AVENUE			CENTRE	11/29/2005	12	3	2 (	0	2	8
1721178	2005	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVENUE				11/29/2005	14	2	3 2	0	2	8
1721178	2005	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVENUE				11/29/2005	14	2	3 2	0	2	8
1721178	2005	3	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVENUE				11/29/2005	14	2	3 2	0	2	8
Z542325	2005	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50TH AVE			@ INTERSECTION NW SIDE HIT L POLE WEST OF HWY 28	11/29/2005	11	3	1 (	0		9
Z542325	2005	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50TH AVE			@ INTERSECTION NW SIDE HIT L POLE WEST OF HWY 28	11/29/2005	11	3	1 (	0		9
1778756	2006	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				5/1/2006	9	3	2 (	•	2	8
1778756	2006	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				5/1/2006	9	3	2 (	0	2	8
1778793	2006	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				7/21/2006	15	2	2 1	0	2	8
1778793	2006	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				7/21/2006	15	2	2 1	0	2	8
1873172	2006	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50TH AVE			MIDDLE OF INTERSECTION	8/29/2006	17	2	2 1	0	1	3
1873172	2006	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50TH AVE			MIDDLE OF INTERSECTION	8/29/2006	17	2	2 1	0	1	3
1528741	2007	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE			CENTRE AVE	10/15/2007	11	3	2 (	0	2	12
1528741	2007	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE			CENTRE AVE	10/15/2007	11	3	2 (	0	2	12
1721047	2007 2007	1	1137 1137	1	COLD LAK	HWY 28 HWY 28	50 AVENUE	28 28	1		50 AVE NB 50 AVE NB				2/23/2007	13	3	2 (	0	2	5
1721047 1873170	2007	2	1137	1	COLD LAK COLD LAK	HWY 28 HWY 28	50 AVENUE 50 AVENUE	28 28	1		50 AVE NB				2/23/2007 3/12/2007	13	3	2 (	0	2	5
1873170	2007	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28 28	1		50 AVE				3/12/2007	7	3	2 (	0	2	5
2040921	2007	4	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				12/16/2007	97	3	2 (	0	2	12
2040921	2007	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				12/16/2007	97	3	2 (		2	12
2040982	2007	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE 50 AVENUE			IN CITY - DO NOT PLOT	2/14/2008	18	3	2 (	0	2	13
2040982	2008	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVENUE			IN CITY - DO NOT PLOT	2/14/2008	18	3	2 (	0	2	13
2041275	2008	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE			IN ONLY DO NOT LEGT	7/31/2008	9	3	2 (	0	2	97
2041275	2008	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE				7/31/2008	9	3	2 (	0	2	97
2209626	2008	1	1137	i	COLD LAK	HWY 28	50 AVENUE	28	i		50 AVE			(IN CITY)	10/30/2008	14	3	2 (	0	2	97
2209626	2008	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE			(IN CITY)	10/30/2008	14	3	2 (	0	2	97
Z586385	2008	1	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE			IN CITY - DO NOT PLOT	12/4/2008	12	3	1 (	0		7
Z586385	2008	2	1137	1	COLD LAK	HWY 28	50 AVENUE	28	1		50 AVE			IN CITY - DO NOT PLOT	12/4/2008	12	3	1 0	0		7

COLLISION CASE NUMBER	SPECIAL FACILITY A	ROAD R ALIGNMENTA ALIGN	OAD		COLLISION	OR IECT TYPI	F OR IECT ID		D DRIVER/PED SEX			LIGHT CONDITION A		TRAFFIC CONTROL DEVIC PRESENT	E TRAFFIC CONDITION PED	DRIVER/PED CONDITION	CONTRIBUTING ROAD	ENVIRONMENTAL CONDITION		I OAD DETAILS A	LOAD DETAILS B ATTACHMENTS TRAILER TYPE	VEHICLE CONDITION CONTRIBUTING FACTORS	N/ UNSAFE SPEEDS
1552836	1		2	3	2	1	2 00000110	34	- C	1111 AO 1	4	2	97	2	1	 1	1	1	2	2	EUAD DETAILU DE ATTAOHMENTO TRAILERTTE	TACTORO	ONOAI E OI EEDO
1552836	1		2	3	2	1	3	41	F	8		3	97	2	1	3	1	1	2	2			
	1					1	3		F	8	4	3	97 97	2	1	3 1	1	1	2				
1637704	1		2	3	2	!	!	41	M	4	!	1	97 97	2	!	1	!	1	3				
1637704	1		2	3	2	1	1	37		8	1	1		2	1	1	1	1	3				
1637727	1	-	2	3	2	1	1	59	М	2	9	3	2 97	2	1	1	1	1	1				
1637727	1		2	3	2	1	2	55	М	8	1	97	97	2	1	1	1	1	1				
1637728	1	•	2	3	2	1	1	42	М	8	1	3	2	2	1	1	1	1	1				
1637728	1		2	3	2	1	2	29	М	8	14	97	97	2	1	1	1	1	1				
1637769	1		2	3	2	1	1	75	M		13	1	1	1		1	1	1	2				
1637769	1		2	3	2	1	1	24	F		1	1	1	1		1	1	1	2				
1637777	1	•	2	3	2	1	1	38	F	9	1	1	1	4	97	1	4	4	3				
1637777	1	•	2	3	2	1	1	28	F		97	97	97	97	97	97	97	4	3				
1637779	97		2	3	2	1	3	38	F	8	99	1	97	2	1	1	1	4	3				
1637779	97	1	2	3	2	1	2	39	M	4	1	1	97	2	1	1	1	4	3				
1637781	1	1	2	3	2	1	3	64	M	4	1	1	97	2	1	1	1	1	3				
1637781	1	1	2	3	2	1	1	24	M	8	1	1	97	2	1	1	1	1	3				
1637841	1	1	2	3	2	1	2	39	M	7	1	1	97	2	1	1	1	1	1				
1637841	1	1	2	3	2	1	3	40	F	7	9	1	97	2	1	1	1	1	1				
1637841	1	1	2	3	2	1	1	25	F	2	1	1	97	2	1	1	1	1	1				
1720871	1	1	2	3	2	1	1	69	M	8	99	1	1	2	1	1	1	1	1				
1720871	1	1	2	3	2	1	3	35	M	4	99	1	1	2	1	1	1	1	1				
1720914	1	1	2	3	2	1	1	16	F	7	1	1	1	2	1	97	1	1	1				
1720914	1	1	2	3	2	1	1	21	М		9	1	1	2	1	97	1	1	1				
1721176	1	1	2	3	2	1	3	49	F	4	1	1	97	2	1	1	4	4	3				
1721176	1	1	2	3	2	1	3	17	М	8	1	1	97	2	1	1	4	4	3				
1721178	1	1	2	2	2	1	1	45	F	4	1	1	97	2	1	1	1	4	3				
1721178	1	1	2	2	2	1	1	18	F	8	1	1	97	2	1	1	1	4	3				
1721178	1	1	2	2	2	1	1	19	M	4	1	1	97	2	1	1	1	4	3				
Z542325	1	1	1	3	2	1	1	33	F	8		1	97	2	•		4	4	3				
Z542325	1	1	1	3	2	9	13	00	•	Ü			0.	-			97	•	Ü				
1778756	1	1	2	3	2	1	1	48	М	4	1	1	97	4	97	1	1	2	2				
1778756	1	1	2	3	2	1	2	48	M		6	1	97	4	97	1	1	2	2				
1778793	1	1	1	3	2	1	1	47	F	4	97	1	97	2	1	97	1	1	1				
1778793	1	1	1	3	2	1	1	19	M		97	1	97	2	1	97	1	1	1				
1873172	1	1	1	3	2	1	1	39	F	7	1	1	1	2	1	1	1	1	1				
1873172	1	1		3	2		2	33		1	11		1	2	1	99	1		1				
1528741	97	1	1	3	1	1	2	48	М	7	4	1	97	4	1	4	1	<u>'</u>	1				
1528741	97	1	1	2	1	1	4	33	M	2	10	1	97	1		1	1	1	1				
1721047	1	1	1	2	2	1	4	71	M	2	97	1	1	1	4	1	1	4	2				
	•	1	4	3	2	1	2	47	M	0	97	1	1	2	1	1	4	4	3				
1721047 1873170	1 97	1	1	3	-	1	2	36	IVI	8	97	1	97	2	1	1	4	4	3				
		1	1	2	2	1	1		M	2	9	1	97	2	1	1	1	1	3				
1873170	97	1	1	2	2	1	1	39	***	8	1	1		2	1	1	1 97	1	3				
2040921	1	1	1	2	2	1	1	48	M	97	97	97	97	97	97	97	97	97	97				
2040921	1	1	1	2	2	1	2	32	М		97	97	97	97	97	97	97	97	97				
2040982	1	1	1	2	2	1	1	33	M	8	1	3	97	2	1	1	1	1	3				
2040982	1	1	1	2	2	1	2	34	M	_	8	3	97	2	1	1	1	1	3				
2041275	97	1	1	3	2	1	1	53	F	8	97	1	97	2	1	1	1	1	1				
2041275	97	1	1	3	2	1	2	87	M	2	97	1	97	2	1	1	1	1	1				
2209626	1	1	1	8	2	1	3	57	M	5	1	1	99	2	1	1	1	1	1				
2209626	1	1	1	8	2	1	3	30	F	7	11	1	99	2	1	1	1	1	1				
Z586385	1	1	1	8	2	1	5	30	М	6		1	1	2			1	1	1				
Z586385	1	1	1	8	2	9	13										97						

	CASE	OBJECT	POLICE			NORTH/SOUTH	EAST/WEST		AT INTERSECTION WITH		AT INTERSECTION WITH	IF NOT AT INTERSECTION			OCCURRENCE DATE	OCCURRENCE	COLLISION N	UMBER OF 1	NUMBER N	UMBER	1	PRIMARY
LLISION CASE NUMBER		NUMBER	SERVICE	IN/ NEAR	CITY NAME	ROAD NAME	ROAD NAME	ON HWY #	HWY #	ON STREET/AVENUE	STREET/AVENUE	DISTANCE	IF NOT AT INTERSECTION - DESCRIPTION	SPECIAL REFERENCE	(CCYY/MM/DD)	HOUR				TALITIES HIT	AND RUN	EVENT
1720945	2006	1	1137	1	COLD LAK	55 STREET	54 AVENUE	28	1		54 AVE			55 ST	6/26/2006	13	2	2	1	0	2	1
1720945	2006	2	1137	1	COLD LAK	55 STREET	54 AVENUE	28	1		54 AVE			55 ST	6/26/2006	13	2	2	1	0	2	1
1873358	2006	1	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE			NEAR OF 5343-55 ST	11/28/2006	19	3	2	0	0	2	97
1873358	2006	2	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE			NEAR OF 5343-55 ST	11/28/2006	19	3	2	0	0	2	97
1873360	2006	1	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				11/29/2006	8	3	2	0	0	2	1
1873360	2006	2	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				11/29/2006	8	3	2	0	0	2	1
1873371	2006	1	1137	1	COLD LAK	55 STREET	54 AVENUE			54 AVE	55 ST			5402 55 ST E	11/30/2006	17	3	2	0	0	2	8
1873371	2006	2	1137	1	COLD LAK	55 STREET	54 AVENUE			54 AVE	55 ST			5402 55 ST E	11/30/2006	17	3	2	0	0	2	8
1873422	2006	1	1137	1	COLD LAK	55 STREET	54 AVENUE			54 AVE		5	55 ST	SERVICE ROAD INTERSECTION	12/15/2006	18	3	2	0	0	2	97
1873422	2006	2	1137	1	COLD LAK	55 STREET	54 AVENUE			54 AVE		5	55 ST	SERVICE ROAD INTERSECTION	12/15/2006	18	3	2	0	0	2	97
1721138	2007	1	1137	1	COLD LAK	55 STREET	54 AVENUE							INTERSECTION@MACH 5451 55 ST	2/8/2007	7	3	2	0	0	2	3
1721138	2007	2	1137	1	COLD LAK	55 STREET	54 AVENUE							INTERSECTION@MACH 5451 55 ST	2/8/2007	7	3	2	0	0	2	3
1721145	2007	1	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE			<del></del>	2/2/2007	18	3	2	0	0	2	5
1721145	2007	2	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				2/2/2007	18	3	2	0	0	2	5
1873011	2007	1	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				5/18/2007	11	3	2	0	0	2	3
1873011	2007	2	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				5/18/2007	11	3	2	0	0	2	3
1873037	2007	1	1137	1	COLD LAK	55 STREET	54 AVENUE			55 STREET	54 AVENUE				7/26/2007	97	3	2	0	0	2	3
1873037	2007	2	1137	1	COLD LAK	55 STREET	54 AVENUE			55 STREET	54 AVENUE				7/26/2007	97	3	2	0	0	2	3
2040903	2007	1	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVENUE				12/31/2007	97	3	2	0	0	2	5
2040903	2007	2	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVENUE				12/31/2007	97	3	2	0	0	2	5
2041289	2008	1	1137	4	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				8/9/2008	17	3	2	0	0	2	_
2041289	2008	,	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				8/9/2008	17	3	2	0	0	2	5
2041359	2008	1	1137	1	COLD LAK	55A STREET	54 AVENUE			54 AVE	55A ST				12/8/2008	17	2	2	1	0	2	6
2041359	2008	,	1137	4	COLD LAK		54 AVENUE			54 AVE	55A ST					17	2	2		0	2	0
2041398	2008	4	1137		COLD LAK	55A STREET 55 STREET	54 AVENUE			55 SE	54 AVE				12/8/2008	15	2	2	1	0	2	0
		1	1137		COLD LAK										12/31/2008	15	3	2	0	0		0
2041398	2008	2	1137	1		55 STREET	54 AVENUE			55 SE	54 AVE				12/31/2008	15 14	3	2	0	0	1	12
2209660	2008	1		1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				11/25/2008	14	3	2	0	0	2	12
2209660	2008	2	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				11/25/2008	14	3	2	0	0	2	12
2209759	2008	1	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				9/8/2008	8	2	2	3	0	2	8
2209759	2008	2	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				9/8/2008	8	2	2	3	0	2	8
2041420	2009	1	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				1/27/2009	15	2	3	3	0	2	8
2041420	2009	2	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				1/27/2009	15	2	3	3	0	2	8
2041420	2009	3	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				1/27/2009	15	2	3	3	0	2	8
2041650	2009	1	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				5/20/2009	8	3	2	0	0	2	6
2041650	2009	2	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				5/20/2009	8	3	2	0	0	2	6
2307010	2009	1	1137	1	COLD LAK	55 STREET	54 AVENUE			ACCESS ROAD	54 AVE			JCT OF HWY 28 AND ACCESS RD AT NO FRILLS	10/13/2009	19	3	2	0	0	2	3
2307010	2009	2	1137	1	COLD LAK	55 STREET	54 AVENUE			ACCESS ROAD	54 AVE			JCT OF HWY 28 AND ACCESS RD AT NO FRILLS	10/13/2009	19	3	2	0	0	2	3
2307067	2009	1	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				9/14/2009	16	3	2	0	0	2	8
2307067	2009	2	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				9/14/2009	16	3	2	0	0	2	8
2307142	2009	1	1137	1	COLD LAK	55A STREET	54 AVENUE			54 AVE	55 A ST				12/7/2009	15	3	2	0	0	2	3
2307142	2009	2	1137	1	COLD LAK	55A STREET	54 AVENUE			54 AVE	55 A ST				12/7/2009	15	3	2	0	0	2	3
2307225	2009	1	1137	1	COLD LAK	55 STREET	54 AVENUE			54 AVE	55 ST				4/18/2009	14	3	2	0	0	2	7
2307225	2009	2	1137	1	COLD LAK	55 STREET	54 AVENUE			54 AVE	55 ST				4/18/2009	14	3	2	0	0	2	7
2307240	2009	1	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				7/3/2009	12	3	2	0	0	2	8
2307240	2009	2	1137	1	COLD LAK	55 STREET	54 AVENUE			55 ST	54 AVE				7/3/2009	12	3	2	0	0	2	8
2307411	2009	1	1137	1	COLD LAK	55 STREET	54 AVENUE			54 AVE	55 ST				9/2/2009	17	3	3	0	0	2	5
2307411	2009	2	1137	1	COLD LAK	55 STREET	54 AVENUE			54 AVE	55 ST				9/2/2009	17	3	3	0	0	2	5
2307411	2009	3	1137	1	COLD LAK	55 STREET	54 AVENUE			54 AVE	55 ST				9/2/2009	17	3	3	0	0	2	5

COLLISION CASE NUMBER	SPECIAL FACILITY	ROAD ALIGNMENT A	ROAD ALIGNMENT B	ROAD CLASS	COLLISION	OBJECT TYPE O	BJECT ID	DRIVER/PED D			DRIVER ACTION CO	LIGHT ONDITION A	LIGHT CONDITION B	TRAFFIC CONTROL DEVICE PRESENT	TRAFFIC CONDITION DEVICE CONDITION	PEDESTRIAN ACTION	DRIVER/PED C	ONTRIBUTING ROAD CONDITION	ENVIRONMENTAL CONDITION	SURFACE CONDITION	LOAD DETAILS A	LOAD DETAILS B	ATTACHMENTS	VEHICLE TRAILER TYPE CONTRIBUT	CONDITION/	UNSAFE SPEEDS
1720945	1	1	1	2	2	1	1	19	F	1	1	1	97	2	1		1	1	1	1						
1720945	1	1	1	2	2	4	8	14	F		97	97	97	97	97		97	97	1	1						
1873358	1	1	1	4	2	1	1	29	F	3	13	3	2	2	1		1	1	4	3	2					
1873358	1	1	1	4	2	1	2	31	M	8	1	3	2	2	1		1	1	4	3	2					
1873360	1	1	1	2	2	1	1	28	M	8	99	1	97	2	1		1	1	1	3						
1873360	1	1	1	2	2	1	3	32	M	4	99	1	97	2	1		1	1	1	3						
1873371	1	1	1	2	2	1	1	50	F	4	1	3	97	2	1		1	1	4	3						
1873371	1	1	1	2	2	1	3	52	F		1	3	97	2	1		1	1	4	3						
1873422	1	1	1	2	2	1	2	55	M	2	97	3	97	1			97	1	4	3						
1873422	1	1	1	2	2	1	2	37	M	8	98	3	97	1			97	1	4	3						
1721138	97	1	1	2	1	1	1	54	M	8	13	1	97	1			1	1	1	3						
1721138	97	1	1	2	1	1	1	49	M	8	1	1	97	3	1		1	1	1	3						
1721145	97	1	1	4	2	1	1	52	M	7	1	1	97	2	1		1	98	1	3						
1721145	97	1	1	4	2	1	2	39	F	7	9	1	97	2	1		1	98	1	3						
1873011	97	1	1	2	2	1	3	46	M	8	97	97	97	97	97		97	97	97	97						
1873011	97	1	1	2	2	1	1	70	F	6	97	97	97	97	97		97	97	97	97						
1873037	97	1	1	2	1	1	2	20	M	3	9	1	1	2	1		1	1	1	1	2					
1873037	97	1	1	2	1	1	1	17	M	8	3	1	1	2	1		1	1	1	1						
2040903	97	1	1	2	2	1	1	17	M	1	1	1	97	2	1		1	1	1	3						
2040903	97	1	1	2	2	1	2	17	F	2	13	1	97	2	1		1	1	1	3						
2041289	97	7	7	97	97	1	2	17	M	8	99	97	97	97	97		97	97	97	97						
2041289	97	7	7	97	97	1	2	24	M		99	97	97	97	97		97	97	97	97						
2041359	97	1	1	2	1	1	1	50	M	7	99	3	2	1			97	1	4	3						
2041359	97	1	1	2	1	1	1	47	F	7	99	3	2	1			97	1	4	3						
2041398	97	1	1	2	2	1	3	49	F	4	97	3	1	97	97		97	1	4	3						
2041398	97	1	1	2	2	1	99				97	97	97	97	97		97	97	4	3						
2209660	97	1	1	2	1	1	9	53	F	1	1	1	97	1			1	1	1	3						
2209660	97	1	1	2	1	1	3	61	М	7	10	1	97	1			1	1	1	3						
2209759	97	1	1	2	2	1	1	19	F	8	99	1	97	97	97		97	1	1	1						
2209759	97	1	1	2	2	1	3	35	۲.	4	99	1	97	97	97		97	!	!	1						
2041420	97	1	1	2	2	1	2	26	M	4	1	1	1	1			1	1	1	3						
2041420	97 97	1	1	2	2	1	3	30		3	6	1	1	1			1 97	1	1	3						
2041420 2041650	97	- 1	1	2	2	1	1	27 17	IVI NA	0	00	1		07	97		97	07	07	3						
2041650	1			2	2	<u> </u>	3	28	IVI	4	99	1		97	97		99	97	97	97						
2307010	97	0	1	2	2	1	3	20	IVI		99	2	1	9/	97		99	97	9/	97						
2307010	97	0	1	2	2	1	2	51	M	1	2	3	2	3	1		1	1	1	1						
2307067	97	1	1	2	2	1	2	45	M	4	1	1	1	2	4				4	1						
2307067	97	4	1	2	2	1	3	28	E	8	6	1	1	2	4		99	1	4	4						
2307142	97	1	1	2	2	1	3	54	Ė	7	99	1	1	2	1		1	1	1	3						
2307142	97	4	1	2	2	1	2	31	M	8	99	1	1	3	1		1	1	4	3						
2307225	97	1	1	2	2	1	2	18	M	5	1	1	1	2	1		1	3	2	2						
2307225	97	1	1	2	2	1	5	32	M	•	14	1	1	1	•		1	3	2	2	1	1	1	2	1	2
2307240	97	1	1	2	2	1	5	45	M	8	99	1	· i	2	1		· i	1	2	2	i	1	1 1	2 5	1	2 97
2307240	97	1	1	2	2	1	2	52	M	4	1	1	i	2	1		i	i	2	2	•	•	•	Ü		٠.
2307411	1	1	1	2	2	1	2	75	M	8	9	1	1	2	1		1	1	1	1						
2307411	1	1	1	2	2	1	2	18	M	11	11	1	1	2	1		i	1	1	1						
2307411	1	1	1	2	2	1	3	21	F	7	1	1	1	2	1		1	1	1	1						

								AT			IF NOT AT										
COLLISION CASE		OBJECT				NORTH/SOUTH	EAST/WEST	INTERSECTION		AT INTERSECTION WITH		NOT AT INTERSECTION -		OCCURRENCE DATE	OCCURRENCE	COLLISION	NUMBER OF	NUMBER	NUMBER		PRIMARY
NUMBER	YEAR I	NUMBER	SERVICE	IN/ NEAR	CITY NAME	ROAD NAME	ROAD NAME ON HWY #	WITH HWY #	ON STREET/AVENUE	STREET/AVENUE	DISTANCE	DESCRIPTION	SPECIAL REFERENCE	(CCYY/MM/DD)	HOUR	SEVERITY	VEHICLES	INJURED	FATALITIES	HIT AND RUN	EVENT
1315702	2005	1	1137	1	COLD LAK	50 STREET	50 AVENUE		50 ST	50 AVE				12/2/2005	17	3	2	0	0	2	3
1315702	2005	2	1137	1	COLD LAK	50 STREET	50 AVENUE		50 ST	50 AVE				12/2/2005	17	3	2	0	0	2	3
1637719	2005	1	1137	1	COLD LAK	50 STREET	50 AVENUE		50 ST	50 AVE				12/24/2005	1	3	2	0	0	2	8
1637719	2005	2	1137	1	COLD LAK	50 STREET	50 AVENUE		50 ST	50 AVE				12/24/2005	1	3	2	0	0	2	8
1721245	2006	1	1137	1	COLD LAK	50 STREET	50 AVENUE		50 AVE	50 ST			IN FRONT OF SCOTIA BANK	5/10/2006	18	3	2	0	0	2	13
1721245	2006	2	1137	1	COLD LAK	50 STREET	50 AVENUE		50 AVE	50 ST			IN FRONT OF SCOTIA BANK	5/10/2006	18	3	2	0	0	2	13
1720941	2007	1	1137	1	COLD LAK	50 STREET	50 AVENUE		50 ST	50 AVE				6/15/2007	18	3	2	0	0	2	12
1720941	2007	2	1137	1	COLD LAK	50 STREET	50 AVENUE		50 ST	50 AVE				6/15/2007	18	3	2	0	0	2	12
1721084	2007	1	1137	1	COLD LAK	50 STREET	50 AVENUE		50 ST	50 AVE			SIDE OF TD BANK	4/11/2007	17	3	2	0	0	1	7
1721084	2007	2	1137	1	COLD LAK	50 STREET	50 AVENUE		50 ST	50 AVE			SIDE OF TD BANK	4/11/2007	17	3	2	0	0	1	7
1721161	2007	1	1137	1	COLD LAK	50 STREET	50 AVENUE		50 AVE WB	50 ST				2/23/2007	11	3	2	0	0	2	8
1721161	2007	2	1137	1	COLD LAK	50 STREET	50 AVENUE		50 AVE WB	50 ST				2/23/2007	11	3	2	0	0	2	8
1873142	2007	1	1137	1	COLD LAK	50 STREET	50 AVENUE		50 ST	50 AVE				8/17/2007	18	3	2	0	0	2	12
1873142	2007	2	1137	1	COLD LAK	50 STREET	50 AVENUE		50 ST	50 AVE				8/17/2007	18	3	2	0	0	2	12
1873463	2007	1	1137	1	COLD LAK	50 STREET	50 AVENUE		50 STREET	50 AVE				1/12/2007	16	2	2	1	0	2	8
1873463	2007	2	1137	1	COLD LAK	50 STREET	50 AVENUE		50 STREET	50 AVE				1/12/2007	16	2	2	1	0	2	8
2041546	2008	1	1137	1	COLD LAK	50 STREET	50 AVENUE		50 ST	50 AVE				6/14/2008	16	3	2	0	0	2	8
2041546	2008	2	1137	1	COLD LAK	50 STREET	50 AVENUE		50 ST	50 AVE				6/14/2008	16	3	2	0	0	2	8
2209766	2008	1	1137	1	COLD LAK	50 STREET	50 AVENUE		50 AVE	50 ST				9/19/2008	12	3	2	0	0	2	13
2209766	2008	2	1137	1	COLD LAK	50 STREET	50 AVENUE		50 AVE	50 ST				9/19/2008	12	3	2	0	0	2	13
Z586393	2008	1	1137	1	COLD LAK	50 STREET	50 AVENUE		50 ST	50 AVE				11/10/2008	24	3	1	0	0		1
Z586393	2008	2	1137	1	COLD LAK	50 STREET	50 AVENUE		50 ST	50 AVE				11/10/2008	24	3	1	0	0		1
2041434	2009	1	1137	1	COLD LAK	50 STREET	50 AVENUE		50 ST	50 AVE				2/7/2009	10	2	2	1	0	2	8
2041434	2009	2	1137	1	COLD LAK	50 STREET	50 AVENUE		50 ST	50 AVE				2/7/2009	10	2	2	1	0	2	8
2307447	2009	1	1137	1	COLD LAK	50 STREET	50 AVENUE		50 AVE		10	50 ST		3/12/2009	14	3	2	0	0	2	13
2307447	2009	2	1137	1	COLD LAK	50 STREET	50 AVENUE		50 AVE		10	50 ST		3/12/2009	14	3	2	0	0	2	13

		ROAD	ROAD															CONTRIBUTING							CONDITION/	
COLLISION CASE		ALIGNMENT	ALIGNMENT					DRIVER/PED			DRIVER		LIGHT		TRAFFIC CONDITION			ROAD	ENVIRONMENTAL						CONTRIBUTING	UNSAFE
NUMBER	FACILITY	A	В	ROAD CLASS	LOCATION	TYPE	ID	AGE	SEX	POINT OF IMPACT	ACTION	CONDITION A		DEVICE PRESENT	DEVICE CONDITION	ACTION	CONDITION	CONDITION	CONDITION	CONDITION	LOAD DETAILS A	LOAD DETAILS	B ATTACHME	NTS TRAILER TYP	E FACTORS	SPEEDS
1315702	1	1	1	2	2	1	1	25	F	8	11	3	97	2	1		1	1	98	3						
1315702	1	1	1	2	2	1	2	25	M	7	1	3	97	2	1		1	1	98	3						
1637719	97	7	7	97	97	1	2	30	M	8	6	1	97	4	1 97		1	1	1	1						
1637719	97	7	7	97 97	97	1	1	44	F	4	1	97	97	97	97		1	97	1	1						
1721245	97	7	7		97	1	1	47	IVI	5	8	1	97	1			1	1	1	1						
1721245	97	1	1	97	97	1	5	43	IVI	4	1	1	97	1	4		1	1	1	1	2			4	4	•
1720941 1720941	97	1	1	2	2	1	5	35 51	F M	3	13	1	1	3	1		1	1	1	1	2		1	4	1	2
1721084	97	1	1	2	1	-	1	51	IVI	0	7	ı	į.	3	ı		1	07	1	1						
1721084	97	1	1	2	1	1	90			0	99	97	97	97	97		99	97	1	1						
1721161	07	7	7	07	97	4	1	39	_	4	97	1	07	07	1		1	4	,	2						
1721161	97	7	7	97	97	1	1	42	M	φ	97	1	97	97	97		1	4	4	3						
1873142	97	1	1	2	1	1	1	42	F	97	8	1	97	1	31		1	1	1	1						
1873142	97	1	1	2	1	1	3	24	M	31	1	1	97	1			1	1	1	1						
1873463	97	0	1	2	2	1	2	25	M	4	1	1	97	3	97		1	1	1	3						
1873463	97	0	1	2	2	8	17	21	M	8	1	1	97	3	97		1	1	1	3						
2041546	97	1	1	2	1	1	3	48	M	4	3	1	97	3	1		97	1	1	1						
2041546	97	1	1	2	1	1	3	27	M	8	1	1	97	3	1		97	1	1	1						
2209766	97	1	1	2	2	1	1	30	M	6	97	1	97	3	1		97	1	1	1						
2209766	97	1	1	2	2	1	2	38	M	5	8	1	97	97	97		97	1	1	1						
Z586393	97	7	7	97	97	1	1	21	M	1		3	2	3				4	1	3						
Z586393	97	7	7	97	97	9	13											97								
2041434	97	1	1	3	2	1	1	52	F	4	1	1	1	2	1		97	1	1	3						
2041434	97	1	1	3	2	1	1	26	M	8	6	1	1	97	97		97	1	1	3						
2307447	97	1	1	2	1	1	1	18	M	3	1	1	1	6	1		1	1	1	1						
2307447	97	1	1	2	1	1	2	25	M	4	8	1	1	97	97		97	1	1	1						

COLLISION CASE	CASE	OBJECT	POLICE			NORTH/SOUTH ROAD	E4.07//4/E07.D0.4D		AT INTERSECTION		AT INTERSECTION WITH	IF NOT AT	IF NOT AT		OCCURRENCE DATE	OCCUPRENCE	COLLISION	NUMBER OF	NUMBER	NUMBER		
NUMBER	YEAR	NUMBER	SERVICE	IN/ NEA	R CITY NAME	NAME	NAME	ON HWY#	WITH HWY #	ON STREET/AVENUE	STREET/AVENUE	DISTANCE	DESCRIPTION	SPECIAL REFERENCE	(CCYY/MM/DD)	HOUR	SEVERITY	VEHICLES	INJURED		HIT AND RUN PRIMA	RYEVENT
1315589	2005	1	1137	1	COLD LAK	50 STREET	46 AVENUE			46 AVE	50 STREET				10/19/2005	12	3	2	0	0	2	1
1315589	2005	2	1137	1	COLD LAK	50 STREET	46 AVENUE			46 AVE	50 STREET				10/19/2005	12	3	2	0	0	2	1
1720806	2005	1	1137	1	COLD LAK	50 STREET	46 AVENUE			46 AVE	EAST SERVICE RD.			SERVICE ROAD IN FRONT OF DAIRY QUEEN	9/1/2005	21	3	2	0	0	1	6
1720806	2005	2	1137	1	COLD LAK	50 STREET	46 AVENUE			46 AVE	EAST SERVICE RD.			SERVICE ROAD IN FRONT OF DAIRY QUEEN	9/1/2005	21	3	2	0	0	1	6
1721290	2005	1	1137	1	COLD LAK	50 STREET	46 AVENUE			50 STREET	46 AVENUE				12/21/2005	17	3	2	0	0	2	8
1721290	2005	2	1137	1	COLD LAK	50 STREET	46 AVENUE			50 STREET	46 AVENUE				12/21/2005	17	3	2	0	0	2	8
Z524717	2005	1	1137	1	COLD LAK	50 STREET	46 AVENUE			50 STREET	46 AVENUE				9/1/2005	13	3	2	0	0		12
Z524717	2005	2	1137	1	COLD LAK	50 STREET	46 AVENUE			50 STREET	46 AVENUE				9/1/2005	13	3	2	0	0		12
1873226	2006	1	1137	1	COLD LAK	50 STREET	46 AVENUE			50 ST	46 AVE			STOP SIGN BY WALMART	11/4/2006	16	3	2	0	0	2	3
1873226	2006	2	1137	1	COLD LAK	50 STREET	46 AVENUE			50 ST	46 AVE			STOP SIGN BY WALMART	11/4/2006	16	3	2	0	0	2	3
1873228	2006	1	1137	1	COLD LAK	50 STREET	46 AVENUE			46 AVE	50 ST			STOP SIGN @ WALMART INTERSECTION	11/4/2006	15	3	2	0	0	2	8
1873228	2006	2	1137	1	COLD LAK	50 STREET	46 AVENUE			46 AVE	50 ST			STOP SIGN @ WALMART INTERSECTION	11/4/2006	15	3	2	0	0	2	8
1873440	2006	1	1137	1	COLD LAK	50 STREET	46 AVENUE							SERVICE ROAD IN FRONT OF DAIRY QUEEN 4605-50 ST	12/19/2006	9	3	2	0	0	2	8
1873440	2006	2	1137	1	COLD LAK	50 STREET	46 AVENUE							SERVICE ROAD IN FRONT OF DAIRY QUEEN 4605-50 ST	12/19/2006	9	3	2	0	0	2	8
1721077	2007	1	1137	1	COLD LAK	50 STREET	46 AVENUE			50 ST	46 AVE				4/13/2007	15	3	3	0	0	2	5
1721077	2007	2	1137	1	COLD LAK	50 STREET	46 AVENUE			50 ST	46 AVE				4/13/2007	15	3	3	0	0	2	5
1721077	2007	3	1137	1	COLD LAK	50 STREET	46 AVENUE			50 ST	46 AVE				4/13/2007	15	3	3	0	0	2	5
2041222	2007	1	1137	1	COLD LAK	50 STREET	46 AVENUE			50 ST		2	46 AVE		11/20/2007	17	3	2	0	0	2	8
2041222	2007	2	1137	1	COLD LAK	50 STREET	46 AVENUE			50 ST		2	46 AVE		11/20/2007	17	3	2	0	0	2	8
2041050	2008	1	1137	1	COLD LAK	50 STREET	46 AVENUE			50 ST	46 AVE			PORK CHOP ON SERVICE ROAD IN FRONT OF DAIRY QUEEN	4/18/2008	18	3	2	0	0	2	8
2041050	2008	2	1137	1	COLD LAK	50 STREET	46 AVENUE			50 ST	46 AVE			PORK CHOP ON SERVICE ROAD IN FRONT OF DAIRY QUEEN		18	3	2	0	0	2	8
2041557	2008	1	1137	1	COLD LAK	50 STREET	46 AVENUE			46 AVE	50 ST			STOP SIGN IN FRONT OF DAIRY QUEEN	5/18/2008	17	3	2	0	0	2	7
2041557	2008	2	1137	1	COLD LAK	50 STREET	46 AVENUE			46 AVE	50 ST			STOP SIGN IN FRONT OF DAIRY QUEEN	5/18/2008	17	3	2	0	0	2	7
2041791	2008	1	1137	1	COLD LAK	50 STREET	46 AVENUE			46 TH AVENUE	50TH STREET				3/3/2008	18	3	2	0	0	2	97
2041791	2008	2	1137	1	COLD LAK	50 STREET	46 AVENUE			46 TH AVENUE	50TH STREET				3/3/2008	18	3	2	0	0	2	97
2307279	2009	1	1137	1	COLD LAK	50 STREET	46 AVENUE			50 ST	46 AVE				7/25/2009	20	3	2	0	0	2	12
2307279	2009	2	1137	1	COLD LAK	50 STREET	46 AVENUE			50 ST	46 AVE				7/25/2009	20	3	2	0	0	2	12

																									VEHICLE CONDITION/	
COLLISION CASE	SPECIAL	ROAD	ROAD		COLLISION			DRIVER/PED D				LIGHT L CONDITION A	IGHT CONDITION		TRAFFIC CONDITION	ACTION		CONTRIBUTING	ENVIRONMENTAL		LOAD DETAILS				CONTRIBUTING	
NUMBER	FACILITY	ALIGNMENTA	ALIGNMENT	B ROAD CLASS	LOCATION	OBJECT TYPE	OBJECTID	AGE	SEX	IMPACT	ACTION	CONDITION A	В	DEVICE PRESENT	DEVICE CONDITION	ACTION	CONDITION	ROAD CONDITION	CONDITION	CONDITION	А	LOAD DETAILS B	ATTACHMENTS	TRAILER TYPE	FACTORS	UNSAFE SPEEDS
1315589	1	1	1	3	2	1	!	52	M	8	9	1	97	1			1	1	97	1						
1315589	1	1	1	3	2	1	!	30	٠.	<u>'</u>	1	1	97	1			1	1	97	1						
1720806	1	1	1	2	2	1	1	20	M	<u>'</u>	1	3	1	3	97		1	1	1	!						
1720806	1	1	1	2	2	1	2	10		,	13	3	1	1			99	97	1	1						
1721290	1	1	1	3	2	1	1	19	IVI	8	6	3	2	1			1	1	1	2						
1721290	1	1	1	3	2	1	2	44	M	4	1	3	2	1			1	1	1	2						
Z524717	1	1	1	2	2	1	2	75	F	,		1	1	2				1	1	1						
Z524717	1	1	1	2	2	1	1	21	IVI .	3		1	1	1				1	1	1						
1873226	9					1	3	68	-	6	07	3	97	1	4		1	3	4	3						
1873226	9					1	1	68	-	8	97	3	97	3	1		1	3	4	3						
1873228	9					1	1	47	F.	4	1	1	97	3	1		97	98	4	3						
1873228	9				0	1	2	20	IVI	8	4	1	97	1	4		97	98	4	3						
1873440	1	1	1	4	2	1	3	56	-	4	1	1	1	3	1		1	1	1	3						
1873440 1721077	7	1	1	4	2	1	3	40	F	8	1	1	1	3	1		1	1	1	3						
1721077	97	1	1	2	1	1	1	20	IVI	6	1	1	1	3	1		1	1	1	1						
	97	1	1	2	1	1	1	17	IVI	1	99	1	1	97	97		97	1	1	1						
1721077	97	1	1	2	1	1	1	34	-	5	07	1	97	1			1	1	1	1						
2041222 2041222	97	1	1	2	1	1	3	31	M	4	97	3	97	1			97	1	1	3						
2041222	97	1		2	1		0	34	IVI	0	97	3	97	07	07		97	07	07	3						
2041050	8	1	1	2	2	1	2	36	-	8	1	97	97	97	97		97	97	97	97						
2041557	0	1	1	2	2	1	1	31	M	4	99	97	97	97	9/		97	97	97	97						
2041557	97	1	1	2	2	1	3	34 70	IVI	0	2	1	97	3	1		00	1	1	1						
2041791	97	1	1	2	2	1	3	27	F	0	00	1	97	3	07		99	1	4	1						
2041791	97	1	1	2	2	1	3	27 55	M	0	4	1	1	3	97		1	98	4	3						
2041791	9/	1	1	2	2	1	2	50 50	IVI	3	10	1	1	9/	9/		99	98	4	3						
2307279	1	1	1	2	2	1	2	25	M	3	10	1	1	2	1		1	1	1	1						
230/2/9	1	1	1	2	2	1	2	20	IVI			1	'	3	ı.		'	1	1	1						

												IF NOT AT	IF NOT AT									
COLLISION CASE	CASE	OBJECT	POLICE			NORTH/SOUTH ROAD	EAST/WEST ROAD		AT INTERSECTION		AT INTERSECTION WITH	INTERSECTION	INTERSECTION -		OCCURRENCE DATE	OCCURRENCE	COLLISION	NUMBER OF	NUMBER	NUMBER		PRIMARY
NUMBER	YEAR	NUMBER	SERVICE	IN/ NEAR	CITY NAME	NAME	NAME	ON HWY #	WITH HWY #	ON STREET/AVENUE	STREET/AVENUE	DISTANCE	DESCRIPTION	SPECIAL REFERENCE	(CCYY/MM/DD)	HOUR	SEVERITY	VEHICLES	INJURED	FATALITIES	HIT AND RUN	EVENT
1637758	2005	1	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50 ST				3/9/2005	12	2	2	1	0	2	8
1637758	2005	2	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50 ST				3/9/2005	12	2	2	1	0	2	8
1720829	2005	1	1137	1	COLD LAK	50 STREET	HWY28	28	1		50 ST				8/9/2005	7	3	2	0	0	2	8
1720829	2005	2	1137	1	COLD LAK	50 STREET	HWY28	28	1		50 ST				8/9/2005	7	3	2	0	0	2	8
1720926	2005	1	1137	1	COLD LAK	50 STREET	HWY28	28	1		50 ST				11/8/2005	13	2	2	1	0	2	3
1720926	2005	2	1137	1	COLD LAK	50 STREET	HWY28	28	1		50 ST				11/8/2005	13	2	2	1	0	2	3
1721003	2005	1	1137	1	COLD LAK	50 STREET	HWY28	28	1		50 ST				11/4/2005	17	3	2	0	0	2	8
1721003	2005	2	1137	1	COLD LAK	50 STREET	HWY28	28	1		50 ST				11/4/2005	17	3	2	0	0	2	8
Z524810	2005	1	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50 STREET				4/29/2005	6	3	2	0	0		8
Z524810	2005	2	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50 STREET				4/29/2005	6	3	2	0	0		8
1721271	2006	1	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50 ST				2/8/2006	16	3	2	0	0	2	3
1721271	2006	2	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50 ST				2/8/2006	16	3	2	0	0	2	3
1778730	2006	1	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50 STREET				3/4/2006	8	3	2	0	0	2	2
1778730	2006	2	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50 STREET				3/4/2006	8	3	2	0	0	2	2
1721045	2007	1	1137	1	COLD LAK	50 STREET	HWY 28	28	1					JUST WEST OF 50 ST	2/23/2007	15	3	2	0	0	2	12
1721045	2007	2	1137	1	COLD LAK	50 STREET	HWY 28	28	1					JUST WEST OF 50 ST	2/23/2007	15	3	2	0	0	2	12
1721079	2007	1	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50 STREET			IN FRONT OF RAILBIRDS	4/15/2007	22	3	1	0	0	2	1
1721079	2007	2	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50 STREET			IN FRONT OF RAILBIRDS	4/15/2007	22	3	1	0	0	2	1
2040963	2008	1	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50 ST			IN CITY - DO NOT PLOT	1/14/2008	8	3	2	0	0	2	8
2040963	2008	2	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50 ST			IN CITY - DO NOT PLOT	1/14/2008	8	3	2	0	0	2	8
2040966	2008	1	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50ST			IN CITY - DO NOT PLOT	1/14/2008	11	3	2	0	0	2	8
2040966	2008	2	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50ST			IN CITY - DO NOT PLOT	1/14/2008	11	3	2	0	0	2	8
Z586694	2008	1	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50 ST			(IN CITY)	3/20/2008	11	3	1	0	0		1
Z586694	2008	2	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50 ST			(IN CITY)	3/20/2008	11	3	1	0	0		1
Z680603	2008	1	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50ST			IN CITY	12/9/2008	10	3	1	0	0		1
Z680603	2008	2	1137	1	COLD LAK	50 STREET	HWY 28	28	1		50ST			IN CITY	12/9/2008	10	3	1	0	0		1

							_							CONTROL	CONDITION										CONDITION	
OLLISION CASE	SPECIAL	ROAD	ROAD		COLLISION			RIVER/PED DR				LIGHT	LIGHT	DEVICE	DEVICE CONDITION	PEDESTRIAN ACTION	DRIVER/PED CONDITION	CONTRIBUTING ROAD CONDITION	ENVIRONMENTAL CONDITION	SURFACE CONDITION	LOAD DETAILS A	LOAD DETAILS	ATTACHMENTS	TRAILER TYPE	CONTRIBUTING	UNSAFE SPEEDS
NUMBER	FACILITY	ALIGNMENT A	ALIGNMENT B	ROAD CLASS	LOCATION	OBJECT TYPE	OBJECTID	AGE	SEX	IMPACT	ACTION	CONDITION A	CONDITION B	PRESENT	CONDITION	ACTION	CONDITION	ROAD CONDITION	CONDITION	CONDITION	LUAD DETAILS A	ь	ATTACHMENTS	IRAILER ITPE	FACTORS	UNSAFE SPEEDS
1637758	1	1	2	2	2	1	1	56	-		ь	1	97	1	97		1	1	1	1						
1637758	1	1	2	2	2	1	1	35			1	97	97	97	97		1	97	1	1						
1720829	1	1	2	2	2	1	1	43	NI .	3	07	1	1	1			1	1	1	1	2					
1720829	1	1	2	2	2	1	3	45	Ę	,	9/		1	1			1	1	1	1	2					
1720926 1720926	1	1	2	2	2	1	3	45 74	Ę	0	2		1	2	97		1	1	1	1						
	1	7	2	2	2	1	1	14	F	ь	2	1	97	3	97		1	1 97	7	1 07						
1721003 1721003	97	7	2	2	2	1	2	45	IVI NA	0	97	97	97	97	97		97	97	97	97						
Z524810	97	1	2	2	2	1	2	62	M	9	97	1	97	97	97		97	97	4	97						
Z524810 Z524810	1	1	2	3	2	1	2	43	M	7		1	1	1				1	4	2						
1721271	1	1	2	2	2	1	1	52	[VI	,	07	1	97	2	97		07	4	1	2						
1721271	1	1	2	2	2	1	1	11	M	5	97	1	97	1	51		97	1	1	3						
1778730	1	1	1	3	2	1	1	18	M	8	97	97	97	97	97		97	07	97	97						
1778730	1	;	1	3	2	1	3	62	M	7	97	97	97	97	97		97	97	97	97						
1721045	1	1	1	1	1	1	3	34	M	97	1	1	1	1	31		1	4	4	3						
1721045	1	1	1	1	1	1	2	61	M	0.	10	1	1	1			1	4	4	3						
1721079	97	1	2	2	1	1	2	20	M	1	1	3	97	1			97	1	1	1						
1721079	97	1	2	2	1	9	13	20	***		•	Ü	0,				٠.	97	1	1						
2040963	97	1	1	4	2	1	2	61	F	8	1	1	1	97	97		1	1	1	3						
2040963	97	1	1	4	2	1	2	50	M	4	1	1	1	97	97		1	1	1	3						
2040966	1	1	1	2	2	1	1	62	F	8	97	97	97	97	97		97	4	1	3						
2040966	1	1	1	2	2	1	2	54	F		97	97	97	97	97		97	97	1	3						
Z586694	97	1	1	2	2	1	5	30	M	12		1	1	2			**	1	1	1						
Z586694	97	1	1	2	2	9	13											97								
Z680603	97	1	1	2	2	1	1	31	F	10		1	1	97				1	4	3						
Z680603	97	1	1	2	2	9	13											97								

COLLISION CASE		OBJECT	POLICE				NORTH/SOUTH	EAST/WEST	Δ-	T INTERSECTION WIT	u	AT INTERSECTION WITH	IF NOT AT INTERSECTION	IF NOT AT INTERSECTION -		OCCURRENCE DATE	OCCUPPENCE	COLLISION	NUMBER OF	NUMBER	NUMBER		PRIMARY
NUMBER	CASE YEAR			IN/ NE	AR CITYN		ROAD NAME	ROAD NAME		HWY#	ON STREET/AVENUE	STREET/AVENUE	DISTANCE	DESCRIPTION	SPECIAL REFERENCE	(CCYY/MM/DD)	HOUR	SEVERITY		INJURED		HIT AND RUN	EVENT
1720951	2005	1	1137	1	COLD	LAK	51 STREET	50 AVENUE			50 AVE	51 ST			AT CIBC	9/9/2005	16	3	2	0	0	2	13
1720951	2005	2	1137	1	COLD		51 STREET	50 AVENUE			50 AVE	51 ST			AT CIBC	9/9/2005	16	3	2	ō	ō	2	13
1720972	2006	1	1137	1	COLD		51 STREET	50 AVENUE			50 AVENUE	51 STREET				2/15/2006	15	3	2	0	0	2	1
1720972	2006	2	1137	1	COLD		51 STREET	50 AVENUE			50 AVENUE	51 STREET				2/15/2006	15	3	2	0	0	2	1
1778753	2006	1	1137	1	COLD	LAK	51 STREET	50 AVENUE			51 ST	50 AVE				5/4/2006	11	3	1	0	0	1	7
1778753	2006	2	1137	1	COLD	LAK	51 STREET	50 AVENUE			51 ST	50 AVE				5/4/2006	11	3	1	0	0	1	7
1778753	2006	3	1137	1	COLD	LAK	51 STREET	50 AVENUE			51 ST	50 AVE				5/4/2006	11	3	1	0	0	1	7
1778872	2006	1	1137	1	COLD	LAK	51 STREET	50 AVENUE			50 AVE		3	51 ST		5/27/2006	14	3	2	0	0	2	13
1778872	2006	2	1137	1	COLD	LAK	51 STREET	50 AVENUE			50 AVE		3	51 ST		5/27/2006	14	3	2	0	0	2	13
1873209	2006	1	1137	1	COLD	LAK	51 STREET	50 AVENUE			50 AVE	51 ST				10/23/2006	19	3	2	0	0	2	5
1873209	2006	2	1137	1	COLD		51 STREET	50 AVENUE			50 AVE	51 ST				10/23/2006	19	3	2	0	0	2	5
1721052	2007	1	1137	1	COLD	LAK	51 STREET	50 AVENUE			50 AVE	51 ST			ON 50 AVE IN FRONT OF MOVIE GALLERY	3/13/2007	18	3	2	0	0	1	97
1721052	2007	2	1137	1	COLD		51 STREET	50 AVENUE			50 AVE	51 ST			ON 50 AVE IN FRONT OF MOVIE GALLERY	3/13/2007	18	3	2	0	0	1	97
1721096	2007	1	1137	1	COLD		51 STREET	50 AVENUE			51 ST	50 AVE				3/26/2007	14	3	2	0	0	1	12
1721096	2007	2	1137	1	COLD	LAK	51 STREET	50 AVENUE			51 ST	50 AVE				3/26/2007	14	3	2	0	0	1	12
2041278	2008	1	1137	1	COLD		51 STREET	50 AVENUE			50 AVE	51 ST			IN FRONT OF IDA PHARMACY (NORLITE) 5016 50 AVE	8/3/2008	12	3	2	0	0	2	13
2041278	2008	2	1137	1	COLD		51 STREET	50 AVENUE			50 AVE	51 ST			IN FRONT OF IDA PHARMACY (NORLITE) 5016 50 AVE	8/3/2008	12	3	2	0	0	2	13
2041593	2008	1	1137	1	COLD		51 STREET	50 AVENUE			50 AVE	51 ST				8/14/2008	14	3	2	0	0	1	13
2041593	2008	2	1137	1	COLD		51 STREET	50 AVENUE			50 AVE	51 ST				8/14/2008	14	3	2	0	0	1	13
2209586	2008	1	1137	1	COLD		51 STREET	50 AVENUE			50 AVE	51 ST				12/16/2008	17	3	2	0	0	2	13
2209586	2008	2	1137	1	COLD		51 STREET	50 AVENUE			50 AVE	51 ST				12/16/2008	17	3	2	0	0	2	13
2209698	2008	1	1137	1	COLD		51 STREET	50 AVENUE			50 AVE	51 ST				10/3/2008	16	3	2	0	0	2	13
2209698	2008	2	1137	1	COLD		51 STREET	50 AVENUE			50 AVE	51 ST				10/3/2008	16	3	2	0	0	2	13
2209729	2008	1	1137	1	COLD		51 STREET	50 AVENUE			50 AVE	51 ST				10/24/2008	16	3	2	0	0	2	13
2209729	2008	2	1137	1	COLD		51 STREET	50 AVENUE			50 AVE	51 ST				10/24/2008	16	3	2	0	0	2	13
2307117	2009	1	1137	1	COLD		51 STREET	50 AVENUE			50 AVE	51 ST				7/8/2009	16	3	2	0	0	1	13
2307117	2009	2	1137	1	COLD	LAK	51 STREET	50 AVENUE			50 AVE	51 ST				7/8/2009	16	3	2	0	0	1	13

COLLISION O			ROAD ALIGNMENT B	ROAD CLASS	COLLISION LOCATION	OBJECT TYPE	E OBJECT ID	DRIVER/PED AGE	DRIVER/PED SEX	POINT OF	DRIVER ACTION	LIGHT CONDITION A	LIGHT CONDITION B	TRAFFIC CONTROL DEVICE PRESENT	TRAFFIC CONDITION DEVICE CONDITION	PEDESTRIAN ACTION	DRIVER/PED CONDITION	CONTRIBUTING ROAD	ENVIRONMENTAL CONDITION	SURFACE CONDITION	LOAD DETAILS	S LOAD DETAILS B ATTACHMENTS	CONDITION/ CONTRIBUTI NG FACTORS	UNSAFE SPEEDS
1720951	97	7	7	97	97	1	2	43	F	3	1	1	97	1			1	1	1	1				
1720951	97	7	7	97	97	1	2	62	F	5	8	1	97	1			1	1	1	1				
1720972	1	1	1	2	1	1	2	40	F	4	1	1	97	1			1	1	1	1				
1720972	1	1	1	2	1	1	2	58	M	8	6	1	97	1			1	1	1	1				
1778753	1	1	1	2	2	1	2	46	F	8	5	1	97	3	97		1	1	1	1				
1778753	1	1	1	2	2	2	7	43	F			1	97	97	97	1	97		1	1				
1778753	1	1	1	2	2	2	7	20	F		97	97	97	97	97		97	97	1	1				
1778872	1	1	1	2	1	1	1	48	F	6	1	1	97	1			1	1	1	1				
1778872	1	1	1	2	1	1	2	40	M	4	8	1	97	1			97	97	1	1				
1873209	1	1	1	2	2	1	2	24	F	3	99	3	2	3	1		1	1	1	1				
1873209	1	1	1	2	2	1	1	20	M	8	99	3	2	3	1		1	1	1	1				
1721052	97	1	1	4	1	5	1			8	7							97	1	3				
1721052	97	1	1	4	1	1	99				99	97	97	97	97		97	97	1	3				
1721096	1	1	1	2	1	5	1			97	7							97	1	1				
1721096	1	1	1	2	1	1	2				13	1	1	97	97		97	1	1	1				
2041278	97	1	1	2	1	1	2	60	M	4	8	1	97	1			97	1	1	1				
2041278	97	1	1	2	1	5	1			8	7							97	1	1				
2041593	97	1	1	2	97	1	2	63	M	97	8	1	1	1			97	1	1	1				
2041593	97	1	1	2	97	5	1			8	7							97	1	1				
2209586	97	1	1	2	2	1	2	18	F	8	1	3	2	3	1		99	1	1	3				
2209586	97	1	1	2	2	1	3	38	F		8	3	2	1			99	1	1	3				
2209698	97	1	1	2	2	1	3	19	F	5	8	1	1	97	97		97	1	1	1				
2209698	97	1	1	2	2	1	3	52	M	3	1	1	1	3	1		97	1	1	1				
2209729		1	1	2	2	1	1	27	F	2	1	1	1	1			97	1	1	1				
2209729		1	1	2	2	1	2	28	M		8	1	1	1			97	1	1	1				
2307117	97	1	1	2	1	1	1	23	M	4	1	1	1	3	1		1	1	1	1				
2307117	97	1	1	2	1	1	99				99	1	1	97	97		99	1	1	1				

												IF NOT AT	IF NOT AT									
COLLISION CASE		OBJECT	POLICE			NORTH/SOUTH	EAST/WEST		AT INTERSECTION		AT INTERSECTION WITH	INTERSECTION	INTERSECTION -		OCCURRENCE DATE	OCCURRENCE	COLLISION	NUMBER OF	NUMBER	NUMBER		PRIMARY
NUMBER	CASE YEAR	NUMBER	SERVICE	IN/ NEA	R CITY NAME	ROAD NAME	ROAD NAME	ON HWY #	WITH HWY#	ON STREET/AVENUE	STREET/AVENUE	DISTANCE	DESCRIPTION	SPECIAL REFERENCE	(CCYY/MM/DD)	HOUR	SEVERITY	VEHICLES	INJURED	FATALITIES	HIT AND RUN	EVENT
1552910	2005	1	1137	1	COLD LAK	50 STREET	43 AVENUE			43 AVE	50 ST			SERVICE RD IN FRONT OF TIM HORTONS	8/26/2005	16	2	2	1	0	2	3
1552910	2005	2	1137	1	COLD LAK	50 STREET	43 AVENUE			43 AVE	50 ST			SERVICE RD IN FRONT OF TIM HORTONS	8/26/2005	16	2	2	1	0	2	3
1637776	2005	1	1137	1	COLD LAK	50 STREET	43 AVENUE			43 AVE	50 ST			SERVICE RD	3/17/2005	15	3	2	0	0	2	1
1637776	2005	2	1137	1	COLD LAK	50 STREET	43 AVENUE			43 AVE	50 ST			SERVICE RD	3/17/2005	15	3	2	0	0	2	1
1720920	2005	1	1137	1	COLD LAK	50 STREET	43 AVENUE			50 ST	43 AVE				7/31/2005	15	3	2	0	0	2	12
1720920	2005	2	1137	1	COLD LAK	50 STREET	43 AVENUE			50 ST	43 AVE				7/31/2005	15	3	2	0	0	2	12
1552869	2006	1	1137	1	COLD LAK	50 STREET	43 AVENUE			50 ST	43 AVE				3/10/2006	17	3	2	0	0	2	3
1552869	2006	2	1137	1	COLD LAK	50 STREET	43 AVENUE			50 ST	43 AVE				3/10/2006	17	3	2	0	0	2	3
Z563442	2006	1	1137	1	COLD LAK	50 STREET	43 AVENUE			50 ST	43 AVE			ON EAST SIDE OF INTERSECTION IN NB LANE	7/30/2006	22	3	2	0	0		5
Z563442	2006	2	1137	1	COLD LAK	50 STREET	43 AVENUE			50 ST	43 AVE			ON EAST SIDE OF INTERSECTION IN NB LANE	7/30/2006	22	3	2	0	0		5
1873314	2007	1	1137	1	COLD LAK	50 STREET	43 AVENUE			43 AVE	50 ST				4/21/2007	15	3	2	0	0	2	1
1873314	2007	2	1137	1	COLD LAK	50 STREET	43 AVENUE			43 AVE	50 ST				4/21/2007	15	3	2	0	0	2	1
1873380	2007	1	1137	1	COLD LAK	50 STREET	43 AVENUE			43 AVE				NEXT TO TIM HORTONS 50 ST	3/28/2007	97	3	2	0	0	2	13
1873380	2007	2	1137	1	COLD LAK	50 STREET	43 AVENUE			43 AVE				NEXT TO TIM HORTONS 50 ST	3/28/2007	97	3	2	0	0	2	13
2040983	2008	1	1137	1	COLD LAK	50 STREET	43 AVENUE			43 AVE	50 ST			EAST ACCESS RD	2/12/2008	6	3	2	0	0	1	3
2040983	2008	2	1137	1	COLD LAK	50 STREET	43 AVENUE			43 AVE	50 ST			EAST ACCESS RD	2/12/2008	6	3	2	0	0	1	3
2138378	2008	1	1137	1	COLD LAK	50 STREET	43 AVENUE			50TH ST				LIGHTS BY WALMART 4301 50 ST	5/31/2008	16	3	2	0	0	2	6
2138378	2008	2	1137	1	COLD LAK	50 STREET	43 AVENUE			50TH ST				LIGHTS BY WALMART 4301 50 ST	5/31/2008	16	3	2	0	0	2	6
2209610	2008	1	1137	1	COLD LAK	50 STREET	43 AVENUE			50 STREET	43 AVENUE				11/9/2008	9	3	2	0	0	2	3
2209610	2008	2	1137	1	COLD LAK	50 STREET	43 AVENUE			50 STREET	43 AVENUE				11/9/2008	9	3	2	0	0	2	3
2041431	2009	1	1137	1	COLD LAK	50 STREET	43 AVENUE			50 ST	43 AVE				2/5/2009	13	2	2	1	0	2	5
2041431	2009	2	1137	1	COLD LAK	50 STREET	43 AVENUE			50 ST	43 AVE				2/5/2009	13	2	2	1	0	2	5

COLLISION CASE NUMBER	SPECIAL FACILITY	ROAD ALIGNMENT A	ROAD ALIGNMENT B	ROAD CLASS	COLLISION LOCATION	OBJECT TYPE		DRIVER/PED AGE	DRIVER/PED SEX	POINT OF	DRIVER ACTION	LIGHT CONDITION A	LIGHT CONDITIO			PEDESTRIAN ACTION	DRIVER/PED CONDITION	CONTRIBUTING ROAD CONDITION	ENVIRONMENTAL CONDITION	SURFACE CONDITION	LOAD DETAILS A	LOAD DETAILS B	ATTACHMENTS	TRAILER TYPE	CONDITION/ CONTRIBUTING FACTORS	UNSAFE SPEEDS	
1552910	1	1	1	2	2	1	1	69	F	6	2	1	1	3	97		1	1	1	1							
1552910	1	1	1	2	2	1	3	32	F	8	1	1	1	1			1	1	1	1							
1637776	1	1	1	2	2	1	2	52	M	8	97	1	97	3	1		1	1	4	3							
1637776	1	1	1	2	2	1	3	35	F	3	97	1	97	3	1		1	1	4	3							
1720920	1	1	1	4	1	1	2	23	M	2	99	1	1	99	97		99	99	99	99							
1720920	1	1	1	4	1	1	1	42	F	6	99	1	1	99	97		99	99	99	99							
1552869	1	1	1	4	2	1	1	57	M	6	1	1	1	2	1		1	98	4	3							
1552869	1	1	1	4	2	1	1	39	F	8	1	97	97	2	1		1	98	4	3							
Z563442	1	1	1	2	2	1	1	69	F	8		3	2	2				1	2	2							
Z563442	1	1	1	2	2	1	3	27	M	3		3	2	2				1	2	2							
1873314	97	1	1	4	2	1	3	48	M	1	1	1	97	1			1	1	1	1							
1873314	97	1	1	4	2	8	20	29	M		4	1	97	1			1	1	1	1							
1873380	1	1	1	2	1	8	17	44	M	2	8	1	97	1			1	2	1	1	2						
1873380	1	1	1	2	1	1	2	69	M	2	1	1	97	1			1	2	1	1							
2040983	97	1	1	2	2	1	2	42	M	2	1	3	97	1			1	1	1	3							
2040983	97	1	1	2	2	1	2	19	M	8	2	3	97	3	1		1	1	1	3							
2138378	9					1	1	34	F	7	1	1	1	1			97	1	1	1							
2138378	9					1	1	19	F	8	97	1	1	1			97	1	1	1							
2209610	1	1	1	2	2	1	3	41	M	3	1	1	1	2	1		1	98	2	2							
2209610	1	1	1	2	2	1	2	32	M	1	98	1	1	2	1		1	98	2	2							
2041431	1	1	1	2	2	1	1	35	F	8	97	1	99	2	1		1	1	1	1							
2041431	1	1	1	2	2	1	1	52	M	1	9	1	99	2	1		1	1	1	1							

												IF NOT AT										
COLLISION CASE	CASE	OBJECT	POLICE			NORTH/SOUTH	EAST/WEST		AT INTERSECTION		AT INTERSECTION WITH	INTERSECTION	IF NOT AT INTERSECTION -		OCCURRENCE DATE	OCCURRENCE	COLLISION	NUMBER OF	NUMBER	NUMBER		PRIMARY
NUMBER	YEAR	NUMBER	SERVICE	IN/ NEAR	CITY NAME	ROAD NAME	ROAD NAME	ON HWY#	WITH HWY #	ON STREET/AVENUE	STREET/AVENUE	DISTANCE	DESCRIPTION	SPECIAL REFERENCE	(CCYY/MM/DD)	HOUR	SEVERITY	VEHICLES	INJURED	FATALITIES	HIT AND RUN	EVENT
1637844	2005	1	1137	1	COLD LAK	52 STREET	50 AVENUE			52 ST	50 AVENUE				5/27/2005	24	3	2	0	0	2	6
1637844	2005	2	1137	1	COLD LAK	52 STREET	50 AVENUE			52 ST	50 AVENUE				5/27/2005	24	3	2	0	0	2	6
1637846	2005	1	1137	1	COLD LAK	52 STREET	50 AVENUE			50 AVE	52 ST			IN FRONT OF T D BANK	5/31/2005	11	3	2	0	0	2	13
1637846	2005	2	1137	1	COLD LAK	52 STREET	50 AVENUE			50 AVE	52 ST			IN FRONT OF T D BANK	5/31/2005	11	3	2	0	0	2	13
1720868	2005	1	1137	1	COLD LAK	52 STREET	50 AVENUE			50 AVE	52 ST				6/10/2005	20	2	2	1	0	2	8
1720868	2005	2	1137	1	COLD LAK	52 STREET	50 AVENUE			50 AVE	52 ST				6/10/2005	20	2	2	1	0	2	8
1720896	2005	1	1137	1	COLD LAK	52 STREET	50 AVENUE			50 AVE	52 ST			ACROSS FROM CREDIT UNION	7/11/2005	13	3	2	0	0	2	13
1720896	2005	2	1137	1	COLD LAK	52 STREET	50 AVENUE			50 AVE	52 ST			ACROSS FROM CREDIT UNION	7/11/2005	13	3	2	0	0	2	13
1778777	2006	1	1137	1	COLD LAK	52 STREET	50 AVENUE			52 ST		10	50 AVE		7/10/2006	14	3	2	0	0	2	13
1778777	2006	2	1137	1	COLD LAK	52 STREET	50 AVENUE			52 ST		10	50 AVE		7/10/2006	14	3	2	0	0	2	13
1873362	2006	1	1137	1	COLD LAK	52 STREET	50 AVENUE			52 ST		2	50 AVE	IN FRONT OF SMOKE DAMAGE	11/29/2006	15	3	2	0	0	2	13
1873362	2006	2	1137	1	COLD LAK	52 STREET	50 AVENUE			52 ST		2	50 AVE	IN FRONT OF SMOKE DAMAGE	11/29/2006	15	3	2	0	0	2	13
1721055	2007	1	1137	1	COLD LAK	52 STREET	50 AVENUE			52 STREET	50 AVE			SIDE STREET OF 5202 50 AVE TD BANK	3/19/2007	12	3	2	0	0	1	13
1721055	2007	2	1137	1	COLD LAK	52 STREET	50 AVENUE			52 STREET	50 AVE			SIDE STREET OF 5202 50 AVE TD BANK	3/19/2007	12	3	2	0	0	1	13
1721088	2007	1	1137	1	COLD LAK	52 STREET	50 AVENUE			50 AVE	52ST			IN FRONT OF ROYAL LEPAGE W/B	4/10/2007	10	3	2	0	0	2	8
1721088	2007	2	1137	1	COLD LAK	52 STREET	50 AVENUE			50 AVE	52ST			IN FRONT OF ROYAL LEPAGE W/B	4/10/2007	10	3	2	0	0	2	8
2041508	2008	1	1137	1	COLD LAK	52 STREET	50 AVENUE			50 AVE		10	52 ST	IN FRONT OF ORBITING TRENDS	5/9/2008	13	3	2	0	0	2	13
2041508	2008	2	1137	1	COLD LAK	52 STREET	50 AVENUE			50 AVE		10	52 ST	IN FRONT OF ORBITING TRENDS	5/9/2008	13	3	2	0	0	2	13
2041413	2009	1	1137	1	COLD LAK	52 STREET	50 AVENUE			50 AVE	52 ST				1/26/2009	9	3	2	0	0	2	3
2041413	2009	2	1137	1	COLD LAK	52 STREET	50 AVENUE			50 AVE	52 ST				1/26/2009	9	3	2	0	0	2	3
2307460	2009	1	1137	1	COLD LAK	52 STREET	50 AVENUE			50 AVE	52 ST				2/24/2009	12	2	2	6	0	2	8
2307460	2009	2	1137	1	COLD LAK	52 STREET	50 AVENUE			50 AVE	52 ST				2/24/2009	12	2	2	6	0	2	8

LLISION CASE NUMBER	SPECIAL FACILITY	ROAD ALIGNMENT A	ROAD ALIGNMENT B	ROAD CLASS	COLLISION LOCATION	OBJECT TYPE	OBJECT ID D	RIVER/PED AGE	DRIVER/PED SEX	POINT OF IMPACT	DRIVER ACTION	LIGHT CONDITION A	LIGHT CONDITION B	TRAFFIC CONTROL DEVICE PRESENT	TRAFFIC CONDITION DEVICE CONDITION	PEDESTRIAN ACTION	DRIVER/PED CONDITION	CONTRIBUTING ROAD CONDITION	ENVIRONMENTAL CONDITION	SURFACE CONDITION	LOAD DETAILS A LOAD DETAILS B	ATTACHMENTS	VEHICLE CONDITION TRAILER TYPE CONTRIBUTING FACTOR	
1637844	1	1	1	2	2	1	1	18	F	7	97	3	2	3	1		1	1	1	1				
1637844	1	1	1	2	2	1	2	21	M	7	97	3	2	3	1		1	1	1	1				
1637846	1	1	1	4	1	1	2	69	M	4	8	1	1	1			1	1	1	1				
1637846	1	1	1	4	1	1	1	25	F	3	1	1	1	1			1	1	1	1				
1720868	1	1	1	2	2	1	1	19	M	8	6	1	1	3	1		1	1	1	1				
1720868	1	1	1	2	2	1	2	20	M	4	1	1	1	3	1		1	1	1	1				
1720896	1	1	1	4	1	1	3	44	F	8	1	1	97	1			1	1	1	1				
1720896	1	1	1	4	1	1	1	68	F	4	8	1	97	1			1	1	1	1				
1778777	97	7	7	97	97	1	1	23	F	2	97	97	97	97	97		97	97	97	97				
1778777	97	7	7	97	97	1	1	17	F	4	97	97	97	97	97		97	97	97	97				
1873362	1	1	1	1	1	1	1	20	M	6	1	1	1	1			1	1	1	3				
1873362	1	1	1	1	1	1	2	37	F	4	8	1	1	1			1	1	1	3				
1721055	97	7	7	97	97	5	1			8	7							97	1	3				
1721055	97	7	7	97	97	1	2	24	F	4	8	1	97	1			1	1	1	3				
1721088	1	1	1	2	2	1	1	63	F	4	1	1	97	3	1		97	1	1	1				
1721088	1	1	1	2	2	1	3	47	F	8	99	1	97	97	97		97	1	1	1				
2041508	97	7	7	97	97	1	1	57	M	2	1	1	97	1			1	1	1	1				
2041508	97	7	7	97	97	1	2	78	F	4	8	1	97	1			1	1	1	1				
2041413	97	1	1	2	2	1	1	31	M	8	99	1	1	3	1		1	1	1	1				
2041413	97	1	1	2	2	1	2	76	M	2	99	1	1	3	1		1	1	1	1				
2307460	97	1	1	2	2	1	3	17	F	8	6	1	1	3	1		1	1	4	3				
2307460	97	1	1	2	2	1	1	17	M	4	1	1	1	3	1		1	1	4	3				

										ON		IF NOT AT	IF NOT AT										
COLLISION CASE	CASE	OBJECT	POLICE			NORTH/SOUTH	EAST/WEST		AT INTERSECTION	N STREET/AV	AT INTERSECTION WITH	INTERSECTION	INTERSECTION -		OCCURRENCE DATE	OCCURRENCE	COLLISION	NUMBER OF	NUMBER	NUMBER		PRIMARY	SPECIAL
NUMBER	YEAR	NUMBER	SERVICE	IN/ NEAR	CITY NAME	ROAD NAME	ROAD NAME	ON HWY #	WITH HWY #	ENUE	STREET/AVENUE	DISTANCE	DESCRIPTION	SPECIAL REFERENCE	(CCYY/MM/DD)	HOUR	SEVERITY	VEHICLES	INJURED	FATALITIES	HIT AND RUN	EVENT	FACILITY
1637732	2005	1	1137	1	COLD LAK	49 STREET	51 AVENUE			49 STREET	51 AVENUE				1/22/2005	17	3	2	0	0	2	1	1
1637732	2005	2	1137	1	COLD LAK	49 STREET	51 AVENUE			49 STREET	51 AVENUE				1/22/2005	17	3	2	0	0	2	1	1
1720872	2005	1	1137	1	COLD LAK	49 STREET	51 AVENUE			49 STREET	51 AVE				6/8/2005	12	2	1	1	0	2	11	97
1720872	2005	2	1137	1	COLD LAK	49 STREET	51 AVENUE			49 STREET	51 AVE				6/8/2005	12	2	1	1	0	2	11	97
1873345	2006	1	1137	1	COLD LAK	49 STREET	51 AVENUE			51 AVE	49 STREET				11/24/2006	12	3	2	0	0	2	3	1
1873345	2006	2	1137	1	COLD LAK	49 STREET	51 AVENUE			51 AVE	49 STREET				11/24/2006	12	3	2	0	0	2	3	1
2040922	2007	1	1137	1	COLD LAK	49 STREET	51 AVENUE			49 ST	51 AVE				12/16/2007	17	3	2	0	0	2	3	97
2040922	2007	2	1137	1	COLD LAK	49 STREET	51 AVENUE			49 ST	51 AVE				12/16/2007	17	3	2	0	0	2	3	97
2041824	2007	1	1137	1	COLD LAK	49 STREET	51 AVENUE			51 AVE	49 ST				12/14/2007	10	2	2	1	0	2	3	97
2041824	2007	2	1137	1	COLD LAK	49 STREET	51 AVENUE			51 AVE	49 ST				12/14/2007	10	2	2	1	0	2	3	97
2306970	2009	1	1137	1	COLD LAK	49 STREET	51 AVENUE			51 AVE	49 ST			4818 51 AVE	2/14/2009	2	3	1	0	0	1	1	97
2306970	2009	2	1137	1	COLD LAK	49 STREET	51 AVENUE			51 AVE	49 ST			4818 51 AVE	2/14/2009	2	3	1	0	0	1	1	97
2307004	2009	1	1137	1	COLD LAK	49 STREET	51 AVENUE			49 ST	51 AVE				10/8/2009	9	3	2	0	0	2	1	97
2307004	2009	2	1137	1	COLD LAK	49 STREET	51 AVENUE			49 ST	51 AVE				10/8/2009	9	3	2	0	0	2	1	97
2307161	2009	1	1137	1	COLD LAK	49 STREET	51 AVENUE			51 AVE	49 ST				12/14/2009	16	3	2	0	0	2	13	97
2307161	2009	2	1137	1	COLD LAK	49 STREET	51 AVENUE			51 AVE	49 ST				12/14/2009	16	3	2	0	0	2	13	97
2307195	2009	1	1137	1	COLD LAK	49 STREET	51 AVENUE			49 ST	51 AVE				12/27/2009	14	3	2	0	0	2	8	97
2307195	2009	2	1137	1	COLD LAK	49 STREET	51 AVENUE			49 ST	51 AVE				12/27/2009	14	3	2	0	0	2	8	97
2307448	2009	1	1137	1	COLD LAK	49 STREET	51 AVENUE			51 AVE	49 ST				3/9/2009	17	3	2	0	0	2	8	97
2307448	2009	2	1137	1	COLD LAK	49 STREET	51 AVENUE			51 AVE	49 ST				3/9/2009	17	3	2	0	0	2	8	97

																								VEHICLE CONDITION/	
COLLISION CASE	ROAD	ROAD	ROAD	COLLISION	OBJECT		DRIVER/PED	DRIVER/PED	POINT OF	DRIVER	LIGHT	LIGHT	TRAFFIC CONTROL	TRAFFIC CONDITION	PEDESTRIAN	DRIVER/PED	CONTRIBUTING	ENVIRONMENTAL	SURFACE		LOAD DETAILS			CONTRIBUTING	UNSAFE
NUMBER	ALIGNMENT A	ALIGNMENT B	CLASS	LOCATION	TYPE	OBJECT ID	AGE	SEX	IMPACT	ACTION	CONDITION A	CONDITION B	DEVICE PRESENT	DEVICE CONDITION	ACTION	CONDITION	ROAD CONDITION	CONDITION	CONDITION	LOAD DETAILS A	В	ATTACHMENTS	TRAILER TYPE	FACTORS	SPEEDS
1637732	1	1	2	2	5	2			6	7							97	1	3						
1637732	1	1	2	2	1	2	21	M	8	98	1	97	3	99		1	1	1	3						
1720872	7	7	97	97	2	7	14	F			1	97	3	97	98	1		1	1						
1720872	7	7	97	97	1	3	60	F	8	1	1	97	3	97		1	1	1	1						
1873345	1	1	2	2	1	3	44	M	8	1	1	97	1			1	1	4	3						
1873345	1	1	2	2	1	1	19	F	1	1	1	97	1			1	1	4	3						
2040922	1	1	2	1	1	1	48	M	8	97	97	97	97	97		97	97	97	97						
2040922	1	1	2	1	1	1	19	F	6	97	97	97	97	97		97	97	97	97						
2041824	1	1	2	2	1	3	29	F	8	1	1	97	1			1	1	1	3						
2041824	1	1	2	2	1	3	60	M	1	3	1	97	4	1		1	1	1	3						
2306970	1	1	2	1	1	2	19	M	8	12	3	2	1			3	1	1	3						
2306970	1	1	2	1	9	13												1	3						
2307004	1	1	2	2	1	1	22	F	1	99	1	1	4	1		1	4	4	3						
2307004	1	1	2	2	1	2	42	M		1	1	1	4	1		1	4	4	3						
2307161	1	1	2	2	1	1	52	M	8	1	1	1	4	1		97	1	1	3						
2307161	1	1	2	2	1	2	83	M	4	8	1	1	97	97		97	97	1	3						
2307195	1	1	2	2	1	3	69	M	6	1	1	1	3	97		1	3	1	3						
2307195	1	1	2	2	1	3	26	F	2	6	1	1	3	97		1	3	1	3						
2307448	1	1	2	2	1	3	48	M	4	6	3	2	4	1		1	1	1	3						
2307448	1	1	2	2	1	2	45	M	8	1	3	2	4	1		1	1	1	3						

COLLISION CASE	CASE	OBJECT	POLICE			NORTH/SOUTH	EAST/WEST		AT INTERSECTION		AT INTERSECTION WITH	IF NOT AT	IF NOT AT INTERSECTION -		OCCURRENCE DATE	OCCURRENCE	COLLISION	NUMBER OF	NUMBER	NUMBER	HIT AND	PRIMARY	SPECIAL
NUMBER	YEAR	NUMBER	SERVICE	IN/ NEAR	CITY NAME	ROAD NAME	ROAD NAME	ON HWY #	WITH HWY #	ON STREET/AVENUE	STREET/AVENUE	DISTANCE	DESCRIPTION	SPECIAL REFERENCE	(CCYY/MM/DD)	HOUR	SEVERITY	VEHICLES	INJURED	FATALITIES	RUN	EVENT	FACILITY
1721201	2005	1	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		43 AVENUE			MACDONALD/TIM HORTON INTERSECTION	12/10/2005	12	2	2	2	0	2	3	1
1721201	2005	2	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		43 AVENUE			MACDONALD/TIM HORTON INTERSECTION	12/10/2005	12	2	2	2	0	2	3	1
1778737	2006	1	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		43 AVE				3/8/2006	8	3	2	0	0	2	8	1
1778737	2006	2	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		43 AVE				3/8/2006	8	3	2	0	0	2	8	1
1778908	2006	1	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		43 AVE				6/9/2006	9	2	3	5	0	2	8	1
1778908	2006	2	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		43 AVE				6/9/2006	9	2	3	5	0	2	8	1
1778908	2006	3	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		43 AVE				6/9/2006	9	2	3	5	0	2	8	1
1873368	2006	1	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		43 AVE				11/30/2006	15	3	2	0	0	2	8	1
1873368	2006	2	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		43 AVE				11/30/2006	15	3	2	0	0	2	8	1
1873390	2006	1	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		50 ST			INTERSECTION @ TIM HORTON'S 43 AVE	11/30/2006	18	3	2	0	0	2	1	1
1873390	2006	2	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		50 ST			INTERSECTION @ TIM HORTON'S 43 AVE	11/30/2006	18	3	2	0	0	2	1	1
1873425	2006	1	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		43 AVE				11/27/2006	14	3	2	0	0	2	8	1
1873425	2006	2	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		43 AVE				11/27/2006	14	3	2	0	0	2	8	1
2040989	2008	1	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		43AVE			AT INTERSECTION OF HWY28& 43AVE	2/17/2008	13	3	2	0	0	2	8	97
2040989	2008	2	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		43AVE			AT INTERSECTION OF HWY28& 43AVE	2/17/2008	13	3	2	0	0	2	8	97
2209616	2008	1	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		43 AVE				11/12/2008	13	3	2	0	0	2	8	1
2209616	2008	2	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		43 AVE				11/12/2008	13	3	2	0	0	2	8	1
2209686	2008	1	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		43 AVE				10/2/2008	18	3	2	0	0	2	5	1
2209686	2008	2	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1		43 AVE				10/2/2008	18	3	2	0	0	2	5	1
2307091	2009	1	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1					TIM HORTONS	11/6/2009	17	3	1	0	0	2	1	97
2307091	2009	2	1137	1	COLD LAK	HWY 28	43 AVENUE	28	1					TIM HORTONS	11/6/2009	17	3	1	0	0	2	1	97

COLLISION CASE NUMBER	ROAD ALIGNMENT A	ROAD ALIGNMENT B	ROAD CLASS	COLLISION LOCATION	OBJECT TYPE	OBJECT ID	DRIVER/PED AGE	DRIVER/PED SEX	POINT OF	DRIVER L	IGHT CONDITION	LIGHT CONDITION B	TRAFFIC CONTROL DEVICE PRESENT	TRAFFIC CONDITION DEVICE CONDITION	PEDESTRIAN ACTION	DRIVER/PED CONDITION	CONTRIBUTING ROAD CONDITION	ENVIRONMENTAL CONDITION	SURFACE CONDITION	LOAD DETAILS A	LOAD DETAILS B	ATTACHMENTS	TRAILER TYPE	VEHICLE CONDITION/ CONTRIBUTING FACTORS	UNSAFE SPEEDS
1721201	1	1	3	2	1	1	26	M	7	1	1	97	2	1		1	1	1	1						
1721201	1	1	3	2	1	2	77	F	1	11	1	97	2	1		1	1	1	1						
1778737	1	1	2	2	1	1	66	M	8	97	97	97	97	97		97	97	97	97						
1778737	1	1	2	2	1	10	58	M	4	97	97	97	97	97		97	97	97	97						
1778908	1	1	2	2	1	1	61	F	4	1	1	1	2	1		1	1	2	2						
1778908	1	1	2	2	1	3	69	M	4	1	1	1	2	1		1	1	2	2						
1778908	1	1	2	2	1	1	45	F	8	98	1	1	2	1		1	1	2	2						
1873368	1	1	4	2	1	1	21	M	8	99	1	1	2	1		1	1	4	3						
1873368	1	1	4	2	1	1	71	M	4	1	1	1	2	1		1	1	4	3						
1873390	1	1	2	1	1	2	36	M	4	1	3	97	2	1		1	4	1	3						
1873390	1	1	2	1	1	1	21	M		97	3	97	2	1		1	4	1	3						
1873425	1	1	3	2	1	3	31	M	8	6	1	1	1			1	1	1	3						
1873425	1	1	3	2	1	2	20	M	4	1	1	1	1			1	1	1	3						
2040989	1	1	2	2	1	1	39	M	4	1	1	97	2	1		1	1	1	3						
2040989	1	1	2	2	1	2	42	M	8	6	1	97	2	1		99	1	1	3						
2209616	1	1	2	2	1	1	46	F	4	1	1	97	2	1		1	1	4	3						
2209616	1	1	2	2	1	1	17	M	8	6	1	97	2	1		1	1	4	3						
2209686	1	1	2	2	1	2	44	M	5	1	1	1	1			1	1	1	1						
2209686	1	1	2	2	1	3	56	M	8	9	2	1	1			1	1	1	1						
2307091	7	7	97	97	1	1	38	M	8	1	3	99	1			1	1	1	1						
2307091	7	7	97	97	7	15												1	1						

										ON	AT INTERSECTION	IF NOT AT	IF NOT AT									
COLLISION CASE	CASE	OBJECT	POLICE			NORTH/SOUTH	EAST/WEST ROAD		AT INTERSECTION	STREET/AV		INTERSECTION	INTERSECTION -		OCCURRENCE DATE		COLLISION	NUMBER OF	NUMBER	NUMBER	HIT AND	PRIMARY
NUMBER	YEAR	NUMBER	SERVICE	IN/NEAR	CITY NAME	ROAD NAME	NAME	ON HWY #	WITH HWY #	ENUE	STREET/AVENUE	DISTANCE	DESCRIPTION	SPECIAL REFERENCE	(CCYY/MM/DD)	HOUR	SEVERITY	VEHICLES	INJURED	FATALITIES	RUN	EVENT
1721004	2005	1	1137	1	COLD LAK	HWY 28	55 AVENUE	28	1		55 AVENUE				11/7/2005	14	3	2	0	0	2	8
1721004	2005	2	1137	1	COLD LAK	HWY28	55 AVENUE	28	1		55 AVENUE				11/7/2005	14	3	2	0	0	2	8
1720965	2006	1	1137	1	COLD LAK	HWY28	55 AVENUE	28	1		55 AVE				11/7/2006	15	3	1	0	0	2	1
1720965	2006	2	1137	1	COLD LAK	HWY28	55 AVENUE	28	1		55 AVE				11/7/2006	15	3	1	0	0	2	1
1721231	2006	1	1137	1	COLD LAK	HWY28	55 AVENUE	28	1		55 AVENUE				1/13/2006	15	3	2	0	0	2	8
1721231	2006	2	1137	1	COLD LAK	HWY 28	55 AVENUE	28	1		55 AVENUE				1/13/2006	15	3	2	0	0	2	8
1873321	2007	1	1137	1	COLD LAK	HWY 28	55 AVENUE	28	1		55 AVE			BY HUSKY SERVICE STATION	4/18/2007	16	2	3	1	0	2	12
1873321	2007	2	1137	1	COLD LAK	HWY 28	55 AVENUE	28	1		55 AVE			BY HUSKY SERVICE STATION	4/18/2007	16	2	3	1	0	2	12
1873321	2007	3	1137	1	COLD LAK	HWY 28	55 AVENUE	28	1		55 AVE			BY HUSKY SERVICE STATION	4/18/2007	16	2	3	1	0	2	12
1954761	2007	1	1137	1	COLD LAK	HWY 28	55 AVENUE	28	1		55 AVE				9/6/2007	16	3	2	0	0	2	8
1954761	2007	2	1137	1	COLD LAK	HWY 28	55 AVENUE	28	1		55 AVE				9/6/2007	16	3	2	0	0	2	8
Z541953	2007	1	1137	1	COLD LAK	HWY 28	55 AVENUE	28	1		55 AVE				11/23/2007	18	3	2	0	0		1
Z541953	2007	2	1137	1	COLD LAK	HWY 28	55 AVENUE	28	1		55 AVE				11/23/2007	18	3	2	0	0		1
2041570	2008	1	1137	1	COLD LAK	HWY 28	55 AVENUE	28	1		55 AVE			IN CITY - DO NOT PLOT	6/26/2008	18	3	2	0	0	2	5
2041570	2008	2	1137	1	COLD LAK	HWY 28	55 AVENUE	28	1		55 AVE			IN CITY - DO NOT PLOT	6/26/2008	18	3	2	0	0	2	5
2041772	2008	1	1137	1	COLD LAK	HWY 28	55 AVENUE	28	1		55 AVE				2/6/2008	8	2	2	1	0	2	5
2041772	2008	2	1137	1	COLD LAK	HWY 28	55 AVENUE	28	1		55 AVE				2/6/2008	8	2	2	1	0	2	5
2209659	2008	1	1137	1	COLD LAK	HWY 28	55 AVENUE	28	1		55 AVE				11/25/2008	12	2	2	2	0	2	3
2209659	2008	2	1137	1	COLD LAK	HWY 28	55 AVENUE	28	1		55 AVE				11/25/2008	12	2	2	2	0	2	3
2307093	2009	1	1137	1	COLD LAK	HWY 28	55 AVENUE	28	1					SB NEAR HUSKY SS	11/7/2009	17	3	2	0	0	2	8
2307093	2009	2	1137	1	COLD LAK	HWY 28	55 AVENUE	28	1					SB NEAR HUSKY SS	11/7/2009	17	3	2	0	0	2	8

														TRAFFIC CONTROL	TRAFFIC CONDITION									VEHICLE CONDITION/	
COLLISION CASE		ROAD ALIGNMENT F	ROAD ALIGNMENT		COLLISION	OBJECT		DRIVER/PED		POINT OF	DRIVER	LIGHT	LIGHT CONDITION	DEVICE	DEVICE	PEDESTRIAN	DRIVER/PED	CONTRIBUTING ROAD	ENVIRONMENTAL	SURFACE				CONTRIBUTING	UNSAFE
NUMBER	FACILITY	Α	В	ROAD CLASS	LOCATION	TYPE	OBJECT ID	AGE	DRIVER/PED SEX	IMPACT	ACTION	CONDITION A	В	PRESENT	CONDITION	ACTION	CONDITION	CONDITION	CONDITION	CONDITION	LOAD DETAILS A LOAD DETAILS B	ATTACHMENTS	TRAILER TYPE	FACTORS	SPEEDS
1721004	1	1	1	2	2	1	3	18	M	8	6	1	97	1			1	1	1	2					
1721004	1	1	1	2	2	1	1	25	M	4	1	1	97	1			1	1	1	2					
1720965	1	1	1	2	1	1	1	27	M	8	98	1	97	1			1	1	1	3					
1720965	1	1	1	2	1	9	13												1	3					
1721231	1	1	1	2	2	1	2	17	M	8	97	1	97	1			1	1	97	3					
1721231	1	1	1	2	2	1	3	32	F	4	1	1	97	1			1	1	97	3					
1873321	97	1	1	2	2	1	1	41	M	1	99	1	97	97	97		1	1	2	2					
1873321	97	1	1	2	2	1	3	31	F	5	1	1	97	97	97		1	1	2	2					
1873321	97	1	1	2	2	1	3	47	F	2	1	1	97	1			1	1	2	2					
1954761	1	1	1	2	2	1	1	17	F	8	6	1	97	1			1	4	2	2					
1954761	1	1	1	2	2	1	2	28	M		1	1	97	1			1	4	2	2					
Z541953	97	7	7	97	97	1	1	45	M	4		3	2	1				1	99	3					
Z541953	97	7	7	97	97	1	2	22	M	8		3	2	1				1	99	3					
2041570	97	1	1	2	2	1	2	47	M	8	1	1	1	1			1	4	2	2					
2041570	97	1	1	2	2	1	2	29	M	7	9	1	1	3	1		1	4	2	2					
2041772	97	1	1	4	2	1	9	72	M	8	99	1	1	2	97		1	1	1	3					
2041772	97	1	1	4	2	1	1	42	F	1	9	1	1	2	97		1	1	1	3					
2209659	1	1	1	2	2	1	1	21	F	6	9	1	99	3	99		1	1	1	3					
2209659	1	1	1	2	2	1	3	27	F		1	1	99	1			1	1	1	3					
2307093	1	1	1	2	1	1	4	16	M	4	1	3	2	1			1	1	1	1					
2307093	1	1	1	2	1	1	4	16	M	8	6	3	2	1			1	1	1	1					

## **TECHNICAL MEMORANDUM**



# **Appendix B - Collision Analysis Summaries**



#### **HWY 28 & 54 AVENUE COLLISION ANALYSIS**

YEAR	NO. OF COLLISIONS	% OF TOTAL
2005	10	24%
2006	11	26%
2007	9	21%
2008	11	26%
2009	1	2%
Total	42	100%

Collision by Month

	MONTH	NO. OF COLLISIONS	% OF TOTAL
1	January	6	14%
2	February	6	14%
3	March	2	5%
4	April	2	5%
5	May	1	2%
6	June	4	10%
7	July	3	7%
8	August	6	14%
9	September	3	7%
10	October	2	5%
11	November	6	14%
12	December	1	2%
	Total	42	100%

Collision by Day

	DAY	NO. OF COLLISIONS	% OF TOTAL
1	Sunday	2	5%
2	Monday	6	14%
3	Tuesday	8	19%
4	Wednesday	7	17%
5	Thursday	6	14%
6	Friday	9	21%
7	Saturday	4	10%
	Total	42	100%

Collision by Hour

	HOUR	NO. OF COLLISIONS	% OF TOTAL
1	1:00 AM	1	2%
2	2:00 AM	0	0%
3	3:00 AM	0	0%
4	4:00 AM	0	0%
5	5:00 AM	0	0%
6	6:00 AM	0	0%
7	7:00 AM	0	0%
8	8:00 AM	0	0%
9	9:00 AM	2	5%
10	10:00 AM	5	12%
11	11:00 AM	2	5%
12	12:00 PM	4	10%
13	1:00 PM	2	5%
14	2:00 PM	5	12%
15	3:00 PM	4	10%
16	4:00 PM	5	12%
17	5:00 PM	7	17%
18	6:00 PM	4	10%
19	7:00 PM	1	2%
20	8:00 PM	0	0%
21	9:00 PM	0	0%
22	10:00 PM	0	0%
23	11:00 PM	0	0%
24	12:00 AM	0	0%
<u> </u>	Total	42	100%

	COLLISION TYPE	NO. OF COLLISIONS	% OF TOTAL
1	Struck Object	2	5%
2	Off Road Left	0	0%
3	Right Angle	2	5%
4	Passing - Left Turn	0	0%
5	Left Turn - Across Path	14	33%
6	Sideswipe	0	0%
7	Other	1	2%
8	Rear End	14	33%
9	Off Road Right	0	0%
10	Head On	0	0%
11	Passing Right Turn	1	2%
12	Sideswipe - Same Direction	4	10%
13	Backing	1	2%
97	Unknown	3	7%
	Total	42	100%

#### **HWY 28 & 54 AVENUE COLLISION ANALYSIS**

Collision by Cause

	COLLISION CAUSE	NO. OF COLLISIONS	% OF TOTAL
1	Driving Properly	2	5%
2	Stop Sign Violation	0	0%
3	Yield Sign Violation	0	0%
4	Fail to Yield Right-of-Way, Uncontrolled Intersection	1	2%
5	Fail to Yield Right-of-Way, Pedestrian	0	0%
6	Followed Too Closely	5	12%
7	Parked Vehicle	0	0%
8	Backed Unsafely	1	2%
9	Left Turn Across Path	9	21%
10	Improper Lane Change	6	14%
11	Disobey Traffic Signal	0	0%
12	Ran off Road	0	0%
13	Improper Turn	2	5%
14	Left of Centre	0	0%
15	Improper Passing	0	0%
97	Blank	15	36%
98	Other	0	0%
99	Unknown	1	2%
	Total	42	100%

Collision by Severity

	COLLISION SEVERITY	NO. OF COLLISIONS	% OF TOTAL
1	Fatal	0	0%
2	Injury	9	21%
3	Property Damage	33	79%
97	Unknown	0	0%
	Total	42	100%

Collision by Weather Condition

E	NVIRONMENTAL CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Clear	30	71%
2	Raining	2	5%
3	Hail/Sleet	0	0%
4	Snow	5	12%
5	Fog/Smog/Smoke/Dust	1	2%
6	High Wind	0	0%
97	Unknown	4	10%
98	Other	0	0%
99	Unknown	0	0%
Total		42	100%

Collision by Surface Condition

	SURFACE CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Dry	19	45%
2	Wet	6	14%
3	Slush/Snow/Ice	13	31%
4	Loose Surface Material	0	0%
5	Muddy	0	0%
97	Unknown	3	7%
98	Other	0	0%
99	Unknown	1	2%
	Total	42	100%

	LIGHT CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Daylight	34	81%
2	Sunglare	0	0%
3	Darkness	6	14%
97	Unknown	2	5%
99	Unknown	0	0%
	Total	42	100%

#### **HWY 28 & TRI CITY MALL COLLISION ANALYSIS**

Collision by Year

YEAR	NO. OF COLLISIONS	% OF TOTAL
2005	6	21%
2006	6	21%
2007	7	24%
2008	9	31%
2009	1	3%
Total	29	100%

	MONTH	NO. OF COLLISIONS	% OF TOTAL
1	January	4	14%
2	February	5	17%
3	March	3	10%
4	April	0	0%
5	May	3	10%
6	June	1	3%
7	July	1	3%
8	August	1	3%
9	September	2	7%
10	October	1	3%
11	November	3	10%
12	December	5	17%
	Total	29	100%

Collision by Day			
	DAY	NO. OF COLLISIONS	% OF TOTAL
1	Sunday	2	7%
2	Monday	5	17%
3	Tuesday	1	3%
4	Wednesday	5	17%
5	Thursday	4	14%
6	Friday	9	31%
7	Saturday	3	10%
	Total	20	100%

Collision by Hour

	HOUR	NO. OF COLLISIONS	% OF TOTAL
1	1:00 AM	0	0%
2	2:00 AM	0	0%
3	3:00 AM	0	0%
4	4:00 AM	0	0%
5	5:00 AM	0	0%
6	6:00 AM	1	3%
7	7:00 AM	1	3%
8	8:00 AM	0	0%
9	9:00 AM	0	0%
10	10:00 AM	1	3%
11	11:00 AM	0	0%
12	12:00 PM	6	21%
13	1:00 PM	4	14%
14	2:00 PM	2	7%
15	3:00 PM	4	14%
16	4:00 PM	3	10%
17	5:00 PM	2	7%
18	6:00 PM	2	7%
19	7:00 PM	1	3%
20	8:00 PM	0	0%
21	9:00 PM	1	3%
22	10:00 PM	0	0%
23	11:00 PM	0	0%
24	12:00 AM	0	0%
97	Unknown	1	3%
	Total	29	100%

Complete by Type	COLLISION TYPE	NO. OF COLLISIONS	% OF TOTAL
1	Struck Object	2	7%
2	Off Road Left	0	0%
3	Right Angle	3	10%
4	Passing - Left Turn	0	0%
5	Left Turn - Across Path	0	0%
6	Sideswipe	0	0%
7	Other	0	0%
8	Rear End	20	69%
9	Off Road Right	1	3%
10	Head On	1	3%
11	Passing Right Turn	0	0%
12	Sideswipe - Same Direction	1	3%
13	Backing	0	0%
97	Unknown	1	3%
	Total	29	100%

#### **HWY 28 & TRI CITY MALL COLLISION ANALYSIS**

Collision by Cause

	COLLISION CAUSE	NO. OF COLLISIONS	% OF TOTAL
1	Driving Properly	5	17%
2	Stop Sign Violation	0	0%
3	Yield Sign Violation	0	0%
4	Fail to Yield Right-of-Way, Uncontrolled Intersection	0	0%
5	Fail to Yield Right-of-Way, Pedestrian	0	0%
6	Followed Too Closely	10	34%
7	Parked Vehicle	0	0%
8	Backed Unsafely	0	0%
9	Left Turn Across Path	0	0%
10	Improper Lane Change	1	3%
11	Disobey Traffic Signal	0	0%
12	Ran off Road	0	0%
13	Improper Turn	0	0%
14	Left of Centre	0	0%
15	Improper Passing	0	0%
97	Blank	9	31%
98	Other	1	3%
99	Unknown	3	10%
	Total	29	100%

Complete by CC	verity		
	COLLISION SEVERITY	NO. OF COLLISIONS	% OF TOTAL
1	Fatal	0	0%
2	Injury	3	10%
3	Property Damage	26	90%
97	Unknown	0	0%
	Total	29	100%

Collision by Weather Conditio

	ENVIRONMENTAL CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Clear	22	76%
2	Raining	2	7%
3	Hail/Sleet	0	0%
4	Snow	4	14%
5	Fog/Smog/Smoke/Dust	0	0%
6	High Wind	0	0%
97	Unknown	1	3%
98	Other	0	0%
99	Unknown	0	0%
	Total	29	100%

Collision by Surface Condition

Collision by Sun	ace Condition		
	SURFACE CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Dry	11	38%
2	Wet	3	10%
3	Slush/Snow/Ice	13	45%
4	Loose Surface Material	0	0%
5	Muddy	0	0%
97	Unknown	2	7%
98	Other	0	0%
99	Unknown	0	0%
	Total	29	100%

Collision by Lig	ht Condition - Natural Light		
	LIGHT CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Daylight	24	83%
2	Sunglare	0	0%
3	Darkness	4	14%
97	Unknown	0	0%
99	Unknown	1	3%
	Total	29	100%

#### **HWY 28 & 50 AVENUE COLLISION ANALYSIS**

YEAR	NO. OF COLLISIONS	% OF TOTAL
2005	14	56%
2006	3	12%
2007	4	16%
2008	4	16%
2009	0	0%
Total	25	100%

Collision by Month

	MONTH	NO. OF COLLISIONS	% OF TOTAL
1	January	1	4%
2	February	4	16%
3	March	5	20%
4	April	0	0%
5	May	2	8%
6	June	1	4%
7	July	3	12%
8	August	1	4%
9	September	1	4%
10	October	2	8%
11	November	3	12%
12	December	2	8%
	Total	25	100%

Collision by Day

	DAY	NO. OF COLLISIONS	% OF TOTAL
1	Sunday	1	4%
2	Monday	4	16%
3	Tuesday	4	16%
4	Wednesday	2	8%
5	Thursday	8	32%
6	Friday	5	20%
7	Saturday	1	4%
	Total	25	100%

Collision by Hour

	HOUR	NO. OF COLLISIONS	% OF TOTAL
1	1:00 AM	0	0%
2	2:00 AM	0	0%
3	3:00 AM	0	0%
4	4:00 AM	0	0%
5	5:00 AM	0	0%
6	6:00 AM	1	4%
7	7:00 AM	3	12%
8	8:00 AM	1	4%
9	9:00 AM	2	8%
10	10:00 AM	0	0%
11	11:00 AM	2	8%
12	12:00 PM	3	12%
13	1:00 PM	1	4%
14	2:00 PM	3	12%
15	3:00 PM	2	8%
16	4:00 PM	1	4%
17	5:00 PM	1	4%
18	6:00 PM	1	4%
19	7:00 PM	1	4%
20	8:00 PM	1	4%
21	9:00 PM	0	0%
22	10:00 PM	0	0%
23	11:00 PM	0	0%
24	12:00 AM	0	0%
97	Unknown	2	8%
	Total	25	100%

	COLLISION TYPE	NO. OF COLLISIONS	% OF TOTAL
1	Struck Object	0	0%
2	Off Road Left	0	0%
3	Right Angle	1	4%
4	Passing - Left Turn	0	0%
5	Left Turn - Across Path	7	28%
6	Sideswipe	0	0%
7	Other	1	4%
8	Rear End	9	36%
9	Off Road Right	1	4%
10	Head On	0	0%
11	Passing Right Turn	0	0%
12	Sideswipe - Same Direction	2	8%
13	Backing	2	8%
97	Unknown	2	8%
	Total	25	100%

#### **HWY 28 & 50 AVENUE COLLISION ANALYSIS**

Collision by Cause

_	COLLISION CAUSE	NO. OF COLLISIONS	% OF TOTAL
1	Driving Properly	4	16%
2	Stop Sign Violation	0	0%
3	Yield Sign Violation	0	0%
4	Fail to Yield Right-of-Way, Uncontrolled Intersection	0	0%
5	Fail to Yield Right-of-Way, Pedestrian	0	0%
6	Followed Too Closely	2	8%
7	Parked Vehicle	0	0%
8	Backed Unsafely	1	4%
9	Left Turn Across Path	4	16%
10	Improper Lane Change	1	4%
11	Disobey Traffic Signal	2	8%
12	Ran off Road	0	0%
13	Improper Turn	1	4%
14	Left of Centre	1	4%
15	Improper Passing	0	0%
97	Blank	7	28%
98	Other	0	0%
99	Unknown	2	8%
	Total	25	100%

Collision by Severity

Collision by Severity				
COLLISION SEVERITY		NO. OF COLLISIONS	% OF TOTAL	
1	Fatal	0	0%	
2	Injury	5	20%	
3	Property Damage	20	80%	
97	Unknown	0	0%	
	Total	25	100%	

Collision by Weather Condition

	ENVIRONMENTAL CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Clear	17	68%
2	Raining	1	4%
3	Hail/Sleet	0	0%
4	Snow	6	24%
5	Fog/Smog/Smoke/Dust	0	0%
6	High Wind	0	0%
97	Unknown	1	4%
98	Other	0	0%
99	Unknown	0	0%
	Total	25	100%

Collision by Surface Condition

	SURFACE CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Dry	11	44%
2	Wet	3	12%
3	Slush/Snow/Ice	10	40%
4	Loose Surface Material	0	0%
5	Muddy	0	0%
97	Unknown	1	4%
98	Other	0	0%
99	Unknown	0	0%
	Total	25	100%

Collision by Light Condition - Natural Light				
	LIGHT CONDITION	NO. OF COLLISIONS	% OF TOTAL	
1	Daylight	20	80%	
2	Sunglare	0	0%	
3	Darkness	4	16%	
97	Unknown	1	4%	
99	Unknown	0	0%	
	Total	25	100%	

#### 55/55A STREET & 54 AVENUE COLLISION ANALYSIS

YEAR	NO. OF COLLISIONS	% OF TOTAL
2005	0	0%
2006	5	22%
2007	5	22%
2008	5	22%
2009	8	35%
Total	23	100%

Collision by Month

	MONTH	NO. OF COLLISIONS	% OF TOTAL
1	January	1	4%
2	February	2	9%
3	March	0	0%
4	April	1	4%
5	May	2	9%
6	June	1	4%
7	July	2	9%
8	August	1	4%
9	September	3	13%
10	October	1	4%
11	November	4	17%
12	December	5	22%
	Total	23	100%

Collision by Day

Comision by Day	DAY	NO. OF COLLISIONS	% OF TOTAL
1	Sunday	0	0%
2	Monday	6	26%
3	Tuesday	4	17%
4	Wednesday	4	17%
5	Thursday	3	13%
6	Friday	4	17%
7	Saturday	2	9%
	Total	23	100%

Collision by Hour

HOUR		NO. OF COLLISIONS	% OF TOTAL
1	1:00 AM	0	0%
2	2:00 AM	0	0%
3	3:00 AM	0	0%
4	4:00 AM	0	0%
5	5:00 AM	0	0%
6	6:00 AM	0	0%
7	7:00 AM	1	4%
8	8:00 AM	3	13%
9	9:00 AM	0	0%
10	10:00 AM	0	0%
11	11:00 AM	1	4%
12	12:00 PM	1	4%
13	1:00 PM	1	4%
14	2:00 PM	2	9%
15	3:00 PM	3	13%
16	4:00 PM	1	4%
17	5:00 PM	4	17%
18	6:00 PM	2	9%
19	7:00 PM	2	9%
20	8:00 PM	0	0%
21	9:00 PM	0	0%
22	10:00 PM	0	0%
23	11:00 PM	0	0%
24	12:00 AM	0	0%
97	Unknown	2	9%
	Total	23	100%

	COLLISION TYPE	NO. OF COLLISIONS	% OF TOTAL
1	Struck Object	2	9%
2	Off Road Left	0	0%
3	Right Angle	5	22%
4	Passing - Left Turn	0	0%
5	Left Turn - Across Path	4	17%
6	Sideswipe	2	9%
7	Other	1	4%
8	Rear End	6	26%
9	Off Road Right	0	0%
10	Head On	0	0%
11	Passing Right Turn	0	0%
12	Sideswipe - Same Direction	1	4%
13	Backing	0	0%
97	Unknown	2	9%
	Total	23	100%

#### 55/55A STREET & 54 AVENUE COLLISION ANALYSIS

	COLLISION CAUSE	NO. OF COLLISIONS	% OF TOTAL
1	Driving Properly	1	4%
2	Stop Sign Violation	1	4%
3	Yield Sign Violation	1	4%
4	Fail to Yield Right-of-Way, Uncontrolled Intersection	0	0%
5	Fail to Yield Right-of-Way, Pedestrian	0	0%
6	Followed Too Closely	2	9%
7	Parked Vehicle	0	0%
8	Backed Unsafely	0	0%
9	Left Turn Across Path	1	4%
10	Improper Lane Change	1	4%
11	Disobey Traffic Signal	1	4%
12	Ran off Road	0	0%
13	Improper Turn	3	13%
14	Left of Centre	1	4%
15	Improper Passing	0	0%
97	Blank	3	13%
98	Other	1	4%
99	Unknown	7	30%
	Total	23	100%

Collision by Severity

Collision by Severity				
	COLLISION SEVERITY	NO. OF COLLISIONS	% OF TOTAL	
1	Fatal	0	0%	
2	Injury	4	17%	
3	Property Damage	19	83%	
97	Unknown	0	0%	
	Total	23	100%	

Collision by Weather Condition

	ENVIRONMENTAL CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Clear	13	57%
2	Raining	2	9%
3	Hail/Sleet	0	0%
4	Snow	5	22%
5	Fog/Smog/Smoke/Dust	0	0%
6	High Wind	0	0%
97	Unknown	3	13%
98	Other	0	0%
99	Unknown	0	0%
	Total	23	100%

Collision by Surface Condition

	SURFACE CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Dry	6	26%
2	Wet	2	9%
3	Slush/Snow/Ice	12	52%
4	Loose Surface Material	0	0%
5	Muddy	0	0%
97	Unknown	3	13%
98	Other	0	0%
99	Unknown	0	0%
	Total	23	100%

Collision by Lig	ht Condition - Natural Light		
	LIGHT CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Daylight	15	65%
2	Sunglare	0	0%
3	Darkness	6	26%
97	Unknown	2	9%
99	Unknown	0	0%
	Total	23	100%

#### **50 STREET & 50 AVENUE COLLISION ANALYSIS**

Collision by Year

YEAR	NO. OF COLLISIONS	% OF TOTAL
2005	2	15%
2006	1	8%
2007	5	38%
2008	3	23%
2009	2	15%
Total	13	100%

Collision by Month

	MONTH	NO. OF COLLISIONS	% OF TOTAL
1	January	1	8%
2	February	2	15%
3	March	1	8%
4	April	1	8%
5	May	1	8%
6	June	2	15%
7	July	0	0%
8	August	1	8%
9	September	1	8%
10	October	0	0%
11	November	1	8%
12	December	2	15%
	Total	13	100%

Collision by Day

	DAY	NO. OF COLLISIONS	% OF TOTAL
1	Sunday	0	0%
2	Monday	1	8%
3	Tuesday	0	0%
4	Wednesday	2	15%
5	Thursday	1	8%
6	Friday	6	46%
7	Saturday	3	23%
	Total	13	100%

Collision by Hour

•	HOUR	NO. OF COLLISIONS	% OF TOTAL
1	1:00 AM	1	8%
2	2:00 AM	0	0%
3	3:00 AM	0	0%
4	4:00 AM	0	0%
5	5:00 AM	0	0%
6	6:00 AM	0	0%
7	7:00 AM	0	0%
8	8:00 AM	0	0%
9	9:00 AM	0	0%
10	10:00 AM	1	8%
11	11:00 AM	1	8%
12	12:00 PM	1	8%
13	1:00 PM	0	0%
14	2:00 PM	1	8%
15	3:00 PM	0	0%
16	4:00 PM	2	15%
17	5:00 PM	2	15%
18	6:00 PM	3	23%
19	7:00 PM	0	0%
20	8:00 PM	0	0%
21	9:00 PM	0	0%
22	10:00 PM	0	0%
23	11:00 PM	0	0%
24	12:00 AM	1	8%
97	Unknown	0	0%
•	Total	13	100%

	COLLISION TYPE	NO. OF COLLISIONS	% OF TOTAL
1	Struck Object	1	8%
2	Off Road Left	0	0%
3	Right Angle	1	8%
4	Passing - Left Turn	0	0%
5	Left Turn - Across Path	0	0%
6	Sideswipe	0	0%
7	Other	1	8%
8	Rear End	5	38%
9	Off Road Right	0	0%
10	Head On	0	0%
11	Passing Right Turn	0	0%
12	Sideswipe - Same Direction	2	15%
13	Backing	3	23%
97	Unknown	0	0%
	Total	13	100%

#### 50 STREET & 50 AVENUE COLLISION ANALYSIS

Collision by Cause

	COLLISION CAUSE	NO. OF COLLISIONS	% OF TOTAL
1	Driving Properly	1	8%
2	Stop Sign Violation	0	0%
3	Yield Sign Violation	1	8%
4	Fail to Yield Right-of-Way, Uncontrolled Intersection	0	0%
5	Fail to Yield Right-of-Way, Pedestrian	0	0%
6	Followed Too Closely	2	15%
7	Parked Vehicle	1	8%
8	Backed Unsafely	4	31%
9	Left Turn Across Path	0	0%
10	Improper Lane Change	0	0%
11	Disobey Traffic Signal	1	8%
12	Ran off Road	0	0%
13	Improper Turn	1	8%
14	Left of Centre	0	0%
15	Improper Passing	0	0%
97	Blank	2	15%
98	Other	0	0%
99	Unknown	0	0%
	Total	13	100%

Complete by CC	oy		
	COLLISION SEVERITY	NO. OF COLLISIONS	% OF TOTAL
1	Fatal	0	0%
2	Injury	2	15%
3	Property Damage	11	85%
97	Unknown	0	0%
	Total	13	100%

Collision by Weather Conditio

Commonding we	eather Condition	NO OF COLUMNS	0/ 05 70741	
	ENVIRONMENTAL CONDITION	NO. OF COLLISIONS	% OF TOTAL	
1	Clear	11	85%	
2	Raining	0	0%	
3	Hail/Sleet	0	0%	
4	Snow	1	8%	
5	Fog/Smog/Smoke/Dust	0	0%	
6	High Wind	0	0%	
97	Unknown	0	0%	
98	Other	1	8%	
99	Unknown	0	0%	
	Total	13	100%	

Collision by Surface Condition

consisting surface condition				
	SURFACE CONDITION	NO. OF COLLISIONS	% OF TOTAL	
1	Dry	8	62%	
2	Wet	0	0%	
3	Slush/Snow/Ice	5	38%	
4	Loose Surface Material	0	0%	
5	Muddy	0	0%	
97	Unknown	0	0%	
98	Other	0	0%	
99	Unknown	0	0%	
	Total	13	100%	

	LIGHT CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Daylight	10	77%
2	Sunglare	0	0%
3	Darkness	2	15%
97	Unknown	1	8%
99	Unknown	0	0%
	Total	13	100%

#### **50 STREET & 46 AVENUE COLLISION ANALYSIS**

Collision by Year

Ochiolon by rou		
YEAR	NO. OF COLLISIONS	% OF TOTAL
2005	4	31%
2006	3	23%
2007	2	15%
2008	3	23%
2009	1	8%
Total	13	100%

Collision by Month

	MONTH	NO. OF COLLISIONS	% OF TOTAL
1	January	0	0%
2	February	0	0%
3	March	1	8%
4	April	2	15%
5	May	1	8%
6	June	0	0%
7	July	1	8%
8	August	0	0%
9	September	2	15%
10	October	1	8%
11	November	3	23%
12	December	2	15%
	Total	13	100%

Collision by Day

Collision by Day	Collision by Day				
	DAY		% OF TOTAL		
1	Sunday	1	8%		
2	Monday	1	8%		
3	Tuesday	2	15%		
4	Wednesday	2	15%		
5	Thursday	2	15%		
6	Friday	2	15%		
7	Saturday	3	23%		
	Total	13	100%		

Collision by Hour

	HOUR	NO. OF COLLISIONS	% OF TOTAL
1	1:00 AM	0	0%
2	2:00 AM	0	0%
3	3:00 AM	0	0%
4	4:00 AM	0	0%
5	5:00 AM	0	0%
6	6:00 AM	0	0%
7	7:00 AM	0	0%
8	8:00 AM	0	0%
9	9:00 AM	1	8%
10	10:00 AM	0	0%
11	11:00 AM	0	0%
12	12:00 PM	1	8%
13	1:00 PM	1	8%
14	2:00 PM	0	0%
15	3:00 PM	2	15%
16	4:00 PM	1	8%
17	5:00 PM	3	23%
18	6:00 PM	2	15%
19	7:00 PM	0	0%
20	8:00 PM	1	8%
21	9:00 PM	1	8%
22	10:00 PM	0	0%
23	11:00 PM	0	0%
24	12:00 AM	0	0%
97	Unknown	0	0%
	Total	13	100%

Complete by Type	COLLISION TYPE	NO. OF COLLISIONS	% OF TOTAL
1	Struck Object	1	8%
2	Off Road Left	0	0%
3	Right Angle	1	8%
4	Passing - Left Turn	0	0%
5	Left Turn - Across Path	1	8%
6	Sideswipe	1	8%
7	Other	1	8%
8	Rear End	5	38%
9	Off Road Right	0	0%
10	Head On	0	0%
11	Passing Right Turn	0	0%
12	Sideswipe - Same Direction	2	15%
13	Backing	0	0%
97	Unknown	1	8%
	Total	13	100%

#### 50 STREET & 46 AVENUE COLLISION ANALYSIS

Collision by Cause

	COLLISION CAUSE	NO. OF COLLISIONS	% OF TOTAL
1	Driving Properly	1	8%
2	Stop Sign Violation	1	8%
3	Yield Sign Violation	0	0%
4	Fail to Yield Right-of-Way, Uncontrolled Intersection	0	0%
5	Fail to Yield Right-of-Way, Pedestrian	0	0%
6	Followed Too Closely	2	15%
7	Parked Vehicle	0	0%
8	Backed Unsafely	0	0%
9	Left Turn Across Path	1	8%
10	Improper Lane Change	1	8%
11	Disobey Traffic Signal	0	0%
12	Ran off Road	0	0%
13	Improper Turn	1	8%
14	Left of Centre	0	0%
15	Improper Passing	0	0%
97	Blank	3	23%
98	Other	1	8%
99	Unknown	2	15%
	Total	13	100%

Collision by Severity

comoion by coroning			
	COLLISION SEVERITY		% OF TOTAL
1	Fatal	0	0%
2	Injury	0	0%
3	Property Damage	13	100%
97	Unknown	0	0%
	Total		100%

l l	ENVIRONMENTAL CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Clear	8	62%
2	Raining	0	0%
3	Hail/Sleet	0	0%
4	Snow	3	23%
5	Fog/Smog/Smoke/Dust	0	0%
6	High Wind	0	0%
97	Unknown	2	15%
98	Other	0	0%
99	Unknown	0	0%
Total		13	100%

Collision by Surface Condition

	SURFACE CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Dry	6	46%
2	Wet	1	8%
3	Slush/Snow/Ice	5	38%
4	Loose Surface Material	0	0%
5	Muddy	0	0%
97	Unknown	1	8%
98	Other	0	0%
99	Unknown	0	0%
	Total	13	100%

	LIGHT CONDITION		% OF TOTAL
1	Daylight	8	62%
2	Sunglare	0	0%
3	Darkness	4	31%
97	Unknown	1	8%
99	Unknown	0	0%
	Total		100%

#### **HWY 28 & 50 STREET COLLISION ANALYSIS**

Collision by Year

YEAR	NO. OF COLLISIONS	% OF TOTAL
2005	5	38%
2006	2	15%
2007	2	15%
2008	4	31%
2009	0	0%
Total	13	100%

Collision by Month

	MONTH	NO. OF COLLISIONS	% OF TOTAL
1	January	2	15%
2	February	2	15%
3	March	3	23%
4	April	2	15%
5	May	0	0%
6	June	0	0%
7	July	0	0%
8	August	1	8%
9	September	0	0%
10	October	0	0%
11	November	2	15%
12	December	1	8%
Total		13	100%

Collision by Day

	DAY	NO. OF COLLISIONS	% OF TOTAL
1	Sunday	1	8%
2	Monday	2	15%
3	Tuesday	3	23%
4	Wednesday	2	15%
5	Thursday	1	8%
6	Friday	3	23%
7	Saturday	1	8%
Total		13	100%

Collision by Hour

	HOUR	NO. OF COLLISIONS	% OF TOTAL
1	1:00 AM	0	0%
2	2:00 AM	0	0%
3	3:00 AM	0	0%
4	4:00 AM	0	0%
5	5:00 AM	0	0%
6	6:00 AM	1	8%
7	7:00 AM	1	8%
8	8:00 AM	2	15%
9	9:00 AM	0	0%
10	10:00 AM	1	8%
11	11:00 AM	2	15%
12	12:00 PM	1	8%
13	1:00 PM	1	8%
14	2:00 PM	0	0%
15	3:00 PM	1	8%
16	4:00 PM	1	8%
17	5:00 PM	1	8%
18	6:00 PM	0	0%
19	7:00 PM	0	0%
20	8:00 PM	0	0%
21	9:00 PM	0	0%
22	10:00 PM	1	8%
23	11:00 PM	0	0%
24	12:00 AM	0	0%
97	Unknown	0	0%
•	Total	13	100%

	COLLISION TYPE	NO. OF COLLISIONS	% OF TOTAL
1	Struck Object	3	23%
2	Off Road Left	1	8%
3	Right Angle	2	15%
4	Passing - Left Turn	0	0%
5	Left Turn - Across Path	0	0%
6	Sideswipe	0	0%
7	Other	0	0%
8	Rear End	6	46%
9	Off Road Right	0	0%
10	Head On	0	0%
11	Passing Right Turn	0	0%
12	Sideswipe - Same Direction	1	8%
13	Backing	0	0%
97	Unknown	0	0%
	Total	13	100%

#### **HWY 28 & 50 STREET COLLISION ANALYSIS**

Collision by Cause

	COLLISION CAUSE	NO. OF COLLISIONS	% OF TOTAL
1	Driving Properly	2	15%
2	Stop Sign Violation	1	8%
3	Yield Sign Violation	0	0%
4	Fail to Yield Right-of-Way, Uncontrolled Intersection	0	0%
5	Fail to Yield Right-of-Way, Pedestrian	0	0%
6	Followed Too Closely	1	8%
7	Parked Vehicle	0	0%
8	Backed Unsafely	0	0%
9	Left Turn Across Path	0	0%
10	Improper Lane Change	1	8%
11	Disobey Traffic Signal	0	0%
12	Ran off Road	0	0%
13	Improper Turn	0	0%
14	Left of Centre	0	0%
15	Improper Passing	0	0%
97	Blank	8	62%
98	Other	0	0%
99	Unknown	0	0%
	Total	13	100%

Collision by Severity

comoion by coroni,			
	COLLISION SEVERITY	NO. OF COLLISIONS	% OF TOTAL
1	Fatal	0	0%
2	Injury	2	15%
3	Property Damage	11	85%
97	Unknown	0	0%
The state of the s	Total	13	100%

Collision by Weather Conditio

	ENVIRONMENTAL CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Clear	8	62%
2	Raining	0	0%
3	Hail/Sleet	0	0%
4	Snow	3	23%
5	Fog/Smog/Smoke/Dust	0	0%
6	High Wind	0	0%
97	Unknown	2	15%
98	Other	0	0%
99	Unknown	0	0%
	Total	13	100%

Collision by Surface Condition

	SURFACE CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Dry	5	38%
2	Wet	0	0%
3	Slush/Snow/Ice	6	46%
4	Loose Surface Material	0	0%
5	Muddy	0	0%
97	Unknown	2	15%
98	Other	0	0%
99	Unknown	0	0%
	Total	13	100%

Collision by Lig	iht Condition - Natural Light		
	LIGHT CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Daylight	9	69%
2	Sunglare	0	0%
3	Darkness	1	8%
97	Unknown	3	23%
99	Unknown	0	0%
	Total	13	100%

#### 51 STREET & 50 AVENUE COLLISION ANALYSIS

Collision by Year

YEAR	NO. OF COLLISIONS	% OF TOTAL
2005	1	8%
2006	4	31%
2007	2	15%
2008	5	38%
2009	1	8%
Total	13	100%

Collision by Month

	MONTH	NO. OF COLLISIONS	% OF TOTAL
1	January	0	0%
2	February	1	8%
3	March	2	15%
4	April	0	0%
5	May	2	15%
6	June	0	0%
7	July	1	8%
8	August	2	15%
9	September	1	8%
10	October	3	23%
11	November	0	0%
12	December	1	8%
	Total	13	100%

Collision by Day

oomsion by Bay			
	DAY	NO. OF COLLISIONS	% OF TOTAL
1	Sunday	1	8%
2	Monday	2	15%
3	Tuesday	2	15%
4	Wednesday	2	15%
5	Thursday	2	15%
6	Friday	3	23%
7	Saturday	1	8%
	Total	13	100%

Collision by Hour

	HOUR	NO. OF COLLISIONS	% OF TOTAL
1	1:00 AM	0	0%
2	2:00 AM	0	0%
3	3:00 AM	0	0%
4	4:00 AM	0	0%
5	5:00 AM	0	0%
6	6:00 AM	0	0%
7	7:00 AM	0	0%
8	8:00 AM	0	0%
9	9:00 AM	0	0%
10	10:00 AM	0	0%
11	11:00 AM	1	8%
12	12:00 PM	1	8%
13	1:00 PM	0	0%
14	2:00 PM	3	23%
15	3:00 PM	1	8%
16	4:00 PM	4	31%
17	5:00 PM	1	8%
18	6:00 PM	1	8%
19	7:00 PM	1	8%
20	8:00 PM	0	0%
21	9:00 PM	0	0%
22	10:00 PM	0	0%
23	11:00 PM	0	0%
24	12:00 AM	0	0%
97	Unknown	0	0%
	Total	13	100%

COMMON BY TYPE	COLLISION TYPE	NO. OF COLLISIONS	% OF TOTAL
1	Struck Object	1	8%
2	Off Road Left	0	0%
3	Right Angle	0	0%
4	Passing - Left Turn	0	0%
5	Left Turn - Across Path	1	8%
6	Sideswipe	0	0%
7	Other	1	8%
8	Rear End	0	0%
9	Off Road Right	0	0%
10	Head On	0	0%
11	Passing Right Turn	0	0%
12	Sideswipe - Same Direction	1	8%
13	Backing	8	62%
97	Unknown	1	8%
	Total	13	100%

#### 51 STREET & 50 AVENUE COLLISION ANALYSIS

Collision by Cause

_	COLLISION CAUSE	NO. OF COLLISIONS	% OF TOTAL
1	Driving Properly	0	0%
2	Stop Sign Violation	0	0%
3	Yield Sign Violation	0	0%
4	Fail to Yield Right-of-Way, Uncontrolled Intersection	0	0%
5	Fail to Yield Right-of-Way, Pedestrian	1	8%
6	Followed Too Closely	1	8%
7	Parked Vehicle	0	0%
8	Backed Unsafely	7	54%
9	Left Turn Across Path	0	0%
10	Improper Lane Change	0	0%
11	Disobey Traffic Signal	0	0%
12	Ran off Road	0	0%
13	Improper Turn	1	8%
14	Left of Centre	0	0%
15	Improper Passing	0	0%
97	Blank	0	0%
98	Other	0	0%
99	Unknown	3	23%
	Total	13	100%

Collision by Severity

Combien by CC	city		
	COLLISION SEVERITY	NO. OF COLLISIONS	% OF TOTAL
1	Fatal	0	0%
2	Injury	0	0%
3	Property Damage	13	100%
97	Unknown	0	0%
	Total	13	100%

Comsion by We	ather Condition	NO OF COLUMNIA	0/ OF TOTAL
	ENVIRONMENTAL CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Clear	13	100%
2	Raining	0	0%
3	Hail/Sleet	0	0%
4	Snow	0	0%
5	Fog/Smog/Smoke/Dust	0	0%
6	High Wind	0	0%
97	Unknown	0	0%
98	Other	0	0%
99	Unknown	0	0%
Total 13 10			

Collision by Surface Condition

	SURFACE CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Dry	11	85%
2	Wet	0	0%
3	Slush/Snow/Ice	2	15%
4	Loose Surface Material	0	0%
5	Muddy	0	0%
97	Unknown	0	0%
98	Other	0	0%
99	Unknown	0	0%
	Total	13	100%

Collision by Lig	ht Condition - Natural Light		
	LIGHT CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Daylight	10	77%
2	Sunglare	0	0%
3	Darkness	2	15%
97	Unknown	1	8%
99	Unknown	0	0%
	Total	13	100%

#### 50 STREET & 43 AVENUE COLLISION ANALYSIS

Complete by Tea		
YEAR	NO. OF COLLISIONS	% OF TOTAL
2005	3	27%
2006	2	18%
2007	2	18%
2008	3	27%
2009	1	9%
Total	11	100%

Collision by Month

	MONTH	NO. OF COLLISIONS	% OF TOTAL
1	January	0	0%
2	February	2	18%
3	March	3	27%
4	April	1	9%
5	May	1	9%
6	June	0	0%
7	July	2	18%
8	August	1	9%
9	September	0	0%
10	October	0	0%
11	November	1	9%
12	December	0	0%
	Total	11	100%

Collision by Day

Collision by Day	DAY	NO. OF COLLISIONS	% OF TOTAL
1	Sunday	3	27%
2	Monday	0	0%
3	Tuesday	1	9%
4	Wednesday	1	9%
5	Thursday	2	18%
6	Friday	2	18%
7	Saturday	2	18%
	Total	11	100%

Collision by Hour

	HOUR	NO. OF COLLISIONS	% OF TOTAL
1	1:00 AM	0	0%
2	2:00 AM	0	0%
3	3:00 AM	0	0%
4	4:00 AM	0	0%
5	5:00 AM	0	0%
6	6:00 AM	1	9%
7	7:00 AM	0	0%
8	8:00 AM	0	0%
9	9:00 AM	1	9%
10	10:00 AM	0	0%
11	11:00 AM	0	0%
12	12:00 PM	0	0%
13	1:00 PM	1	9%
14	2:00 PM	0	0%
15	3:00 PM	3	27%
16	4:00 PM	2	18%
17	5:00 PM	1	9%
18	6:00 PM	0	0%
19	7:00 PM	0	0%
20	8:00 PM	0	0%
21	9:00 PM	0	0%
22	10:00 PM	1	9%
23	11:00 PM	0	0%
24	12:00 AM	0	0%
97	Unknown	1	9%
	Total	11	100%

	COLLISION TYPE	NO. OF COLLISIONS	% OF TOTAL
1	Struck Object	2	18%
2	Off Road Left	0	0%
3	Right Angle	4	36%
4	Passing - Left Turn	0	0%
5	Left Turn - Across Path	2	18%
6	Sideswipe	1	9%
7	Other	0	0%
8	Rear End	0	0%
9	Off Road Right	0	0%
10	Head On	0	0%
11	Passing Right Turn	0	0%
12	Sideswipe - Same Direction	1	9%
13	Backing	1	9%
97	Unknown	0	0%
	Total	11	100%

#### 50 STREET & 43 AVENUE COLLISION ANALYSIS

Collision by Cause

	COLLISION CAUSE	NO. OF COLLISIONS	% OF TOTAL
1	Driving Properly	1	9%
2	Stop Sign Violation	2	18%
3	Yield Sign Violation	0	0%
4	Fail to Yield Right-of-Way, Uncontrolled Intersection	1	9%
5	Fail to Yield Right-of-Way, Pedestrian	0	0%
6	Followed Too Closely	0	0%
7	Parked Vehicle	0	0%
8	Backed Unsafely	1	9%
9	Left Turn Across Path	1	9%
10	Improper Lane Change	0	0%
11	Disobey Traffic Signal	0	0%
12	Ran off Road	0	0%
13	Improper Turn	0	0%
14	Left of Centre	0	0%
15	Improper Passing	0	0%
97	Blank	3	27%
98	Other	1	9%
99	Unknown	1	9%
	Total	11	100%

Collision by Severity

Collision by Severity			
	COLLISION SEVERITY	NO. OF COLLISIONS	% OF TOTAL
1	Fatal	0	0%
2	Injury	2	18%
3	Property Damage	9	82%
97	Unknown	0	0%
	Total	11	100%

Collision by Weather Condition

	ENVIRONMENTAL CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Clear	6	55%
2	Raining	2	18%
3	Hail/Sleet	0	0%
4	Snow	2	18%
5	Fog/Smog/Smoke/Dust	0	0%
6	High Wind	0	0%
97	Unknown	0	0%
98	Other	0	0%
99	Unknown	1	9%
	Total	11	100%

Collision by Surface Condition

	SURFACE CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Dry	5	45%
2	Wet	2	18%
3	Slush/Snow/Ice	3	27%
4	Loose Surface Material	0	0%
5	Muddy	0	0%
97	Unknown	0	0%
98	Other	0	0%
99	Unknown	1	9%
	Total	11	100%

Collision by Light Condition - Natural Light				
	LIGHT CONDITION	NO. OF COLLISIONS	% OF TOTAL	
1	Daylight	9	82%	
2	Sunglare	0	0%	
3	Darkness	2	18%	
97	Unknown	0	0%	
99	Unknown	0	0%	
	Total	11	100%	

#### 52 STREET & 50 AVENUE COLLISION ANALYSIS

YEAR	NO. OF COLLISIONS	% OF TOTAL
2005	4	36%
2006	2	18%
2007	2	18%
2008	1	9%
2009	2	18%
Total	11	100%

Collision by Month

	MONTH	NO. OF COLLISIONS	% OF TOTAL
1	January	1	9%
2	February	1	9%
3	March	1	9%
4	April	1	9%
5	May	3	27%
6	June	1	9%
7	July	2	18%
8	August	0	0%
9	September	0	0%
10	October	0	0%
11	November	1	9%
12	December	0	0%
	Total	11	100%

	DAY	NO. OF COLLISIONS	% OF TOTAL
1	Sunday	0	0%
2	Monday	4	36%
3	Tuesday	3	27%
4	Wednesday	1	9%
5	Thursday	0	0%
6	Friday	3	27%
7	Saturday	0	0%
	Total	11	100%

Collision by Hour

HOUR		NO. OF COLLISIONS	% OF TOTAL
1	1:00 AM	0	0%
2	2:00 AM	0	0%
3	3:00 AM	0	0%
4	4:00 AM	0	0%
5	5:00 AM	0	0%
6	6:00 AM	0	0%
7	7:00 AM	0	0%
8	8:00 AM	0	0%
9	9:00 AM	1	9%
10	10:00 AM	1	9%
11	11:00 AM	1	9%
12	12:00 PM	2	18%
13	1:00 PM	2	18%
14	2:00 PM	1	9%
15	3:00 PM	1	9%
16	4:00 PM	0	0%
17	5:00 PM	0	0%
18	6:00 PM	0	0%
19	7:00 PM	0	0%
20	8:00 PM	1	9%
21	9:00 PM	0	0%
22	10:00 PM	0	0%
23	11:00 PM	0	0%
24	12:00 AM	1	9%
97	Unknown	0	0%
	Total	11	100%

	COLLISION TYPE	NO. OF COLLISIONS	% OF TOTAL
1	Struck Object	0	0%
2	Off Road Left	0	0%
3	Right Angle	1	9%
4	Passing - Left Turn	0	0%
5	Left Turn - Across Path	0	0%
6	Sideswipe	1	9%
7	Other	0	0%
8	Rear End	3	27%
9	Off Road Right	0	0%
10	Head On	0	0%
11	Passing Right Turn	0	0%
12	Sideswipe - Same Direction	0	0%
13	Backing	6	55%
97	Unknown	0	0%
	Total	11	100%

#### 52 STREET & 50 AVENUE COLLISION ANALYSIS

	Cause

	COLLISION CAUSE	NO. OF COLLISIONS	% OF TOTAL
1	Driving Properly	0	0%
2	Stop Sign Violation	0	0%
3	Yield Sign Violation	0	0%
4	Fail to Yield Right-of-Way, Uncontrolled Intersection	0	0%
5	Fail to Yield Right-of-Way, Pedestrian	0	0%
6	Followed Too Closely	2	18%
7	Parked Vehicle	0	0%
8	Backed Unsafely	5	45%
9	Left Turn Across Path	0	0%
10	Improper Lane Change	0	0%
11	Disobey Traffic Signal	0	0%
12	Ran off Road	0	0%
13	Improper Turn	0	0%
14	Left of Centre	0	0%
15	Improper Passing	0	0%
97	Blank	2	18%
98	Other	0	0%
99	Unknown	2	18%
	Total	11	100%

Collision by Severity

Collision by Seventy			
	COLLISION SEVERITY	NO. OF COLLISIONS	% OF TOTAL
1	Fatal	0	0%
2	Injury	2	18%
3	Property Damage	9	82%
97	Unknown	0	0%
Total		11	100%

Collision by Weather Condition

	ENVIRONMENTAL CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Clear	9	82%
2	Raining	0	0%
3	Hail/Sleet	0	0%
4	Snow	1	9%
5	Fog/Smog/Smoke/Dust	0	0%
6	High Wind	0	0%
97	Unknown	1	9%
98	Other	0	0%
99	Unknown	0	0%
	Total	11	100%

Collision by Surface Condition

	SURFACE CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Dry	7	64%
2	Wet	0	0%
3	Slush/Snow/Ice	3	27%
4	Loose Surface Material	0	0%
5	Muddy	0	0%
97	Unknown	1	9%
98	Other	0	0%
99	Unknown	0	0%
	Total	11	100%

	LIGHT CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Daylight	9	82%
2	Sunglare	0	0%
3	Darkness	1	9%
97	Unknown	1	9%
99	Unknown	0	0%
Total		11	100%

#### 49 STREET & 51 AVENUE COLLISION ANALYSIS

-	lision	hw	Vaar

YEAR	NO. OF COLLISIONS	% OF TOTAL
2005	2	20%
2006	1	10%
2007	2	20%
2008	0	0%
2009	5	50%
Total	10	100%

Collision by Month

	MONTH	NO. OF COLLISIONS	% OF TOTAL
1	January	1	10%
2	February	1	10%
3	March	1	10%
4	April	0	0%
5	May	0	0%
6	June	1	10%
7	July	0	0%
8	August	0	0%
9	September	0	0%
10	October	1	10%
11	November	1	10%
12	December	4	40%
Total		10	100%

Collision by Day

Complete by Eay	DAY	NO. OF COLLISIONS	% OF TOTAL
1	Sunday	2	20%
2	Monday	2	20%
3	Tuesday	0	0%
4	Wednesday	1	10%
5	Thursday	1	10%
6	Friday	2	20%
7	Saturday	2	20%
	Total	10	100%

Collision by Hour

	HOUR	NO. OF COLLISIONS	% OF TOTAL
1	1:00 AM	0	0%
2	2:00 AM	1	10%
3	3:00 AM	0	0%
4	4:00 AM	0	0%
5	5:00 AM	0	0%
6	6:00 AM	0	0%
7	7:00 AM	0	0%
8	8:00 AM	0	0%
9	9:00 AM	1	10%
10	10:00 AM	1	10%
11	11:00 AM	0	0%
12	12:00 PM	2	20%
13	1:00 PM	0	0%
14	2:00 PM	1	10%
15	3:00 PM	0	0%
16	4:00 PM	1	10%
17	5:00 PM	3	30%
18	6:00 PM	0	0%
19	7:00 PM	0	0%
20	8:00 PM	0	0%
21	9:00 PM	0	0%
22	10:00 PM	0	0%
23	11:00 PM	0	0%
24	12:00 AM	0	0%
97	Unknown	0	0%
	Total	10	100%

	COLLISION TYPE	NO. OF COLLISIONS	% OF TOTAL
1	Struck Object	3	30%
2	Off Road Left	0	0%
3	Right Angle	3	30%
4	Passing - Left Turn	0	0%
5	Left Turn - Across Path	0	0%
6	Sideswipe	0	0%
7	Other	0	0%
8	Rear End	2	20%
9	Off Road Right	0	0%
10	Head On	0	0%
11	Passing Right Turn	1	10%
12	Sideswipe - Same Direction	0	0%
13	Backing	1	10%
97	Unknown	0	0%
	Total	10	100%

#### 49 STREET & 51 AVENUE COLLISION ANALYSIS

	COLLISION CAUSE	NO. OF COLLISIONS	% OF TOTAL
1	Driving Properly	2	20%
2	Stop Sign Violation	0	0%
3	Yield Sign Violation	1	10%
4	Fail to Yield Right-of-Way, Uncontrolled Intersection	0	0%
5	Fail to Yield Right-of-Way, Pedestrian	0	0%
6	Followed Too Closely	2	20%
7	Parked Vehicle	1	10%
8	Backed Unsafely	1	10%
9	Left Turn Across Path	0	0%
10	Improper Lane Change	0	0%
11	Disobey Traffic Signal	0	0%
12	Ran off Road	1	10%
13	Improper Turn	0	0%
14	Left of Centre	0	0%
15	Improper Passing	0	0%
97	Blank	1	10%
98	Other	0	0%
99	Unknown	1	10%
	Total	10	100%

Collision by Severity

Collision by Seventy			
	COLLISION SEVERITY	NO. OF COLLISIONS	% OF TOTAL
1	Fatal	0	0%
2	Injury	2	20%
3	Property Damage	8	80%
97	Unknown	0	0%
	Total	10	100%

Collision by Weather Condition

	ENVIRONMENTAL CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Clear	7	70%
2	Raining	0	0%
3	Hail/Sleet	0	0%
4	Snow	2	20%
5	Fog/Smog/Smoke/Dust	0	0%
6	High Wind	0	0%
97	Unknown	1	10%
98	Other	0	0%
99	Unknown	0	0%
	Total	10	100%

Collision by Surface Condition

	SURFACE CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Dry	1	10%
2	Wet	0	0%
3	Slush/Snow/Ice	8	80%
4	Loose Surface Material	0	0%
5	Muddy	0	0%
97	Unknown	1	10%
98	Other	0	0%
99	Unknown	0	0%
	Total	10	100%

Collision by Lig	ht Condition - Natural Light		
LIGHT CONDITION		NO. OF COLLISIONS	% OF TOTAL
1	Daylight	7	70%
2	Sunglare	0	0%
3	Darkness	2	20%
97	Unknown	1	10%
99	Unknown	0	0%
	Total	10	100%

#### **HWY 28 & 43 AVENUE COLLISION ANALYSIS**

YEAR	NO. OF COLLISIONS	% OF TOTAL
2005	1	10%
2006	5	50%
2007	0	0%
2008	3	30%
2009	1	10%
Total	10	100%

Collision by Month

	MONTH	NO. OF COLLISIONS	% OF TOTAL
1	January	0	0%
2	February	1	10%
3	March	1	10%
4	April	0	0%
5	May	0	0%
6	June	1	10%
7	July	0	0%
8	August	0	0%
9	September	0	0%
10	October	1	10%
11	November	5	50%
12	December	1	10%
	Total	10	100%

Collision by Day

Comision by Buy	DAY	NO. OF COLLISIONS	% OF TOTAL
1	Sunday	1	10%
2	Monday	1	10%
3	Tuesday	0	0%
4	Wednesday	2	20%
5	Thursday	3	30%
6	Friday	2	20%
7	Saturday	1	10%
	Total	10	100%

Collision by Hour

	HOUR	NO. OF COLLISIONS	% OF TOTAL
1	1:00 AM	0	0%
2	2:00 AM	0	0%
3	3:00 AM	0	0%
4	4:00 AM	0	0%
5	5:00 AM	0	0%
6	6:00 AM	0	0%
7	7:00 AM	0	0%
8	8:00 AM	1	10%
9	9:00 AM	1	10%
10	10:00 AM	0	0%
11	11:00 AM	0	0%
12	12:00 PM	1	10%
13	1:00 PM	2	20%
14	2:00 PM	1	10%
15	3:00 PM	1	10%
16	4:00 PM	0	0%
17	5:00 PM	1	10%
18	6:00 PM	2	20%
19	7:00 PM	0	0%
20	8:00 PM	0	0%
21	9:00 PM	0	0%
22	10:00 PM	0	0%
23	11:00 PM	0	0%
24	12:00 AM	0	0%
97	Unknown	0	0%
	Total	10	100%

	COLLISION TYPE	NO. OF COLLISIONS	% OF TOTAL
1	Struck Object	2	20%
2	Off Road Left	0	0%
3	Right Angle	1	10%
4	Passing - Left Turn	0	0%
5	Left Turn - Across Path	1	10%
6	Sideswipe	0	0%
7	Other	0	0%
8	Rear End	6	60%
9	Off Road Right	0	0%
10	Head On	0	0%
11	Passing Right Turn	0	0%
12	Sideswipe - Same Direction	0	0%
13	Backing	0	0%
97	Unknown	0	0%
	Total	10	100%

#### **HWY 28 & 43 AVENUE COLLISION ANALYSIS**

	COLLISION CAUSE	NO. OF COLLISIONS	% OF TOTAL
1	Driving Properly	1	10%
2	Stop Sign Violation	0	0%
3	Yield Sign Violation	0	0%
4	Fail to Yield Right-of-Way, Uncontrolled Intersection	0	0%
5	Fail to Yield Right-of-Way, Pedestrian	0	0%
6	Followed Too Closely	3	30%
7	Parked Vehicle	0	0%
8	Backed Unsafely	0	0%
9	Left Turn Across Path	1	10%
10	Improper Lane Change	0	0%
11	Disobey Traffic Signal	1	10%
12	Ran off Road	0	0%
13	Improper Turn	0	0%
14	Left of Centre	0	0%
15	Improper Passing	0	0%
97	Blank	2	20%
98	Other	1	10%
99	Unknown	1	10%
	Total	10	100%

Collision by Severity

Collision by Sev	Collision by Severity			
	COLLISION SEVERITY	NO. OF COLLISIONS	% OF TOTAL	
1	Fatal	0	0%	
2	Injury	2	20%	
3	Property Damage	8	80%	
97	Unknown	0	0%	
	Total	10	100%	

Collision by Weather Condition

	ENVIRONMENTAL CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Clear	6	60%
2	Raining	1	10%
3	Hail/Sleet	0	0%
4	Snow	2	20%
5	Fog/Smog/Smoke/Dust	0	0%
6	High Wind	0	0%
97	Unknown	1	10%
98	Other	0	0%
99	Unknown	0	0%
	Total	10	100%

Collision by Surface Condition

	SURFACE CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Dry	3	30%
2	Wet	1	10%
3	Slush/Snow/Ice	5	50%
4	Loose Surface Material	0	0%
5	Muddy	0	0%
97	Unknown	1	10%
98	Other	0	0%
99	Unknown	0	0%
	Total	10	100%

	LIGHT CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Daylight	6	60%
2	Sunglare	1	10%
3	Darkness	2	20%
97	Unknown	1	10%
99	Unknown	0	0%
	Total	10	100%

#### **HWY 28 & 55 AVENUE COLLISION ANALYSIS**

Collision by Year

YEAR	NO. OF COLLISIONS	% OF TOTAL
2005	1	10%
2006	2	20%
2007	3	30%
2008	3	30%
2009	1	10%
Total	10	100%

Collision by Month

	MONTH	NO. OF COLLISIONS	% OF TOTAL
1	January	1	10%
2	February	1	10%
3	March	0	0%
4	April	1	10%
5	May	0	0%
6	June	1	10%
7	July	0	0%
8	August	0	0%
9	September	1	10%
10	October	0	0%
11	November	5	50%
12	December	0	0%
	Total	10	100%

Collision by Day

Collision by Day			
	DAY	NO. OF COLLISIONS	% OF TOTAL
1	Sunday	0	0%
2	Monday	1	10%
3	Tuesday	2	20%
4	Wednesday	2	20%
5	Thursday	2	20%
6	Friday	2	20%
7	Saturday	1	10%
	Total	10	100%

Collision by Hour

	HOUR	NO. OF COLLISIONS	% OF TOTAL
1	1:00 AM	0	0%
2	2:00 AM	0	0%
3	3:00 AM	0	0%
4	4:00 AM	0	0%
5	5:00 AM	0	0%
6	6:00 AM	0	0%
7	7:00 AM	0	0%
8	8:00 AM	1	10%
9	9:00 AM	0	0%
10	10:00 AM	0	0%
11	11:00 AM	0	0%
12	12:00 PM	1	10%
13	1:00 PM	0	0%
14	2:00 PM	1	10%
15	3:00 PM	2	20%
16	4:00 PM	2	20%
17	5:00 PM	1	10%
18	6:00 PM	2	20%
19	7:00 PM	0	0%
20	8:00 PM	0	0%
21	9:00 PM	0	0%
22	10:00 PM	0	0%
23	11:00 PM	0	0%
24	12:00 AM	0	0%
97	Unknown	0	0%
	Total	10	100%

	COLLISION TYPE	NO. OF COLLISIONS	% OF TOTAL
1	Struck Object	2	20%
2	Off Road Left	0	0%
3	Right Angle	1	10%
4	Passing - Left Turn	0	0%
5	Left Turn - Across Path	2	20%
6	Sideswipe	0	0%
7	Other	0	0%
8	Rear End	4	40%
9	Off Road Right	0	0%
10	Head On	0	0%
11	Passing Right Turn	0	0%
12	Sideswipe - Same Direction	1	10%
13	Backing	0	0%
97	Unknown	0	0%
	Total	10	100%

#### **HWY 28 & 55 AVENUE COLLISION ANALYSIS**

	COLLISION CAUSE	NO. OF COLLISIONS	% OF TOTAL	
1	Driving Properly	0	0%	
2	Stop Sign Violation	0	0%	
3	Yield Sign Violation	0	0%	
4	Fail to Yield Right-of-Way, Uncontrolled Intersection	0	0%	
5	Fail to Yield Right-of-Way, Pedestrian	0	0%	
6	Followed Too Closely	3	30%	
7	Parked Vehicle	0	0%	
8	Backed Unsafely	Backed Unsafely 0		
9	Left Turn Across Path	3	30%	
10	Improper Lane Change	0	0%	
11	Disobey Traffic Signal	0	0%	
12	Ran off Road	0	0%	
13	Improper Turn	0	0%	
14	Left of Centre	0	0%	
15	Improper Passing	0	0%	
97	Blank	2	20%	
98	Other	1	10%	
99	Unknown	1	10%	
	Total		100%	

Collision by Severity

Complete by Co	city		
COLLISION SEVERITY		NO. OF COLLISIONS	% OF TOTAL
1	Fatal	0	0%
2	Injury	3	30%
3	Property Damage	7	70%
97	Unknown	0	0%
Total		10	100%

Collision by Weather Condition

ENVIRONMENTAL CONDITION		NO. OF COLLISIONS	% OF TOTAL
1	Clear	5	50%
2	Raining	3	30%
3	Hail/Sleet	0	0%
4	Snow	0	0%
5	Fog/Smog/Smoke/Dust	0	0%
6	High Wind	0	0%
97	Unknown	1	10%
98	Other	0	0%
99	Unknown	1	10%
•	Total	10	100%
	Total	10	100%

Collision by Surface Condition

	SURFACE CONDITION	NO. OF COLLISIONS	% OF TOTAL
1	Dry	1	10%
2	Wet	4	40%
3	Slush/Snow/Ice	5	50%
4	Loose Surface Material	0	0%
5	Muddy	0	0%
97	Unknown	0	0%
98	Other	0	0%
99	Unknown	0	0%
	Total	10	100%

Comsion by Light Condition - Natural Light				
	LIGHT CONDITION	NO. OF COLLISIONS	% OF TOTAL	
1	Daylight	8	80%	
2	Sunglare	0	0%	
3	Darkness	2	20%	
97	Unknown	0	0%	
99	Unknown	0	0%	

## **FINAL REPORT**



**Appendix C - Cold Lake Transportation Study Traffic Volume Forecast and Analysis** 

# **Technical Memorandum**

# **City of Cold Lake**

**Cold Lake Transportation Study Traffic Volume Forecast and Analysis** 

## **April 2011**



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Appendix A - ASP, ARP and Outline Plan Information

#### City of Cold Lake

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1

## Introduction

The City of Cold Lake (City) is located 287 km northeast of the City of Edmonton and was formed in 1996 by merging three municipalities, namely Grand Centre, Medley (Canadian Forces Base W4) and Cold Lake. Grand Centre was subsequently renamed Cold Lake South (CLS) and the original Cold Lake is now known as Cold Lake North (CLN).

The City has experienced noticeable growth in recent years. According to municipal census the City had a population of 11,991 in 2006 and 13,924 in 2009. This corresponds to a 5.4% linear growth annually. Current transportation improvements within the City have been based on the previous transportation study completed in 2000 (2000 Transportation Study) and is no longer considered representative of the actual transportation network required to address current and future transportation needs.

In light of the continuing accelerated pace of development in the region and the need to rationalize and identify the transportation network requirements for the City, including surrounding rural municipalities and counties, the existing transportation plan requires a comprehensive update.

#### 1.1 STUDY BACKGROUND

Associated Engineering (AE) was retained by the City to update the existing transportation study. The purpose of the transportation study is to provide a comprehensive long-range plan that integrates the transportation infrastructure with requirements of the existing and future land uses. The transportation study will provide the City with a blueprint on which to plan and implement specific transportation network improvement projects over the next 20 years in 5-year, 10-year, 15-year, and 20-year planning horizons.

One component of the transportation study was to forecast the future traffic volumes for the next 20 years. Traffic volumes were forecasted for the 5-year, 10-year, 15-year, and 20-year planning horizons, and analyzed to determine the future road classification and the number of lanes required to accommodate the future traffic volumes. This technical memorandum presents the methodology used and the results from the traffic volume forecast and analysis.

#### 1.2 STUDY AREA

The traffic volume forecast will encompass the area bounded by the current city limits. Figure 1.1 presents the study area.



#### 1.3 STUDY OBJECTIVE

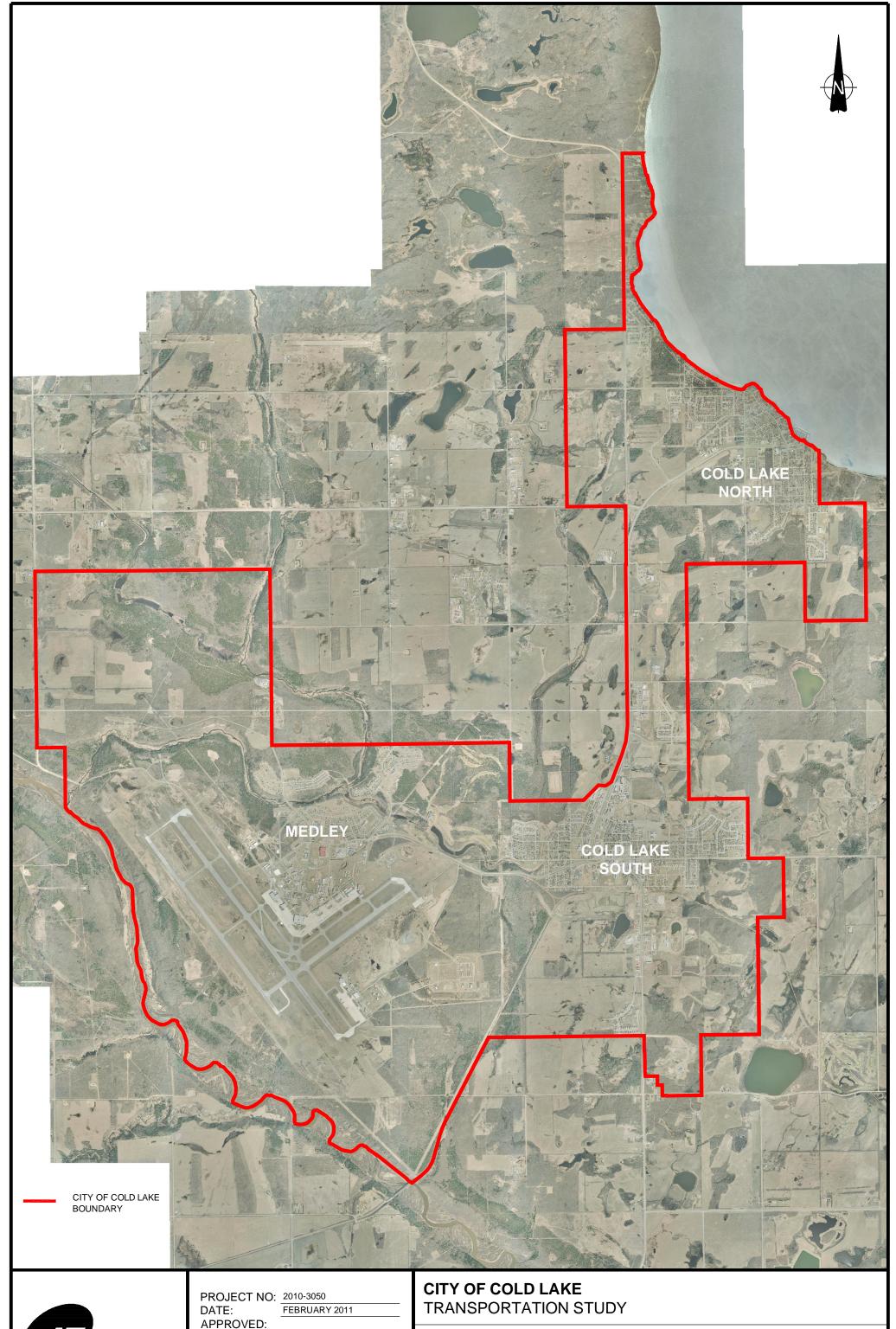
The objectives for the traffic volume forecast and analysis were:

- Forecast the future traffic volumes over the next 20 years, in 5-year planning horizons
- Establish the required road classification for the City's major road network over the next 20 years
- Determine the number of lanes required for the City's major road network over the next 20 years.

For the purpose of the study, the afternoon (p.m.) peak hour traffic volume forecast was considered to be the most critical and was selected for the analysis. The following planning horizons were analyzed:

- 5-year (2015) horizon
- 10-year (2020) horizon
- 15-year (2025) horizon
- 20-year (2030) horizon.

The road classification and number of lanes required to accommodate traffic under the 20-year (2030) horizon will be used by the City to preserve the right-of-way required to accommodate future roadway expansion.





APPROVED: SCALE: DWG NO:

NTS

FIGURE 1.1 TRAFFIC FORECAST STUDY AREA 2

## **Traffic Forecast: Methodology**

A spreadsheet model, following the four-step planning process, was used to forecast the future traffic volumes. To complete the spreadsheet model, a skeletal road network was developed for each planning horizon to represent the anticipated road network. The major road network (collectors and arterials) identified for each planning horizon in the 2000 Transportation Study were used to represent the skeletal road network for the respective planning horizon. Aside from alignment changes to English Bay Road/28 Street/25 Street in the northwest quadrant and the classification of Centre Avenue, between 57 Street and Highway 28, as a four-lane arterial, the major road networks presented in the 2000 Transportation Study were considered to be valid. Changes were made to the 5-year and 10-year road networks to reflect the current alignment of English Bay Road/28 Street/25 Street.

Future traffic within the City will be comprised of background traffic and development traffic. Background traffic represents the growth in existing traffic reflecting the additional trips generated in the surrounding areas and in the City's existing subdivisions. Future background traffic volumes for each planning horizon were estimated by applying an annual growth rate of 2% to the existing (2010) traffic volumes, over a 5-year, 10-year, 15-year, and 20-year period.

Development traffic represents traffic generated by new subdivisions or area redevelopment. The information about future development or redevelopment within the City was obtained from the City's Area Structure Plans (ASP), Area Redevelopment Plans (ARP) and Outline Pans, and from the Municipal District (MD) of Bonnyville's Intermunicipal Development Plan (IDP). Future development traffic volumes for each planning horizon were estimated using a four-step process, which involved:

- Trip Generation: Estimate the number of trips generated from and attracted to each development/redevelopment
- Trip Distribution: Estimate the origin and destination of trips to and from each development/redevelopment
- Modal Split: Not within the scope of the study
- **Trip Assignment**: Select the routes to and from the developments/redevelopments and assign the development traffic volumes to the City's road network.



3

## **Traffic Forecast: Background Traffic Volumes**

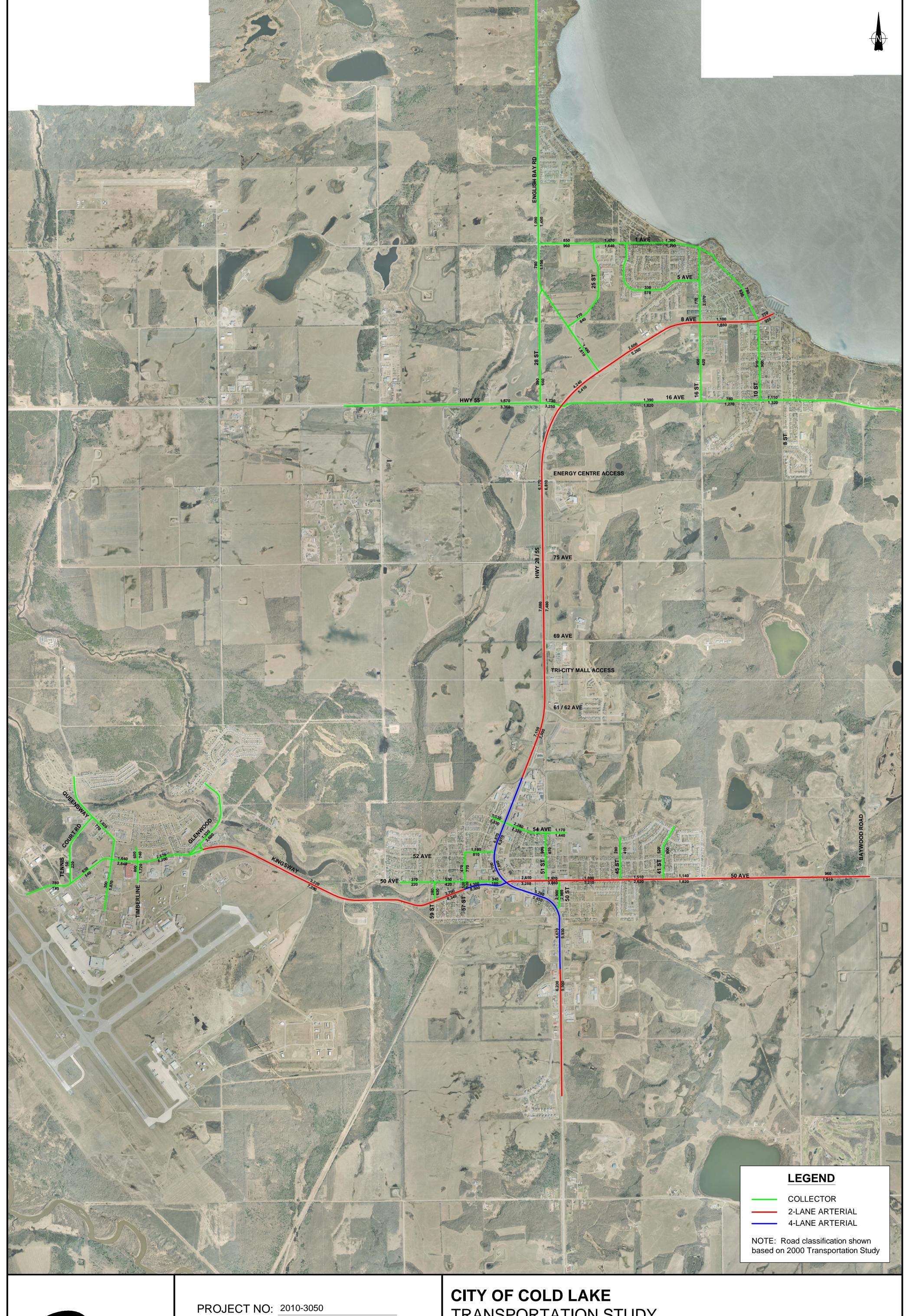
Growth rate calculations were completed as part of the existing condition analysis for the transportation study update and an annual, non-compounding, growth rate of 2.0% was established. The growth rate was selected after discussions with the City. The detailed growth rate calculations can be referenced in the technical memorandum titled Existing (2010) Traffic Operational Analysis.

#### 3.1 FORECASTED BACKGROUND TRAFFIC VOLUMES

Background traffic volumes for the planning horizons were estimated by applying the 2.0% annual growth rate to the existing (2010) traffic volumes, over a 5-year, 10-year, 15-year and 20-year period. The existing (2010) daily traffic volumes were obtained from the Existing (2010) Traffic Operational Analysis technical memorandum and presented in Figure 3.1.

Figure 3.2 through Figure 3.5 present the forecasted daily background traffic volumes for the 5-year, 10-year, 15-year and 20-year horizons.





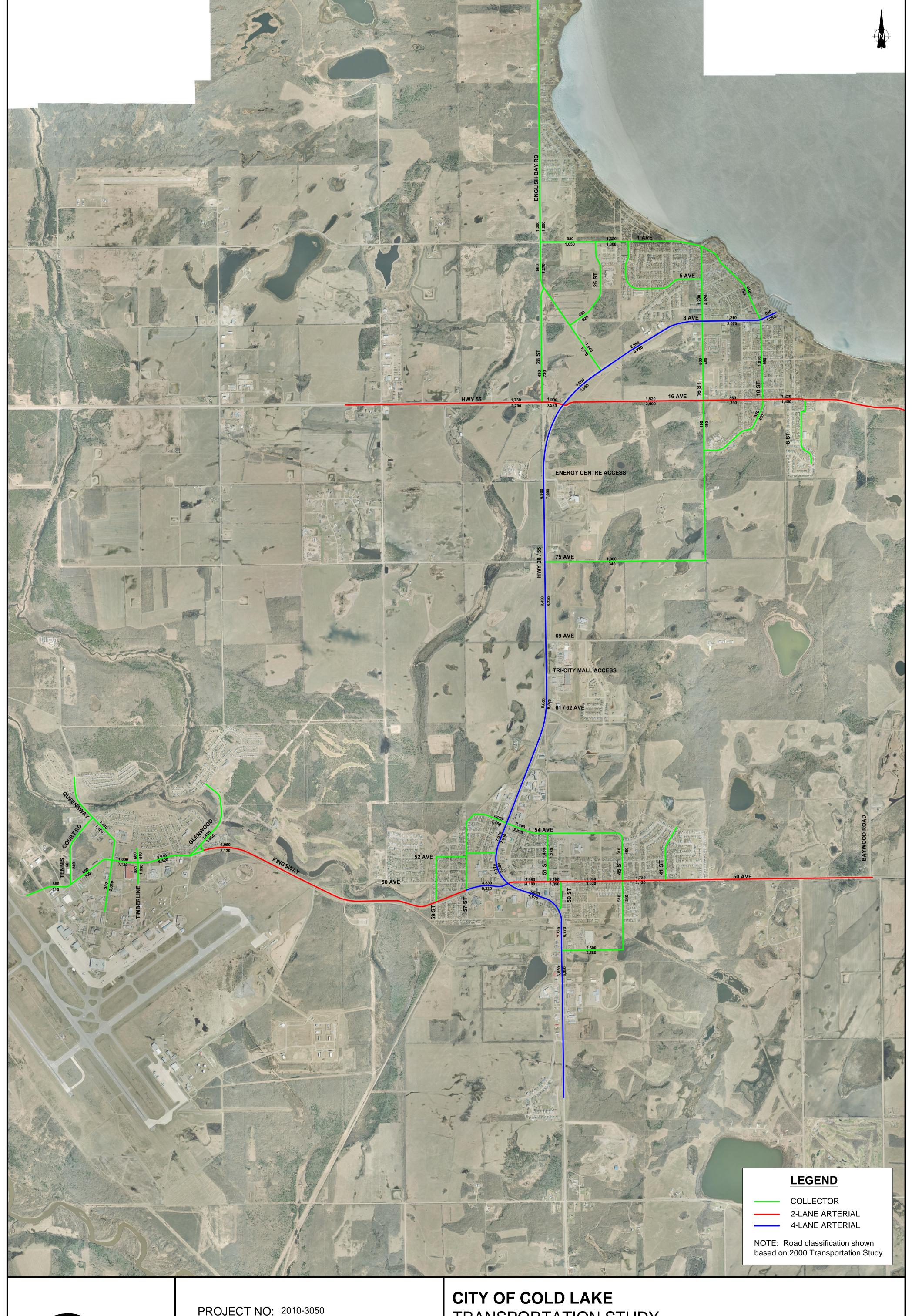


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TRANSPORTATION STUDY

FIGURE 3.1 EXISTING (2010) DAILY TRAFFIC VOLUMES



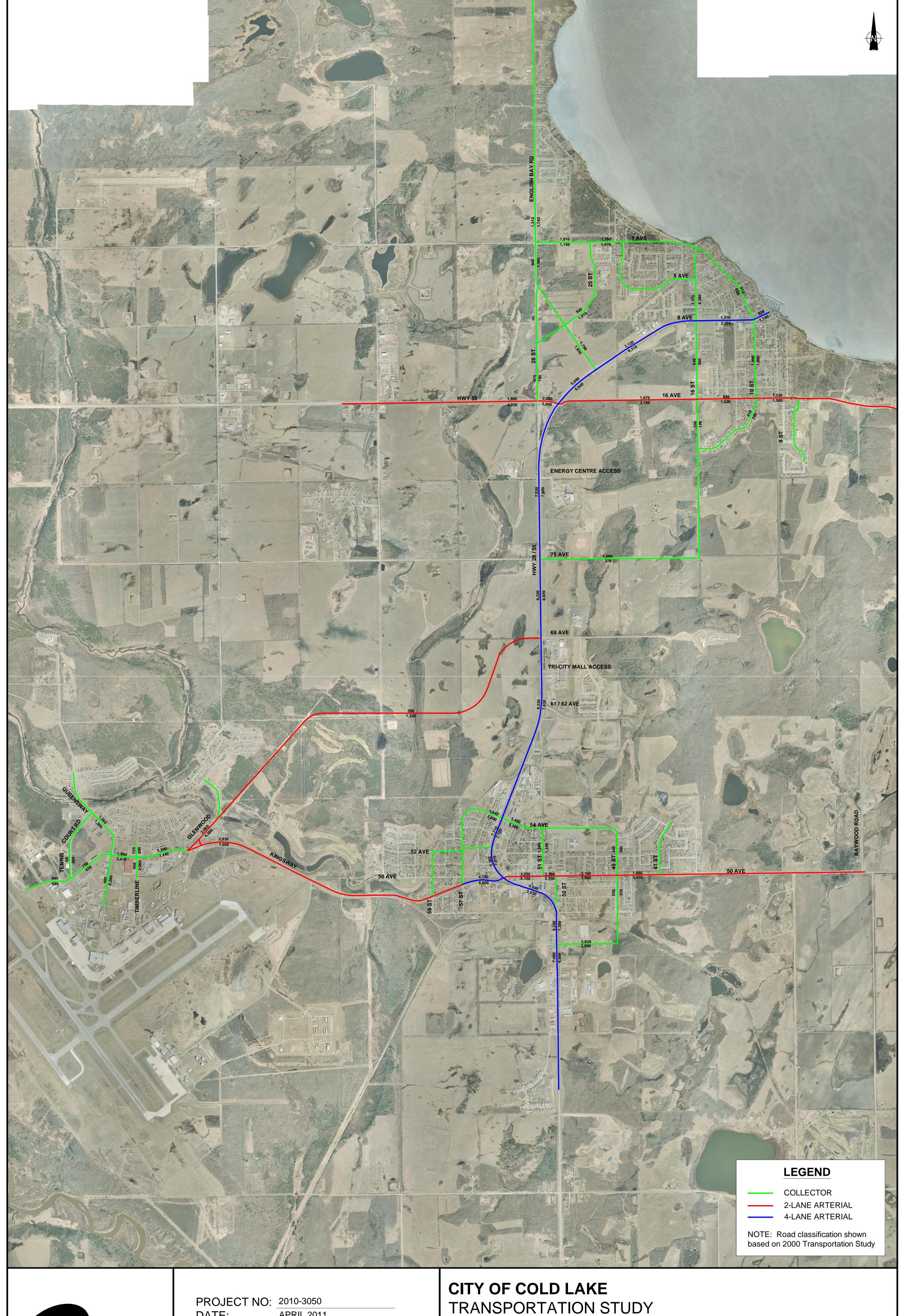


PROJECT NO: 2010-3050 DATE: **APRIL 2011** APPROVED: NTS SCALE:

DWG NO:

TRANSPORTATION STUDY

FIGURE 3.2 5 YEAR (2015) DAILY BACKGROUND TRAFFIC VOLUMES

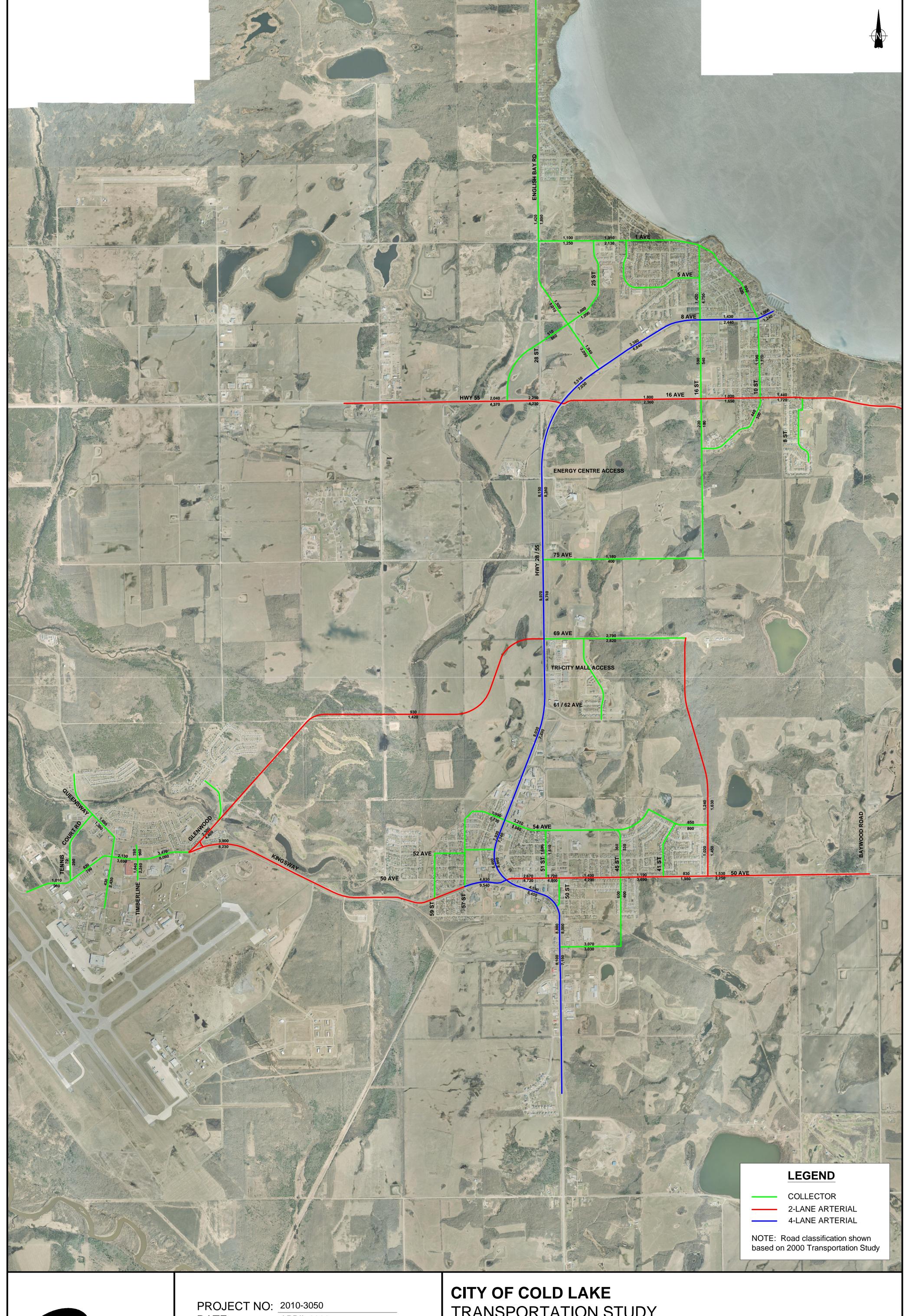




DATE: **APRIL 2011** APPROVED: NTS SCALE:

DWG NO:

FIGURE 3.3 10 YEAR (2020) DAILY BACKGROUND TRAFFIC VOLUMES



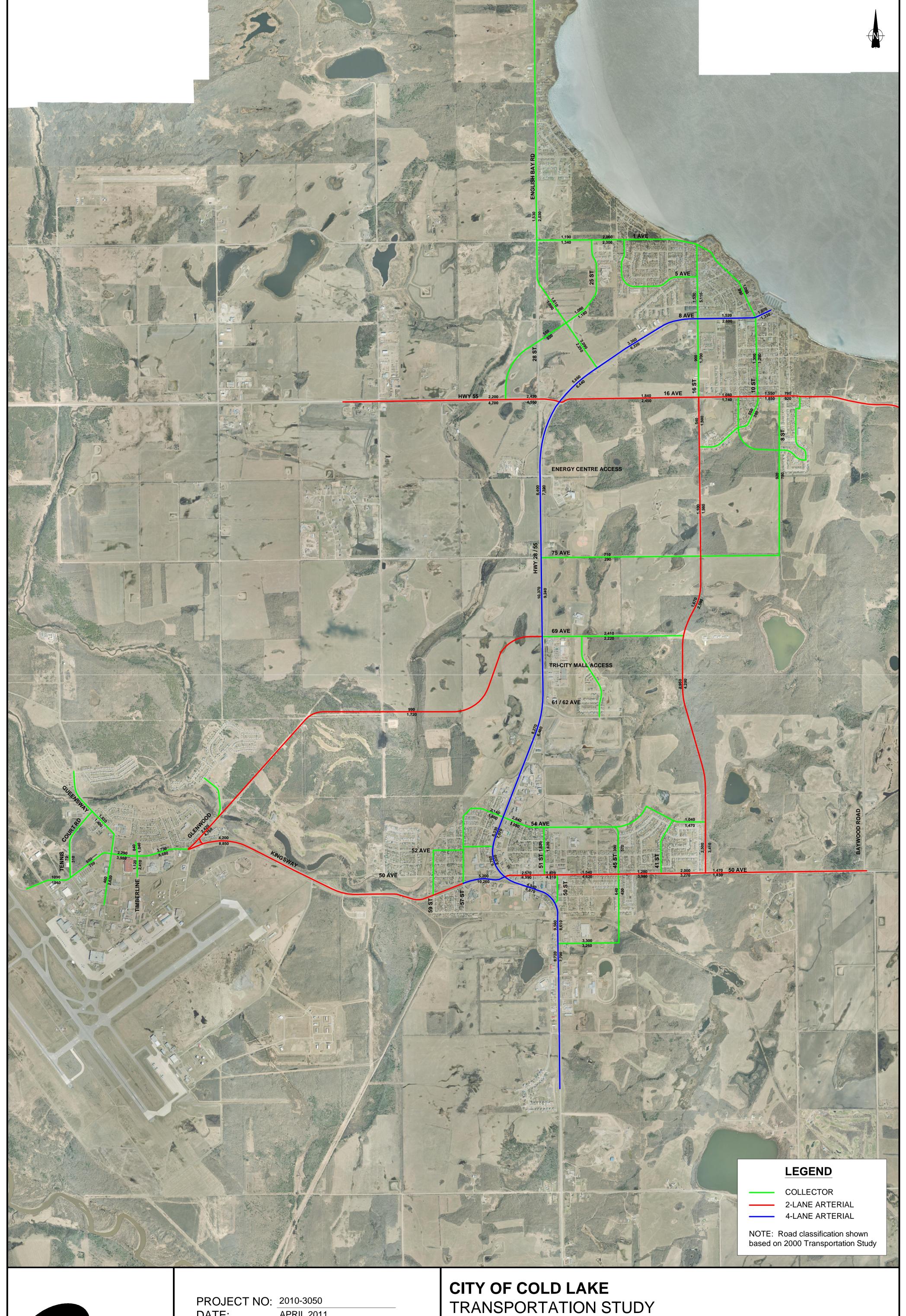


DATE: **APRIL 2011** APPROVED:

NTS SCALE: DWG NO:

# TRANSPORTATION STUDY

FIGURE 3.4 15 YEAR (2025) DAILY BACKGROUND TRAFFIC VOLUMES





DATE: **APRIL 2011** 

APPROVED: NTS SCALE: DWG NO:

FIGURE 3.5 20 YEAR (2030) DAILY BACKGROUND TRAFFIC VOLUMES

## 4

## **Traffic Forecast: Future Development Traffic Volumes**

#### 4.1 FUTURE DEVELOPMENTS

There are 20 development/redevelopment projects identified for the City and surrounding area in the next 20 years; 13 are located within the City and seven are located outside the City, in the MD of Bonnyville. The development/redevelopment projects are shown in Figure 4.1 and are discussed in the following sections.

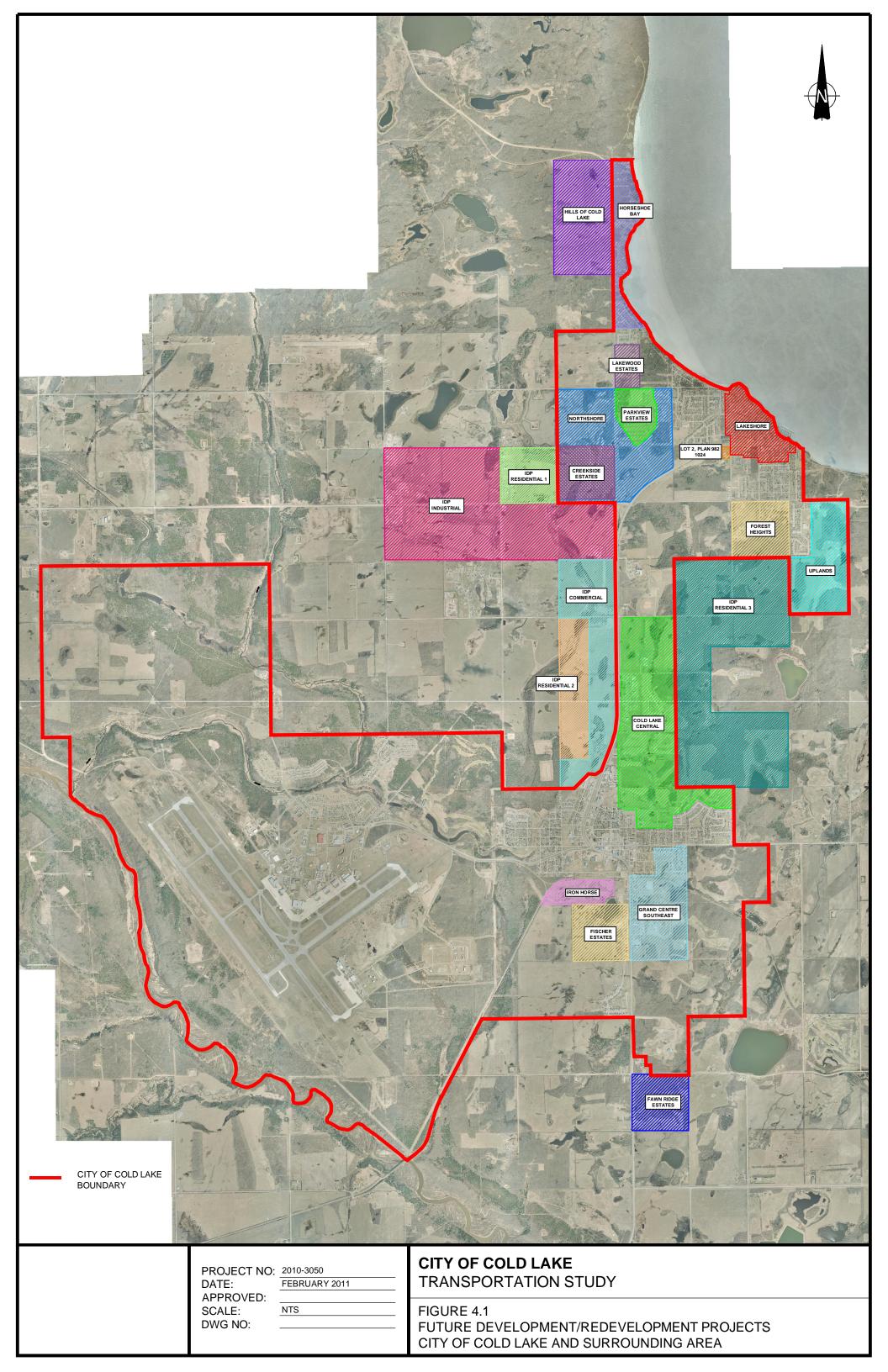
AE contacted the Department of Defence in Medley to obtain information about future land use growth or change within the base, and was informed that there would be no expected changes or growth within the 20-year planning horizon of the transportation study.

#### 4.1.1 City of Cold Lake

The 13 development/redevelopment projects expected within the City are:

- Fischer Estates: 63.5 hectares located in Cold Lake South (SE 1/4 34-62-2-4)
- Iron Horse: 30.77 hectares located in Cold Lake South (N ½ f 34-62-2-4)
- Cold Lake Central: 248.0 hectares located between Cold Lake North and Cold Lake South (W ½ 11-63-2-4, W ¼ 2-63-2-4, and S ½ 2-63-11-4)
- Grand Centre SE: 105.0 hectares located in Cold Lake South (W ½ 35-62-2-4)
- Forest Heights: 64.0 hectares located in Cold Lake North (NW ¼ o13-63-2-4)
- Northshore: 244.0 hectares located in Cold Lake North (NE ¼ 22-63-2-4, SE ¼ 22-63-2-4, SW ¼ 23-63-2-4, and NW ¼ 23-63-2-4)
- Lot 2, Plan 982 1024: 1.81 hectares located in Cold Lake North (SE ½ 23-63-2-4)
- Horseshoe Bay: 77.7 hectares located in Cold Lake North (NW ¼ 26-63-2-4, SW ¼ 35-63-2-4, and NW ¼ 35-63-2-4)
- Uplands: 101.9 hectares located in Cold Lake north (NE 13-63-2-4 and SE 13-63-2-4)
- Lakeshore Redevelopment: 66.0 hectares located in Cold Lake North.
- Lakewood Estates: 21.3 hectares located in Cold Lake North (SW ½ 26-63-2-4)
- Creekside Estates: 60.5 hectares located in Cold Lake North (SE 1/4 22-63-2-4)
- Parkview Estates: 36.8 hectares located in Cold Lake North (NW 23-63-2-4).





#### 4.1.2 MD of Bonnyville

The seven development projects expected outside the City, in the MD of Bonnyville are:

- **Hills of Cold Lake**: 119.3 hectares located northwest of Cold Lake North (SE, NE ¼ 34-63-2-4)
- Fawn Ridge Estates: 34.9 hectares located south of Cold Lake South (NW ½ 23-62-2-4)
- **IDP Residential Development 1:** 63 hectares located along the north side of Highway 55, west of the City
- **IDP Residential Development 2:** 84 hectares located west of the IDP Commercial Development, from 75 Avenue and south of 61/62 Avenue
- **IDP Residential Development 3:** 418 hectares located east of Cold Lake Central, from Energy Centre and 55 Avenue
- IDP Industrial Development: 392 hectares located along both sides of Highway 55, west of the City
- **IDP Commercial Development:** 157 hectares located along the west side of Highway 28, from Energy Centre to 55 Avenue.

#### 4.1.3 Development Phasing

The ASP, ARP and Outline Plans for the development/redevelopment projects provided by the City and the IDP did not discuss the expected timing or staging for the projects. Most of the documents stated that the timing would be dictated by market conditions and the availability of municipal servicing capacity.

To forecast the future development traffic volumes for each planning horizon, the following assumptions were made:

- Each development/redevelopment plan will experience 25% growth in each planning horizon with full build-out by 2030 except for Fischer Estates, Iron Horse, Forest Heights and the IDP developments
- Development of Fischer Estates, Iron Horse and Forest Heights will be delayed until 2020.
   By 2030, these three developments will be 50% developed
- Development of the residential land from the IDP will be delayed until 2015. By 2030, the three residential developments will be 30% developed
- Development of the industrial land from the IDP will be delayed until 2015. By 2030, the industrial development will be 20% developed
- Development of the commercial land from the IDP will be delayed until 2015. By 2030, the commercial development will be 30% developed.

The development phasing assumptions were established through discussions with the City's Planning Department. Table 4.1 summarizes the development phasing assumed for each planning horizon.



Table 4-1
Development Phasing Assumption

Development / Redevelopment	Land Use	5-year (2015) Horizon	10-year (2020) Horizon	15-year (2025) Horizon	20-year (2030) Horizon
Fischer Estates	Residential	0%	0%	25%	50%
FISCHEL ESTATES	Commercial	0%	0%	25%	50%
Iron Horse	Residential	0%	0%	25%	50%
Cold Loke Control	Residential	25%	50%	75%	100%
Cold Lake Central	Commercial	50%	100%	100%	100%
Grand Centre	Residential	25%	50%	75%	100%
Southeast	Industrial	25%	50%	75%	100%
Forest Heights	Residential	0%	0%	25%	50%
	Residential	25%	50%	75%	100%
Nowholowa	Commercial	25%	50%	75%	100%
Northshore	Institutional	25%	50%	75%	100%
	School	0%	0%	100%	100%
Lot 2, Plan 982 1024	Commercial	100%	100%	100%	100%
Horseshoe Bay	Residential	50%	100%	100%	100%
	Residential	25%	50%	75%	100%
Uplands	Health Services & Mixed Use	25%	50%	75%	100%
Lakeshore Area Redevelopment	All	25%	50%	75%	100%

Development / Redevelopment	Land Use	5-year (2015) Horizon	10-year (2020) Horizon	15-year (2025) Horizon	20-year (2030) Horizon
Lakewood Estates	Residential	25%	50%	75%	100%
Creekside Estates	Residential	25%	50%	75%	100%
Parkview Estates	Residential	25%	50%	75%	100%
Parkview Estates	Commercial	25%	50%	75%	100%
Hills of Cold Lake	Residential	25%	50%	75%	100%
Fawn Ridge Estates Development	Residential	25%	50%	75%	100%
IDP Residential Development 1	Residential	0%	10%	20%	30%
IDP Residential Development 2	Residential	0%	10%	20%	30%
IDP Residential Development 3	Residential	0%	10%	20%	30%
IDP Industrial Development	Industrial	0%	5%	10%	20%
IDP Commercial Development	Commercial	0%	10%	20%	30%

#### 4.2 TRIP GENERATION

The ASP, ARP and Outline Plans for the development/redevelopment projects were reviewed to obtain information regarding the future land uses and the associated developable area. Of particular relevance was the residential, commercial, industrial and institutional land uses.

Portions of the above mentioned subdivisions are currently developed. Traffic volumes from the developed portions are captured by existing (2010) traffic volumes; therefore, these areas were not included in the forecast for the future development traffic volumes. The breakdown of the future land uses and areas for each development/redevelopment project are presented in Appendix A.

Table 4.2 summarizes the trip generation calculations for each development/redevelopment project. The Institute of Transportation Engineer (ITE) Trip Generation Handbook (7th Edition) was referenced to obtain trip rates for each land use. The maximum site coverage assumptions listed in Table 4.2 reflect those stated in the Traffic Demand Forecast Work Plan established at project initiation and attached in Appendix B. Some site coverage assumptions were revised using engineering judgement to reflect more practical trip estimates.



#### City of Cold Lake

Figure 4.2 through Figure 4.5 present the expected p.m. peak hour trips generated by each development/redevelopment project, for the 5-year, 10-year, 15-year and 20-year horizon, respectively.

In order to establish the trip distribution within the City, the study area was broken into nine traffic analysis zones (TAZ), and is illustrated in Figure 4.6. Zone boundaries were selected to encompass areas with relatively homogenous land use types (e.g., business zones and residential zones).

107,639.1 City of Cold Lake Transportation Study 1 hectare = sq.ft. Project No: 2010-3050 1 hectare = 2.4711 acre Date: April 9, 2011

#### **Table 4.2 Future Developments Trip Generation**

Trip Generation - ITE Trip Generation Handbook

Fischer Co						Independent	Variable			ITE	Data					Trips	s (T)		
	Description	# of	Unit	Land Use Description	Maximum Lot	maepenaem	variable	AM P	eak		PM P	eak		ı	AM Peak			PM Peak	
	Description	# 01	Onne	Land 036 Description	Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	ln	Out
	Low Density Residential	449	du	210: Single Family Residential	-	Dwelling Units	449	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	324	81	243	414	261	153
Eischor	Multi Family Residential	295	du	230: Residential Condo/Townhouse	-	Dwelling Units	295	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	123	21	102	146	98	48
rischei	Commercial - Arterial	0.0	ha	770: Business Park	80%	1000 sq.ft	0.0	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	-	-	-	-	-	-
	Commercial - Neighbourhood	5.8	ha	770: Business Park	50%	1000 sq.ft	313.3	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	438	368	70	431	99	332
-					Tota	I Dwelling Units	744						Total Trips:	884	470	415	992	458	534
				Total Co	ommercial/Indus	trial (1000 sq.ft)	313						•						

						Independent	Variable			ITE	Data					Trips	(T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM Po	eak		PM Pe	eak		J	AM Peak			PM Peak	
					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
	Low Density Residential	323	du	210: Single Family Residential	-	Dwelling Units	323	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	236	59	177	308	194	114
Iron Horse	Medium Density Residential	18	du	230: Residential Condo/Townhouse	-	Dwelling Units	18	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	13	2	11	15	10	5
	High Density Residential	45	du	223: Mid-Rise Apartment	-	Dwelling Units	45	T = 0.41(X) - 13.06	31%	69%	T = 0.48(X) - 11.07	58%	42%	5	2	4	11	6	4
					Tota	I Dwelling Units	386				·		Total Trips:	254	63	191	333	210	123

						Indonendent	Variable			ITE	Data					Trips	(T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot Coverage         Independent Variable           Description         Units (X)           -         Dwelling Units         1,354           -         Dwelling Units         578           -         Dwelling Units         602           80%         1000 sq.ft         1,612.7	AM P	eak		PM P	Peak			AM Peak			PM Peak			
<b>-</b>	1				Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	ln	Out	Total	In	Out
	Low Density Residential	1,354	du	210: Single Family Residential	-	Dwelling Units	1,354	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	957	239	718	1,118	705	414
Cold Lake Central Area	Medium Density Residential	578	du	230: Residential Condo/Townhouse	-	Dwelling Units	578	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	210	36	174	253	170	84
Cold Lake Certifal Area	High Density Residential	602	du	223: Mid-Rise Apartment	-	Dwelling Units	602	T = 0.41(X) - 13.06	31%	69%	T = 0.48(X) - 11.07	58%	42%	234	72	161	278	161	117
	Commercial - Arterial	18.7	ha	770: Business Park	80%	Description   Units	1,612.7	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	2,182	1,833	349	1,949	448	1,500
					Tota	I Dwelling Units	2,534				_		Total Trips:	3,583	2,180	1,403	3,598	1,484	2,114

Total Commercial/Industrial (1000 sq.ft) 1,612.7

Grand Centre SE Mo						Indopendent	Variable			ITE	Data					Trip	s (T)		
Development	Description	# of	Unit	Land Use Description	- Dwelling Units 28 - Dwelling Units 150	variable	AM F	eak		PM F	Peak			AM Peak			PM Peak		
2010100	25551, <b>4</b> 11511	" "	J	24.14 000 2000 (2000)		Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out	
	Low Density Residential	281	du	210: Single Family Residential	-	Dwelling Units	281	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	206	52	155	272	171	101
Grand Centre SE	Mobile home	150	du	240: Mobile Home Park	-	Dwelling Units	150	Ln(T) = 0.64Ln(X) + 0.96	20%	80%	T = 0.57(X) + 2.06	62%	38%	65	13	52	88	54	33
	Industrial - Light Industrial	5.9	ha	130: Industrial Park	60%	1000 sq.ft	380.1	Ln(T) = 0.77Ln(X) + 1.09	82%	18%	T = 0.77(X) + 42.11	21%	79%	288	236	52	335	70	265
					Tota	I Dwelling Units	431						Total Trips:	559	301	258	694	296	398
Low Density Residential   281   du   210: Single Family Residential   -   D	trial (1000 sq.ft)	380.1						-											

						Independent	Variable			ITE	Data					Trips	(T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot Coverage	Variable	AM P	eak		PM F	Peak			AM Peak			PM Peak		
	2333,				Coverage	Coverage         Description         Units           -         Dwelling Units         345           -         Dwelling Units         248	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	ln	Out	Total	In	Out
Forest Heights	Single Family Residential	345	du	210: Single Family Residential	-	Dwelling Units	345	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	251	63	188	327	206	121
i orest Heights	Multi Family Residential	248	du	230: Residential Condo/Townhouse	-	Dwelling Units	248	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	107	18	89	127	85	42
					Tota	al Dwelling Units	593						Total Trips:	358	81	277	453	291	163

						Independent	Variable			ITE	Data					Trip	s (T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM P	eak		PM F	Peak			AM Peak			PM Peak	
					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
	Low Density Residential	537	du	210: Single Family Residential	-	Dwelling Units	537	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	385	96	289	487	307	180
	Medium Density Residential	475	du	230: Residential Condo/Townhouse	-	Dwelling Units	475	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	180	31	149	216	145	71
Northshore	Mixed Use Commercial <sup>1</sup>	547	du	223: Mid-Rise Apartment	-	Dwelling Units	547	T = 0.41(X) - 13.06	31%	69%	T = 0.48(X) - 11.07	58%	42%	211	65	146	251	146	10€
	iviixed use commercial	6.5	ha	770: Business Park	25%	1000 sq.ft	174.9	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	247	208	40	252	58	194
Northshore	Mixed Use Institutional <sup>2</sup>	157	du	223: Mid-Rise Apartment	-	Dwelling Units	157	T = 0.41(X) - 13.06	31%	69%	T = 0.48(X) - 11.07	58%	42%	51	16	35	64	37	27
	iviixed ose iristitutional	2.6	ha	720: Medical-Dental Office Building	15%	1000 sq.ft	42.0	2.48	79%	21%	3.72	27%	73%	104	82	22	156	42	11/
	School Site	1,958	students	520: Elementary School	-	Students	1,958	Ln(T) = 1.11Ln(X) - 1.73	55%	45%	Ln(T) = 1.08Ln(X) - 1.90	45%	55%	799	439	360	537	242	29F
	Commercial - Neighbourhood	1.8	ha	770: Business Park	50%	1000 sq.ft	96.9	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	139	116	22	147	34	11?
	Commercial - Arterial	11.9	ha	770: Business Park	80%	1000 sq.ft	1,024.7	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	1,399	1,175	224	1,284	295	98¢
ium lot coverage for co	mmercial portion of mix use commercial assum	ned to be 25%			Tota	I Dwelling Units	1,716						Total Trips:	3,516	2,229	1,286	3,394	1,305	2,08

Maximum lot coverage for commercial portion of mix use commercial assumed to be 25%
 Maximum lot coverage for institutional portion of mix use insitutuional assumed to be 15%

Total Dwelling Units 1,716

Total Commercial/Instutional (1000 sq.ft) 1,338.5 School Site (Students)

City of Cold Lake Transportation Study 1 hectare = 107,639.1 sq.ft. Project No: 2010-3050 1 hectare = 2.4711 acre Date: April 9, 2011

#### **Table 4.2 Future Developments Trip Generation**

Trip Generation - ITE Trip Generation Handbook

						Independent	Variable			ITE	Data					Trips	s (T)		
Development  Lot 2 Commen	Description	# of	Unit	Land Use Description	Maximum Lot	independen	variable	AM P	eak		PM P	eak			AM Peak			PM Peak	
					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	ln	Out	Total	ln	Out
Lot 2	Commercial - Arterial	15,365	sq.ft	770: Business Park	-	1000 sq.ft	15.4	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	23	19	4	27	6	21
				Total Co	ommercial/Indus	trial (1000 sq.ft)	15.4						Total Trips:	23	19	4	27	6	21

						Independent	Variable			ITE	Data					Trips	s (T)		
Development	Development Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM F	Peak		PM P	eak			AM Peak			PM Peak	
				, , , , , , , , , , , , , , , , , , , ,	Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	ln	Out
Horseshoe Bay	Low Density Residential	42	du	210: Single Family Residential	-	Dwelling Units	42	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	39	10	29	49	31	18
•					Tota	al Dwelling Units	42						Total Trips:	39	10	29	49	31	18

						Independent	Variable			ITE	Data					Trip	s (T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM P	eak		PM P	eak			AM Peak			PM Peak	
	233314				Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
	Single Family Residential	904	du	210: Single Family Residential	-	Dwelling Units	904	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	642	161	482	778	490	288
Uplands	Multi Family Residential	480	du	230: Residential Condo/Townhouse	-	Dwelling Units	480	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	181	31	150	218	146	72
	Health Services and Mixed Use 3,4	5.0	ha	620: Nursing Home	50%	1000 sq.ft	269.1	0.38	53%	47%	0.42	47%	53%	102	54	48	113	53	60
3. Maximum site coverage for Hea	Ith Care & Mixed Use assumed to be 50%				Tota	I Dwelling Units	1,384						Total Trips:	926	246	680	1,108	689	419
4. AM directional split for Nursing H	Home, by 1000 sq. ft., not available. Assume reverse	of PM direct	ional split	Total Hea	alth Care & Mixed	Use (1000 sq.ft)	269.1						•						

#### TO SUBTRACT

						Independent	Variable			ITE	Data					Trip	s (T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM P	Peak		PM P	eak			AM Peak			PM Peak	
	25551				Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
	902 10 Street	0.11	hec	770: Business Park	50%	1000 sq.ft	5.9	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	9	8	1	11	3	9
	904 10 Street	0.07	hec	770: Business Park	50%	1000 sq.ft	3.7	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	6	5	1	7	2	6
Lakeshore ARP	901 9 Avenue	0.11	hec	770: Business Park	50%	1000 sq.ft	6.0	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	9	8	1	11	3	9
	803 10 Avenue	0.22	hec	770: Business Park	50%	1000 sq.ft	12.1	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	18	15	3	22	5	17
	Fire Hall / Community Hall	0.34	hec	495: Recreational Community Center <sup>5</sup>	50%	1000 sq.ft	18.4	22.88	50%	50%	10 % of a	All Day		422	211	211	42	21	21
<ol><li>For Fire Hall, assumed same trip</li></ol>	rate as future community centre. Trips will remain	the same esse	entially.		Total Comme	rcial (1000 sq.ft)	27.7						Total Trips:	464	246	218	94	33	61
				Total Fire	Hall / Community	Hall (1000 sq.ft)	18.4						•						

<sup>5.</sup> For Fire Hall, assumed same trip rate as future community centre. Trips will remain the same essentially.

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						Independent	Variable			ITE	Data					Trip	s (T)		
902 904 901	Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM F	Peak		PM F	Peak			AM Peak			PM Peak	
					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	ln	Out	Total	In	Out
	Vacant parcel on 12 Street and 8 Avenue	38	du	230: Residential Condo/Townhouse	-	Dwelling Units	38	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	24	4	20	27	18	9
	902 10 Street	15	du	223: Mid-Rise Apartment <sup>6</sup>	-	Dwelling Units	15	0.3	31%	69%	0.39	58%	42%	4	1	3	6	3	2
	904 10 Street	9	du	223: Mid-Rise Apartment	-	Dwelling Units	9	0.3	31%	69%	0.39	58%	42%	3	1	2	4	2	2
	901 9 Avenue	15	du	223: Mid-Rise Apartment	-	Dwelling Units	15	0.3	31%	69%	0.39	58%	42%	5	1	3	6	3	2
Lakeshore ARP	803 10 Avenue	3	du	210: Single Family Residential	-	Dwelling Units	3	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	12	3	9	5	3	2
	Triangle Park	0.11	hec	411: City Park 7	100%	Acres	0.3	1.59	50%	50%	10 % of	All Day		0	0	0	0	0	0
	Bibeau Park	1.16	hec	411: City Park	100%	Acres	2.9	1.59	50%	50%	10 % of	All Day		5	2	2	0	0	0
	Centoaph Park	0.26	hec	411: City Park	100%	Acres	0.6	1.59	50%	50%	10 % of	All Day		1	1	1	0	0	0
	Fire Hall / Community Hall	0.34	hec	495: Recreational Community Center 8	50%	1000 sq.ft	18.4	22.88	50%	50%	10 % of	All Day		422	211	211	42	21	21
verage rate used instead of	f equation since equation results in negative trip valu	ies.			Tota	I Dwelling Units	80						Total Trips:	475	225	251	90	51	38

<sup>7.</sup> Trip rate for park is for all-day, weekday. PM trips assumed to be 10% of all-day trips (presented under AM Trips)
8. Trip rate for community centre is for all-day, weekday. PM trips assumed to be 10% of all-day trips (presented under AM Trips). To be used for special events only.

Total City Park (Acres)	3.8
Total Fire Hall / Community Hall (1000 sq.ft)	18.4

						Independent	Variable			ITE	Data					Trips	s (T)		
Development	Development Description	# of	Unit	Land Use Description	Maximum Lot	maepenaem	Variable	AM P	eak		PM P	eak			AM Peak			PM Peak	
				, , , , , , , , , , , , , , , , , , , ,	Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	ln	Out	Total	In	Out
Lakewood	Low Density Residential	153	du	210: Single Family Residential	-	Dwelling Units	153	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	117	29	87	157	99	58
					Tota	I Dwelling Units	153						Total Trips:	117	29	87	157	99	58

						Independent	Variable			ITE	Data					Trips	s (T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	maepenaem	variable	AM P	eak		PM Pe	eak			AM Peak			PM Peak	
2010lopillolli	2000.	<i>"</i> 6.	· · · · ·		Coverage	Description	Units (X)	Equation or Average Rate	% Inbound %	Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
Creekside	Low Density Residential	594	du	210: Single Family Residential	-	Dwelling Units	594	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	425	106	319	533	336	197
Creekside	Medium Density Residential	196	du	230: Residential Condo/Townhouse	-	Dwelling Units	196	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	88	15	73	104	70	34
•					Total	Dwelling Units	790						Total Trips:	514	121	392	637	406	232

<sup>4.</sup> AM directional split for Nursing Home, by 1000 sq. ft., not available. Assume reverse of PM directional split

 City of Cold Lake Transportation Study
 1 hectare =
 107,639.1
 sq.ft.

 Project No: 2010-3050
 1 hectare =
 2.4711
 acre

 Date: April 9, 2011

#### **Table 4.2 Future Developments Trip Generation**

Trip Generation - ITE Trip Generation Handbook

						Indonendent	Variable			ITE	Data					Trips	s (T)		
Parkview	Description	# of	Unit	Land Use Description		independent	variable	AM F	Peak		PM I	Peak			AM Peak			PM Peak	
	2333.				Coverage   Description   Units (X)   Equation or Average Rate   % Inbound   % Outbound   Equation or Average Rate   % Inbound   % Outbound   Total   In   Out   Total   Lind   Units (X)   Equation or Average Rate   % Inbound   % Outbound   Equation or Average Rate   % Inbound   % Outbound   Total   In   Out   Total   Lind   Units (X)   Equation or Average Rate   % Inbound   % Outbound   Equation or Average Rate   % Inbound   % Outbound   Total   In   Out   Total   Lind   Units (X)   Equation or Average Rate   % Inbound   % Outbound   Equation or Average Rate   % Inbound   % Outbound   Total   In   Out   Total   Units (X)   Equation or Average Rate   % Inbound   % Outbound   Equation or Average Rate   % Inbound   % Outbound   Total   In   Out   Total   Units (X)   Equation or Average Rate   % Inbound   % Outbound   Equation or Average Rate   % Inbound   % Outbound   Total   Units (X)   Equation or Average Rate   % Inbound   % Outbound   Equation or Average Rate   % Inbound   % Outbound   Total   Units (X)   Equation or Average Rate   % Inbound   % Outbound   Equation or Average Rate   % Inbound   % Outbound   Total   Units (X)   Equation or Average Rate   % Inbound   % Outbound   Equation or Average Rate   % Inbound   % Outbound   Total   Units (X)   Equation or Average Rate   % Inbound   % Outbound   Equation or Average Rate   % Inbound   % Outbound   Total   Units (X)   Equation or Average Rate   % Inbound   % Outbound   Equation or Average Rate   % Inbound   % Outbound   %	ln	Out												
Dorlavious	Low Density Residential	367	du	210: Single Family Residential	-	Dwelling Units	367	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	266	67	200	345	218	128
Parkview	Commercial	2.6	ha	770: Business Park	50%	1000 sq.ft	141.7	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	201	169	32	208	48	160
					Tota	l Dwelling Units	367						Total Tring:	467	236	232	553	265	288
				T-1-16				1					Total Imps.	401	200	202	000	203	200
				Total C		trial (1000 sq.ft)	141.7	<u> </u>	_	ITE	Data	_	Total Trips.	401	200			203	200
Development	Description	# of	Unit		Commercial/Indus	trial (1000 sq.ft)	141.7	AM F	Peak	ITE		Peak	Total IIIps.			Trips	s (T)	PM Peak	200
Development	Description	# of	Unit	Total C	Commercial/Indus	trial (1000 sq.ft)	141.7 Variable				PM F				AM Peak	Trips	s (T)	PM Peak	Out
Development  MD - Hills of Cold Lake	Description Option 2 - 300 1/2 Acre Lot Subdivision	# <b>of</b>			Commercial/Indus	trial (1000 sq.ft)	141.7 Variable				PM F				AM Peak	Trips	s (T)	PM Peak	

					iota	Dwelling Units	300	<u>l</u>					rotai irips:	219	55	105	288	182	107
						Independent	Variable			ITE	Data					Trips	(T)		
Development	Development Description # of Unit	Land Use Description	Maximum Lot	independent	variable	AM P	eak		PM P	eak			AM Peak			PM Peak			
		Description # of Unit Land Use Description		Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	ln	Out	
MD - Fawn Ridge Estates <sup>9</sup>	Country Residential Lots	0	du	210: Single Family Residential	-	Dwelling Units	54	0.77	26%	74%	1.02	64%	36%	42	11	31	55	35	20
9. Information, including dwelling ur	nits, trip generation rates and percentage splits, as p			Tota	l Dwelling Units	54						Total Trips:	42	11	31	55	35	20	

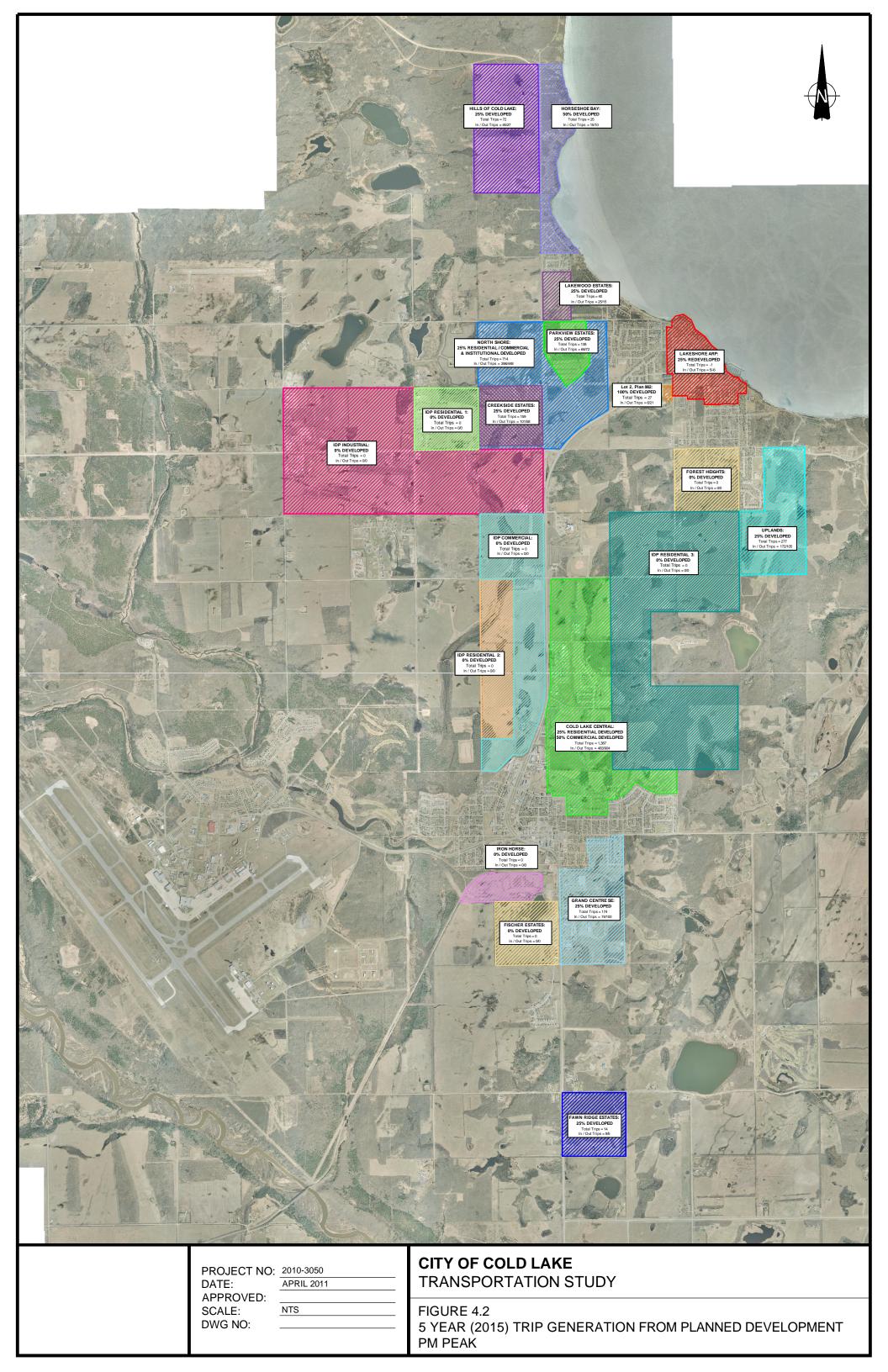
						Independent	Variable			ITE	Data					Trip	s (T)		
Development	Description	# of Unit Land Use Description  283 du 210: Single Family Residential	Land Use Description	Maximum Lot	independent	variable	AM Po	eak		PM P	eak			AM Peak			PM Peal	(	
2010юро.	2000	0.	<b></b>		Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
IDP - Residential Development 1	Low Density Residential	283	du	210: Single Family Residential	-	Dwelling Units	283	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	208	52	156	273	172	101
IDF - Residential Development 1	Multi Family Residential	236	du	230: Residential Condo/Townhouse	-	Dwelling Units	236	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	103	17	85	122	81	40
•					Tota	I Dwelling Units	519						Total Trips:	310	69	241	395	254	141

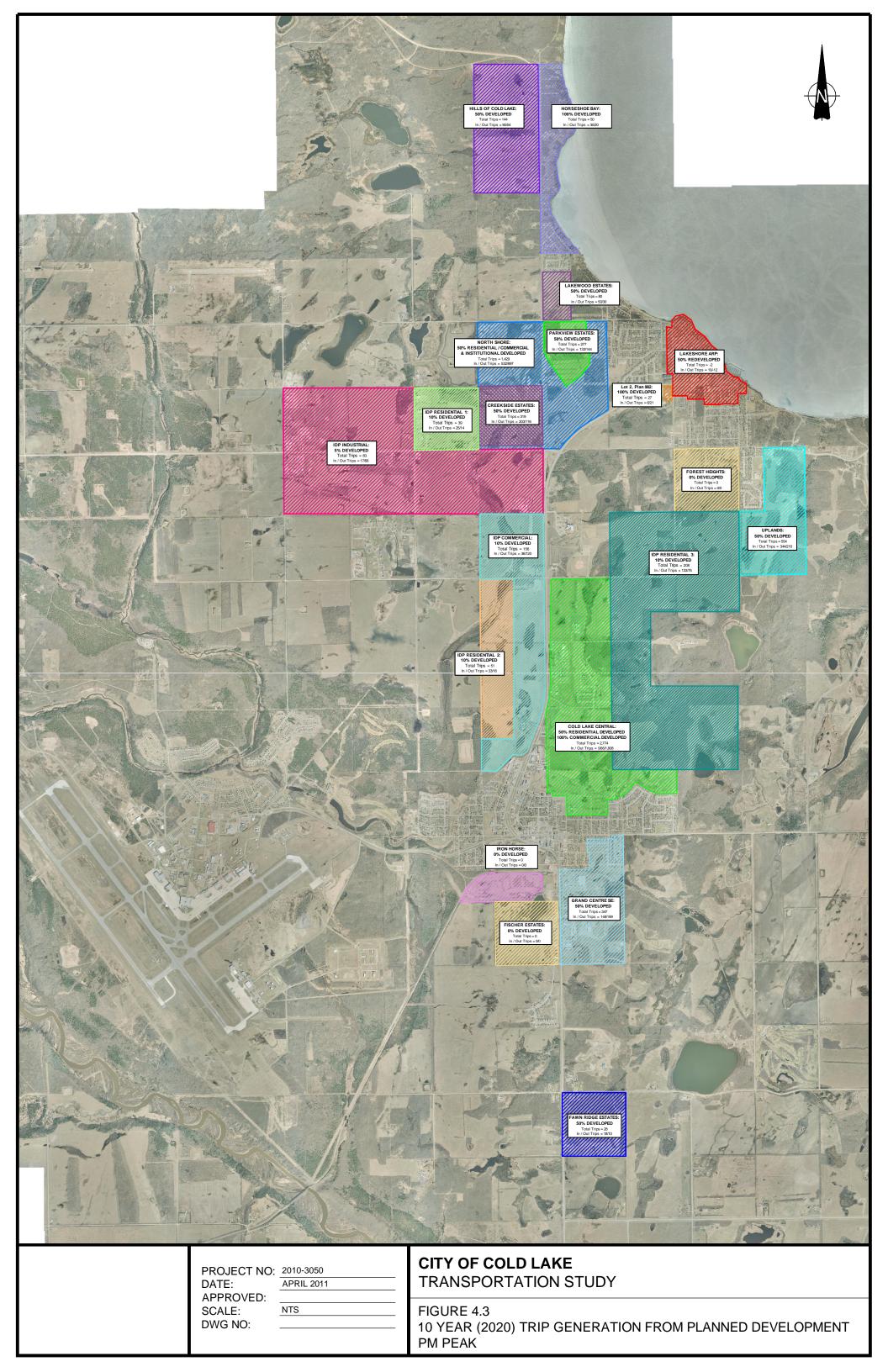
						Independent	Variable			ITE I	Data					Trips	(T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM P	eak		PM P	Peak			AM Peak			PM Peak	
					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
IDP - Residential Development 2	Low Density Residential	379	du	210: Single Family Residential	-	Dwelling Units	379	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	275	69	206	356	224	132
IDP - Residential Development 2	Multi Family Residential	316	du	230: Residential Condo/Townhouse	-	Dwelling Units	316	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	130	22	108	154	104	51
					Tota	I Dwelling Units	696						Total Trips:	405	91	314	510	328	183

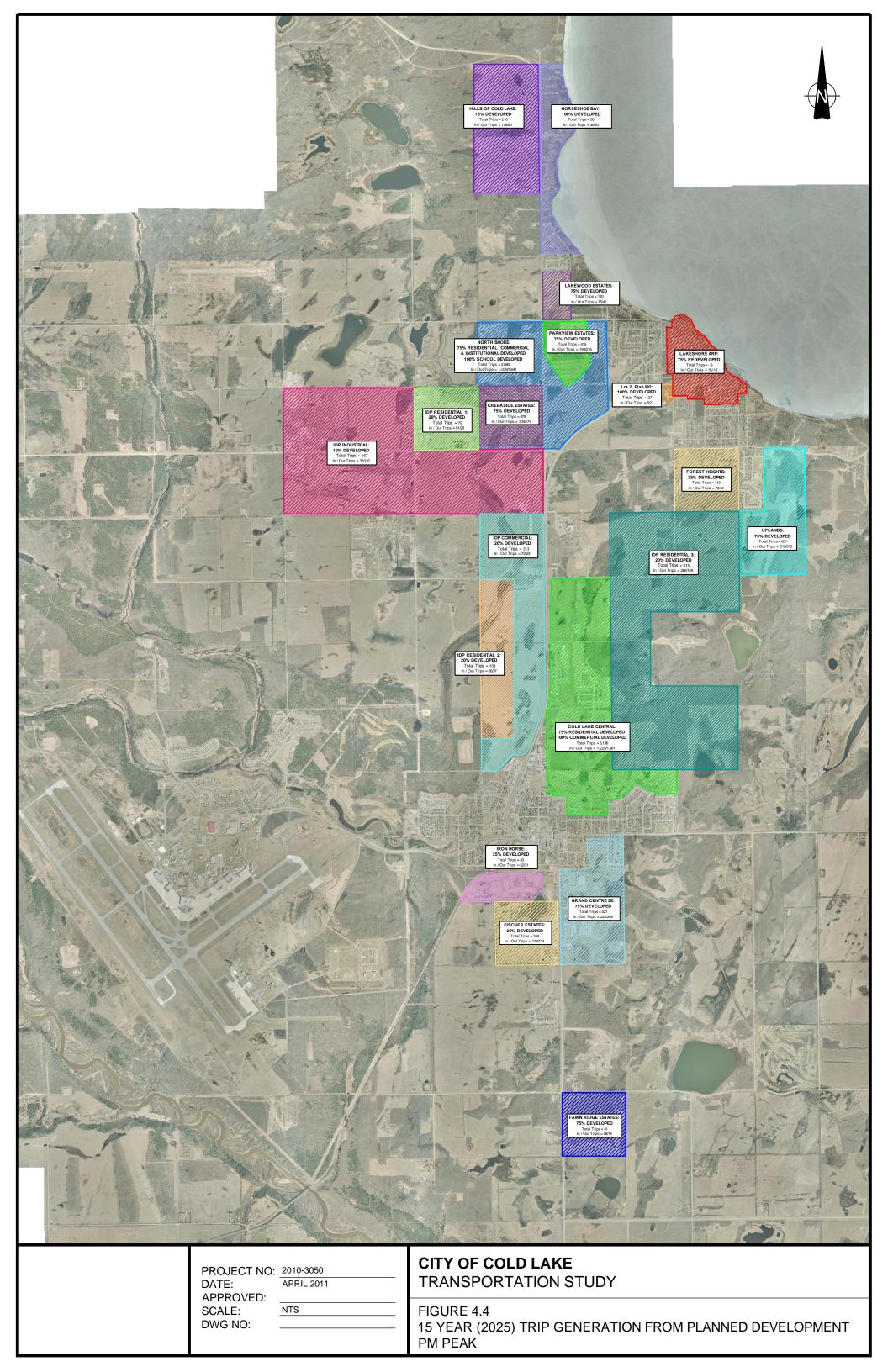
					Land Use Description Maximum Lot		Independent Variable		ITE Data						Trips (T)					
Development	Description	# of	Unit	Land Use Description			variable	AM Peak			PM P	eak			AM Peak	Il Peak		PM Peak		
233334	,				Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out	
IDP - Residential Development 3	Low Density Residential	1,880	du	210: Single Family Residential	-	Dwelling Units	1,880	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	1326	331	994	1503	947	556	
IDF - Residential Development S	Multi Family Residential	1,567	du	230: Residential Condo/Townhouse	-	Dwelling Units	1,567	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	467	79	387	574	385	189	
					Tota	I Dwelling Units	3,447						Total Trips:	1,792	411	1,381	2,077	1,331	746	

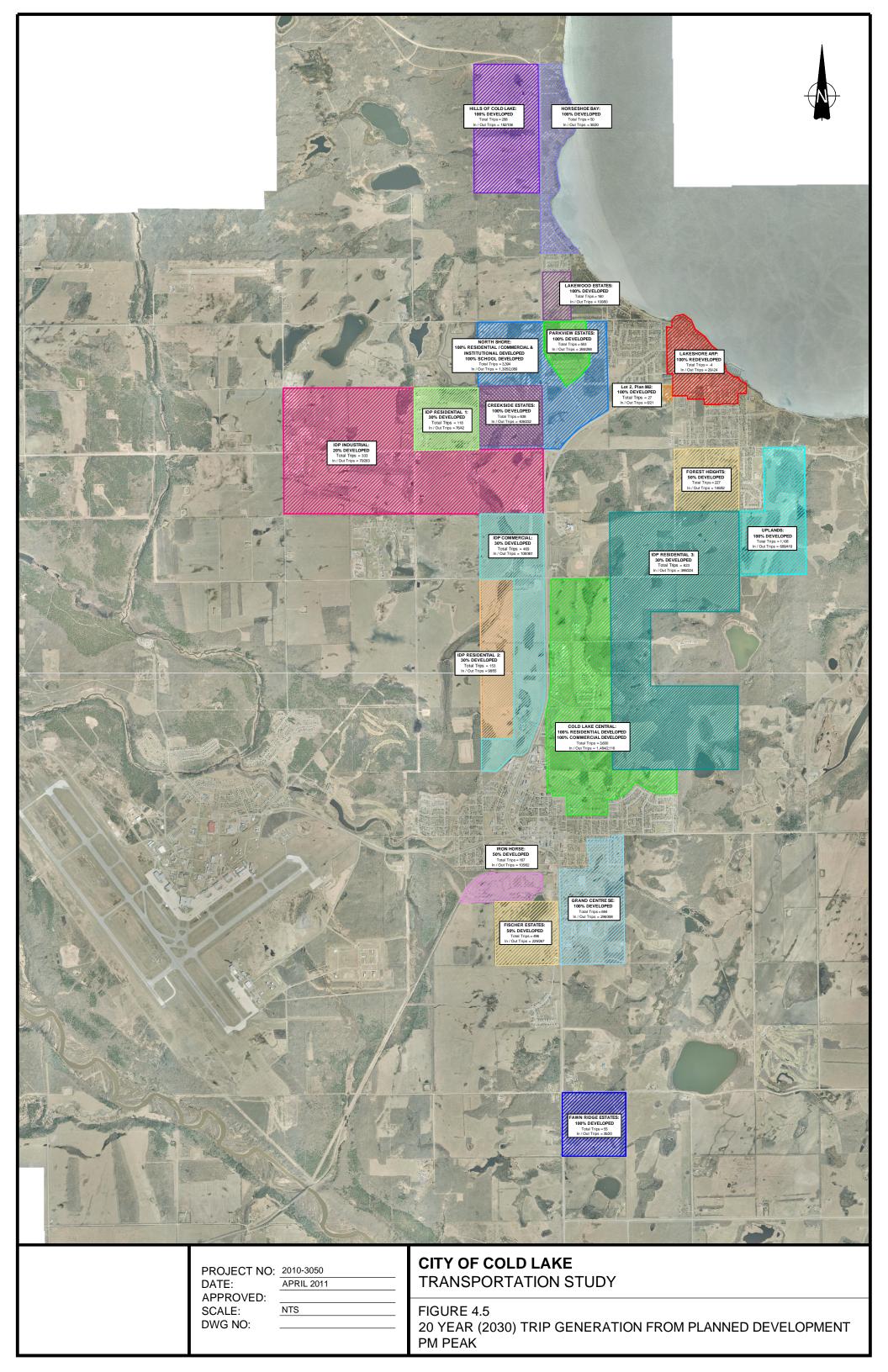
					Land Use Description Maximum Lot		Independent Variable		ITE Data								Trips	(T)		
Development	Description	# of	Unit	Land Use Description			Variable	AM P	AM Peak		PM P	Peak			AM Peak		PM Peak			
201010p0111	2000.1511011	<i>"</i> •	J		Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	ln	Out	Total	ln	Out	
IDP - Industrial Developments 10	Industrial - Light & Heavy Industrial	78	ha	130: Industrial Park	25%	1000 sq.ft	2,109.4	Ln(T) = 0.77Ln(X) + 1.09	82%	18%	T = 0.77(X) + 42.11	21%	79%	1,079	885	194	1,666	350	1,316	
10. Maximum site coverage for IDP	Industial Developments assumed to be 25%			Total C	ommercial/Indus	strial (1000 sq.ft)	2,109						Total Trips:	1,079	885	194	1,666	350	1,316	

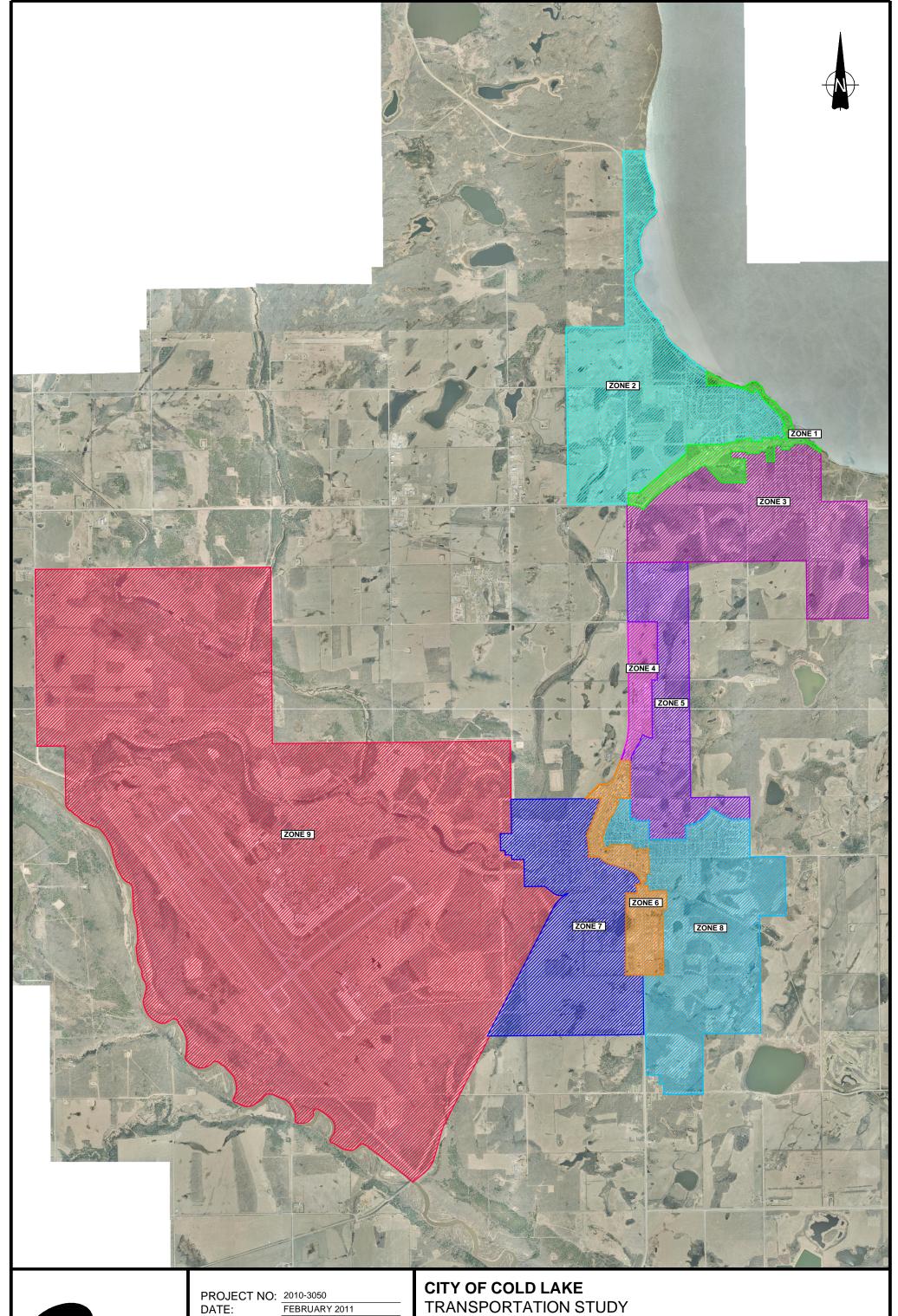
					nd Use Description		. Independe		Variable	ITE Data								Trips	rips (T)			
Development	Description	# of	Unit	Land Use Description			variable	AM Peak			PM P	PM Peak			AM Peak	Peak		PM Peak				
					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	ln	Out			
IDP - Commercial 11	Commercial - Arterial	47.2	ha	770: Business Park	25%	1000 sq.ft	1,270.8	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	1,727	1,451	276	1,565	360	1,205			
11. Maximum site coverage for IDP	- Industial Developments assumed to be 25%		·	Total Co	ommercial/Indus	trial (1000 sq.ft)	1,271			•	•		Total Trips:	1,727	1,451	276	1,565	360	1,205			













DATE: APPROVED:

SCALE:

DWG NO:

FEBRUARY 2011

NTS

FIGURE 4.6 TRAFFIC ANALYSIS ZONES WITHIN CITY OF COLD LAKE The development trips presented above in Figure 4.2 through Figure 4.5 were broken down by location and the corresponding TAZ. Table 4.3 through Table 4.6 summarizes the p.m. peak hour trips generated by each zone, for the 5-year, 10-year, 15-year, and 20-year planning horizons respectively.

Table 4.3 5-year (2015) - Trip Generation from Planned Developments by Zone

Zone	Total Trips	Inbound Trips	Outbound Trips
1	347	85	262
2	755	400	355
3	277	172	105
4	4 974		750
5	412	259	154
6	84	18	66
7	0	0	0
8	90	56	33
9	0	0	0
External Zones	External Zones 86		32
Total	3,025	1,268	1,757

Table 4.4
10-year (2020) - Trip Generation from Planned Developments by Zone

Zone	Total Trips	Inbound Trips	Outbound Trips		
1	667	163	504		
2	1,510	800	710		
3	554	344	210		
4	1,949	448	1,500		
5	825	518	307		
6	167	35	132		
7	0	0	0		
8	180	113	67		
9	0	0	0		
External Zones	External Zones 710		356		
Total	6,023	2,530	3,493		

Table 4.5
15-year (2025) - Trip Generation from Planned Developments by Zone

Zone	Total Trips	Inbound Trips	Outbound Trips
1	987	241	745
2	2,777	1,426	1,351
3	944	589	355
4	1,949	448	1,500
5	1,237	777	461
6	359	78	281
7	223	142	81
8	270	169	100
9	0	0	0
External Zones	1,334	652	681
Total	9,003	4,033	4,970

Table 4.6 20-year (2030) - Trip Generation from Planned Developments by Zone

Zone	Total Trips	Inbound Trips	Outbound Trips
1	1,307	320	987
2	3,507	1,811	1,696
3	1,335	834	501
4	1,949	448	1,500
5	1,650	1,036	614
6	551	120	431
7	447	284	162
8	360	226	134
9	0	0	0
External Zones	2,041	969	1,072
Total	11,447	5,295	6,152

#### 4.3 TRIP DISTRIBUTION

A simplified gravity model was originally selected to determine the distribution of trips within the City generated by the proposed development/redevelopment. The gravity model assumes that the number of trips between two zones is directly proportional to the trips produced and attracted by both zones and inversely proportional to the square of travel time between the two zones. The procedure for the simplified gravity model was illustrated in detail in the Traffic Demand Forecast Work Plan (included in Appendix B) and the trip distribution calculations for the 5-year planning horizon are presented in Appendix C.

A trip distribution table was established for the 5-year planning horizon using the simplified gravity model. The trip distribution established for each TAZ was not reflective of the local travel patterns within the City; therefore, the trip distribution was revised after discussions with the City to reflect the local characteristic of the City. Table 4.7 presents the trip distribution used for the traffic volume forecast. The same trip distribution was used for each planning horizon (5-year, 10-year, 15-year, and 20-year).

Table 4.7
Trip Distribution Table (Within City Limits)

Erom					То					CLIM
From	1	2	3	4	5	6	7	8	9	SUM
1	5%	23%	23%	5%	8%	5%	10%	8%	13%	100%
2	20%	15%	10%	15%	10%	8%	7%	5%	10%	100%
3	20%	10%	15%	15%	10%	8%	7%	5%	10%	100%
4	6%	15%	15%	5%	15%	8%	12%	12%	12%	100%
5	16%	12%	12%	15%	15%	8%	7%	5%	10%	100%
6	5%	5%	5%	5%	15%	5%	25%	25%	10%	100%
7	10%	5%	5%	15%	10%	18%	12%	15%	10%	100%
8	10%	5%	5%	15%	10%	20%	10%	15%	10%	100%
9	8%	10%	10%	10%	7%	20%	10%	10%	15%	100%
SUM	100%	100%	100%	100%	100%	100%	100%	100%	100%	-

The trip distribution presented above is only applicable to development trips within the City. The developments located outside the City, within the MD of Bonnyville, were distributed using the trip distribution presented in Table 4.8.

Table 4.8

Trip Distribution Table (Outside City Limits)

Development	Inbound Trips	Outbound Trips
Hills of Cold	20% from Cold Lake North business area	5% to Cold Lake North business area
Lake	20% from Tri-City Mall area	25% to Tri-City Mall area
	20% from Cold Lake South business area	25% to Cold Lake South business area
	20% from the commercial area in the south	40% to the commercial area in the south
	(near 43 Avenue)	(near 43 Avenue)
	20% from Medley	5% to Medley
Fawn Ridge	20% from Cold Lake North business area	5% to Cold Lake North business area
Estates	20% from Tri-City Mall area	25% to Tri-City Mall area
	20% from Cold Lake South business area	25% to Cold Lake South business area
	20% from the commercial area in the south	40% to the commercial area in the south
	(near 43 Avenue)	(near 43 Avenue)
	20% from Medley	5% to Medley
IDP	25% Internal	30% Internal
Residential 1	15% from Cold Lake North business area	5% to Cold Lake North business area
IDP	15% from Tri-City Mall area	20% to Tri-City Mall area
Residential 2	15% from Cold Lake South business area	20% to Cold Lake South business area
IDP	15% from the commercial area in the south	20% to the commercial area in the south
Residential 3	(near 43 Avenue)	(near 43 Avenue)
	15% from Medley	5% to Medley
IDP Industrial	25% Internal	25% Internal
	25% Cold Lake North residential	25% Cold Lake North residential
	10% Residential behind Tri-City Mall	10% Residential behind Tri-City Mall
	35% Cold Lake South residential	35% Cold Lake South residential
	5% Medley	5% Medley
IDP	25% Internal	25% Internal
Commercial	25% Cold Lake North residential	25% Cold Lake North residential
	10% Residential behind Tri-City Mall	10% Residential behind Tri-City Mall
	35% Cold Lake South residential	35% Cold Lake South residential
	5% Medley	5% Medley

#### 4.4 TRIP ASSIGNMENT

The development trips were assigned onto the future road network by considering the logical routes that would be taken by the commuters between the origin and destinations, on the basis of impedance and travel time. To capture worst-case traffic scenarios, the development trips were primarily assigned to the skeletal road network established for the planning horizon.

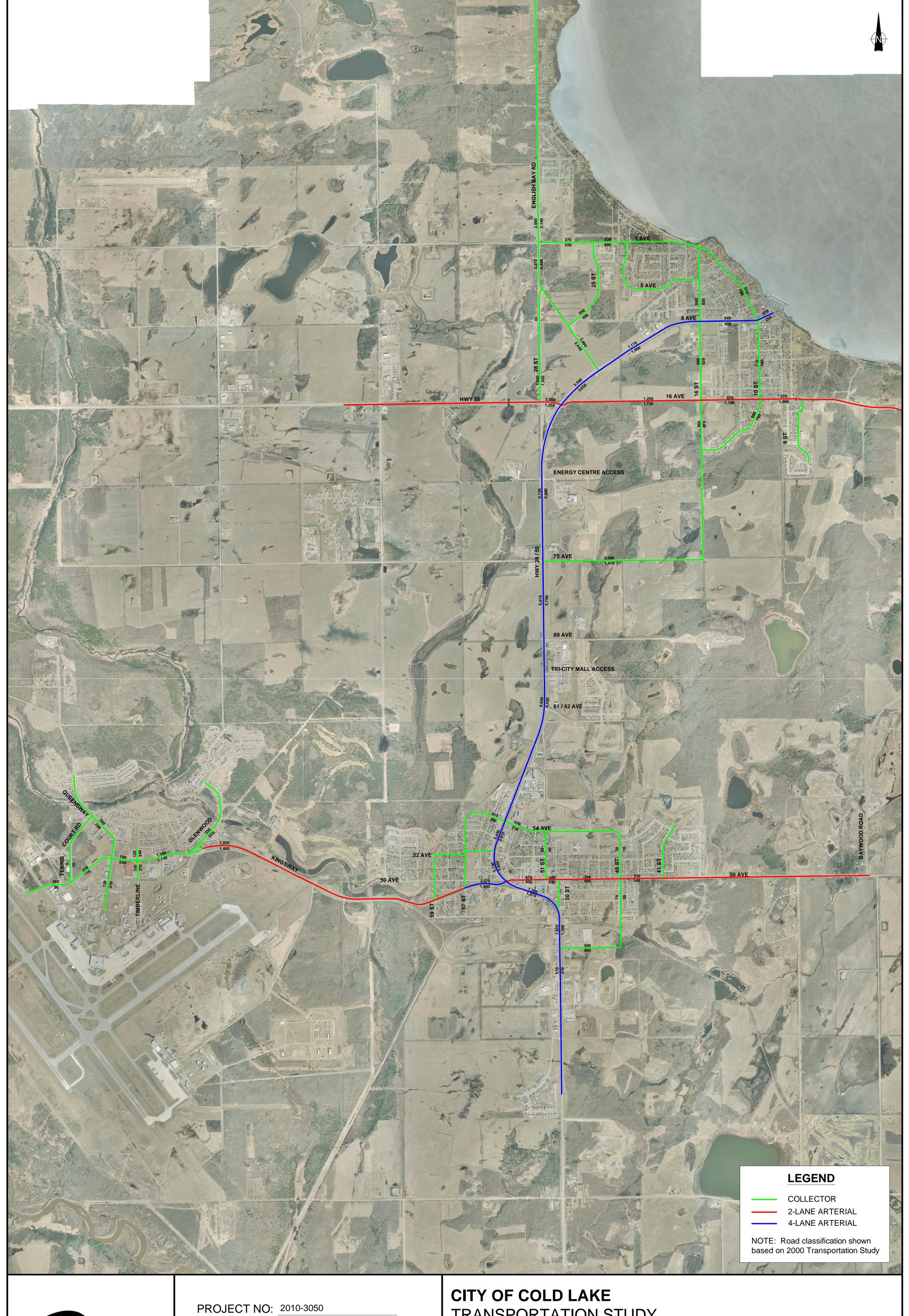
#### 4 - Traffic Forecast: Future Development Traffic Volumes

To simplify the trip assignment process, selected intersections were used to represent each study zone and development trips were assumed to enter/exit the zone from those intersections. As a result, some roadways within the skeletal road network do not appear to have background or development traffic assigned to it. The traffic volumes on these roadways were forecasted using growth patterns established from the other roadways.

#### 4.5 FORECASTED DEVELOPMENT TRAFFIC VOLUMES

Figure 4.7 presents the forecasted daily development traffic volumes for the 5-year (2015) Horizon. Figure 4.8 presents the forecasted daily development traffic volumes for the 10-year (2020) Horizon. Figure 4.9 presents the forecasted daily development traffic volumes for the 15-year (2025) Horizon. Figure 4.10 presents the forecasted daily development traffic volumes for the 20-year (2030) Horizon.





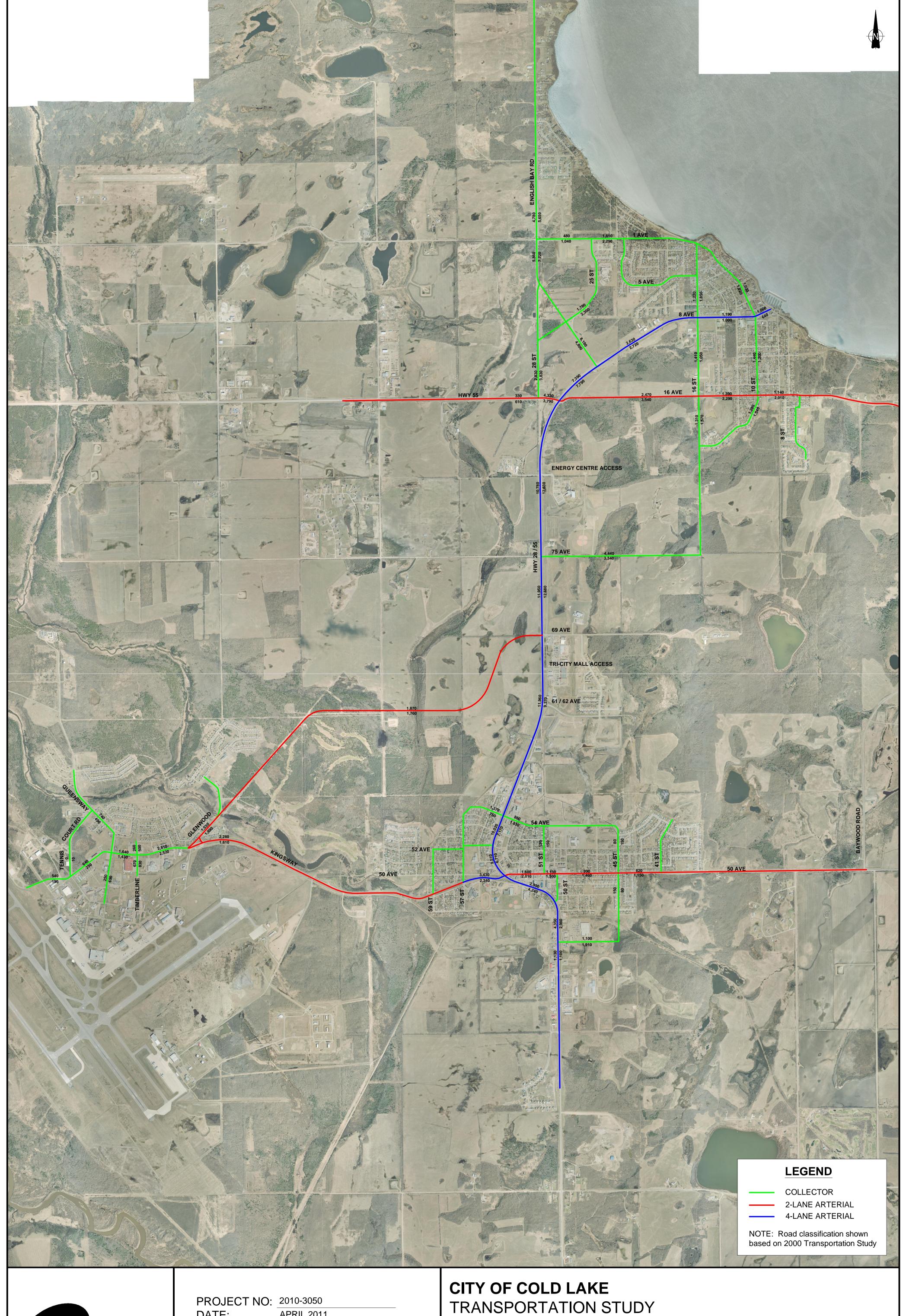


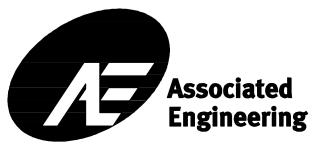
DATE: **APRIL 2011** 

APPROVED: NTS SCALE: DWG NO:

TRANSPORTATION STUDY

FIGURE 4.7 5 YEAR (2015) DAILY DEVELOPMENT TRAFFIC VOLUMES

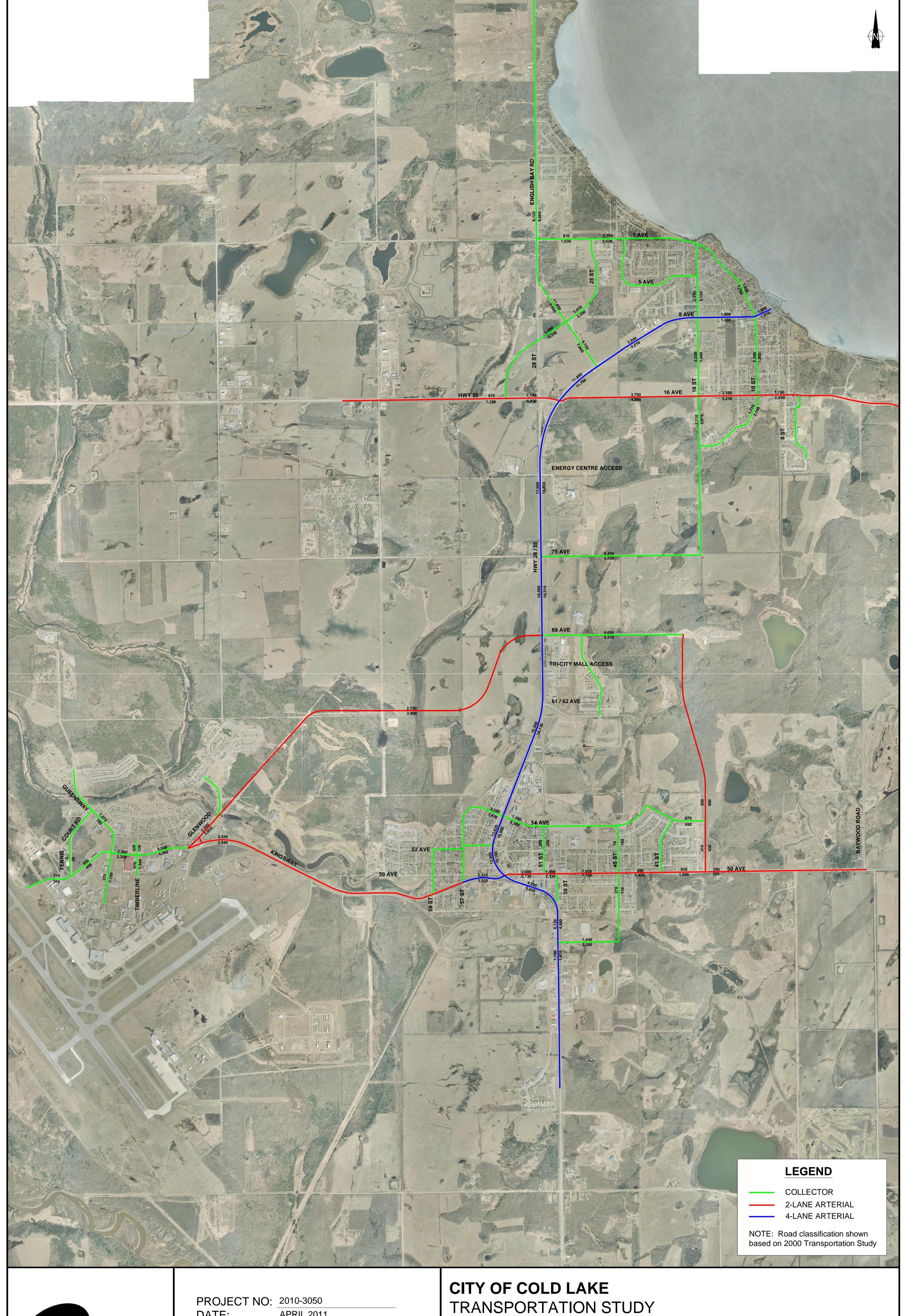




DATE: **APRIL 2011** APPROVED:

NTS SCALE: DWG NO:

FIGURE 4.8 10 YEAR (2020) DAILY DEVELOPMENT TRAFFIC VOLUMES



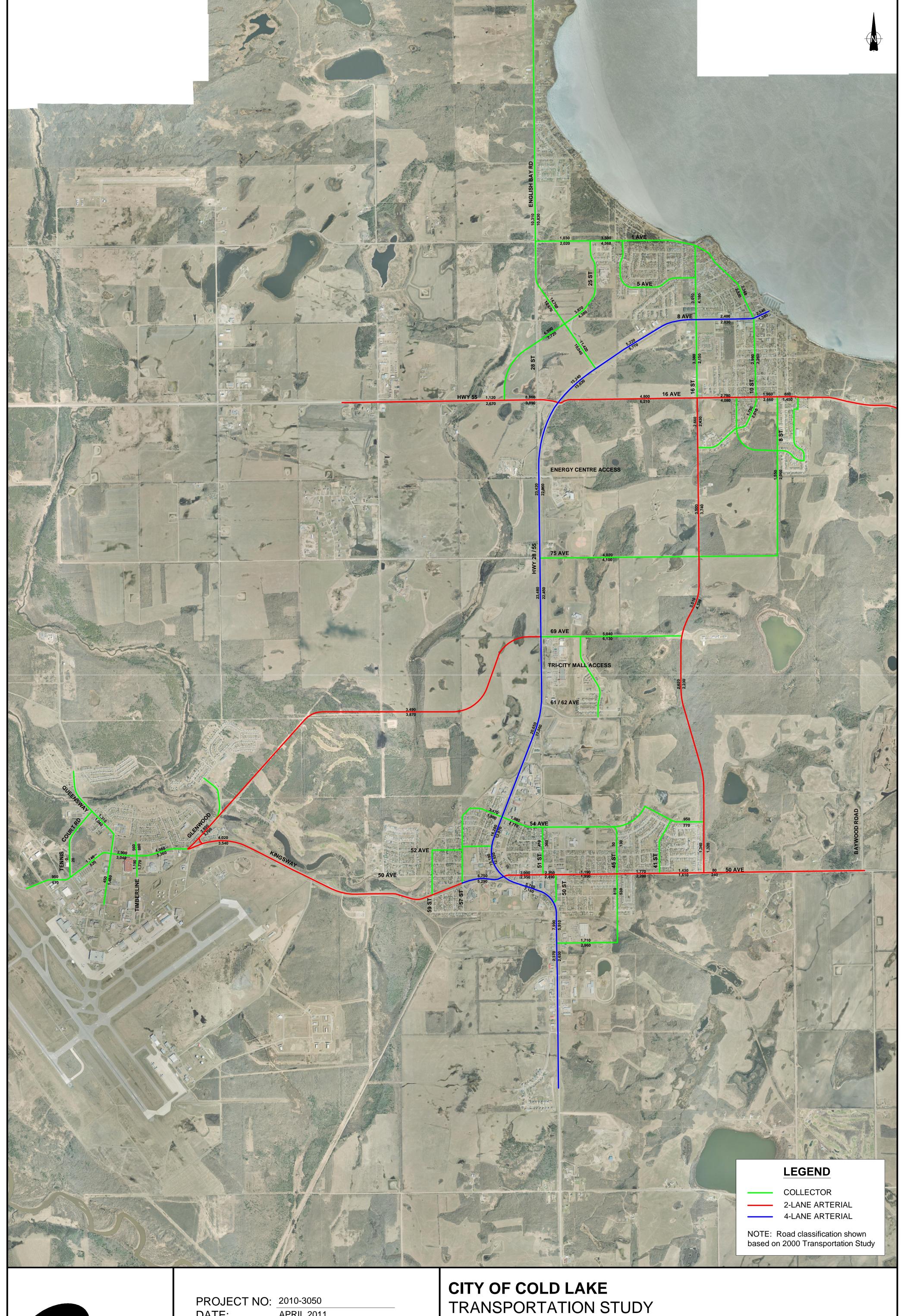


DATE: **APRIL 2011** APPROVED: SCALE:

DWG NO:

NTS

FIGURE 4.9 15 YEAR (2025) DAILY DEVELOPMENT TRAFFIC VOLUMES





DATE: **APRIL 2011** APPROVED: NTS SCALE:

DWG NO:

FIGURE 4.10 20 YEAR (2030) DAILY DEVELOPMENT TRAFFIC VOLUMES 5

# **Traffic Forecast: Total Traffic Volumes**

Total traffic volumes were calculated by combining the background traffic volumes with the development traffic volumes for common planning horizons. The average traffic growth for each road classification established in the 2000 Transportation Study (collectors, two-lane arterials, and four-lane arterials) was used to forecast future traffic volumes on the roadways which were not included in the trip assignment. The total traffic volumes for each planning horizon are presented in the following section.

#### 5.1 FORECASTED TOTAL TRAFFIC VOLUMES

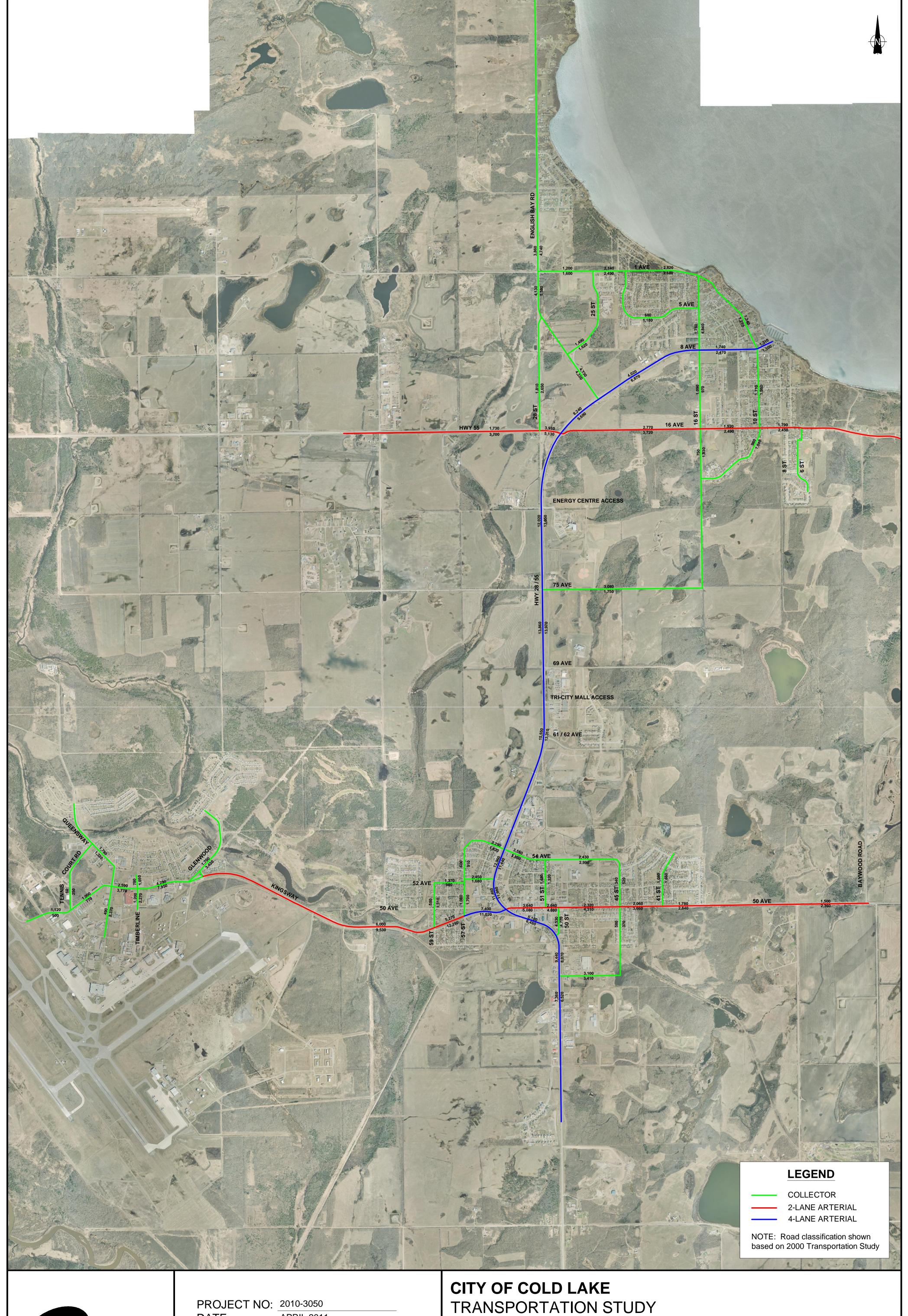
Figure 5.1 presents the forecasted daily total traffic volumes for the 5-year (2015) Horizon.

Figure 5.2 presents the forecasted daily total traffic volumes for the 10-year (2020) Horizon.

Figure 5.3 presents the forecasted daily total traffic volumes for the 15-year (2025) Horizon.

Figure 5.4 presents the forecasted daily total traffic volumes for the 20-year (2030) Horizon



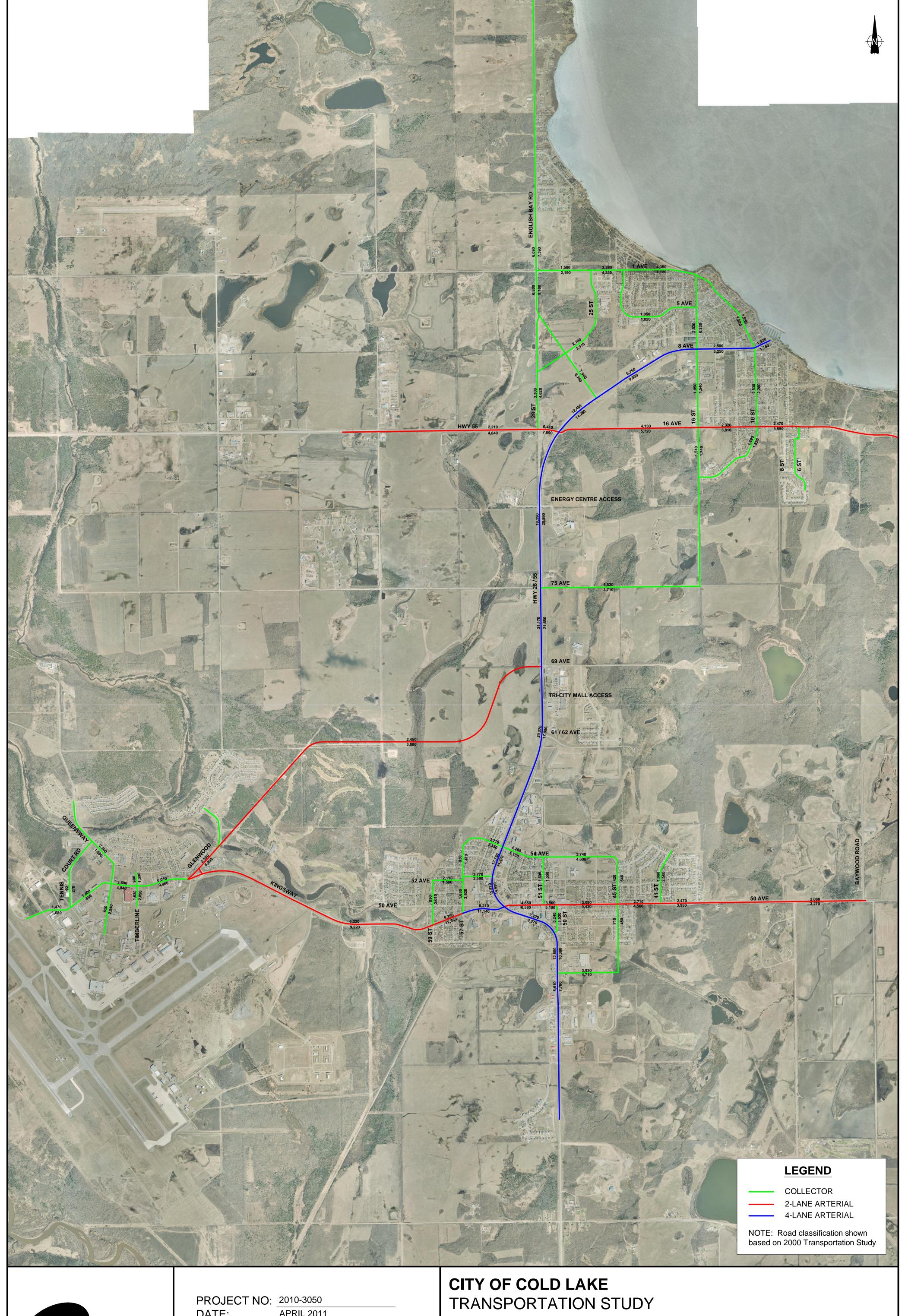




PROJECT NO: 2010-3050
DATE: APRIL 2011
APPROVED:

APPROVED:
SCALE:
NTS
DWG NO:

FIGURE 5.1 5 YEAR (2015) DAILY TOTAL TRAFFIC VOLUMES

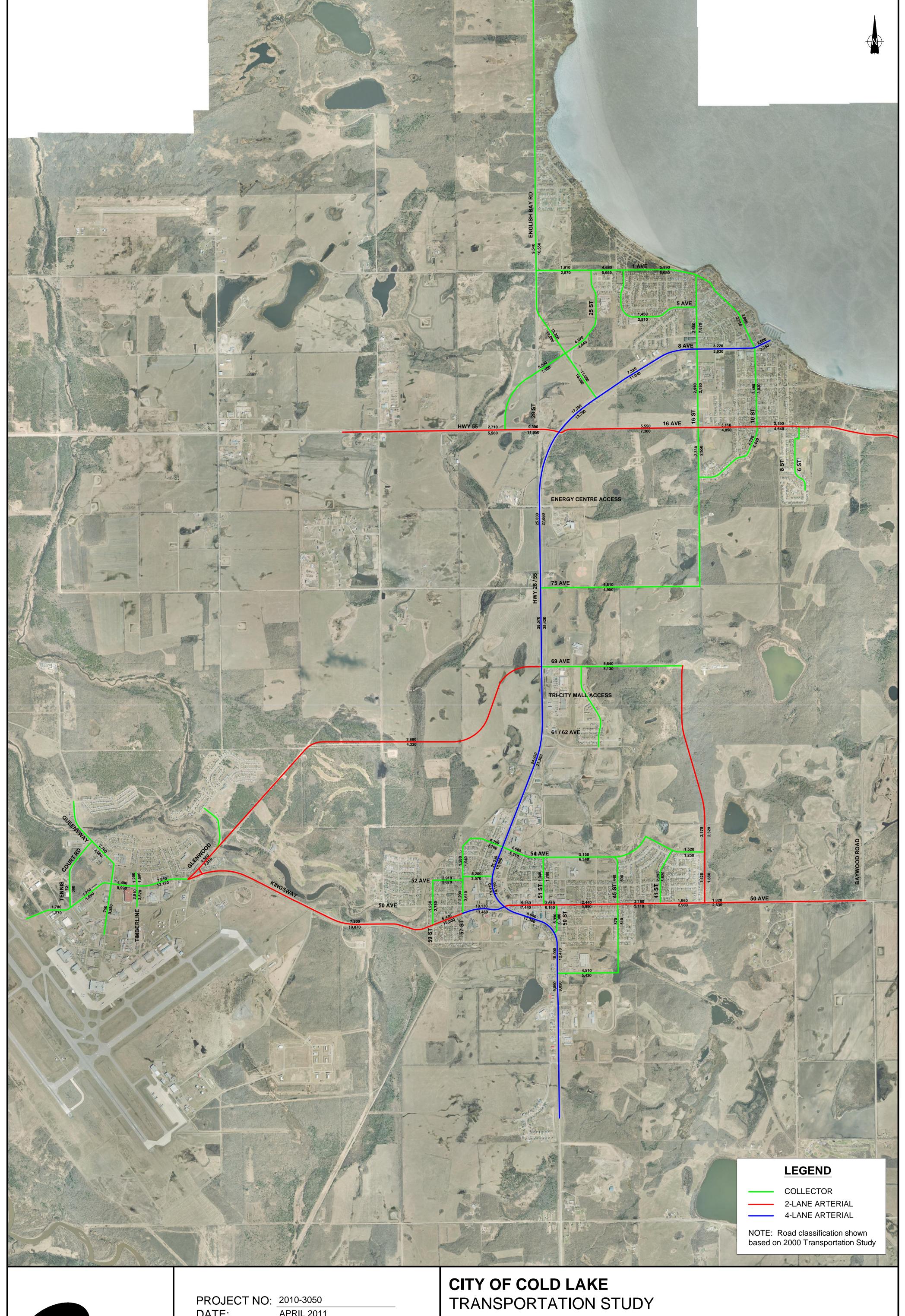




DATE: **APRIL 2011** APPROVED: NTS

SCALE: DWG NO:

FIGURE 5.2 10 YEAR (2020) DAILY TOTAL TRAFFIC VOLUMES

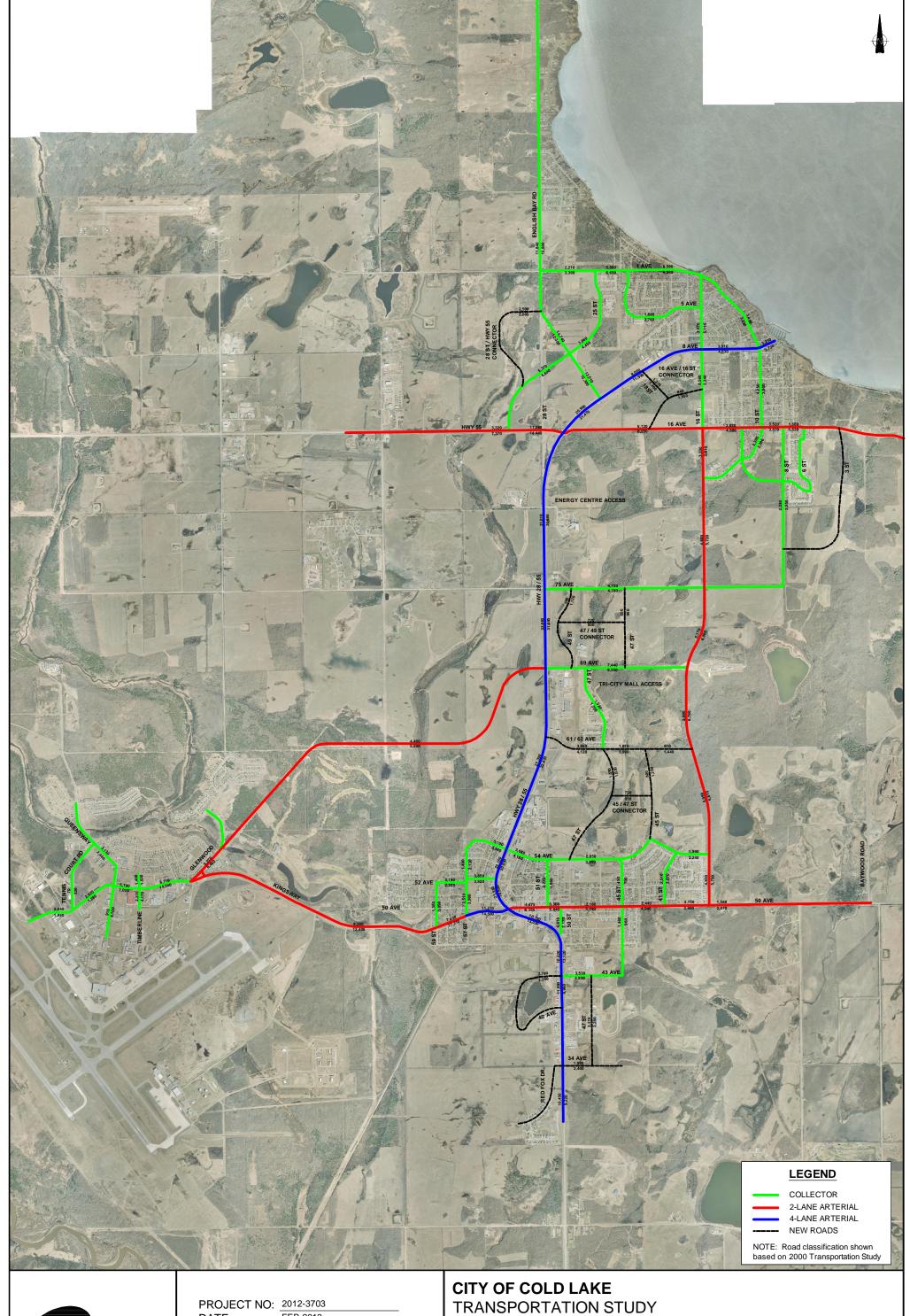




DATE: **APRIL 2011** APPROVED:

NTS SCALE: DWG NO:

FIGURE 5.3 15 YEAR (2025) DAILY TOTAL TRAFFIC VOLUMES





PROJECT NO: 2012-3703

DATE: FEB 2013

APPROVED: NTS

DWG NO: NTS

FIGURE 5.4 (REVISED) 20 YEAR (2030) DAILY TOTAL TRAFFIC VOLUMES



# **Roadway Requirements**

#### 6.1 METHODOLOGY

Table 6.1 presents the City's roadway design standards obtained from the Municipal Engineering Servicing Standards And Standard Construction Specifications (2008). The table presents the City's roadway designation/classification and the daily service volumes for each roadway classification.

Table 6-1
City of Cold Lake – Roadway Classification and Daily Service Volumes

Roadway Designation	Daily Service Volume (vpd)	Daily Service Volume Range (vpd)
Urban Expressway	>30,000	>30,000
Divided Arterial	>20,000	20,000 - 30,000
Undivided Arterial	<20,000	10,000 - 20,000
Divided Residential Collector	<10,000	3,000 - 10,000
Undivided Residential Collector	<10,000	3,000 - 10,000
Divided Residential Local	<3,000	500 - 3,000
11m Undivided Residential Local	<3,000	500 - 3,000
10m Undivided Residential Local	<3,000	500 - 3,000
Rural Industrial Collector	<10,000	3,000 - 10,000
Urban Industrial Collector	<10,000	3,000 - 10,000
Rural Industrial Local	<3,000	500 - 3,000
Urban Industrial Local	<3,000	500 - 3,000
Frontage (Service) Road	<3,000	500 - 3,000
Lanes	<500	<500

Table 6.2 presents typical lane capacities, in vehicles per hour, for various road classifications.



Table 6-2
Lane Capacity by Road Classification

Road Classification	City of Cold Lake Road Classification	Capacity (vehicles per hour, per lane)	Capacity (vehicles per day, per lane)
Provincial Controlled Access Highway	Expressway	1,800	18,000
County Arterial Road	Divided Arterial	1,000	10,000
Local Major and Minor Arterial Roads	Undivided Arterial	800	8,000
Local Collector Road	Collector (Residential or Industrial)	400	4,000
Local Road (Other)	Local (Residential or Industrial)	100	1,000

NOTE: Capacities are generalized based on typical engineering design standards.

Lane capacity per day based on assumption that peak hour traffic volumes are 10% of daily traffic volumes.

The forecasted total traffic volumes for each planning horizon were compared with the daily service volumes provided in Table 6.1 to determine the required roadway classification, as per the City's standards. The lane volumes were also compared with the lane capacity for the given road classification provided in Table 6.2, to determine the number of lanes required along each roadway.

#### 6.2 RESULTS

The required road classification and number of lanes for the future road network are summarized and provided in Appendix D for each planning horizon. The results presented in the appendix were determined from the forecasted traffic volumes and does not account for continuous roadway functionality and lane balancing along a single corridor.

As mentioned, the road classification and number of lanes required to accommodate traffic under the 20-year (2030) horizon will be used by the City to retain the right-of-way required to accommodate future roadway expansion. All the major corridors under the 20-year (2030) horizon were reviewed independently to establish continuous roadway function and lane balance along the corridor, where possible.

Figure 6.1 presents the recommended road classification and number of lanes, for the 20-year (2030) horizon.

According to the preliminary analysis completed for the 20-year planning horizon, 1 Avenue (25 Street to 16 Street) should be classified as a two-lane undivided arterial roadway. However, in order to maintain continuous roadway functionality, AE recommends that the road segment be classified as a collector roadway. This portion of 1 Avenue is adjacent to Kinosoo Beach, a major tourist attraction in Cold Lake North, and has been identified in the In-Service Road Safety Review technical memorandum, as an ideal location to implement traffic calming and beautification measures. A collector road classification would better suit the functionality of the area. Table 6.3 summarizes the road corridors in the 20-year planning horizon, along with the recommended road classification, number of lanes, and expected capacities. Table 6.4 summarizes the major road network in the 20-year planning horizon, along with a comparison of the existing and future road classification and number of lanes. The table also summarizes the improvements required to upgrade the corridors from the existing horizon to the 20-year planning horizon.



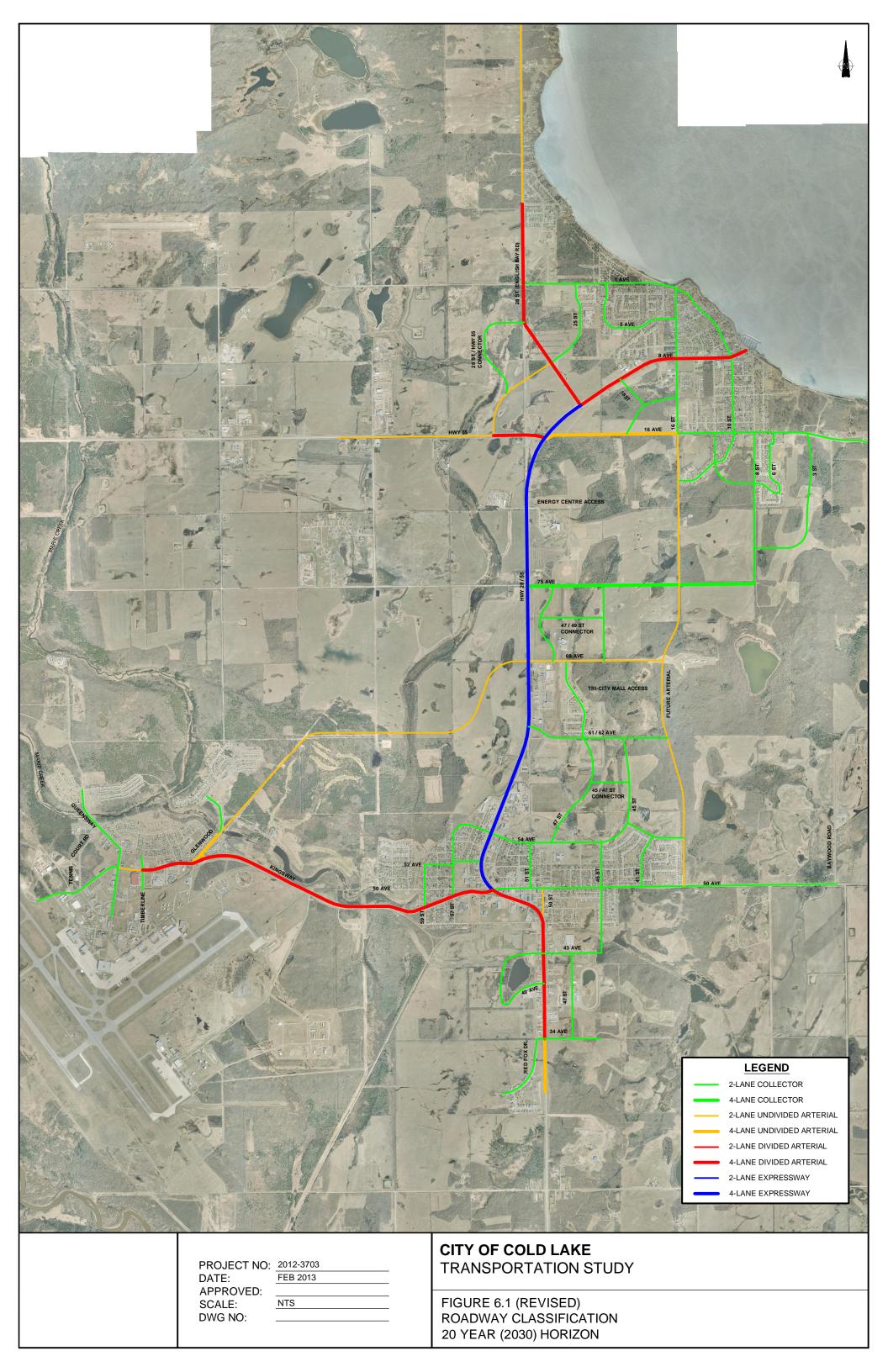


TABLE 6.3: CAPACITY ANALYSIS - 20 YEAR (2030) HORIZON - ROAD CLASSIFICATION, NUMBER OF LANES AND CAPACITIES

									Lane Capacity	Lane Capacity	Number of Lanes
	Corridor			Direction					for Road Classification	for Road Classification	Required (One
Tomos	1 Avenue						Collector				1
Column											
Prop				Westbound	5,560					,	
Prop   Program				Westbound	6,580	** *				-,	
Victor   V	Hwy 28	· ·		Southbound	20,900	42,370					2
	8 Avenue	25 Street	16 Street	Westbound	8,060	20,050	4-Lane Arterial	Divided Arterial	1,000	10,000	1
March   Marc	8 Avenue	16 Street	10 Street	Westbound	3,910	8,540	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	
March   Marc	8 Avenue	10 Street	Lakeshore Drive	Westbound	3,210	5,930	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	
Michael   Mich	Hwy 55	West City Limit	25 Street	Westbound	3,320	10,690	2-Lane Arterial	Undivided Arterial	800	8,000	1
1.	Hwy 55	25 Street	Hwy 28			25,720	2-Lane Arterial	Divided Arterial	1,000	10,000	
M. Service   19 Stand	16 Avenue	Hwy 28	16 Street			14,370	2-Lane Arterial	Undivided Arterial	800	8,000	
15   Security   15   Securit	16 Avenue <sup>5</sup>	16 Street	10 Street	Eastbound	4,880	8,700	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	
State   Stat	16 Avenue	10 Street	8 Street	Eastbound	3,670	7,190	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
	16 Avenue	8 Street	East City Limit	Eastbound	1,530	3,190	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
Seption   Design	English Bay Road <sup>6</sup>	North City Limit	Lake Avenue	Northbound	6,430	12,350	Collector	Undivided Arterial	800	8,000	
Popular Beynolds	* '		1 Avenue	Northbound	12,860	24.700	Collector	Divided Arterial	1,000	10.000	
Company   Comp				Northbound	14,740						
2   Street   Company   C											2
15 Stant   1-	- '				12,300 6,690				,	-,	
Secure   Content											
Select Concessor   Opt				Southbound	3,960						1
10   10   10   10   10   10   10   10	Street Connector			Westbound	2,080					,	1
18 Based   Punnes   P	Nelson Street			Westbound	1,600	,					1
1. Billion   1.	16 Street <sup>8</sup>	1 Avenue	8 Avenue	Southbound	3,570	8,680	Collector	Collector (Residential or Industrial)	400	4,000	1
1.	16 Street	8 Avenue	16 Avenue	Southbound	3,580	6,820	Collector	Collector (Residential or Industrial)	400	4,000	1
Concession   Survey		16 Avenue	75 Avenue	Southbound	3,200	7,010	Collector	Collector (Residential or Industrial)	400	4,000	1
Comment   15 Annual   15 Street   Machinery   15 Annual   15 Street   15 Annual   15 Ann	Connector	8 Avenue	16 Avenue		1,490	2,760	-	Local (Residential or Industrial)	150	1,500	
1		16 Avenue	16 Street			2,390	-	Local (Residential or Industrial)	150	1,500	
10 Servel   16 Annua   15 Annua		1 Avenue	8 Avenue			6,720	Collector	Collector (Residential or Industrial)	400	4,000	
10 Street   16 Annual   16 Street   17 Annual   27 Annual   2,200   4,700   Calector   Calector Residented in Industrial)   400   4,000   1	10 Street <sup>9</sup>	8 Avenue	16 Avenue	Northbound		7,810	Collector	Collector (Residential or Industrial)	400	4,000	•
8 Breef   19 Annum   75 Annum   25 Annum   35500   4.780   Calector   Calector   Calector (Residented or Inclusions)   450   4.500   1	10 Street	16 Avenue	16 Street	Northbound	2,380	4,680	Collector	Collector (Residential or Industrial)	400	4,000	1
6 State     16 American   24 American   2-2800   4,880   Collector   Collector (Residential or Industrial)   400   4,000   1	8 Street	16 Avenue	75 Avenue	Northbound	2,550	4,780	Collector	Collector (Residential or Industrial)	400	4,000	1
Hely 2855	6 Street 10	16 Avenue	21 Avenue	Northbound	2,380	4,680	Collector	Collector (Residential or Industrial)	400	4,000	1
Normal   Personal		Hwy 55/16 Avenue	75 Avenue	Northbound	30,680	62,490	4-Lane Arterial	Expressway	1.800	18.000	
Normal   Hay 2865   Futus Andread   22,000   50,210   4-Lane Andread   Egressway   1,800   18,000   2   2   2   2   2   2   2   2   2	•		69 Avenue	Northbound	31,690	65 550	4-I ane Arterial				2 2
Seminor   14					22,820						2 2
69 Avenue	,										
On Annual   Hay 2855   Faura Artifal   Eatliboard   4.480   15,700   Collector   Undivided Anterial   800   8,000   2   1   1   1   1   1   1   1   1   1		,				-, -				,	
49 Street   77 Avenue				Westbound	4,480						
47 Street   75 Avenue				Westbound	7,440					,	
# Street   61 Avenue   62 Avenue   63 Avenue   64 Avenue   64 Avenue   65 Aven	49 Street	75 Avenue	69 Avenue	Southbound	680	1,800	Collector	Local (Residential or Industrial)	150	1,500	
A Street   A Street		75 Avenue	69 Avenue	Southbound	600	1,560	Collector	Local (Residential or Industrial)	150	1,500	1
# 7 Street   61 Avenue®		47 Street		Westbound	720	1,670	-	Local (Residential or Industrial)	150	1,500	1
All Street   Street	47 Street	69 Avenue		Southbound	1,750	3,270	Collector	Collector (Residential or Industrial)	400	4,000	1
State   Manual   State   Manual   State   Manual   State   Manual   State   Manual   State   Manual   State   Manual   State	47 Street <sup>11</sup>	61 Avenue/62 Avenue	54 Avenue	Southbound	1,590	2,620	Collector	Local (Residential or Industrial)	150	1,500	1
## Annune@2 Avenue ## Af Street ## Street ## Street ## Eastbound ## 1.810 ## Annune@2 Avenue ## Af Street ## Future Anterial ## Eastbound ## 1.810	61 Avenue/62 Avenue <sup>12</sup>	Hwy 28/55	47 Street	Eastbound		8,000	Collector	Collector (Residential or Industrial)	400	4,000	1
## Street ## Str	61 Avenue/62 Avenue	47 Street	45 Street	Eastbound	3,990	5,800	Collector	Collector (Residential or Industrial)	400	4,000	1
47 Street	61 Avenue/62 Avenue	45 Street	Future Arterial	Eastbound	1,440	2,350	Collector	Collector (Residential or Industrial)	400	4,000	
45 Street   61 Avenue <sup>62</sup>   Avenue   54 Avenue   54 Avenue   54 Avenue   56 Street   Hwy 28/55   Eastbound   800   2,080   -   Local (Residential or Industrial)   150   1,500   1		47 Street	45 Street	Eastbound	950	1,670	-	Local (Residential or Industrial)	150	1,500	1
SourceOuth   Sou		61 Avenue/62 Avenue	54 Avenue	Northbound	1,280	2,080	-	Local (Residential or Industrial)	150	1,500	1
Standard   Standard   Street   Street   Street   Street   Standard   Street   Standard				Eastbound	3,380		Collector				1
Standard   Street				Eastbound	4,160						1
State				Eastbound	3,950						1
Section				Westbound Eastbound	2,850 3,095	-,		,		,	1
Section   Sect											
Section   Sect				Westbound	1,990						1
Section   Sect				Westbound	3,190	-,				,	1
Centre Avenue   59 Street   57 Street   Westbound   7,430   24,440   2-Lane Anterial   Divided Anterial   1,000   10,000   1			,	Westbound	3,850						i
Centre Avenue   57 Street   Filtre Avenue   1   Street   Filtre Avenue   1   Street   1   Stre	Centre Avenue	59 Street	57 Street	Westbound	7,430	24,740	2-Lane Arterial	Divided Arterial	1,000	10,000	1
50 Avenue	Centre Avenue	57 Street	Hwy 28/55	Westbound	11,410	25,910	4-Lane Arterial	Divided Arterial	1,000	10,000	2
50 Avenue	50 Avenue <sup>16</sup>	Hwy 28/55	51 Street	Westbound	4,475	10,830	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue <sup>19</sup> 50 Street 45 Street Eastbound 5,290 7,470 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue <sup>19</sup> 45 Street 41 Street Eastbound 2,180 7,470 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue <sup>19</sup> 45 Street 41 Street Westbound 2,440 7,380 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue <sup>19</sup> 45 Street 41 Street Estitude Arterial South 2,440 7,380 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue <sup>19</sup> 45 Street 41 Street Estitude Arterial 3,900 6,650 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue <sup>19</sup> 45 Street 41 Street Estitude Arterial 3,900 6,650 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue <sup>19</sup> 45 Street 41 Street Estitude Arterial 2,900 7,380 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue <sup>19</sup> 45 Street 41 Street Estitude Arterial 2,900 7,380 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue <sup>19</sup> 45 Street 41 Street Estitude Arterial 2,900 7,380 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue <sup>19</sup> 45 Street 41 Street Estitude Arterial 2,900 7,380 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue <sup>19</sup> 45 Street 41 Street Estitude Arterial 2,900 7,380 2-Lane Arterial 2,	50 Avenue <sup>17</sup>	51 Street	50 Street	Eastbound Westbound	5,440 3,300	8,740	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue <sup>19</sup> 45 Street 41 Street Eastbound 4,940 7,380 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue 41 Street Future Arterial 3,900 6,650 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue 41 Street Future Arterial 3,900 6,650 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1	50 Avenue <sup>18</sup>	50 Street	45 Street	Eastbound	5,290	7,470	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	
50 Avenue 41 Street Future Arterial Eastbound 3,900 6,650 2,Lane Arterial Collector (Recidential or Industrial) 400 4,000 1	50 Avenue <sup>19</sup>	45 Street	41 Street	Eastbound	4,940	7,380	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
Westbound 2,750 Vertuel Vestbound 2,750 Vertuel Vestbound 1	50 Avenue	41 Street	Future Arterial	Eastbound	3,900	6,650	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1

TABLE 6.3: CAPACITY ANALYSIS - 20 YEAR (2030) HORIZON - ROAD CLASSIFICATION, NUMBER OF LANES AND CAPACITIES

50 Avenue 43 Avenue 59 Street 57 Street	From Future Arterial Hwy 28/55	To Baywood Road	- Direction Eastbound	Daily Traffic - Directional	Daily Traffic -	Road Classification 2000 TPS <sup>1</sup>	Road Classification City of Cold Lake <sup>2</sup>	for Road Classification (veh/hour/lane) 3	for Road Classification (veh/day/lane) 4	Required (One Direction)
43 Avenue <sup>20</sup> 59 Street 57 Street		Baywood Road	Fastbound					(veri/flour/faile)	(veri/day/larie)	Directions
59 Street 57 Street	Hwy 28/55			2,070 1,560	3,630	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
57 Street		45 Street	Westbound Eastbound	4,830 3,530	8,360	Collector	Collector (Residential or Industrial)	400	4,000	1
	52 Avenue	Centre Avenue	Westbound Northbound	3,050	4.400	Collector	Collector (Residential or Industrial)	400	4.000	1
	54 Avenue	52 Avenue	Southbound Northbound	1,350 2,130	3,530	Collector	Collector (Residential or Industrial)	400	4,000	1
	52 Avenue	Centre Avenue	Southbound Northbound	1,400 3,960	6.470	Collector	Collector (Residential or Industrial)	400	4,000	1
57 Street			Southbound Northbound	2,510 20,930	43.080			1.800	18.000	1 2
Hwy 28/55	54 Avenue	52 Avenue	Southbound Northbound	22,150 18,500	.,	4-Lane Arterial	Expressway	,	.,	2
Hwy 28/55	52 Avenue	50 Avenue	Southbound Northbound	21,950 10,700	40,450	4-Lane Arterial	Expressway	1,800	18,000	2
Hwy 28/55	50 Avenue	50 Street	Southbound Northbound	13,030 13,120	23,730	4-Lane Arterial	Divided Arterial	1,000	10,000	2
Hwy 28/55	50 Street	43 Avenue	Southbound	16,420	29,540	4-Lane Arterial	Divided Arterial	1,000	10,000	2
Hwy 28/55	43 Avenue	34 Avenue	Northbound Southbound	9,960 11,690	21,650	4-Lane Arterial	Divided Arterial	1,000	10,000	2
Hwy 28/55	34 Avenue	South City Limit	Northbound Southbound	9,220 10,450	19,670	4-Lane Arterial	Undivided Arterial	800	8,000	2
40 Avenue	43 Avenue	Hwy 28/55	Eastbound Westbound	2,260 2,790	5,050	-	Collector (Residential or Industrial)	400	4,000	1 1
34 Avenue	Hwy 28/55	47 Street	Eastbound Westbound	2,400 1,990	4,390	-	Collector (Residential or Industrial)	400	4,000	1
47 Street	43 Avenue	34 Avenue	Northbound Southbound	2,280 2,210	4,490	-	Collector (Residential or Industrial)	400	4,000	1
51 Street	54 Avenue	50 Avenue	Northbound Southbound	1,980 2,230	4,210	Collector	Collector (Residential or Industrial)	400	4,000	1
50 Street	50 Avenue	Hwy 28/55	Northbound Southbound	7,170 6,810	13,980	Collector	Undivided Arterial	800	8,000	1
45 Street	54 Avenue	50 Avenue	Northbound	700 450	1,150	Collector	Local (Residential or Industrial)	150	1,500	1
45 Street	50 Avenue	43 Avenue	Southbound Northbound	960	2,420	Collector	Local (Residential or Industrial)	150	1,500	1
41 Street	54 Avenue	50 Avenue	Southbound Northbound	1,460 3,870	6.380	Collector	Collector (Residential or Industrial)	400	4.000	1
Future Arterial	75 Avenue	69 Avenue	Southbound Northbound	2,510 6,900	12,070	2-Lane Arterial	Undivided Arterial	800	8.000	1
Future Arterial	69 Avenue	61 Avenue/62	Southbound Northbound	5,170 6,760	12,660	2-Lane Arterial	Undivided Arterial	800	8,000	1
		Avenue	Southbound Northbound	5,900 6,810	,				-,	1
Future Arterial	61 Avenue/62 Avenue	54 Avenue	Southbound Northbound	5,990 5,730	12,800	2-Lane Arterial	Undivided Arterial	800	8,000	1
Future Arterial	54 Avenue	50 Avenue	Southbound Eastbound	4,620 12,400	10,350	2-Lane Arterial	Undivided Arterial	800	8,000	1 2
Kingsway	59 Street	Glenwood	Westbound Fastbound	8,220 14,040	20,620	2-Lane Arterial	Divided Arterial	1,000	10,000	1 2
Kingsway	Timberline	Glenwood	Westbound	8,710	22,750	Collector	Divided Arterial	1,000	10,000	1
Kingsway	Queensway	Timberline	Eastbound Westbound	7,020 5,190	12,210	Collector	Undivided Arterial	800	8,000	1
Kingsway	Tennis Court Road	Queensway	Eastbound Westbound	1,280 2,020	3,300	Collector	Collector (Residential or Industrial)	400	4,000	1
Kingsway	End of Road	Tennis Court Road	Eastbound Westbound	1,440 2,040	3,480	Collector	Collector (Residential or Industrial)	400	4,000	1
Tennis Court Road	Queensway	Kingsway	Northbound Southbound	330 70	400	Collector	Lane	N/A	N/A	N/A N/A
Queensway	Tennis Court Road	Kingsway	Northbound Southbound	3,130 2,440	5,570	Collector	Collector (Residential or Industrial)	400	4,000	1
Queensway	Kingsway	Hanger Ln	Northbound Southbound	4,100 910	5,010	Collector	Collector (Residential or Industrial)	400	4,000	2
Timberline	Juniper Avenue	Kingsway	Northbound Southbound	1,920	3,320	Collector	Collector (Residential or Industrial)	400	4,000	1
Timberline	Kingsway	Athabasca Road	Northbound	4,170	6,480	Collector	Collector (Residential or Industrial)	400	4,000	2
Glenwood Drive	Glenwood	Kingsway	Northbound Southbound	2,310 8,460 5,270	13.730	2-Lane Arterial	Undivided Arterial	800	8,000	2
Road classification based on 2001 Transportation Study Road classification based on 2002 Service Volumes stpulated in City's Roadway Design Standards (Municipal Engineering Servicing Standards and Standard Construction Specifications, Jan 2008) Based on Lane Capacity Table (lattached). Using road classification according to City's standards.  A 2-lane collector cross section would be appropriate for 16 Avenue between 16 Street and 10 Street as the AADT is less than 9000.  A 2-lane collector cross section would be appropriate for 16 Fixer between 1 Avenue and APADT is less than 9000.  A 2-lane collector cross section would be appropriate for 16 Street between 1 Avenue and APADT is less than 9000.  A 2-lane collector cross section would be appropriate for 16 Street between 1 Avenue and APADT is less than 9000  A 2-lane collector cross section would be appropriate for 16 Street between 1 Avenue and APADT is less than 9000  A 2-lane collector cross section would be appropriate for 16 Street between 14 Avenue as the AADT is less than 9000  A 2-lane collector cross section would be appropriate for 18 Street between 16 Avenue as the AADT is less than 9000  A 2-lane collector cross section would be appropriate for 18 Street between 62 Avenue and 16 Street)  1. A 2-lane collector cross section would be appropriate for 47 Street between 62 Avenue and 5 Street and 45 Street an										

TABLE 6.4: COMPARISON OF EXISTING AND 20-YEAR ROAD NETWORK

Corridor	Interse	ection	Existing (2010) Road	Existing (2010) Number of	Recommended 20-Year (2030) Road	Recommended 20-Year (2030) Number of Lanes	Improvements Required
	From	То	Classification <sup>1</sup>	Lanes (One Direction)	Classification	(One Direction)	
8 Avenue	10 Street	Lakeshore Drive	Undivided Arterial	1	Divided Arterial	2 2	Widen to provide centre median and 2 travel lanes in each direction
8 Avenue	25 Street	10 Street	Divided Arterial	2 2	Divided Arterial	2 2	•
Hwy 28/55	Hwy 55/16 Avenue	53 Avenue	Divided Arterial	2 2	Expressway	2 2	•
Hwy 28/55	53 Avenue	52 Avenue	Undivided Arterial	2 2	Expressway	2 2	Widen to provide centre median
Hwy 28/55	52 Avenue	50 Avenue	Divided Arterial	2 2	Expressway	2 2	-
Hwy 28/55	50 Avenue	52 Street	Divided Arterial	2 2	Divided Arterial	2 2	-
Hwy 28/55	52 Street	47 Avenue	Undivided Arterial	2 2	Divided Arterial	2 2	Widen to provide centre median
Hwy 28/55	47 Avenue	40 Avenue	Divided Arterial	2 2	Divided Arterial	2 2	-
Hwy 28/55	40 Avenue	South City Limit	Undivided Arterial	1	Undivided Arterial	1	-
1 Avenue	28 Street	1 Avenue	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1	-
Hwy 55 <sup>2</sup>	West City Limit	28 Street	Collector (Residential or Industrial)	1	Undivided Arterial	1	-
Hwy 55	28 Street	Hwy 28	Collector (Residential or Industrial)	1	Divided Arterial	2 2	Build pavement structure to Arterial standard (centre median and 2 trave lanes in each direction)
16 Avenue	Hwy 28	16 Street	Collector (Residential or Industrial)	1	Undivided Arterial	2 2	Build pavement structure to Arterial standard (2 travel lanes in each direction)
16 Avenue	16 Street	8 Street	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	2 2	Widen to provide 2 travel lanes in each direction
16 Avenue	8 Street	East City Limit	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1	-
English Bay Road	North City Limit	Lake Avenue	Collector (Residential or Industrial)	1	Undivided Arterial	1 1	
English Bay Road	Lake Avenue	Hwy 28	Collector (Residential or Industrial)	1	Divided Arterial	2 2	Build pavement structure to Arterial standard (centre median and 2 trave lanes in each direction)
28 Street	English Bay Road	Hwy 55	Collector (Residential or Industrial)	1 1	Undivided Arterial	1 1	Realign 28 Street and build pavement structure to Arterial standard
25 Street	1 Avenue	English Bay Road	Collector (Residential or Industrial)	1	Undivided Arterial	1	
Nelson Street	1 Avenue	16 Street	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1	
16 Street	1 Avenue	16 Avenue	Collector (Residential or Industrial)	1	Undivided Arterial	1	
16 Street	16 Avenue	75 Avenue	Local	1 1	Undivided Arterial	1	Build pavement structure to Arterial standard
Future Arterial	75 Avenue	50 Avenue	Non-existant	- :	Undivided Arterial	1	Build out as per 20-year horizon
10 Street	1 Avenue	16 Avenue	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1	
10 Street	16 Avenue	16 Street	Local	1	Collector (Residential or Industrial)	1	
8 Street	16 Avenue	75 Avenue	Local	1	Collector (Residential or Industrial)	1	Build pavement structure to Collector standard
6 Street	16 Avenue	21 Avenue	Local	1	Collector (Residential or Industrial)	1	-
20 Avenue	12 Street	8 Street	Non-existant		Collector (Residential or Industrial)	1	Build out as per 20-year horizon
75 Avenue	Hwy 28/55	Future Arterial	Local	1 1	Collector (Residential or Industrial)	2	Build pavement structure to Collector standard (2 travel lanes in each direction)
69 Avenue	Glenwood	Hwy 28/55	Non-existant	-	Undivided Arterial	1	Build out as per 20-year horizon
69 Avenue	Hwy 28/55	Future Arterial	Local	1	Undivided Arterial	1	-
47 Street	69 Avenue	61/62 Avenue	Local	1	Collector (Residential or Industrial)	1	-
54 Avenue	56 Street	49 Street	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	2	Widen to provide 2 travel lanes in each direction
54 Avenue	49 Street	Future Arterial	Non-existant	-	Collector (Residential or Industrial)	2	Build out as per 20-year horizon
52 Avenue	59 Street	57 Street	Collector (Residential or Industrial)	1 1	Collector (Residential or Industrial)	1	
52 Avenue	57 Street	Hwy 28/55	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	2	Widen to provide 2 travel lanes in each direction
Centre Avenue	59 Street	57 Street	Undivided Arterial	1	Divided Arterial	2	Widen to provide centre median and 2 travel lanes in each direction
Centre Avenue	57 Street	Hwy 28/55	Undivided Arterial	1 2	Divided Arterial	2	Widen to provide centre median
50 Avenue	Hwy 28/55	Future Arterial	Undivided Arterial	1	Undivided Arterial	1	
50 Avenue	Future Arterial	Baywood Road	Undivided Arterial	1 1	Collector (Residential or Industrial)	1	
43 Avenue	Hwy 28/55	45 Street	Local	1	Collector (Residential or Industrial)	2	Build pavement structure to Collector standard (2 travel lanes in each
59 Street	52 Avenue	Centre Avenue	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	2	direction)
57 Street	54 Avenue	52 Avenue	Local	1	Collector (Residential or Industrial)	1	
57 Street	52 Avenue	Centre Avenue	Collector (Residential or Industrial)	1 1	Collector (Residential or Industrial)	1	
51 Street	54 Avenue	50 Avenue	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1	
50 Street	50 Avenue	Hwy 28/55	Collector (Residential or Industrial)  Collector (Residential or Industrial)	1 1	Undivided Arterial	1	
45 Street	54 Avenue	50 Avenue	Collector (Residential or Industrial)  Collector (Residential or Industrial)	1 1	Collector (Residential or Industrial)	1	
45 Street	50 Avenue	43 Avenue	Local	1	Collector (Residential or Industrial)  Collector (Residential or Industrial)	1	
				1		1	
41 Street	54 Avenue	50 Avenue	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1 2	
Kingsway	59 Street	Glenwood	Undivided Arterial	1 1	Divided Arterial	2 2	<ul> <li>Widen to provide centre median and 2 travel lanes in each direction</li> <li>Build pavement structure to Arterial standard (centre median and 2 trave</li> </ul>
Kingsway	Timberline	Glenwood	Collector (Residential or Industrial)	1	Divided Arterial	2	lanes in each direction)
Kingsway	Queensway	Timberline	Collector (Residential or Industrial)	1 1	Undivided Arterial	1 1	-
Kingsway	Queensway	End of Road	Collector (Residential or Industrial)	1 1	Collector (Residential or Industrial)	1	-
Queensway	Tennis Court Road	Hanger Ln	Collector (Residential or Industrial)	1 1	Collector (Residential or Industrial)	1 1	
Timberline	Juniper Avenue	Athabasca Road	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1 1 2	Pullid payment structure to Astocial standard (Astro-Aller)
Glenwood Drive	Glenwood	Kingsway	Collector (Residential or Industrial)	1	Undivided Arterial	2 2	Build pavement structure to Arterial standard (2 travel lanes in each direction)

Based on 2000 Transportation Study Road Classifications with consideration for Highway 28 Twinning (10 Street and 54 Avenue)
 Following the reclassification of a roadway, no improvements are required to upgrade the pavement structure unless widening is also required.

# TECHNICAL MEMORANDUM

7

# **Summary of Findings**

AE was retained by the City to forecast the future traffic volumes for the next 20 years. Traffic volumes were forecasted for the 5-year, 10-year, 15-year, and 20-year planning horizons and analyzed to determine roadway classification and number of lanes required to accommodate the future traffic volumes.

Figure 5.1 through Figure 5.4 present the forecasted daily traffic volumes for the 5-year, 10-year, 15-year, and 20-year planning horizons respectively.

The forecasted total traffic volumes for each planning horizon were compared with the City's daily service volumes to determine the required roadway classification. The lane volumes were also compared with the lane capacity for the given road classification, to determine the required number of lanes required along each roadway. The results of the analysis are summarized in Appendix D for each planning horizon.

The 20-year (2030) road classification and number of lanes will be used by the City to determine the right-of-way that should be retained to accommodate future expansion of the road network. The major corridors in the 20-year road network were reviewed independently to establish consistent road classification and numbers of lanes along the corridor, where possible. The recommended road classification and number of lanes is presented in Figure 6.1.

Table 6.4 summarizes the major road network in the 20-year planning horizon, along with a comparison of the existing and future road classification and number of lanes. The table also summarizes the improvements required to upgrade the corridors from the existing horizon to the 20-year planning horizon.



# **TECHNICAL MEMORANDUM**



# **Appendix A - ASP, ARP and Outline Plan Information**



Project No: 2010-3050 Date: January 28, 2011

# FISCHER ESTATES - LAND USE INFORMATION

Land Use Type	Total Developable		Develope	ed in 2010	Undeveloped in 2010	
Land Ose Type	Area (ha.)	Dwelling Units	Areas (ha)	Dwelling Units	Area (ha)	Dwelling Units
Low-Density Residential	25.0	449	-	0	-	449
Multi-Family Residential	5.9	295	-	0	-	295
Commercial - Arterial	3.6	-	3.6	-	0.0	-
Commercial - Neighbourhood	6.7	-	0.9	-	5.8	-
Municipal Reserve	5.0	-	0.0	-	5.0	-
Stormwater	4.7	-	0.0	-	4.7	-
Other (Roadway/Pathway)	12.6	-	0.0	-	12.6	-
Total	63.5	744	4.5	0	28.1	744

Project No: 2010-3050 Date: January 28, 2011

## **IRON HORSE - LAND USE INFORMATION**

Land Use	Total Developable		Develope	ed in 2010	Undeveloped in 2010	
Land USe	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units
Low-Density Residential	19.7	323	-	0	-	323
Medium-Density Residential	0.6	18	-	0	-	18
High-Density Residential	0.9	45	-	0	-	45
Municipal Reserve	2.24	-	0.0	-	0.0	-
Other (Roadways)	7.36	-	0.0	-	0.0	-
Total	30.8	386	0.0	0	0.0	386

City of Cold Lake Transportation Study Traffic Volume Forecast Project No: 2010-3050

Date: January 28, 2011

## **COLD LAKE CENTRAL - LAND USE INFORMATION**

	Total Developable		Develope	ed in 2010	Undeveloped in 2010	
Land Use	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units
Low-Density Residential	90.1	1,559	•	205	-	1,354
Medium-Density Residential	20.7	622	•	44	•	578
High-Density Residential	10.5	1,046	•	444	•	602
Manufactured Housing	12.3	243	•	243	-	0
Commercial - Arterial	37.7	-	18.9	-	18.7	-
Institutional	2.6	-	2.6	-	0.0	-
Parks/Municipal Reserve	25.8	-	0.0	-	0.0	-
Stormwater Facility/PUL (Sanitary Forcemain)	13.9	-	0.0	-	0.0	-
Circulation	36.9	=	0.0	-	0.0	-
Total	250.6	3,470	21.6	936	18.7	2,534

Project No: 2010-3050 Date: January 28, 2011

# **GRAND CENTRE SE - LAND USE INFORMATION**

	Total Developable		Develope	ed in 2010	Undeveloped in 2010	
Land Use	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units
Low-Density Residential	21.6	370	-	89	-	281
Mobile Home	8.4	240	-	90	-	150
Commercial - Arterial	10.1	-	10.1	-	0.0	-
Industrial	15.8	-	9.9	-	5.9	-
Utility	6.4	-	0.0	-	6.4	-
Open Space	1.8	-	0.0	-	1.8	-
Fairgrounds	40.1	-	0.0	-	40.1	-
Cementary	0.8	-	0.8	-	0.0	-
Total	105.0	610	20.8	179	54.3	431

City of Cold Lake Transportation Study

Traffic Volume Forecast Project No: 2010-3050 Date: January 28, 2011

#### FOREST HEIGHTS - LAND USE INFORMATION

	Total Developable		Develope	ed in 2010	Undeveloped in 2010	
Land Use	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units
Developed in 2007 (120 Residential Lots & School Site)	19.6	120	19.6	120	0.0	0
Single-Family Residential	20.6	345	0.0	0	20.6	345
Multi-Family Residential	8.3	248	0.0	0	8.3	248
Municipal Reserve	4.4	-	0.0	-	4.4	-
Storm Water Management	1.7	-	0.0	-	1.7	-
Roadways	9.4	-	0.0	-	9.4	-
Total Residential	64.0	713	19.6	120	44.4	593

City of Cold Lake Transportation Study Traffic Volume Forecast Project No: 2010-3050 Date: January 28, 2011

#### **NORTHSHORE - LAND USE INFORMATION**

\* Land uses highlighted in yellow will be used to generate development traffic volumes \*

#### **TOTAL AREA**

Land Use	Total Area <sup>1</sup> (ha)	Creekside ASP <sup>2</sup> (ha)	Parkview ASP <sup>3</sup> (ha)	Remaining Area (ha)
		60.5	` '	` ,
Gross Area	244.1		36.8	146.8
Non-Residential Subtotal	125.6	17.8	13.2	94.6
Linear Parks (Parkways/Trails)	4.9	0.0	0.0	4.9
Local Parks	10.4	1.3	5.7	3.5
Special Study Area	20.8	9.8	0.0	11.0
Stormwater Management Facilities	10.1	3.6	0.0	6.5
Public Utility Lots	1.6	0.0	0.0	1.6
Roads	53.6	2.8	7.5	43.3
School Site	4.6	0.0	0.0	4.6
Institutional	5.6	0.0	0.0	5.6
Religious Assembly	0.3	0.3	0.0	0.0
Neighbourhood Commercial	1.8	0.0	0.0	1.8
Highway Commercial	11.9	0.0	0.0	11.9
Residential	118.5	42.7	23.6	52.2
Low Density Residential	91.9	38.4	21.0	32.6
Medium Density Residential	14.9	4.4	0.0	10.6
Mixed Use Commercial	9.1	0.0	2.6	6.5
Mixed Use Institutional	2.6	0.0	0.0	2.6

<sup>1.</sup> From Northshore ASP

#### 2007 HORIZON - DEVELOPED

Land Use	Total Area (ha)	Creekside ASP (ha)	Parkview ASP (ha)	Remaining Area (ha)
Gross Area	244.1	60.5	36.8	146.8
Non-Residential Subtotal	5.9	0.3	0.0	5.6
Linear Parks (Parkways/Trails)	0.0	0.0	0.0	0.0
Local Parks	0.0	0.0	0.0	0.0
Special Study Area	0.0	0.0	0.0	0.0
Stormwater Management Facilities	0.0	0.0	0.0	0.0
Public Utility Lots	0.0	0.0	0.0	0.0
Roads	0.0	0.0	0.0	0.0
School Site	0.0	0.0	0.0	0.0
Institutional	5.6	0.0	0.0	5.6
Religious Assembly	0.3	0.3	0.0	0.0
Neighbourhood Commercial	0.0	0.0	0.0	0.0
Highway Commercial	0.0	0.0	0.0	0.0
Residential	5.1	0.0	0.0	5.1
Low Density Residential	5.1	0.0	0.0	5.1
Medium Density Residential	0.0	0.0	0.0	0.0
Mixed Use Commercial	0.0	0.0	0.0	0.0
Mixed Use Institutional	0.0	0.0	0.0	0.0
Developable Area	233.1	60.2	36.8	136.1

<sup>2.</sup> From Creekside ASP

<sup>3.</sup> From Parkview ASP

Project No: 2010-3050 Date: January 28, 2011

#### **NORTHSHORE - LAND USE INFORMATION**

\* Land uses highlighted in yellow will be used to generate development traffic volumes \*

#### 2010 HORIZON - DEVELOPED

Land Use	Total Area (ha)	Creekside ASP (ha)	Parkview ASP (ha)	Remaining Area (ha)
Gross Area	233.1	60.2	36.8	136.1
Non-Residential Subtotal	0.9	0.9	0.0	0.0
Linear Parks (Parkways/Trails)	0.0	0.0	0.0	0.0
Local Parks	0.0	0.0	0.0	0.0
Special Study Area	0.0	0.0	0.0	0.0
Stormwater Management Facilities	0.9	0.9	0.0	0.0
Public Utility Lots	0.0	0.0	0.0	0.0
Roads	0.0	0.0	0.0	0.0
School Site	0.0	0.0	0.0	0.0
Institutional	0.0	0.0	0.0	0.0
Religious Assembly	0.0	0.0	0.0	0.0
Neighbourhood Commercial	0.0	0.0	0.0	0.0
Highway Commercial	0.0	0.0	0.0	0.0
Residential	7.4	5.7	1.7	0.0
Low Density Residential	7.4	5.7	1.7	0.0
Medium Density Residential	0.0	0.0	0.0	0.0
Mixed Use Commercial <sup>1</sup>	0.0	0.0	0.0	0.0
Mixed Use Institutional	0.0	0.0	0.0	0.0
Developable Area	224.8	53.7	35.1	136.1

#### 2010 - DEVELOPABLE

	Total Area	Creekside ASP	Parkview ASP	Remaining Area
Land Use	(ha)	(ha)	(ha)	(ha)
Gross Area	224.8	53.7	35.1	136.1
Non-Residential Subtotal	118.8	16.6	13.2	89.0
Linear Parks (Parkways/Trails)	4.9	0.0	0.0	4.9
Local Parks	10.4	1.3	5.7	3.5
Special Study Area	20.8	9.8	0.0	11.0
Stormwater Management Facilities	9.2	2.8	0.0	6.5
Public Utility Lots	1.6	0.0	0.0	1.6
Roads	53.6	2.8	7.5	43.3
School Site	4.6	0.0	0.0	4.6
Institutional	0.0	0.0	0.0	0.0
Religious Assembly	0.0	0.0	0.0	0.0
Neighbourhood Commercial	1.8	0.0	0.0	1.8
Commercial - Arterial	11.9	0.0	0.0	11.9
Residential	106.0	37.1	21.9	47.1
Low Density Residential	79.4	32.7	19.2	27.5
Medium Density Residential	14.9	4.4	0.0	10.6
Mixed-Use Commercial 4	9.1	0.0	2.6	6.5
Mixed-Use Institutional	2.6	0.0	0.0	2.6

<sup>4.</sup> To maintain consistency with Northshore ASP, Parkview's Commercial as been considered as Mixed Use Commercial - To account for total land area

Project No: 2010-3050 Date: January 28, 2011

#### **NORTHSHORE - LAND USE INFORMATION**

\* Land uses highlighted in yellow will be used to generate development traffic volumes \*

#### TOTAL DEVELOPABLE - SCHOOL/RESIDENTIAL

	Total	Area	Creekside ASP	Parkview ASP	Remaining Area	
Land Use	Unit	# of	# of	# of	# of	
School	Students	1,958	0	0	1,958	
Low-Density Residential	Dwelling Units	1,654	659	401	594	
Medium-Density Residential	Dwelling Units	671	196	0	475	
Mixed-Use Commercial	Dwelling Units	547	0	0	547	
Mixed-Use Institutional	Dwelling Units	157	0	0	157	

#### 2007 DEVELOPED - SCHOOL/RESIDENTIAL

	Total	Area	Creekside ASP	Parkview ASP	Remaining Area
Land Use	Unit	Unit # of		# of	# of
School	Students	0	0	0	0
Low-Density Residential	Dwelling Units	57	0	0	57
Medium-Density Residential	Dwelling Units	0	0	0	0
Mixed-Use Commercial	Dwelling Units	0	0	0	0
Mixed-Use Institutional	Dwelling Units	0	0	0	0

#### 2010 DEVELOPED - SCHOOL/RESIDENTIAL

	Total	Area	Creekside ASP	Parkview ASP	Remaining Area
Land Use	Unit	# of	# of	# of	# of
School	Students	0	0	0	0
Low-Density Residential	Dwelling Units	99	65	34	0
Medium-Density Residential	Dwelling Units	0	0	0	0
Mixed-Use Commercial	Dwelling Units	0	0	0	0
Mixed-Use Institutional	Dwelling Units	0	0	0	0

#### 2010 DEVELOPABLE - SCHOOL/RESIDENTIAL

	Total	Area	Creekside ASP	Parkview ASP	Remaining Area	
Land Use	Unit	# of	# of	# of	# of	
School	Students	1,958	0	0	1,958	
Low-Density Residential	Dwelling Units	1,498	594	367	537	
Medium-Density Residential	Dwelling Units	671	196	0	475	
Mixed-Use Commercial	Dwelling Units	547	0	0	547	
Mixed-Use Institutional	Dwelling Units	157	0	0	157	

Project No: 2010-3050 Date: January 28, 2011

# LOT 2, PLAN 982 1024 - LAND USE INFORMATION

	Total Developable		Develope	ed in 2010	Undeveloped in 2010	
Land Use	Area (s.ft.)	Dwelling Units	Area (s.ft.)	Dwelling Units	Area (s.ft.)	Dwelling Units
Residential						
Building 1	5,506.0	12	5,506.0	12	0.0	0
Building 2	5,506.0	12	5,506.0	12	0.0	0
Building 3	19,394.0	54	19,394.0	54	0.0	0
Building 6 - Will not be built	0.0	0	0.0	0	0.0	0
Total Residential	30,406.0	78	30,406.0	78	0.0	0
Commercial						
Building 4	11,295.9	0	0.0	0	11,295.9	0
Building 5	4,068.6	0	0.0	0	4,068.6	0
Total Commercial	15,364.5	0	0.0	0	15,364.5	0

City of Cold Lake Transportation Study

Traffic Volume Forecast Project No: 2010-3050 Date: January 28, 2011

#### **HORSESHOE BAY - LAND USE INFORMATION**

Land Use	Area (acres)	Area (ha)	% of Planned Area	Lots	Population Estimate
Existing 50 ft width lot (Beach Avenue)	4.0	1.6	2.0%	21	65
Existing 0.5 acre lots	3.7	1.5	2.0%	7	22
Existing 1.0 acre lots	14.0	5.7	7.4%	11	34
Potential Serviced Residential Estates	105.0	42.5	55.3%	182	564
Natural Area Park	5.0	2.0	2.6%		
Lakeshore Trail System	5.0	2.0	2.6%		
Environmental Reserve	26.0	10.5	13.7%		
English Bay Road	7.0	2.8	3.7%		
Local Roads (by dedication)	20.0	8.0	10.5%		
Total	190.0	77.0	100.0%	219	651

	Total Developable	Developed in 2010	Undeveloped in 2010
Land Use	(Dwelling Unit)	(Dwelling Unit)	(Dwelling Unit)
Low Density Residential	219	177	42

Project No: 2010-3050 Date: January 28, 2011

# **UPLANDS - LAND USE INFORMATION**

	Total Developable		Develope	ed in 2010	Undeveloped in 2010		
Land Use	Area (ha)	Area (ha) Dwelling Units		Dwelling Units	Area (ha)	Dwelling Units	
Single-Family Residential	45.2	904	0.0	0	45.2	904	
Multi-Family Residential	9.6	480	0.0	0	9.6	480	
Health Services and Mixed Use	5.0	•	0.0	-	5.0	-	
Municipal Reserve	12.7	•	0.0	-	12.7	-	
SWMF and Existing Wetlands	7.9	•	0.0	-	7.9	-	
Roads and Lanes	21.5	-	0.0	-	21.5	-	
Total	101.9	1,384	0.0	0	101.9	1,384	

City of Cold Lake Transportation Study Traffic Volume Forecast Project No: 2010-3050

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#### LAKESHORE REDEVELOPMENT - LAND USE INFORMATION

	Description	Existing Land Use	Area (s.m.)	Area (hec)	Max. Site Coverage	Developable Area (hec)	Area (sq.ft)
1	Vacant parcel on 12 Street and 8 Avenue	Vacant	16,938.6	1.7	0%	0.0	0.0
2	902 10 Street <sup>1</sup>	Commercial	1,097.2	0.1	50%	0.1	5,905.0
3	904 10 Street	Commercial	690.4	0.1	50%	0.0	3,715.9
4	901 9 Avenue	Commercial	1,118.8	0.1	50%	0.1	6,021.3
5	803 10 Avenue	Commercial	2,248.6	0.2	50%	0.1	12,102.1
6	Triangle Park <sup>2</sup>	Park / Open Space	1,135.7	0.1	100%	0.1	12,224.7
7	Bibeau Park	Park / Open Space	11,648.9	1.2	100%	1.2	125,387.8
8	Centoaph Park	Park / Open Space	2,605.4	0.3	100%	0.3	28,044.2
9	Fire Hall <sup>3</sup>	Fire Hall	3,427.9	0.3	50%	0.2	18,449.1

<sup>1.</sup> Assume maximum site coverage for HDR is the same for MDR (50%)

<sup>3.</sup> From address map, fire hall building is approximately 50% of site.

	Description	Future Land Use	Area (s.m.)	Area (hec)	Max. Site Coverage	Developable Area (hec)	Area (sq.ft)	Dwelling Units
1	Vacant parcel on 12 Street and 8 Avenue <sup>4</sup>	Medium Density Residential	16,938.6	1.7	50%	0.8	91,162.6	38
2	902 10 Street <sup>5</sup>	High Density Residential	1,097.2	0.1	50%	0.1	5,905.0	15
3	904 10 Street <sup>5</sup>	High Density Residential	690.4	0.1	50%	0.0	3,715.9	9
4	901 9 Avenue <sup>5</sup>	High Density Residential	1,118.8	0.1	50%	0.1	6,021.3	15
5	803 10 Avenue <sup>6</sup>	Low Density Residential	2,248.6	0.2	45%	0.1	10,891.9	3
6	Triangle Park	Park / Open Space	1,135.7	0.1	100%	0.1	12,224.7	-
7	Bibeau Park	Park / Open Space	11,648.9	1.2	100%	1.2	125,387.8	-
8	Centoaph Park	Park / Open Space	2,605.4	0.3	100%	0.3	28,044.2	-
9	Fire Hall	Community Hall	3,427.9	0.3	50%	0.2	18,449.1	-

<sup>4.</sup> Maximum Density of 45 units/ha

<sup>2.</sup> Will not include in existing trip generation since not currently used. Following redevelopment, parks will be used and generate traffic.

<sup>5.</sup> HDR assumed to have 1 dwelling unit per 400 sq.ft of building footprint. Derived using ratio from Lot 2 buildings.

<sup>6. 803 10</sup> Avenue can be subdivided into three single family lots

City of Cold Lake Transportation Study

Traffic Volume Forecast Project No: 2010-3050 Date: January 28, 2011

#### **LAKEWOOD ESTATES - LAND USE INFORMATION**

	Total Developable			Develop	ed in 2010	Undeveloped in 2010	
Land Use	Area (s.m.)	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units
Total ASP Area	213,281.7	21.3	198				
Low-Density Residential	103,315.2	10.3					
Phase I		-	45	-	45	-	0
Phase II		-	32	-	0	-	32
Phase III		-	21	-	0	-	21
Phase IV		-	31	-	0	-	31
Phase V		-	21	-	0	-	21
Phase VI		-	28	-	0	-	28
Phase VII		-	20	-	0	-	20
Municipal Reserve	25,619.5	2.6	-	0.0	-	2.6	-
Others (Roadway/Pathways)	84,347.0	8.4	-	0.0	-	8.4	-
Total	213,281.7	21.3	198	0.0	45	11.0	153

City of Cold Lake Transportation Study Traffic Volume Forecast Project No: 2010-3050

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#### **CREEKSIDE ESTATES - LAND USE INFORMATION**

	Total Developable				Developed in 2010		Undeveloped in 2010	
Land Use	Area (s.m.)	Area (ha)	Density (Units/ha) <sup>1</sup>	Dwelling Units	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units
Total ASP Area	605,034.5	60.5						
Developed - In 2010								
Low-Density Residential	56,734.3	5.7	-	65	-	65	-	0
SWMF	-	0.9	-	-	0.9	-		
Undeveloped - In 2010								
Low-Density Residential	330,059.3	33.0	18.0	594	-	0	-	594
Medium-Density Residential	-	4.4	45.0	196	-	0	-	196
Park	-	1.3	-	-	0.0	-	1.3	-
SWMF	-	2.8	-		0.0	-	2.8	-
Special Study Area	-	9.8	-	-	0.0	-	9.8	-
Other (Roadways / Pathways)	-	2.8					2.8	
Total		60.5		855	0.9	65	16.6	790

<sup>1.</sup> From Northshore ASP

City of Cold Lake Transportation Study Traffic Volume Forecast Project No: 2010-3050

Date: January 28, 2011

#### PARKVIEW ESTATES - LAND USE INFORMATION

		Total Developable			2010 Developed		2010 Undeveloped	
Land Use	Area (s.m.)	Area (ha)	Density (Units/ha) <sup>1</sup>	Dwelling Units	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units
Total ASP Area	367,975.6	36.8	-	-				
Developed - In 2010								
Low-Density Residential (R1B)	17,287.5	1.7	-	34	-	34	-	0
Undeveloped - In 2010								
Low-Density Residential - Divided Lots	51,906.5	5.2	-	114	-	0		114
Low-Density Residential - Undivided Lots	140,448.2	14.0	18.0	253	-	0		253
Neighbourhood Commercial <sup>2</sup>	26,325.8	2.6	-	-	0.0	-	2.6	
Open Space	56,814.5	5.7	-	-	0.0	-	5.7	
Other (Roadways / Pathways)	75,193.1	7.5	-	-	0.0	-	7.5	
Total	292,782.5	29.3		401	0.0	34	15.8	367

<sup>1.</sup> From Northshore ASP

<sup>2.</sup> Outline Plan shows this area as C3 - Neighbourhood Commercial but Northshore ASP shows as Mixed Use Commercial

Project No: 2010-3050 Date: January 28, 2011

# **HILLS OF COLD LAKE - LAND USE INFORMATION**

\* Land uses highlighted in yellow will be used to generate development traffic volumes \*

Description of Land Use	Unserviced Lots (Acres)	Serviced Lots (Acres)
Total area available for development	294.9	294.9
Land to be allocated to the MD. Land marked as MR reserve.	45.7	64.5
Area of road reserve and public utility lanes	46.3	59.5
Area planned for establishment of building lots	202.9	170.9

**Phasing** 

Phase	Unserviced Lot Subdivision	Serviced Lot Subdivision
Phase A - Year 1	40	40
Phase B - Year 2-3	40	40
Phase C - Year 4-5	40	40
Phase D - Year 6-8	60	60
Phase E - Year 9	20	20
Phase F - Year 10-11	-	40
Phase G - Year 12-13	-	40
Phase H - Year 14	-	20
Total	200	300

City of Cold Lake Transportation Study Traffic Volume Forecast Project No: 2010-3050 Date: January 28, 2011

#### FAWN RIDGE ESTATES - LAND USE INFORMATION

		Developable Area	
Subdivisions and Legal Description	Land Use	(Acres)	Dwelling Units
NW 23-62-3-4 (Fawn Ridge Estates Subdivision)	Country Residential (CR)	86.3	54

			Direction Distribution (%)		Direction Dis	tribution (%)
	Trip Generation Rate (Trips per Dwelling					
Time Period	Units)	Generated Trips	Inbound	Outbound	Inbound	Outbound
NW 23-62-3-4 (Fawn Ridge Estates Subdivision)						
Country Residential (Fawn Ridge Estates Subdivision) Code 210						
Weekday (AADT)	9.57	517	50%	50%	258	258
AM Peak Hour	0.77	42	26%	74%	11	31
PM Peak Hour	1.02	55	64%	36%	35	20

City of Cold Lake Transportation Study Project No: 2010-3050

Date: April 9, 2011

#### MD BONNYVILLE - RESIDENTIAL

Source: Intermunicipal Development Plan (Feb 2009)

#### Future Land Uses:

- Residential developments

Land Use Location		Developable Area (m²)	Developable Area (Hec)	Developed Area by 2030 (Hec)
Residential Development 1 - 30% Developed by 2030	Along north side of Highway 55, west of Cold Lake	629,116.85	63	19
Residential Development 2 - 30% Developed by 2030	West of IDP Commercial Development, between 75 Avenue and south of 61/62 Avenue	843,132.02	84	25
Residential Development 3 - 30% Developed by 2030	East of Cold Lake Central, between Energy Centre to 55 Avenue	4,178,346.10	418	125

#### Assumed:

- Single family: 20 dwelling units/ha
- Multi family: 50 dwelling units/ha
- 75/25 split between single family and multi family residential developments

Land Use	Building Type	Developable Area (Hec)	Dwelling Units
	75% - Single Family Residential	14	283
Residential Development 1	25% - Multi Family Residential	5	236
	Total	19	519
	75% - Single Family Residential	19	379
Residential Development 2	25% - Multi Family Residential	6	316
	Total	25	696
	75% - Single Family Residential	94	1,880
Residential Development 3	25% - Multi Family Residential	31	1,567
	Total	125	3,447

**City of Cold Lake Transportation Study** 

Project No: 2010-3050 Date: April 9, 2011

# **MD BONNYVILLE - IDP INDUSTRIAL**

Source: Intermunicipal Development Plan (Feb 2009)

## Location:

- Either side of Highway 55, west of Cold Lake

# Future Land Uses:

- Industrial

			Developed Area
Land Use	Developable Area (m²)	Developable Area (Hec)	by 2030 (Hec)
Industrial Development - 20% Developed by 2030	3,919,353.21	392	78

## Assumed:

- 60% max site coverage (as per City of Cold Lake Bylaw)

**City of Cold Lake Transportation Study** 

Project No: 2010-3050 Date: April 9, 2011

# MD BONNYVILLE - IDP COMMERCIAL

Source: Intermunicipal Development Plan (Feb 2009)

## Location:

- Along west side of Highway 28, from Energy Centre to 55 Avenue

# **Future Land Uses:**

- Commercial

Land Use	Developable Area (m²)	Developable Area (Hec)	Developed Area by 2030 (Hec)
Commercial Development - 30% developed by 2030	1,574,100.10	157	47

# Assumed:

- Arterial Commercial: 80% site coverage (as per City of Cold Lake Bylaw)

# **TECHNICAL MEMORANDUM**



# **Appendix B - Traffic Demand Forecast Work Plan**





**Date:** February 25, 2011 **File:** 20103050.00.01.10

To: Bob Kitchen

From: Rohit Vij

**Project:** City of Cold Lake Transportation Study

Subject: Traffic Demand Forecast Work Plan

**MEMO** 

#### 1 OBJECTIVE

The objective of this work plan is to develop a methodology by which to forecast future traffic demand within Cold Lake, using the ASP, ARP and Outline Plans. **Highlighted text illustrates our assumptions for Cold Lake Transportation Study.** Please review the assumptions and provide your consensus.

#### 2 BACKGROUND INFORMATION

Associated Engineering (AE) has obtained the following information from the City of Cold Lake.

# 2.1 AREA STRUCTURE PLANS (ASP)

- Fischer Estates
- Horseshoe Bay
- Iron Horse
- Cold Lake Central
- Southeast
- Forest Heights
- North Shore
- Lot 2, Plan 982 1024
- Uplands

# 2.2 AREA REDEVELOPMENT PLAN (ARP)

Cold Lake Downtown (Cold Lake North)

#### 2.3 OUTLINE PLANS

- Lakewood Estates
- Creekside Estates
- Parkview Estates

**Figure 1** presents the Outline Plan for Lakewood Estates. The Outline Plan presents the breakdown of the subdivision to parcels and indicates the phasing anticipated; however, the land use is not indicated. Based on the layout, **it will be assumed that subdivision will be solely low-density residential**.



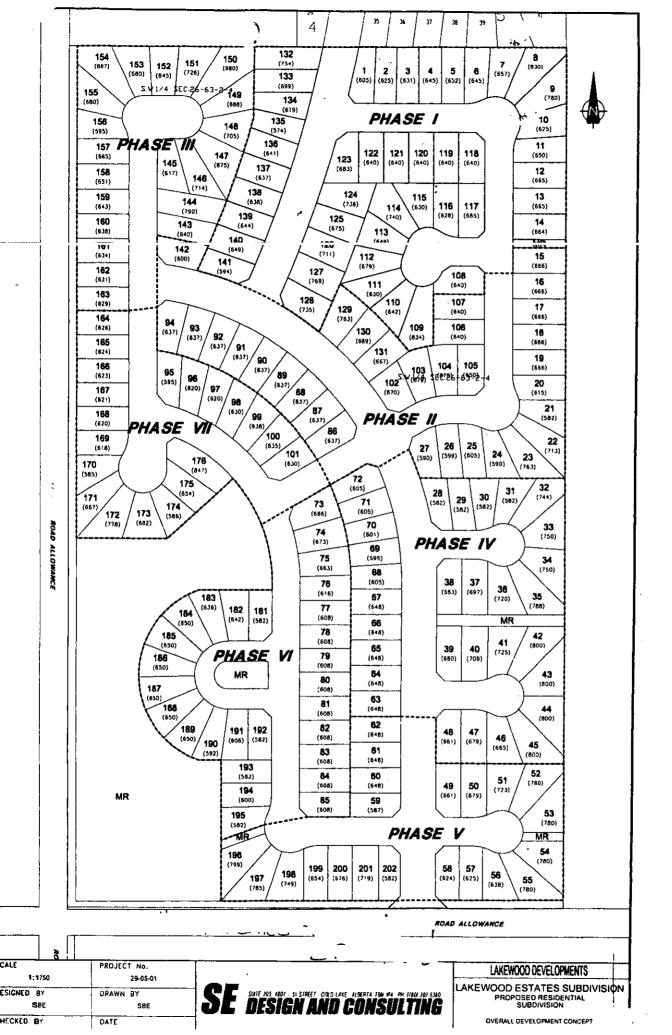
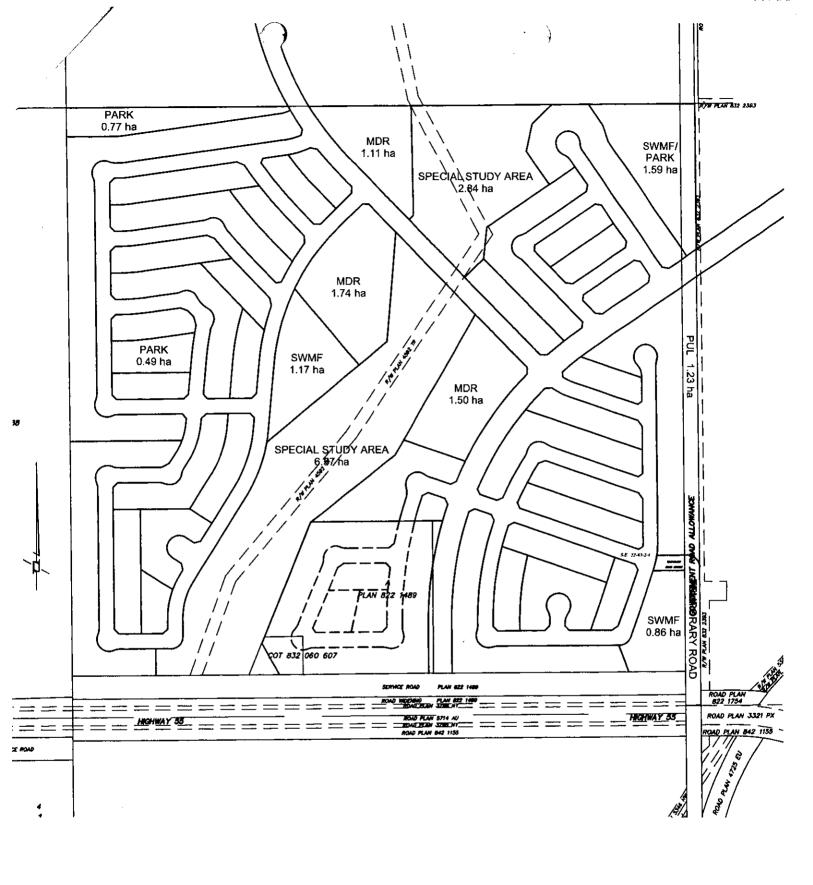


Figure 1

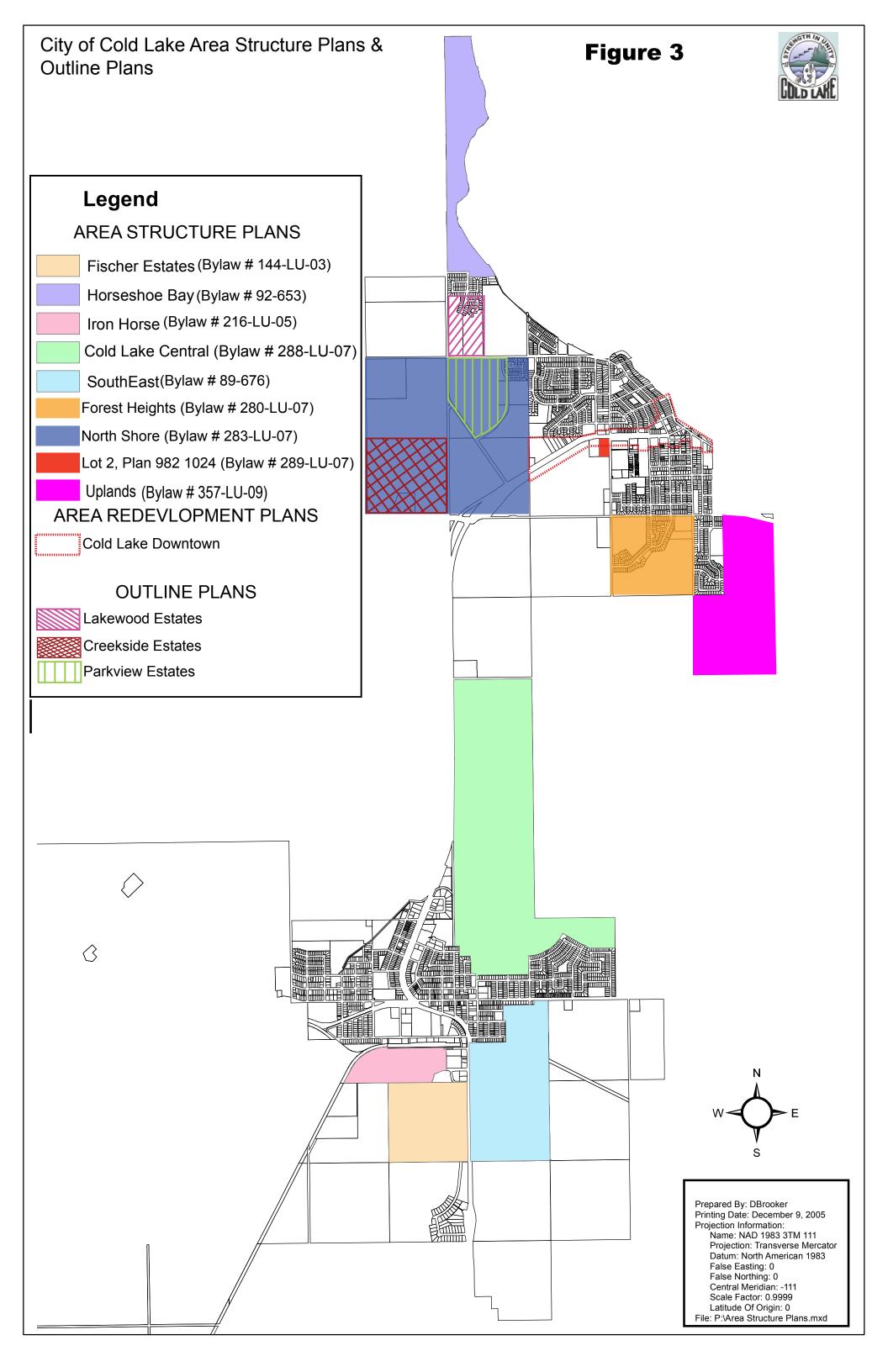


CREEKSIDE

OVERALL DESIGN CONCEPT

Figure 2







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**Figure 2** presents the Outline Plan for Creekside Estates. The Outline Plan presents the breakdown of the subdivision into parcels and indicates the land use. **For parcels where the land use is not indicated, low-density residential will be assumed.** 

The available ASP, ARP and Outline Plans are shown in Figure 3.

#### 2.4 CITY OF COLD LAKE LAND USE BY-LAW

The different land use districts within the City of Cold Lake was presented and described in the Land Use By-law. The following table summarizes the information of interest for the purpose of the Traffic Demand Forecast.

Table 2.1 - Information from City of Cold Lake Land Use Bylaw

Land Use District	Maximum Lot Coverage	Minimum Floor Area (m²)	Maximum Floor Area	Maximum Density
RE – Residential Estates District	35%	108.0	-	1 unit/lot
R1A – Residential District (Single Detached)	45%	84.0	-	1 unit/lot
R1B – Residential District (Single Detached – Small Lots)	45%	72.0	-	1 unit/lot
R1B-1 – Residential District (Single Detached – Small Lots)	45%	72.0	-	1 unit/lot
R2 – Residential District (Semi- Detached/Duplex)	45%	72.0	-	2 units/lot
R3 – Medium Density Residential (Row Housing)	50%	63.0	-	42 units/ha
R4 – High Density Residential	-	-	1.3	95 units/ha
RMX – Residential Mixed Use	-	At discretion of Development Authority	-	-





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Land Use District	Maximum Lot Coverage	Minimum Floor Area (m²)	Maximum Floor Area	Maximum Density
RMHC – Residential Manufactured Home Community District	40%	a) single wide – 65.0 b) double wide – 85.0	-	16 units/ha
RMHS – Residential Manufactured Home Subdivision	40%	49.5	-	-
C1 – Downtown Commercial (Central Business District)	80%	At discretion of Development Authority	-	-
C2 – Arterial Commercial (Along Major Arterial Roads, Highway 28)	80%	At discretion of Development Authority	-	-
C3 – Neighbourhood Commercial	50%	Permitted Use – 250.0 Discretionary Use – 1000.0	-	-
LC – Lakeshore Commercial	80%	Commercial – Min. 30% of all floors, 50% of ground floor Residential – Max. 70% of all, 50% of ground floor	-	-
BD – Beach District	At discretion of Development Authority			
LI – Light Industrial	60%	-	-	-
HI – Heavy Industrial	60%	-	-	-





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Land Use District	Maximum Lot Coverage	Minimum Floor Area (m²)	Maximum Floor Area Ratio	Maximum Density		
PS – Public Service (Educational, government, health care and recreational services)	At discretion of Development Authority					
IP – Imperial Park District		At discretion of	of Development Authority			
UR – Urban Reserve	At discretion of Development Authority					
CON - Conservation	At discretion of Development Authority					
DC – Direct Control District	-	-	-	-		
DC-SR – Spinnaker Ridge Direct Control District	-	-	-	45 units/ha 8 units/row house		
DC-TCE – Tri City Estates Direct Control District	40%	63.0	-	40 units/ha		
DC-RMHC – Residential Manufactured Home Community Direct Control District	45% 49.5 -		25.2 units/ha or 19.76 per gross ha.			
FW – National Defense	At discretion of Department of National Defense					

# 2.5 MD OF BONNYVILLE NO. 87 LAND USE BYLAW

The different land use districts within the MD of Bonnyville was presented and described in the Land Use Bylaw. The following table summarizes the information of interest for the purpose of the Traffic Demand Forecast.

Table 2.2 - Information from MD of Bonnyville Land Use Bylaw

Land Use District	Maximum Lot Coverage	Minimum Floor Area (m²)	Maximum Floor Area Ratio	Maximum Density
A – Agricultural	-	-	-	1 unit/lot
CR – Country Residential (Resort)	-	-	-	1 unit/lot





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Land Use District	Maximum Lot Coverage	Minimum Floor Area (m²)	Maximum Floor Area Ratio	Maximum Density
CR1 – Country Residential	-	-	-	1 unit/lot
CR2 – Country Residential (Large Lot)	-	-	-	1 unit/lot
CUD – Controlled Urban Development	-	-	-	-
DC – Direct Control	-	-	-	-
HG – Hamlet General	-	Unserviced – 1860.0 Serviced – 420.0 Sewer only – 930.0 Water only – 1400.0	-	1 unit/lot
HR1 – Hamlet Single Family Residential	-	Unserviced – 1860.0 Serviced – 560.0 Sewer only – 930.0 Water only – 1400.0	-	1 unit/lot
HR2 – Hamlet Multi Family Residential (Duplex)	35%	Interior Site – 697.0 Corner Site – 744.0	-	2 units/lot
HR2 – Hamlet Multi Family Residential (Triplex/Fourplex)	-	297.0 / unit	At discretion of Development Authority	At discretion of Development Authority
HR2 – Hamlet Multi Family Residential (Townhouse)	At discretion of Development Authority	Interior Lot – 185.5 Corner Lot – 297.0	At discretion of Development Authority	30 units/ha
HR2 – Hamlet Multi Family Residential (Apartment)	30%	800.0	0.60	At discretion of Development Authority
HUR – Hamlet Urban Reserve District	-	-	-	-
IR – Intensive Recreation		At discretion of Deve	elopment Authority	





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Land Use District	Maximum Lot Coverage	Minimum Floor Are	Maximum Floor Area Ratio	Maximum Density		
MHC – Manufactured Home Community	-	a) single wide – 465.0 b) double wide – 510.0	-	20 units/ha		
RC – Rural Commercial	At discretion of Development Authority					
RI – Rural Industrial	At discretion of Development Authority					

#### 3 WORKPLAN

A spreadsheet model will be utilized to forecast the future traffic demand of Cold Lake in the 5-year, 10-year, 15-year, and 20-year horizons. The spreadsheet model will comprise of 9 steps, which are explained in detail below.

#### 3.1 STEP 1: DEVELOP NETWORK

- Draw road network. The road network for the City of Cold Lake will consist of collector and arterial roads within the existing City limits.
- Divide study areas into traffic analysis zones (TAZ). The City will be divided into different TAZs that are homogenous in terms of land use. **AE anticipates that nine TAZs will be established for Cold Lake to represent the following:** 
  - TAZ 1: Cold Lake North Commercial
  - TAZ 2: Cold Lake North Residential North/West
  - TAZ 3: Cold Lake North Residential South/East
  - TAZ 4: Central Corridor Commercial (between Cold Lake North and Cold Lake South, including 75 Avenue and 61/62 Avenue)
  - TAZ 5: Central Corridor Residential (between Cold Lake North and Cold Lake South, including Energy Centre Access and 61/62 Avenue)
  - TAZ 6: Cold Lake South Commercial
  - TAZ 7: Cold Lake South Residential West
  - TAZ 8: Cold Lake South Residential East
  - TAZ 9: Medley
- Identify intersections with counts within each zone.
- Determine the centroid for each zone. The centroid may be chosen to be the geographic center or the "center" of the road network.



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- Calculate the area of each zone (A<sub>1</sub>, A<sub>2</sub>, etc.)
- Calculate the distance between the centroid of each zone and the furthest point of the zone (d<sub>ii</sub>) and calculate the distance between each centroid (d<sub>ij</sub>). Where i denotes the study zone and j denotes the destination zone.

# 3.2 STEP 2 – EXISTING VOLUME

• Build a spreadsheet in Excel to summarize the existing traffic volumes. The table would be similar to the following table.

# **Existing Traffic Volumes (2010 Horizon)**

		Zone 1		Zone 2			Zor	Zone 9	
Intersec	etion	I <sub>1,1</sub>	I <sub>1,2</sub>	I <sub>1,x</sub>	l <sub>2,1</sub>	l <sub>2,2</sub>	l <sub>i,x</sub>	<b>I</b> <sub>9,1</sub>	I <sub>9,x</sub>
	Left	5	9	15					
NB	Through	85	211	150					
	Right	3	15	7					
	Left								
SB	Through								
	Right								
	Left								
EB	Through								
	Right								
	Left								
WB	Through								
	Right								

Where  $I_{i,x}$  denotes intersection number x in Zone i.





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#### 3.3 STEP 3 – FUTURE BACKGROUND VOLUMES

- Grow the existing traffic volumes using an annual growth factor to the 5-year, 10-year, 15-year, and 20-year horizons.
- An annual growth rate of 2.0% has been chosen to since it was used in the Municipal Development Plan and represents the median between the moderate (1.5%) and high (2.5%) projection growth in the Inter-municipal Development Plan.
- Future traffic volume n years = Existing traffic volumes + (Existing traffic volume x n x growth %).

  Therefore for n = 5 years, Future traffic volume = Existing traffic volume + (Existing traffic volume x 5 x 0.02).
- Build spreadsheets similar to Step 2 to summarize the Future Background Traffic.
- Four future background traffic volume spreadsheets will be developed for Cold Lake to represent the future background traffic volumes in the 5-year, 10-year, 15-year, and 20-year horizons.

5-year Background Traffic Volumes (2015)





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			Zone 1		Zone 2			Zor	ne 9
Intersec	etion	I <sub>1,1</sub>	I <sub>1,2</sub>	I <sub>1,x</sub>	<b>I</b> <sub>2,1</sub>	l <sub>2,2</sub>	l <sub>i,x</sub>	<b>l</b> <sub>9,1</sub>	I <sub>9,x</sub>
ND	Left	6 = 5+(5x5x 0.02)	10 = 9+(9x5x 0.02)	17 =15+(15 x5x0.02)					
NB	Through	94	232	165					
	Right	3	17	8					
	Left								
SB	Through								
	Right								
	Left								
EB	Through								
	Right								
	Left								
WB	Through								
	Right								

# 3.4 STEP 4 – FUTURE PRODUCTION

- Calculate the trip production for each horizon using the information provided by the City in the ASPs, ARPs and Outline Plans.
- The City does not have projected growth information available for the four horizons. The City of Cold Lake
  is subject to boom/bust cycles of population growth or contraction tied to the resource section. This
  makes growth forecasting difficult.
- For the transportation study, growth assumptions are necessary. **Associated Engineering assumed** that the following development staging would be implemented for each study horizon:





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# **Assumed Development Staging by Study Horizon**

Development / Redevelopment	Land Use	5-Year (2015) Horizon	10-Year (2020) Horizon	15-Year (2025) Horizon	20-Year (2030) Horizon	Total % Developed
Final on Fototon	Residential	0%	0%	25%	25%	50%
Fischer Estates	Commercial	0%	0%	25%	25%	50%
Iron Horse	Residential	0%	0%	25%	25%	50%
Cold Lake Central	Residential	25%	25%	25%	25%	100%
Cold Lake Certifal	Commercial	50%	50%	0%	0%	100%
Grand Centre	Residential	25%	25%	25%	25%	100%
Southeast	Industrial	25%	25%	25%	25%	100%
Forest Heights	Residential	0%	0%	25%	25%	50%
	Residential	25%	25%	25%	25%	100%
Northshore	Commercial	25%	25%	25%	25%	100%
Nottristiore	Institutional	25%	25%	25%	25%	100%
	School	0%	0%	100%	0%	100%
Lot 2, Plan 982 1024	Commercial	100%	0%	0%	0%	100%
Horseshoe Bay	Residential	50%	50%	0%	0%	100%
Linianda	Residential	25%	25%	25%	25%	100%
Uplands	Health Services & Mixed Use	25%	25%	25%	25%	100%





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Development / Redevelopment	Land Use	5-Year (2015) Horizon	10-Year (2020) Horizon	15-Year (2025) Horizon	20-Year (2030) Horizon	Total % Developed
Lakeshore Area Redevelopment	All	25%	25%	25%	25%	100%
Lakewood Estates	Residential	25%	25%	25%	25%	100%
Creekside Estates	Residential	25%	25%	25%	25%	100%
Parkview Estates	Residential	25%	25%	25%	25%	100%
Parkview Estates	Commercial	25%	25%	25%	25%	100%
Hills of Cold Lake	Residential	25%	25%	25%	25%	100%
Fawn Ridge Estates Development	Residential	25%	25%	25%	25%	100%

- The trips produced by the new developments for each future horizon will be generated using the Institute of Transportation Engineers (ITE) Trip Generation Handbook, 7<sup>th</sup> Edition and summarized in a spreadsheet similar to the one below.
- Four trip production spreadsheets will be produced to represent the trip production in the 5-year, 10-year, 15-year, and 20-year horizons.

**Trip Production (Pi) - Horizon** 

	,
Zone (i)	Trip Produced
1	
2	
3	
4	
5	
6	



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7	
8	
9	
Total (∑Pi)	

# 3.5 STEP 5 – FUTURE ATTRACTION

- Calculate attraction from land use and origin-destination (OD) surveys.
- For the spreadsheet model this is normally not available and this step is bypassed.
- This is the case for the Cold Lake project.

# 3.6 STEP 6 – TRIP TABLE

- Calculate the trips T<sub>ij</sub> between origin zone i and destination zone j, using the gravity model illustrated in the following table.
- The gravity model states that the interaction (trips) between two zones declines with increasing distance between them.

From/To Zone	Weight	%	Final Trip
Zone 1 to Zone 1	$A_1/(d_{11})^2$	$[A_1/(d_{11})^2] / \sum_1$	$P_1 \times [A_1/(d_{11})^2] / \sum_1$
Zone 1 to Zone 2	$A_1/(d_{12})^2$	$[A_1/(d_{12})^2] / \sum_1$	$P_1 \times [A_1/(d_{12})^2] / \sum_1$
Zone 1 to Zone 3	A <sub>1</sub> /(d <sub>13</sub> ) <sup>2</sup>	$[A_1/(d_{13})^2] / \sum_1$	$P_1 \times [A_1/(d_{13})^2] / \sum_1$
Zone 1 to Zone 4	A <sub>1</sub> /(d <sub>14</sub> ) <sup>2</sup>	$[A_1/(d_{14})^2] / \sum_1$	$P_1 \times [A_1/(d_{14})^2] / \sum_1$
Zone 1 to Zone 5	A <sub>1</sub> /(d <sub>15</sub> ) <sup>2</sup>	$[A_1/(d_{15})^2] / \sum_1$	$P_1 \times [A_1/(d_{15})^2] / \sum_1$
Zone 1 to Zone 6	A <sub>1</sub> /(d <sub>16</sub> ) <sup>2</sup>	$[A_1/(d_{16})^2] / \sum_1$	$P_1 \times [A_1/(d_{16})^2] / \sum_1$
Zone 1 to Zone 7	A <sub>1</sub> /(d <sub>17</sub> ) <sup>2</sup>	$[A_1/(d_{17})^2] / \sum_1$	$P_1 \times [A_1/(d_{17})^2] / \sum_1$
Zone 1 to Zone 8	A <sub>1</sub> /(d <sub>18</sub> ) <sup>2</sup>	$[A_1/(d_{18})^2] / \sum_1$	$P_1 \times [A_1/(d_{18})^2] / \sum_1$
Zone 1 to Zone 9	A <sub>1</sub> /(d <sub>19</sub> ) <sup>2</sup>	$[A_1/(d_{19})^2] / \sum_1$	$P_1 \times [A_1/(d_{19})^2] / \sum_1$





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Total $\Sigma_1$ 100% $P_1$
-----------------------------

- Repeat above table for all zones (Zone 1 through Zone 9).
- Compile the final trip table, similar to the one shown below, for each time horizon.
- Four trip tables will be produced to represent the trip distribution in the 5-year, 10-year, 15-year and 20-year horizons.

0 D	1	2	3	4	5	6	7	8	9	Total
1	$P_1 x$ $[A_1/(d_{11})^2] /$ $\Sigma_1$	$P_1 x$ $[A_1/(d_{12})^2] / \sum_1$	$P_1 x$ $[A_1/(d_{13})^2] / $ $\sum_1$	$P_1 x$ $[A_1/(d_{14})^2] / $ $\sum_1$	$P_1 x$ $[A_1/(d_{15})^2] / \sum_1$	$P_1 x$ $[A_1/(d_{16})^2] / \sum_1$	$P_1 x$ $[A_1/(d_{17})^2] / \sum_1$	$P_1 x$ $[A_1/(d_{18})^2] / \sum_1$	$P_1 x$ $[A_1/(d_{19})^2] / $ $\Sigma_1$	$\Sigma = P_1$
2										$\sum = P_2$
3										$\sum = P_3$
4										$\sum = P_4$
5										$\sum = P_5$
6										$\Sigma = P_6$
7										$\Sigma = P_7$
8										∑ = P <sub>8</sub>
9										$\Sigma = P_9$
Total	$\Sigma = A_1$	$\Sigma = A_2$	$\Sigma = A_3$	$\Sigma = A_4$	$\Sigma = A_5$	$\Sigma = A_6$	$\Sigma = A_7$	∑ = A <sub>8</sub>	$\Sigma = A_9$	

• If Step 5 had been completed, the sum of the attraction for each zone should equal the sum of production for the same zone.

#### 3.7 STEP 7 – ASSIGNMENT

Assign the trips to intersections using the minimum path algorithm for each zone, for each study horizon.

# 3.8 STEP 8 – EXTERNAL TRIP (IF AVAILABLE)

Collect external trips from Cordon points





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- Distribute external-external trips to intersections.
- This step will be bypassed for Cold Lake as there is no information available for external trips

# 3.9 STEP 9 – ADD DEVELOPMENT TRIPS TO BACKGROUND TRIPS

- Add the trip distribution from Step 7 to the future background traffic volumes established in Step 3.
- The volumes provided at each intersection are the total traffic volume anticipated for each of the study horizons and can be used to analysis the future intersection capacity.



# **TECHNICAL MEMORANDUM**



# **Appendix C - Gravity Model (Trip Distribution) Calculations**



Project No: 2010-3050 Date: February 28, 2011

#### **Traffic Demand Model: Zones & Intersections**

Zone	Description		Intersection with Counts
Zone	Description	Node #	Intersection
		104	1 Avenue & 16 Street
		105	1 Avenue / 2 Avenue & 10 Street
		106	8 Avenue & Lakeshore Drive
1	Cold Lake North - Commercial/Recreational	107	8 Avenue & 10 Street
		108	8 Avenue & 16 Street
		109	Highway 28 & 25 Street
		111	Highway 55/ 16 Avenue & Highway 28
		101	1 Avenue & 28 Street / English Bay Road
2	Cold Lake North - Residential (North of Hwy 28)	102	1 Avenue & 25 Street
2	Cold Lake North - Residential (North of Hwy 26)	103	1 Avenue & Nelson Street
		110	Highway 55 & 28 Street / English Bay Road
3	Cold Lake North - Residential (South of Hwy 28)	112	16 Avenue & 16 Street
3	Cold Lake North - Residential (South of Hwy 26)	113	16 Avenue & 10 Street
		202	Highway 28 / 55 & 75 Avenue
4	Cold Lake Central - Commercial	203	Highway 28 / 55 & 69 Avenue / Museum Road
4	Cold Lake Central - Commercial	204	Highway 28 / 55 & Tri-City Mall Access
		205	Highway 28 / 55 & 62 Avenue / 61 Avenue
5	Cold Lake Central - Residential	201	Highway 28 / 55 & Energy Centre Access
		301	Highway 28 / 55 & 54 Avenue
		302	Highway 28 / 55 & 52 Avenue
	Cold Lake South - CBD/Commercial	303	Highway 28 / 55 & 50 Avenue
		304	Highway 28 / 55 & 52 Street
		305	Highway 28 / 55 & 51 Street
		306	Highway 28 / 55 & 50 Street
6		307	Highway 28 / 55 & 46 Avenue
		308	Highway 28 / 55 & 43 Avenue
		316	50 Avenue & 53 Street
		317	50 Avenue & 52 Street
		318	50 Avenue & 51 Street
		319	50 Avenue & 50 Street
		320	50 Avenue & 49 Street
		309	57 Street & 52 Avenue (North)
		310	57 Street & 52 Avenue (South)
7	Cold Lake Couth Decidential (Mart of the CC)	311	50 Avenue & 59 Street
7	Cold Lake South - Residential (West of Hwy 28)	312	50 Avenue & 57 Street
		313	Centre Avenue & 59 Street
		314	Centre Avenue & 57 Street
		315	54 Avenue & 51 Street
_	Cold Labor Courts Double 12 1/E 1/1/E 200	321	50 Avenue & 45 Street
8	Cold Lake South - Residential (East of Hwy 28)	322	50 Avenue & 41 Street
		323	50 Avenue / Twp Rd 630 & Baywood Road / RR 20
		401	Kingsway & Medley Road
		402	Kingsway & Glenwood Drive (West)
		403	Kingsway & Glenwood Drive (East)
9	Medley	404	Kingsway & Timberline Drive
		405	Kingsway & Queensway
		406	Kingsway & Tennis Court Road
		407	Queensway & Tennis Court Road
	1	707	additional a rolling doubt rodu

# City of Cold Lake Transportation Study Project No: 2010-3050

Date: February 28, 2011

# **Traffic Demand Model: Zones & Areas**

Zone	Description	Area (m <sup>2</sup> )	Area (hec)
1	Cold Lake North - Commercial/Recreational	818,544	81.9
2	Cold Lake North - Residential (North of Hwy 28)	5,302,120	530.2
3	Cold Lake North - Residential (South of Hwy 28)	4,499,137	449.9
4	Cold Lake Central - Commercial	622,964	62.3
5	Cold Lake Central - Residential	2,710,956	271.1
6	Cold Lake South - CBD/Commercial	1,171,259	117.1
7	Cold Lake South - Residential (West of Hwy 28)	4,519,673	452.0
8	Cold Lake South - Residential (East of Hwy 28)	5,295,608	529.6
9	Medley	34,603,627	3,460.4
	Total	59,543,887	5,954.4

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**Traffic Demand Model: Distances** 

	Distance from Zone X to Zone Y (m)								
	1	2	3	4	5	6	7	8	9
1	2,190	1,674	1,348	3,747	4,019	6,077	6,321	6,077	7,799
2	1,674	3,277	2,896	4,055	4,535	6,504	6,521	6,691	7,336
3	1,348	2,896	2,729	3,251	3,312	5,299	5,706	5,161	7,650
4	3,747	4,055	3,251	1,144	649	2,450	2,576	2,688	4,476
5	4,019	4,535	3,312	649	2,191	2,058	2,395	2,155	4,704
6	6,077	6,504	5,299	2,450	2,058	1,701	888	747	3,890
7	6,321	6,521	5,706	2,576	2,395	888	2,813	1,630	3,002
8	6,077	6,691	5,161	2,688	2,155	747	1,630	3,068	4,632
9	7,799	7,336	7,650	4,476	4,704	3,890	3,002	4,632	5,380

Distance from Centroid to furthest point in same zone

# Traffic Demand Model - Distances^2

	Distance from Zone X to Zone Y (m)								
	1	2	3	4	5	6	7	8	9
1	4,796,930	2,801,310	1,817,598	14,039,176	16,154,592	36,928,963	39,957,131	36,929,607	60,825,891
2	2,801,310	10,741,309	8,388,242	16,441,732	20,568,375	42,307,265	42,528,271	44,762,992	53,815,958
3	1,817,598	8,388,242	7,449,558	10,572,188	10,970,754	28,074,900	32,557,480	26,631,180	58,527,941
4	14,039,176	16,441,732	10,572,188	1,309,087	421,173	6,002,192	6,637,264	7,225,079	20,033,666
5	16,154,592	20,568,375	10,970,754	421,173	4,801,520	4,235,053	5,734,169	4,645,231	22,125,180
6	36,928,963	42,307,265	28,074,900	6,002,192	4,235,053	2,893,811	789,007	557,471	15,134,766
7	39,957,131	42,528,271	32,557,480	6,637,264	5,734,169	789,007	7,911,770	2,658,492	9,014,252
8	36,929,607	44,762,992	26,631,180	7,225,079	4,645,231	557,471	2,658,492	9,410,559	21,456,287
9	60,825,891	53,815,958	58,527,941	20,033,666	22,125,180	15,134,766	9,014,252	21,456,287	28,949,736

Distance from Centroid to furthest point in same zone

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# Traffic Demand Model: Future Production, 5 Year (2015)

Zone	Total Trips	In Trips	Out Trips
1	347	85	262
2	755	400	355
3	277	172	105
4	974	224	750
5	412	259	154
6	84	18	66
7	0	0	0
8	90	56	33
9	0	0	0
Outside of City	86	54	32
Total	3,025	1,268	1,757

# Traffic Demand Model: Future Production, 10 Year (2020)

Zone	Total Trips	In Trips	Out Trips
1	667	163	504
2	1,510	800	710
3	554	344	210
4	1,949	448	1,500
5	825	518	307
6	167	35	132
7	0	0	0
8	180	113	67
9	0	0	0
Outside of City	172	108	63
Total	6,023	2,530	3,493

# Traffic Demand Model: Future Production, 15 Year (2025)

Zone	Total Trips	In Trips	Out Trips
1	987	241	745
2	2,777	1,426	1,351
3	944	589	355
4	1,949	448	1,500
5	1,237	777	461
6	359	78	281
7	223	142	81
8	270	169	100
9	0	0	0
Outside of City	257	163	95
Total	9,003	4,033	4,970

# Traffic Demand Model: Future Production, 20 Year (2030)

Zone	Total Trips	In Trips	Out Trips
1	1,307	320	987
2	3,507	1,811	1,696
3	1,335	834	501
4	1,949	448	1,500
5	1,650	1,036	614
6	551	120	431
7	447	284	162
8	360	226	134
9	0	0	0
Outside of City	343	217	126
Total	11,447	5,295	6,152

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# **Traffic Demand Model, 5 Year Horizon: Future Production**

Weight based on Gravity Model. Weight of Zone X to Zone Y = (Area of Zone Y) / (Distance between Zone X & Y  $^2$ )

From Zone	To Zone	Weight <sup>1</sup>	%	Final Trip
	1	0.17	3.0%	11
	2	1.89	33.8%	117
	3	2.48	44.1%	153
	4	0.04	0.8%	3
_	5	0.17	3.0%	10
1	6	0.03	0.6%	2
	7	0.11	2.0%	7
	8	0.14	2.6%	9
	9	0.57	10.1%	35
	SUM	5.61	100.0%	347
	1	0.29	12.2%	92
	2	0.49	20.7%	156
	3	0.54	22.5%	170
	4	0.04	1.6%	12
}	5	0.13	5.5%	42
2	6	0.03	1.2%	9
<u> </u>	7	0.03	4.5%	34
<u> </u>	8	0.12	5.0%	37
}	9	0.64	26.9%	203
	SUM	2.39	100.0%	<b>755</b>
	1	0.45	15.2%	42
	2	0.63	21.3%	59
	3	0.60	20.4%	56
	4	0.06	2.0%	6
-	5	0.00	8.3%	23
3	6	0.23	1.4%	4
-	7	0.14	4.7%	13
	8	0.14	6.7%	19
	9	0.59	20.0%	55
-	SUM	2.96	100.0%	277
	1	0.06	0.5%	5
	2	0.32	2.9%	28
	3	0.43	3.8%	38
	4	0.48	4.3%	42
4	5	6.44	58.2%	567
	<u>6</u> 7	0.20	1.8%	17
	8	0.68	6.2% 6.6%	60 65
-		0.73		
-	9	1.73	15.6%	152
	SUM	11.06	100.0%	974
-	<u> </u>	0.05	0.8%	3
-		0.26	3.9%	16
-	3 4	0.41	6.3%	26
-		1.48	22.6%	93
5	5	0.56	8.6%	36
<u> </u>	6	0.28	4.2%	17
<u> </u>	7	0.79	12.1%	50
<u> </u>	8	1.14	17.5%	72
ļ	9	1.56	23.9%	99
	SUM	6.53	100.0%	412

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**Traffic Demand Model, 5 Year Horizon: Future Production**Weight based on Gravity Model. Weight of Zone X to Zone Y = (Area of Zone Y) / (Distance between Zone X & Y ^ 2)

From Zone	To Zone	Weight <sup>1</sup>	%	Final Trip
	1	0.02	0.1%	0
	2	0.13	0.7%	1
	3	0.16	0.8%	1
	4	0.10	0.5%	0
6	5	0.64	3.4%	3
O	6	0.40	2.1%	2
	7	5.73	30.2%	25
	8	9.50	50.1%	42
	9	2.29	12.1%	10
	SUM	18.97	100.0%	84
	1	0.02	0.2%	0
	2	0.12	1.4%	0
	3	0.14	1.6%	0
	4	0.09	1.1%	0
7	5	0.47	5.4%	0
'	6	1.48	17.0%	0
	7	0.57	6.5%	0
	8	1.99	22.8%	0
	9	3.84	43.9%	0
	SUM	8.74	100.0%	0
	1	0.02	0.3%	0
	2	0.12	1.7%	2
	3	0.17	2.4%	2
	4	0.09	1.2%	1
8	5	0.58	8.4%	8
O	6	2.10	30.2%	27
	7	1.70	24.4%	22
	8	0.56	8.1%	7
	9	1.61	23.2%	21
	SUM	6.96	100.0%	90
	1	0.01	0.6%	0
	2	0.10	4.2%	0
	3	0.08	3.3%	0
	4	0.03	1.3%	0
9	5	0.12	5.2%	0
	6	0.08	3.3%	0
	7	0.50	21.2%	0
	8	0.25	10.4%	0
	9	1.20	50.6%	0
	SUM	2.36	100.0%	0

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# **Traffic Demand Model, 5 Year Horizon: Future Production**

					Trips					
From	1	2	3	4	5	6	7	8	9	SUM
1	11	117	153	3	10	2	7	9	35	347
2	92	156	170	12	42	9	34	37	203	755
3	42	59	56	6	23	4	13	19	55	277
4	5	28	38	42	567	17	60	65	152	974
5	3	16	26	93	36	17	50	72	99	412
6	0	1	1	0	3	2	25	42	10	84
7	0	0	0	0	0	0	0	0	0	0
8	0	2	2	1	8	27	22	7	21	90
9	0	0	0	0	0	0	0	0	0	0
SUM	154	379	445	157	688	78	211	251	576	

Note: Sum of column for Zone 1 must match sum of row for Zone 1 IF both production and attraction information available. No attraction information.

Project No: 2010-3050 Date: February 28, 2011

# TRIP DISTRIBUTION FROM GRAVITY MODEL

					Trips					
From	1	2	3	4	5	6	7	8	9	SUM
1	3%	34%	44%	1%	3%	1%	2%	3%	10%	100%
2	12%	21%	22%	2%	6%	1%	4%	5%	27%	100%
3	15%	21%	20%	2%	8%	1%	5%	7%	20%	100%
4	1%	3%	4%	4%	58%	2%	6%	7%	16%	100%
5	1%	4%	6%	23%	9%	4%	12%	17%	24%	100%
6	0%	1%	1%	1%	3%	2%	30%	50%	12%	100%
7	0%	1%	2%	1%	5%	17%	7%	23%	44%	100%
8	0%	2%	2%	1%	8%	30%	24%	8%	23%	100%
9	1%	4%	3%	1%	5%	3%	21%	10%	51%	100%

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# **ADJUSTED TRIP DISTRIBUTION**

					Trips					
From	1	2	3	4	5	6	7	8	9	SUM
1	5%	23%	23%	5%	8%	5%	10%	8%	13%	100%
2	20%	15%	10%	15%	10%	8%	7%	5%	10%	100%
3	20%	10%	15%	15%	10%	8%	7%	5%	10%	100%
4	6%	15%	15%	5%	15%	8%	12%	12%	12%	100%
5	16%	12%	12%	15%	15%	8%	7%	5%	10%	100%
6	5%	5%	5%	5%	15%	5%	25%	25%	10%	100%
7	10%	5%	5%	15%	10%	18%	12%	15%	10%	100%
8	10%	5%	5%	15%	10%	20%	10%	15%	10%	100%
9	8%	10%	10%	10%	7%	20%	10%	10%	15%	100%
SUM	100%	100%	100%	100%	100%	100%	100%	100%	100%	_

# **TECHNICAL MEMORANDUM**



# **Appendix D - Capacity Analysis**



# CAPACITY ANALYSIS - EXISTING (2010) HORIZON

	Inters	ection		Forecaste	ed Volumes	Band Classification	David Classification	Lane Capacity	Lane Capacity	
Corridor	From	То	Direction	Daily Traffic -	Daily Traffic - Two	Road Classification 2000 TPS <sup>1</sup>	Road Classification City of Cold Lake <sup>2</sup>	for Road Classification (veh/hour/lane) 3	for Road Classification (veh/day/lane) 4	Number of Lanes Required (One Direction)
1 Avenue	28 Street	25 Street	Eastbound	Directional 960	1,810	Collector	Local (Residential or Industrial)	100	1,000	1
1 Avenue	25 Street	Nelson Street	Westbound	850 1,640 1,470	3,110	Collector	Collector (Residential or Industrial)	400	4,000	1
1 Avenue	Nelson Street	16 Street	Westbound Eastbound	1,290	2,650	Collector	Local (Residential or Industrial)	100	1,000	2 2
Hwy 28	Hwy 55/16 Avenue	25 Street	Northbound	1,360 5,410	9,650	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
8 Avenue	25 Street	16 Street	Southbound Eastbound	4,240 5,260	7,860	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2 2
8 Avenue	16 Street	10 Street	Westbound Eastbound	2,600 1,880	2,980	2-Lane Arterial	Local (Residential or Industrial)	100	1,000	1 2
8 Avenue	10 Street	Lakeshore Drive	Westbound Eastbound	1,100 950	1,720	2-Lane Arterial	Local (Residential or Industrial)	100	1,000	2
Hwy 55	West City Limit	28 Street	Westbound Eastbound	770 3,360	4,930	Collector	Collector (Residential or Industrial)	400	4,000	1 1
Hwy 55	28 Street	Hwy 28	Westbound Eastbound	1,570 3,250	4,980	Collector	Collector (Residential or Industrial)	400	4,000	1 1
16 Avenue	Hwy 28	16 Street	Westbound Eastbound	1,730 1,820	3,210	Collector	Collector (Residential or Industrial)	400	4,000	1
16 Avenue	16 Street	10 Street	Westbound Eastbound	1,390 1,270	2,050	Collector	Local (Residential or Industrial)	100	1,000	1 2
16 Avenue	10 Street	8 Street	Westbound Eastbound	780 1,320	2,430	Collector	Local (Residential or Industrial)	100	1,000	1 2
English Bay Road	North City Limit	1 Avenue	Westbound Northbound	1,110 1,450	2,540	Collector	Local (Residential or Industrial)	100	1,000	2 2
English Bay Road	1 Avenue	25 Street	Southbound Northbound	1,090 1,150	1,930	Collector	Local (Residential or Industrial)	100	1,000	2 2
	25 Street	Hwy 28	Southbound Northbound	780 1,490	3,100	Collector	Collector (Residential or Industrial)	400	4,000	1
English Bay Road 28 Street	English Bay Road	Hwy 55	Southbound Northbound	1,610 660	1,050	Collector	Local (Residential or Industrial)	100	1,000	1
		,	Southbound Northbound	390 840						1
25 Street	1 Avenue	English Bay Road	Southbound Eastbound	770 570	1,610	Collector	Local (Residential or Industrial)	100	1,000	1
Nelson Street	1 Avenue	16 Street	Westbound Northbound	330 2,070	900	Collector	Local (Residential or Industrial)	100	1,000	1 3
16 Street	1 Avenue	8 Avenue	Southbound Northbound	770 420	2,840	Collector	Local (Residential or Industrial)	100	1,000	1
16 Street	8 Avenue	16 Avenue	Southbound Northbound	450 780	870	Collector	Local (Residential or Industrial)	100	1,000	1
10 Street	1 Avenue	8 Avenue	Southbound Northbound	830 900	1,610	Collector	Local (Residential or Industrial)	100	1,000	1
10 Street	8 Avenue	16 Avenue	Southbound Northbound	910 6,810	1,810	Collector	Local (Residential or Industrial)	100	1,000	1
Hwy 28/55	Hwy 55/16 Avenue	75 Avenue	Southbound Northbound	6,170 7,480	12,980	2-Lane Arterial	Undivided Arterial	800	8,000	1
Hwy 28/55	75 Avenue	69 Avenue	Southbound	7,680	15,160	2-Lane Arterial	Undivided Arterial	800	8,000	1
Hwy 28/55	69 Avenue	54 Avenue	Northbound Southbound	7,500 7,150	14,650	2-Lane Arterial	Undivided Arterial	800	8,000	1 2
54 Avenue	56 Street	Hwy 28/55	Westbound	1,330 1,530	2,860	Collector	Local (Residential or Industrial)	100	1,000	2
54 Avenue	Hwy 28/55	51 Street	Westbound	2,320 2,280	4,600	Collector	Collector (Residential or Industrial)	400	4,000	1 1 2
54 Avenue	51 Street	45 Street	Westbound Vestbound	1,440 1,170	2,610	Collector	Local (Residential or Industrial)	100	1,000	2 2
52 Avenue	57 Street	Hwy 28/55	Westbound Vestbound	810 1,180	1,990	Collector	Local (Residential or Industrial)	100	1,000	2
50 Avenue	62 Street	59 Street	Westbound	220 370	590	Collector	Local (Residential or Industrial)	100	1,000	1
50 Avenue	59 Street	57 Street	Westbound Westbound	420 530	950	Collector	Local (Residential or Industrial)	100	1,000	1
50 Avenue	57 Street	55 Street	Westbound Westbound	180 340	520	Collector	Local (Residential or Industrial)	100	1,000	1 1
Centre Avenue	59 Street	57 Street	Westbound Westbound	8,340 3,700	12,040	2-Lane Arterial	Undivided Arterial	800	8,000	2 1
Centre Avenue	57 Street	Hwy 28/55	Eastbound Westbound	8,500 4,300	12,800	4-Lane Arterial	Undivided Arterial	800	8,000	2 1
50 Avenue	Hwy 28/55	51 Street	Eastbound Westbound	3,280 2,610	5,890	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue	51 Street	50 Street	Eastbound Westbound	3,080 1,970	5,050	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue	50 Street	45 Street	Eastbound Westbound	3,210 1,890	5,100	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue	45 Street	41 Street	Eastbound Westbound	2,620 1,510	4,130	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue	41 Street	Future Arterial	Eastbound Westbound	1,820 1,140	2,960	2-Lane Arterial	Local (Residential or Industrial)	100	1,000	2 2
50 Avenue	Future Arterial	Baywood Road	Eastbound Westbound	1,510 960	2,470	2-Lane Arterial	Local (Residential or Industrial)	100	1,000	2
59 Street	50 Avenue	Centre Avenue	Northbound Southbound	630 280	910	Collector	Local (Residential or Industrial)	100	1,000	1 1
57 Street	52 Avenue	50 Avenue	Northbound Southbound	770 470	1,240	Collector	Local (Residential or Industrial)	100	1,000	1
57 Street	50 Avenue	Centre Avenue	Northbound Southbound	820 520	1,340	Collector	Local (Residential or Industrial)	100	1,000	1
Hwy 28/55	54 Avenue	52 Avenue	Northbound Southbound	6,970 6,870	13,840	4-Lane Arterial	Undivided Arterial	800	8,000	1
Hwy 28/55	52 Avenue	50 Avenue	Northbound Southbound	7,640 5,700	13,340	4-Lane Arterial	Undivided Arterial	800	8,000	1
Hwy 28/55	50 Avenue	50 Street	Northbound Southbound	5,280 5,450	10,730	4-Lane Arterial	Undivided Arterial	800	8,000	1
Hwy 28/55	50 Street	43 Avenue	Northbound Southbound	5,430 5,930 6,670	12,600	4-Lane Arterial	Undivided Arterial	800	8,000	1
Hwy 28/55	43 Avenue	South City Limit	Northbound	5,500	11,730	2-Lane Arterial	Undivided Arterial	800	8,000	1
51 Street	54 Avenue	50 Avenue	Northbound Southbound	6,230 870	1,770	Collector	Local (Residential or Industrial)	100	1,000	1 1
50 Street	50 Avenue	Hwy 28/55	Northbound Couthbound	900 2,980	5,880	Collector	Collector (Residential or Industrial)	400	4,000	1
45 Street	54 Avenue	50 Avenue	Northbound Northbound	2,900 410	690	Collector	Local (Residential or Industrial)	100	1,000	1
41 Street	54 Avenue	50 Avenue	Northbound Northbound	280 800	1,320	Collector	Local (Residential or Industrial)	100	1,000	1
511001	2		Southbound	520	.,520	2230101	(	.50	.,500	1

City of Cold Lake - Transportation Study Project No: 2010-3050 Date: April 11, 2011

#### CAPACITY ANALYSIS - EXISTING (2010) HORIZON

Corridor	Interse	ection	Direction	Forecaste	ed Volumes	Road Classification	Road Classification	Lane Capacity for Road Classification	Lane Capacity for Road Classification	Number of Lanes
Corridor	From	То	Direction	Daily Traffic - Directional	Daily Traffic - Two Way	2000 TPS <sup>1</sup>	City of Cold Lake <sup>2</sup>	(veh/hour/lane) 3	(veh/day/lane) 4	Required (One Direction)
Kingsway	59 Street	Glenwood	Eastbound	7,290	10.510	2-Lane Arterial	Undivided Arterial	800	8.000	1
Kingsway	39 311661	Gleriwood	Westbound	3,220	10,510	Z*Lane Antenai	Ondivided Arterial	800	8,000	1
Kingsway	Timberline	Glenwood	Eastbound	6,200	8.870	Collector	Collector (Residential or Industrial)	400	4,000	2
Kiligsway	Timberine	Gieriwood	Westbound	2,670	0,070	Collector	Collector (Resideritial or Industrial)	400	4,000	1
Kingsway	Queensway	Timberline	Eastbound	2,840	4.480	Collector	Collector (Residential or Industrial)	400	4,000	1
Kiligsway	Queerisway	Tittibetilite	Westbound	1,640	4,400	Collector	Collector (Resideritial or Industrial)	400	4,000	1
Kingsway	Tennis Court Road	Queensway	Eastbound	540	1.170	Collector	Local (Residential or Industrial)	100	1,000	1
Kingsway	Tennis Court Road	Queensway	Westbound	630	1,170	Collector	Local (Residential of Industrial)	100	1,000	1
Kingsway	End of Road	Tennis Court Road	Eastbound	740	1,520	Collector	Local (Residential or Industrial)	100	1,000	1
Kiligsway	Eliu di Rodu	Termis Court Road	Westbound	780	1,320	Collector	Local (Residential of Industrial)	100	1,000	1
Tennis Court Road	Queensway	Kingsway	Northbound	220	270	Collector	Lane	N/A	N/A	N/A
Terms Court Road	Queerisway	KiligSway	Southbound	50	210	Collector	Laire	N/A	IVA	N/A
Queensway	Tennis Court Road	Kingsway	Northbound	1,020	1,790	Collector	Local (Residential or Industrial)	100	1,000	2
Queerisway	Termis Court Road	KiligSway	Southbound	770	1,750	Collector	Local (Residential of Industrial)	100	1,000	1
Queensway	Kingsway	Hanger Ln	Northbound	1,870	2,200	Collector	Local (Residential or Industrial)	100	1,000	2
Queensway	rungsway	rianger En	Southbound	330	2,200	Collector	Local (residential of Industrial)	100	1,000	1
Timberline	Juniper Avenue	Kingsway	Northbound	740	1,340	Collector	Local (Residential or Industrial)	100	1,000	1
THIDDINIO	Julipoi Averiue	rungsway	Southbound	600	1,540	Collector	Local (Nesideridal of Industrial)	100	1,000	1
Timberline	Kingsway	Athabasca Road	Northbound	1,720	2,520	Collector	Local (Residential or Industrial)	100	1.000	2
rimberline	niiysway	Alliabasca Road	Southbound	800	2,320	Conector	Local (Nesideridal of Industrial)	100	1,000	1
Glenwood Drive	Glenwood	Kingsway	Northbound	2,600	3,920	Collector	Collector (Residential or Industrial)	400	4,000	1
Bend election be		• •	Southbound	1,320	3,920	Conector	Collector (Nesidefillal of Industrial)	400	4,000	1

<sup>1.</sup>Road classification based on 2000 Transportation Study
2. Road classification based on 2000 Transportation Study
2. Road classification based on Daily Service Volumes stipulated in City's Roadway Design Standards (Municipal Engineering Servicing Standards and Standard Construction Specifications, Jan 2008)
3. Based on Lane Capacity Table (lattached). Using road classification according to City's standards.
4. Based on assumption that PM peak hour traffic is 10% of the daily traffic

# CAPACITY ANALYSIS - 5 YEAR (2015) HORIZON

Corridor	Interse	ection	Direction	Forecasted		Road Classification	Road Classification	Lane Capacity for Road Classification	Lane Capacity for Road Classification	Number of Lanes
	From	То		Daily Traffic - Directional	Daily Traffic - Two Way	2000 TPS 1	City of Cold Lake <sup>2</sup>	(veh/hour/lane) 3	(veh/day/lane) 4	Required (One Direction)
1 Avenue	28 Street	25 Street	Eastbound Westbound	1,600 1,200	2,800	Collector	Local (Residential or Industrial)	100	1,000	2 2
1 Avenue	25 Street	Nelson Street	Eastbound Westbound	2,490 2,140	4,630	Collector	Collector (Residential or Industrial)	400	4,000	1
1 Avenue	Nelson Street	16 Street	Eastbound Westbound	2,680 2,820	5,500	Collector	Collector (Residential or Industrial)	400	4,000	1
Hwy 28	Hwy 55/16 Avenue	25 Street	Northbound Southbound	9,600 8,240	17,840	4-Lane Arterial	Undivided Arterial	800	8,000	2
8 Avenue	25 Street	16 Street	Eastbound Westbound	6,870 4,020	10,890	4-Lane Arterial	Undivided Arterial	800	8,000	1
8 Avenue	16 Street	10 Street	Eastbound	2,470	4,210	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
8 Avenue	10 Street	Lakeshore Drive	Westbound Eastbound	1,740 1,300	2,610	4-Lane Arterial	Local (Residential or Industrial)	100	1,000	2
Hwy 55	West City Limit	28 Street	Westbound Eastbound	1,310 3,700	5,430	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
Hwy 55	28 Street	Hwy 28	Westbound Eastbound	1,730 5,130	9,040	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2
16 Avenue	Hwy 28	16 Street	Westbound Eastbound	3,910 3,720	6,490	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
	-		Westbound Eastbound	2,770 2,490						1 1
16 Avenue	16 Street	10 Street	Westbound Eastbound	1,520 2,450	4,010	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
16 Avenue	10 Street	8 Street	Westbound Northbound	1,790 4.740	4,240	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2
English Bay Road	North City Limit	1 Avenue	Southbound Northbound	3,860 5,580	8,600	Collector	Collector (Residential or Industrial)	400	4,000	1 2
English Bay Road	1 Avenue	25 Street	Southbound	4,130 4,730	9,710	Collector	Collector (Residential or Industrial)	400	4,000	2 2
English Bay Road	25 Street	Hwy 28	Southbound	4,220	8,950	Collector	Collector (Residential or Industrial)	400	4,000	2
28 Street	English Bay Road	Hwy 55	Northbound Southbound	2,650 1,810	4,460	Collector	Collector (Residential or Industrial)	400	4,000	1
25 Street	1 Avenue	English Bay Road	Northbound Southbound	1,620 1,400	3,020	Collector	Collector (Residential or Industrial)	400	4,000	1
Nelson Street	1 Avenue	16 Street	Eastbound Westbound	1,180 680	1,860	Collector	Local (Residential or Industrial)	100	1,000	2
16 Street	1 Avenue	8 Avenue	Northbound Southbound	4,840 1,760	6,600	Collector	Collector (Residential or Industrial)	400	4,000	2
16 Street	8 Avenue	16 Avenue	Northbound Southbound	970 1,090	2,060	Collector	Local (Residential or Industrial)	100	1,000	1 2
16 Street	16 Avenue	10 Street	Northbound Southbound	1,030 1,030 750	1,780	Collector	Local (Residential or Industrial)	100	1,000	2
10 Street	1 Avenue	8 Avenue	Northbound	1,340	2.610	Collector	Local (Residential or Industrial)	100	1.000	2
10 Street	8 Avenue	16 Avenue	Southbound Northbound	1,270 1,580	3.290	Collector	Collector (Residential or Industrial)	400	4.000	2
10 Street	16 Avenue	16 Street	Southbound Northbound	1,710 1,040	2.020	Collector	Local (Residential or Industrial)	100	1.000	1 2
	16 Avenue	21 Avenue	Southbound Northbound	980 1,040	2.020	Collector	Local (Residential or Industrial)	100	1.000	1 2
6 Street 5			Southbound Northbound	980 13,050	25.070	4-Lane Arterial	Divided Arterial	1.000	10.000	1 2
Hwy 28/55	Hwy 55/16 Avenue	75 Avenue	Southbound Northbound	12,020 13,970	-,			,	-,	2 2
Hwy 28/55	75 Avenue	69 Avenue	Southbound Northbound	13,860 13,310	27,830	4-Lane Arterial	Divided Arterial	1,000	10,000	2
Hwy 28/55	69 Avenue	54 Avenue	Southbound	15,550	28,860	4-Lane Arterial	Divided Arterial	1,000	10,000	2
75 Avenue	Hwy 28/55	Future Arterial	Westbound	1,750 3,080	4,830	Collector	Collector (Residential or Industrial)	400	4,000	1
54 Avenue	56 Street	Hwy 28/55	Westbound Westbound	1,820 2,290	4,110	Collector	Collector (Residential or Industrial)	400	4,000	1
54 Avenue	Hwy 28/55	51 Street	Eastbound Westbound	3,800 3,550	7,350	Collector	Collector (Residential or Industrial)	400	4,000	1
54 Avenue	51 Street	45 Street	Eastbound Westbound	2,990 2,430	5,420	Collector	Collector (Residential or Industrial)	400	4,000	1
52 Avenue	59 Street	57 Street	Eastbound Westbound	980 1,370	2,350	Collector	Local (Residential or Industrial)	100	1,000	1 2
52 Avenue	57 Street	Hwy 28/55	Eastbound Westbound	1,680 2,450	4,130	Collector	Collector (Residential or Industrial)	400	4,000	1
Centre Avenue	59 Street	57 Street	Eastbound Westbound	12,290 5,270	17,560	2-Lane Arterial	Undivided Arterial	800	8,000	2
Centre Avenue	57 Street	Hwy 28/55	Eastbound Westbound	11,020 7,400	18,420	4-Lane Arterial	Undivided Arterial	800	8,000	2
50 Avenue	Hwy 28/55	51 Street	Eastbound	5,080	8,720	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
50 Avenue	51 Street	50 Street	Westbound Eastbound	3,640 4,080	6,720	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
50 Avenue	50 Street	45 Street	Westbound Eastbound	2,640 4,310	6,630	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
50 Avenue	45 Street	41 Street	Westbound Eastbound	2,320 3,660	5,720	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue	41 Street	Future Arterial	Westbound Eastbound	2,060 2,840	4,620	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
			Westbound Eastbound	1,780 2,360			Collector (Residential or Industrial)  Collector (Residential or Industrial)		4,000	1
50 Avenue	Future Arterial	Baywood Road	Westbound Eastbound	1,500 3,410	3,860	2-Lane Arterial	, ,	400		1
43 Avenue	Hwy 28/55	45 Street	Westbound	3,100 1,310	6,510	Collector	Collector (Residential or Industrial)	400	4,000	1 2
59 Street	52 Avenue	Centre Avenue	Southbound	580 910	1,890	Collector	Local (Residential or Industrial)	100	1,000	1
57 Street	54 Avenue	52 Avenue	Southbound	600	1,510	Collector	Local (Residential or Industrial)	100	1,000	1 2
57 Street	52 Avenue	Centre Avenue	Northbound Southbound	1,700 1,080	2,780	Collector	Local (Residential or Industrial)	100	1,000	2
Hwy 28/55	54 Avenue	52 Avenue	Northbound Southbound	11,440 12,960	24,400	4-Lane Arterial	Divided Arterial	1,000	10,000	2 2
Hwy 28/55	52 Avenue	50 Avenue	Northbound Southbound	11,980 11,460	23,440	4-Lane Arterial	Divided Arterial	1,000	10,000	2 2
Hwy 28/55	50 Avenue	50 Street	Northbound Southbound	5,180 6,480	11,660	4-Lane Arterial	Undivided Arterial	800	8,000	1
Hwy 28/55	50 Street	43 Avenue	Northbound Southbound	8,070 9,440	17,510	4-Lane Arterial	Undivided Arterial	800	8,000	2 2
Hwy 28/55	43 Avenue	South City Limit	Northbound Southbound	6,520 7,360	13,880	4-Lane Arterial	Undivided Arterial	800	8,000	1 1
51 Street	54 Avenue	50 Avenue	Northbound Southbound	1,320 1,480	2,800	Collector	Local (Residential or Industrial)	100	1,000	2 2
50 Street	50 Avenue	Hwy 28/55	Northbound	4,770	9,300	Collector	Collector (Residential or Industrial)	400	4,000	2
45 Street	54 Avenue	50 Avenue	Southbound Northbound	4,530 520	860	Collector	Local (Residential or Industrial)	100	1,000	2
45 Street	50 Avenue	43 Avenue	Southbound Northbound	340 370	950	Collector	Local (Residential or Industrial)	100	1,000	1
			Southbound Northbound	580 1,660						1 2
41 Street	54 Avenue	50 Avenue	Southbound	1,080	2,740	Collector	Local (Residential or Industrial)	100	1,000	2

City of Cold Lake - Transportation Study Project No: 2010-3050 Date: April 11, 2011

#### CAPACITY ANALYSIS - 5 YEAR (2015) HORIZON

Corridor	Interse	ection	Direction	Forecasted		Road Classification	Road Classification	Lane Capacity for Road Classification	Lane Capacity for Road Classification	Number of Lanes
Corridor	From	То	Direction	Daily Traffic - Directional	Daily Traffic - Two Way	2000 TPS <sup>1</sup>	City of Cold Lake <sup>2</sup>	(veh/hour/lane) 3	(veh/day/lane) 4	Required (One Direction)
Kingsway	59 Street	Glenwood	Eastbound	9,530	15,530	2-I ane Arterial	Undivided Arterial	800	8.000	2
rungaway	33 011661	Gieriwood	Westbound	6,000	13,330	2-Lane Arterial	Olidivided Alterial	000	0,000	1
Kingsway	Timberline	Glenwood	Eastbound	7,950	12.240	Collector	Undivided Arterial	800	8,000	1
rungaway	Tillibellille	Gieriwood	Westbound	4,290	12,240	Collector	Ondivided Arterial	000	0,000	1
Kingsway	Queensway	Timberline	Eastbound	3,770	6.360	Collector	Collector (Residential or Industrial)	400	4.000	1
rungaway	Quociisway	TITIDETIIIE	Westbound	2,590	0,300	Collector	Collector (residential or middstrial)	400	4,000	1
Kingsway	Tennis Court Road	Queensway	Eastbound	770	1.770	Collector	Local (Residential or Industrial)	100	1.000	1
rangonay	TOTALIO COURT TOUG	quononay	Westbound	1,000	1,110	Colloctor	Lood (Nooldonida or madorida)	100	1,000	1
Kingsway	End of Road	Tennis Court Road	Eastbound	900	2,020	Collector	Local (Residential or Industrial)	100	1,000	1
9)			Westbound	1,120	_,			100	1,000	2
Tennis Court Road	Queensway	Kingsway	Northbound	250	310	Collector	Lane	N/A	N/A	N/A
		3,	Southbound	60						N/A
Queensway	Tennis Court Road	Kingsway	Northbound	1,790	3.110	Collector	Collector (Residential or Industrial)	400	4,000	1
		3,	Southbound	1,320	-, -				,	1
Queensway	Kingsway	Hanger Ln	Northbound	2,370	2,860	Collector	Local (Residential or Industrial)	100	1,000	3
	97	9	Southbound	490	_,				.,,	1
Timberline	Juniper Avenue	Kingsway	Northbound	1,050	1.830	Collector	Local (Residential or Industrial)	100	1.000	2
		3,	Southbound	780	,		,		,	1
Timberline	Kingsway	Athabasca Road	Northbound	2,270	3.470	Collector	Collector (Residential or Industrial)	400	4.000	1
	· · ·		Southbound	1,200			,			1
Glenwood Drive	Glenwood	Kingsway	Northbound	3,430	5,130	Collector	Collector (Residential or Industrial)	400	4,000	1
L			Southbound	1,700			,,			1

Southbound 1,700 5,100 Concern

#### CAPACITY ANALYSIS - 10 YEAR (2020) HORIZON

						w.,			Lane Capacity	Lane Capacity	
Second   S	Corridor		1	Direction		Daily Traffic - Two			for Road Classification	for Road Classification	
1-1-9999   1-1-9999	1 Avenue				2,190		Collector	Collector (Residential or Industrial)			1
1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-	1 Avenue	25 Street	Nelson Street	Eastbound	4,250	7,630	Collector	Collector (Residential or Industrial)	400	4,000	2
1.00	1 Avenue	Nelson Street	16 Street	Eastbound	4,120	8,470	Collector	Collector (Residential or Industrial)	400	4,000	
Service   15 Store	Hwy 28	Hwy 55/16 Avenue	25 Street	Northbound	14,280	26,760	4-Lane Arterial	Divided Arterial	1,000	10,000	
A-barrean   1-5 force   1-5	8 Avenue	25 Street	16 Street	Eastbound	9,030	14,780	4-Lane Arterial	Undivided Arterial	800	8,000	2
March   15   15   15   15   15   15   15   1	8 Avenue	16 Street	10 Street	Eastbound	3,250	5,750	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	·
May   May	8 Avenue	10 Street	Lakeshore Drive	Eastbound	1,780	3,700	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	
Prop	Hwy 55	West City Limit	28 Street	Eastbound	4,640	6,850	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
15 American   16 American   15 American	Hwy 55	28 Street	Hwy 28	Eastbound	7,690	14,100	2-Lane Arterial	Undivided Arterial	800	8,000	
Manual   M	16 Avenue	Hwy 28	16 Street	Eastbound	5,720	9,850	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	
15   March   15	16 Avenue	16 Street	10 Street	Eastbound	3,810	6,130	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
Egyptis Dept 2000   1	16 Avenue	10 Street	8 Street	Eastbound	3,590	6,060	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	
Cycle Page Name   1 Amount   25 Date   1990   25 Date	English Bay Road	North City Limit	1 Avenue	Northbound	7,390	13,480	Collector	Undivided Arterial	800	8,000	1
Page   Page	English Bay Road	1 Avenue	25 Street	Northbound	9,100	15,900	Collector	Undivided Arterial	800	8,000	
28 Sept	English Bay Road	25 Street	Hwy 28	Northbound	7,980	14,720	Collector	Undivided Arterial	800	8,000	1
23 Part   1 Annual   Cappe Bay Rose   Processor   Cappe Bay Rose   Cappe	28 Street	English Bay Road	Hwy 55	Northbound	4,620	7,920	Collector	Collector (Residential or Industrial)	400	4,000	2
Network Steel   1 America   16 Steel   1.00   1.00   2.877   Collector   Cooled Presidential in Number   1.00   1.00   2   1.00   2   1.00   1.00   2   1.00   1.00   2   1.00   1.00   2   1.00   1.00   2   1.00   1.00   2   1.00   1.00   2   1.00   1.00   2   1.00   1.00   2   1.00   1.00   2   1.00   1.00   2   1.00   1.00   2   1.00   1.00   1.00   2   1.00	25 Street	1 Avenue	English Bay Road	Northbound	3,310	6,010	Collector	Collector (Residential or Industrial)	400	4,000	1
16 Street   1 American   1 American   2 Am		1 Avenue		Eastbound	1,820						2
16   Daniel   P. America   16   America   1.560   3.00	16 Street	1 Avenue	8 Avenue	Northbound	6,230						2
15 Street				Northbound	1,540						
10   Street	16 Street	16 Avenue	10 Street	Northbound	1,740	3,250	Collector	Collector (Residential or Industrial)	400	4.000	
10   Direct		1 Avenue		Northbound	1,950						1
10   Date   16   Annua   16   Bree   16   Subfection   1,00   1,250					2,280					· ·	1
Compact   15 Amenina			16 Street	Northbound	1,820	,, ,		Collector (Residential or Industrial)		,,,,,	1
New y 2015   New 2015   New y 2015   New y 2015   New y 2015   New y 2015   New y				Northbound	1,820						1
Mary 2005   73 Annuals   60 Annuals   1.200											1 2
Mary 2855   GO Annua   SA SA Annua   SA SA Annua   SA SA Annua   SA SA Annua   SA SA Annua   SA SA Annua   SA SA SA Annua   SA SA SA Annua   SA SA SA Annua   SA SA SA Annua   SA SA SA SA Annua   SA SA SA SA SA SA SA SA SA SA SA SA SA	,				21,650	,				· ·	_
Section   Sect						,, ,					
Westbook   1499 2855   Section   1499 2855   Section   2,550   2,550   2,1 are Anterior   Collector (Residential or Industrial)   400   4,000   1   1   1   1   1   1   1   1   1											_
Section   Sect		,			5,530 3,080					·	2
Second   S			·	Westbound		-,		,		,,,,,	
Set Avenue			·	Westbound	3,210			,			1 2
S2 Avenue										· ·	2 2
S2 Annual   S7 Street   Hwy 2855   Earthound   2,210   5,300   Collector (Residential or Industrial)   400   4,000   1					3,740					· ·	
Centre Avenue				Westbound	2,110	-				· ·	1
Centre Avenue			·	Westbound							
Solution   Solution				Westbound	5,390						1 2
Solution   Color   C			•	Westbound	8,210	-,				-,	_
So Avenue		,		Westbound	4,850						1 2
Solvente				Westbound	3,500			,		· ·	1 2
Solvenue				Westbound	3,090			,		·	
So Avenue				Westbound	2,710	, ,		1 1			1
Solitor   Soli				Westbound	2,470						1
Separate   Separate			-	Westbound	2,080						
Southbound   Sou				Westbound	3,930			1			1
Southbound   Sou				Southbound	980	,		,			1
Sumbound   1,660   1				Southbound	930						1
Hwy 28/55   52 Avenue   50 Avenue   Southbound   17,730   32,000   4-Lane Anterial   Expressway   1,800   18,000   1				Southbound	1,660					· ·	1
Hwy 28/55   50 Avenue   Southbound   15,870   Southbound   1,890   Southbound   1				Southbound	17,730	- ,		1		·	1
Hwy 28/55   50 Avenue   50 Street   43 Avenue   Sumbound   9,270   16,490   4-Lane Arterial   Undivided Arterial   800   8,000   2   2   2   2   2   2   2   2   2				Southbound	15,870						1
No.   No.	Hwy 28/55	50 Avenue	50 Street	Southbound	9,270	16,490	4-Lane Arterial	Undivided Arterial	800	8,000	2
Southbound   Sou	Hwy 28/55	50 Street	43 Avenue	Southbound	12,550	22,910	4-Lane Arterial	Divided Arterial	1,000	10,000	
51 Street   54 Avenue   50 Avenue	Hwy 28/55	43 Avenue	South City Limit	Southbound	8,610	16,310	4-Lane Arterial	Undivided Arterial	800	8,000	
Southound   Sout	51 Street	54 Avenue	50 Avenue	Southbound	1,690	3,240	Collector	Collector (Residential or Industrial)	400	4,000	1
45 Street 55 Avenue 55 Avenue 55 Avenue 75 Ave	50 Street	50 Avenue	Hwy 28/55	Southbound	5,240	10,760	Collector	Undivided Arterial	800	8,000	1
43 Street 54 August 50 Aug	45 Street	54 Avenue	50 Avenue	Southbound	420	1,060	Collector	Local (Residential or Industrial)	100	1,000	1
	45 Street	50 Avenue	43 Avenue	Southbound	710	1,160	Collector	Local (Residential or Industrial)	100	1,000	1
	41 Street	54 Avenue	50 Avenue			4,220	Collector	Collector (Residential or Industrial)	400	4,000	

City of Cold Lake - Transportation Study Project No: 2010-3050 Date: April 11, 2011

#### CAPACITY ANALYSIS - 10 YEAR (2020) HORIZON

Corridor	Interse	ection	Direction	Forecasted	Volumes	Road Classification	Road Classification	Lane Capacity for Road Classification	Lane Capacity for Road Classification	Number of Lanes
Corridor	From	То		Daily Traffic - Directional	Daily Traffic - Two Way	2000 TPS <sup>1</sup>	City of Cold Lake <sup>2</sup>	(veh/hour/lane) 3	(veh/day/lane) 4	Required (One Direction)
Kingsway	59 Street	Glenwood	Eastbound	9,220	15,440	2-Lane Arterial	Undivided Arterial	800	8,000	2
KillySway	39 311661	Gleriwood	Westbound	6,220	13,440	Z-Lane Artenal	Oridivided Arterial	800	8,000	1
Kingsway	Timberline	Glenwood	Eastbound	9,960	15.970	Collector	Undivided Arterial	800	8,000	2
KillySway	rimbelline	Gieriwood	Westbound	6,010	15,970	Collector	Oridivided Arterial	800	8,000	1
Kingsway	Queensway	Timberline	Eastbound	4,840	8,440	Collector	Collector (Residential or Industrial)	400	4,000	2
Kingsway	Queensway	imberine	Westbound	3,600	8,440	Collector	Collector (Residential or Industrial)	400	4,000	1
Kingsway	Tennis Court Road	Queensway	Eastbound	890	2,290	Collector	Local (Residential or Industrial)	100	1,000	1
KillySway	Terris Court Road	Queensway	Westbound	1,400	2,290	Collector	Local (Residential of Industrial)	100	1,000	2
Kingsway	End of Road	Tennis Court Road	Eastbound	1,080	2,550	Collector	Local (Residential or Industrial)	100	1,000	2
KillySway	Elid di Road	Terris Court Road	Westbound	1,470	2,330	Collector	Local (Residential of Industrial)	100	1,000	2
Tennis Court Road	Queensway	Kingsway	Northbound	270	330	Collector	Lane	N/A	N/A	N/A
Terris Court Road	Queerisway	KillySway	Southbound	60	330	Collector	Laite	INA	IN/A	N/A
Queensway	Tennis Court Road	Kingsway	Northbound	2,300	3,990	Collector	Collector (Residential or Industrial)	400	4,000	1
Queensway	Terris Court Road	KillySway	Southbound	1,690	3,990	Collector	Collector (Residential of Industrial)	400	4,000	1
0	Kingsway	Hanger Ln	Northbound	2,940	3,590	Collector	Collector (Residential or Industrial)	400	4,000	1
Queensway	Killysway	nanger Lin	Southbound	650	3,390	Collector	Collector (Residential or Industrial)	400	4,000	1
Timberline	Juniper Avenue	V	Northbound	1,390	2,370	Collector	Local (Residential or Industrial)	100	1,000	2
i imbenine	Juniper Avenue	Kingsway	Southbound	980	2,370	Collector	Local (Residential or Industrial)	100	1,000	1
Timberline	Manager .	Athabasca Road	Northbound	2,890	4.500	Collector	Collector (Residential or Industrial)	400	4.000	1
i imperline	Kingsway	Athabasca Road	Southbound	1,630	4,520	Collector	Collector (Residential of Industrial)	400	4,000	1
Olemand Brins	01	VI	Northbound	6,290	0.000	O I amandadada	Callantes (Basidantial as Industrial)	400	4 000	2
Glenwood Drive	Glenwood	Kingsway	Southbound	3,590	9,880	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1

#### CAPACITY ANALYSIS - 15 YEAR (2025) HORIZON

Corridor	Interse	ection To	- Direction -	Forecasted  Daily Traffic - Directional	Volumes  Daily Traffic - Two Way	Road Classification 2000 TPS <sup>1</sup>	Road Classification City of Cold Lake <sup>2</sup>	Lane Capacity for Road Classification (veh/hour/lane) <sup>3</sup>	Lane Capacity for Road Classification (veh/day/lane) <sup>4</sup>	Number of Lanes Required (One Direction)
1 Avenue	28 Street	25 Street	Eastbound Westbound	2,870 1,910	4,780	Collector	Collector (Residential or Industrial)	400	4,000	1 1
1 Avenue	25 Street	Nelson Street	Eastbound Westbound	5,660 4,680	10,340	Collector	Undivided Arterial	800	8,000	1
1 Avenue	Nelson Street	16 Street	Eastbound Westbound	5,680 5,990	11,670	Collector	Undivided Arterial	800	8,000	1
Hwy 28	Hwy 55/16 Avenue	25 Street	Northbound	18,790	36,170	4-Lane Arterial	Expressway	1,800	18,000	2
8 Avenue	25 Street	16 Street	Southbound Eastbound	17,380 11,040	18,360	4-Lane Arterial	Undivided Arterial	800	8,000	2
8 Avenue	16 Street	10 Street	Westbound Eastbound	7,320 3,930	7,150	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
			Westbound Eastbound	3,220 2,250					· ·	1
8 Avenue	10 Street	Lakeshore Drive	Westbound Eastbound	2,600 5,560	4,850	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2
Hwy 55	West City Limit	28 Street	Westbound Eastbound	2,710 11,050	8,270	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2
Hwy 55	28 Street	Hwy 28	Westbound	9,390	20,440	2-Lane Arterial	Divided Arterial	1,000	10,000	1
16 Avenue	Hwy 28	16 Street	Westbound Westbound	7,360 5,550	12,910	2-Lane Arterial	Undivided Arterial	800	8,000	1
16 Avenue	16 Street	10 Street	Eastbound Westbound	4,850 3,110	7,960	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
16 Avenue	10 Street	8 Street	Eastbound Westbound	4,640 3,190	7,830	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
English Bay Road	North City Limit	1 Avenue	Northbound Southbound	10,550 9,540	20,090	Collector	Divided Arterial	1,000	10,000	2
English Bay road	1 Avenue	25 Street	Northbound	13,350 11,000	24,350	Collector	Divided Arterial	1,000	10,000	2
English Bay road	25 Street	Hwy 28	Southbound Northbound	11,090	21,150	Collector	Divided Arterial	1,000	10,000	2
28 Street	English Bay Road	Hwy 55	Southbound Northbound	10,060 7,100	12.690	Collector	Undivided Arterial	800	8.000	1
25 Street	1 Avenue	English Bay Road	Southbound Northbound	5,590 4,640	8,710	Collector	Collector (Residential or Industrial)	400	4,000	1 2
		-	Southbound Eastbound	4,070 2,510						2
Nelson Street	1 Avenue	16 Street	Westbound Northbound	1,450 7,870	3,960	Collector	Collector (Residential or Industrial)	400	4,000	1
16 Street	1 Avenue	8 Avenue	Southbound Northbound	3,660	11,530	Collector	Undivided Arterial	800	8,000	1
16 Street	8 Avenue	16 Avenue	Southbound	2,180 2,910	5,090	Collector	Collector (Residential or Industrial)	400	4,000	1
16 Street	16 Avenue	10 Street	Northbound Southbound	2,550 2,330	4,880	Collector	Collector (Residential or Industrial)	400	4,000	1
10 Street	1 Avenue	8 Avenue	Northbound Southbound	2,800 2,710	5,510	Collector	Collector (Residential or Industrial)	400	4,000	1
10 Street	8 Avenue	16 Avenue	Northbound Southbound	3,020 3,480	6,500	Collector	Collector (Residential or Industrial)	400	4,000	1
10 Street	16 Avenue	16 Street	Northbound Southbound	2,440 2,150	4,590	Collector	Collector (Residential or Industrial)	400	4,000	1
6 Street <sup>5</sup>	16 Avenue	75 Avenue	Northbound	2,440	4,590	Collector	Collector (Residential or Industrial)	400	4,000	1
Hwy 28/55	Hwy 55/16 Avenue	75 Avenue	Southbound Northbound	2,150 27,060	52,890	4-Lane Arterial	Expressway	1,800	18,000	1 2
Hwy 28/55	75 Avenue	69 Avenue	Southbound Northbound	25,830 28,420	56.990	4-Lane Arterial	Expressway	1.800	18.000	2 2
			Southbound Northbound	28,570 21,360	,			,,,,	-,	2 2
Hwy 28/55	69 Avenue	54 Avenue	Southbound Eastbound	24,930 4,950	46,290	4-Lane Arterial	Expressway	1,800	18,000	2
75 Avenue	Hwy 28/55	Future Arterial	Westbound Eastbound	6,610 4,320	11,560	Collector	Undivided Arterial	800	8,000	1 2
69 Avenue	Glenwood	Hwy 28/55	Westbound	3,680	8,000	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2
69 Avenue	Hwy 28/55	Future Arterial	Westbound Westbound	8,130 8,840	16,970	Collector	Undivided Arterial	800	8,000	2
47 Street <sup>6</sup>	69 Avenue	61/62 Avenue	Northbound Southbound	4,420 4,065	8,485	Collector	Collector (Residential or Industrial)	400	4,000	2
54 Avenue	56 Street	Hwy 28/55	Westbound Westbound	3,160 4,360	7,520	Collector	Collector (Residential or Industrial)	400	4,000	1 2
54 Avenue	Hwy 28/55	51 Street	Eastbound Westbound	5,310 4.600	9,910	Collector	Collector (Residential or Industrial)	400	4,000	2
54 Avenue	51 Street	45 Street	Eastbound Westbound	6,340 5,150	11,490	Collector	Undivided Arterial	800	8,000	1
54 Avenue 7	45 Street	41 Street	Eastbound	3,795	7,130	Collector	Collector (Residential or Industrial)	400	4,000	1
54 Avenue	41 Street	Future Arterial	Westbound Eastbound	3,335 1,250	2,770	Collector	Local (Residential or Industrial)	100	1,000	2
52 Avenue	59 Street	57 Street	Westbound Eastbound	1,520 2,070	4,980	Collector	Collector (Residential or Industrial)	400	4.000	2
52 Avenue	59 Street	Hwy 28/55	Westbound Eastbound	2,910 3,570	8,770	Collector	Collector (Residential or Industrial)  Collector (Residential or Industrial)	400	4,000	1
			Westbound Eastbound	5,200 15,000						2 2
Centre Avenue	59 Street	57 Street	Westbound	6,440 13,460	21,440	2-Lane Arterial	Divided Arterial	1,000	10,000	1 2
Centre Avenue	57 Street	Hwy 28/55	Westbound Vestbound	10,130	23,590	4-Lane Arterial	Divided Arterial	1,000	10,000	2
50 Avenue	Hwy 28/55	51 Street	Westbound	7,440 5,260	12,700	2-Lane Arterial	Undivided Arterial	800	8,000	1
50 Avenue	51 Street	50 Street	Eastbound Westbound	6,140 3,610	9,750	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue	50 Street	45 Street	Eastbound Westbound	5,990 2,440	8,430	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
50 Avenue	45 Street	41 Street	Eastbound Westbound	5,110 2,180	7,290	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
50 Avenue	41 Street	Future Arterial	Eastbound	2,980	4,640	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue	Future Arterial	Baywood Road	Westbound Eastbound	1,660 2,630	4,450	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
43 Avenue	Hwy 28/55	45 Street	Westbound Eastbound	1,820 5,430	9,940	Collector	Collector (Residential or Industrial)	400	4,000	1 2
59 Street	52 Avenue	Centre Avenue	Westbound Northbound	4,510 2,780	4,010	Collector	Collector (Residential or Industrial)	400	4,000	2
			Southbound Northbound	1,230 1,940						1
57 Street	54 Avenue	52 Avenue	Southbound Northbound	1,280 3,610	3,220	Collector	Collector (Residential or Industrial)	400	4,000	1
57 Street	52 Avenue	Centre Avenue	Southbound	2,290	5,900	Collector	Collector (Residential or Industrial)	400	4,000	1 2
Hwy 28/55	54 Avenue	52 Avenue	Northbound Southbound	18,090 21,530	39,620	4-Lane Arterial	Expressway	1,800	18,000	2
Hwy 28/55	52 Avenue	50 Avenue	Northbound Southbound	18,190 19,410	37,600	4-Lane Arterial	Expressway	1,800	18,000	2 2
Hwy 28/55	50 Avenue	50 Street	Northbound Southbound	9,090 11,300	20,390	4-Lane Arterial	Divided Arterial	1,000	10,000	1 2
11Wy 20/33		43 Avenue	Northbound	12,470 15.000	27,470	4-Lane Arterial	Divided Arterial	1,000	10,000	2 2
Hwy 28/55	50 Street	45 Avenue	Southhound							
Hwy 28/55	50 Street 43 Avenue	South City Limit	Northbound Southbound	9,020	18,910	4-Lane Arterial	Undivided Arterial	800	8,000	2
			Northbound Southbound Northbound	9,020 9,890 1,760	18,910 3,700	4-Lane Arterial  Collector	Undivided Arterial  Collector (Residential or Industrial)	800 400	8,000 4,000	2
Hwy 28/55 Hwy 28/55	43 Avenue	South City Limit	Northbound Southbound	9,020 9,890						2

#### CAPACITY ANALYSIS - 15 YEAR (2025) HORIZON

Corridor	Interse	ection	Direction	Forecasted '	Volumes	Road Classification	Road Classification	Lane Capacity for Road Classification	Lane Capacity for Road Classification	Number of Lanes
Corridor	From	То		Daily Traffic - Directional	Daily Traffic - Two Way	2000 TPS <sup>1</sup>	City of Cold Lake <sup>2</sup>	(veh/hour/lane) 3	(veh/day/lane) 4	Required (One Direction)
45 Street	50 Avenue	43 Avenue	Northbound Southbound	510 870	1,380	Collector	Local (Residential or Industrial)	100	1,000	1
41 Street	54 Avenue	50 Avenue	Northbound Southbound	3,520 2,290	5,810	Collector	Collector (Residential or Industrial)	400	4,000	1
Future Arterial	69 Avenue	54 Avenue	Northbound Southbound	2,320 2,170	4,490	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
Future Arterial	54 Avenue	50 Avenue	Northbound Southbound	1,880 1,420	3,300	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
Kingsway	59 Street	Glenwood	Eastbound Westbound	10,870 7,200	18,070	2-Lane Arterial	Undivided Arterial	800	8,000	2
Kingsway	Timberline	Glenwood	Eastbound Westbound	12,120 7,510	19,630	Collector	Undivided Arterial	800	8,000	2
Kingsway	Queensway	Timberline	Eastbound Westbound	5,990 4,480	10,470	Collector	Undivided Arterial	800	8,000	1
Kingsway	Tennis Court Road	Queensway	Eastbound Westbound	1,090 1,750	2,840	Collector	Local (Residential or Industrial)	100	1,000	2 2
Kingsway	End of Road	Tennis Court Road	Eastbound Westbound	1,270 1,780	3,050	Collector	Collector (Residential or Industrial)	400	4,000	1
Tennis Court Road	Queensway	Kingsway	Northbound Southbound	300 70	370	Collector	Lane	N/A	N/A	N/A N/A
Queensway	Tennis Court Road	Kingsway	Northbound Southbound	2,760 2,080	4,840	Collector	Collector (Residential or Industrial)	400	4,000	1
Queensway	Kingsway	Hanger Ln	Northbound Southbound	3,550 790	4,340	Collector	Collector (Residential or Industrial)	400	4,000	1 1
Timberline	Juniper Avenue	Kingsway	Northbound Southbound	1,680 1,200	2,880	Collector	Local (Residential or Industrial)	100	1,000	2 2
Timberline	Kingsway	Athabasca Road	Northbound Southbound	3,570 2,010	5,580	Collector	Collector (Residential or Industrial)	400	4,000	1
Glenwood Drive	Glenwood	Kingsway	Northbound Southbound	7,310 4,550	11,860	2-Lane Arterial	Undivided Arterial	800	8,000	1

Assumed to the Street to be also fall yraffic for 54 Avenue (45 Street to 41 Street) to be average of daily traffic on 54 Avenue (51 Street to 45 Street) and 54 Avenue (41 Street to Future Arterial)

#### CAPACITY ANALYSIS - 20 YEAR (2030) HORIZON

	Interse	ection	Di-	Forecasted	Volumes	Road Classification	Road Classification	Lane Capacity	Lane Capacity	Number of Lanes
Corridor	From	То	Direction	Daily Traffic - Directional	Daily Traffic - Two Way	2000 TPS <sup>1</sup>	City of Cold Lake <sup>2</sup>	for Road Classification (veh/hour/lane) 3	for Road Classification (veh/day/lane) 4	Required (One Direction)
1 Avenue	28 Street	25 Street	Westbound Westbound	3,360 2,210	5,570	Collector	Collector (Residential or Industrial)	400	4,000	1
1 Avenue	25 Street	Nelson Street	Westbound Westbound	6,650 5,560	12,210	Collector	Undivided Arterial	800	8,000	1
1 Avenue	Nelson Street	16 Street	Westbound Westbound	6,240 6,580	12,820	Collector	Undivided Arterial	800	8,000	1
Hwy 28	Hwy 55/16 Avenue	25 Street	Northbound Southbound	21,470 20,900	42,370	4-Lane Arterial	Expressway	1,800	18,000	2 2
8 Avenue	25 Street	16 Street	Westbound Westbound	11,990 8,580	20,570	4-Lane Arterial	Divided Arterial	1,000	10,000	2 1
8 Avenue	16 Street	10 Street	Westbound Westbound	4,630 3,910	8,540	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2 1
8 Avenue	10 Street	Lakeshore Drive	Westbound Westbound	2,720 3,210	5,930	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
Hwy 55	West City Limit	28 Street	Westbound Westbound	7,370 3,320	10,690	2-Lane Arterial	Undivided Arterial	800	8,000	1
Hwy 55	28 Street	Hwy 28	Westbound Westbound	14,440 11,280	25,720	2-Lane Arterial	Divided Arterial	1,000	10,000	2 2
16 Avenue	Hwy 28	16 Street	Westbound Westbound	8,660 6,650	15,310	2-Lane Arterial	Undivided Arterial	800	8,000	2 1
16 Avenue	16 Street	10 Street	Westbound Westbound	5,820 3,820	9,640	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
16 Avenue	10 Street	8 Street	Westbound Westbound	4,510 3,520	8,030	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2 1
16 Avenue	8 Street	East City Limit	Westbound Westbound	2,370 1,660	4,030	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
English Bay Road <sup>5</sup>	North City Limit	Lake Avenue	Northbound Southbound	6,430 5,920	12,350	Collector	Undivided Arterial	800	8,000	1
English Bay Road	Lake Avenue	1 Avenue	Northbound Southbound	12,860 11,840	24,700	Collector	Divided Arterial	1,000	10,000	2 2
English Bay Road	1 Avenue	25 Street	Northbound Southbound	16,370 13,760	30,130	Collector	Expressway	1,800	18,000	1
English Bay Road	25 Street	Hwy 28	Northbound Southbound	13,510 12,300	25,810	Collector	Divided Arterial	1,000	10,000	2 2
28 Street	English Bay Road	Hwy 55	Northbound Southbound	8,640 7,040	15,680	Collector	Undivided Arterial	800	8,000	2
25 Street	1 Avenue	English Bay Road	Northbound Southbound	5,560 4,950	10,510	Collector	Undivided Arterial	800	8,000	1 1
Nelson Street	1 Avenue	16 Street	Eastbound Westbound	2,760 1,600	4,360	Collector	Collector (Residential or Industrial)	400	4,000	1 1
16 Street	1 Avenue	8 Avenue	Northbound Southbound	9,290 4,530	13,820	Collector	Undivided Arterial	800	8,000	2
16 Street	8 Avenue	16 Avenue	Northbound Southbound	3,850 4,280	8,130	Collector	Collector (Residential or Industrial)	400	4,000	1 2
16 Street	16 Avenue	10 Street	Northbound Southbound	3,810 3,200	7,010	Collector	Collector (Residential or Industrial)	400	4,000	1
16 Street	10 Street	75 Avenue	Northbound Southbound	5,720 4,600	10,320	Collector	Undivided Arterial	800	8,000	1
10 Street	1 Avenue	8 Avenue	Northbound Southbound	3,400 3,320	6,720	Collector	Collector (Residential or Industrial)	400	4,000	1
10 Street	8 Avenue	16 Avenue	Northbound Southbound	3,560 4,250	7,810	Collector	Collector (Residential or Industrial)	400	4,000	1 2
10 Street	16 Avenue	16 Street	Northbound Southbound	2,380 2,300	4,680	Collector	Collector (Residential or Industrial)	400	4,000	1
8 Street	16 Avenue	75 Avenue	Northbound Southbound	2,840 2,480	5,320	Collector	Collector (Residential or Industrial)	400	4,000	1
6 Street <sup>6</sup>	16 Avenue	21 Avenue	Northbound Southbound	2,380 2,300	4,680	Collector	Collector (Residential or Industrial)	400	4,000	1
Hwy 28/55	Hwy 55/16 Avenue	75 Avenue	Northbound Southbound	30,680 31,810	62,490	4-Lane Arterial	Expressway	1,800	18,000	2 2
Hwy 28/55	75 Avenue	69 Avenue	Northbound Southbound	31,690 33,860	65,550	4-Lane Arterial	Expressway	1,800	18,000	2 2
Hwy 28/55	69 Avenue	54 Avenue	Northbound Southbound	24,020 28,920	52,940	4-Lane Arterial	Expressway	1,800	18,000	2 2
75 Avenue	Hwy 28/55	Future Arterial	Eastbound Westbound	4,390 4,730	9,120	Collector	Collector (Residential or Industrial)	400	4,000	2 2
69 Avenue	Glenwood	Hwy 28/55	Eastbound Westbound	5,590 4,480	10,070	2-Lane Arterial	Undivided Arterial	800	8,000	1
69 Avenue	Hwy 28/55	Future Arterial	Eastbound Westbound	8,350 7,440	15,790	Collector	Undivided Arterial	800	8,000	2
47 Street 7	69 Avenue	61/62 Avenue	Northbound Southbound	3,720 4,175	7,895	Collector	Collector (Residential or Industrial)	400	4,000	1 2
54 Avenue	56 Street	Hwy 28/55	Eastbound Westbound	3,850 5,610	9,460	Collector	Collector (Residential or Industrial)	400	4,000	1 2
54 Avenue	Hwy 28/55	51 Street	Eastbound Westbound	5,850 4,710	10,560	Collector	Undivided Arterial	800	8,000	1 1
54 Avenue	51 Street	45 Street	Eastbound Westbound	6,960	12,620	Collector	Undivided Arterial	800	8,000	1
54 Avenue 8	45 Street	41 Street	Eastbound Westbound	5,660 4,600 3,825	8,425	Collector	Collector (Residential or Industrial)	400	4,000	1 2 1
54 Avenue	41 Street	Future Arterial	Eastbound	2,240 1,990	4,230	Collector	Collector (Residential or Industrial)	400	4,000	1 1
52 Avenue	59 Street	57 Street	Westbound Eastbound	2,270	5,460	Collector	Collector (Residential or Industrial)	400	4,000	1
52 Avenue	57 Street	Hwy 28/55	Westbound Eastbound	3,190 3,920 5,710	9,630	Collector	Collector (Residential or Industrial)	400	4,000	1
Centre Avenue	59 Street	57 Street	Westbound Westbound	17,310	24,740	2-Lane Arterial	Divided Arterial	1,000	10,000	2
Centre Avenue	57 Street	Hwy 28/55	Westbound Eastbound	7,430 15,540 11,950	27,490	4-Lane Arterial	Divided Arterial	1,000	10,000	2
50 Avenue	Hwy 28/55	51 Street	Westbound Eastbound	7,940	13,510	2-Lane Arterial	Undivided Arterial	800	8,000	2 1 1
50 Avenue	51 Street	50 Street	Westbound Eastbound	5,570 6,800	10,920	2-Lane Arterial	Undivided Arterial	800	8,000	1
50 Avenue	50 Street	45 Street	Westbound Eastbound	4,120 6,610	9,340	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2
50 Avenue	45 Street	41 Street	Westbound Eastbound	2,730 6,170	9,220	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2
50 Avenue	41 Street	Future Arterial	Westbound Eastbound	3,050 4,880	8,320	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2
50 Avenue	Future Arterial	Baywood Road	Westbound Eastbound	3,440 2,070	3,630	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
43 Avenue	Hwy 28/55	45 Street	Westbound Eastbound	1,560 6,220	11,240	Collector	Undivided Arterial	800	8,000	1
59 Street	52 Avenue	Centre Avenue	Westbound Northbound	5,020 3,050	4,400	Collector	Collector (Residential or Industrial)	400	4,000	1
57 Street	54 Avenue	52 Avenue	Southbound Northbound	1,350 2,130	3,530	Collector	Collector (Residential or Industrial)	400	4,000	1
57 Street	52 Avenue	Centre Avenue	Southbound Northbound	1,400 3,960	6,470	Collector	Collector (Residential or Industrial)	400	4,000	1
Hwy 28/55	54 Avenue	52 Avenue	Southbound Northbound	2,510 20,930	45,540	4-Lane Arterial	Expressway	1,800	18,000	1 2
Hwy 28/55	52 Avenue	50 Avenue	Southbound Northbound	24,610 20,540	42,490	4-Lane Arterial	Expressway	1,800	18,000	2
			Southbound Northbound	21,950 10,700	,			,	.,	2 2
Hwy 28/55	50 Avenue	50 Street	Southbound Northbound	13,030 14,520	23,730	4-Lane Arterial	Divided Arterial	1,000	10,000	2
Hwy 28/55	50 Street 43 Avenue	43 Avenue 40 Avenue	Southbound Northbound	17,550 10,330	32,070 21,420	4-Lane Arterial 4-Lane Arterial	Expressway  Divided Arterial	1,800	18,000	1 2
Hwy 28/55			Southbound Northbound	11,090 5,165	, ,			1,000	.,	2
Hwy 28/55 9	40 Avenue	South City Limit	Southbound	5,545	10,710	4-Lane Arterial	Undivided Arterial	800	8,000	1

#### CAPACITY ANALYSIS - 20 YEAR (2030) HORIZON

	Intersection			Forecasted	d Volumes	Road Classification	Road Classification	Lane Capacity	Lane Capacity	Number of Lane
Corridor	From	То	Direction	Daily Traffic - Directional	Daily Traffic - Two Way	2000 TPS <sup>1</sup>	City of Cold Lake <sup>2</sup>	(veh/hour/lane) 3	for Road Classification (veh/day/lane) 4	Required (One Direction)
51 Street	54 Avenue	50 Avenue	Northbound	1,980	4,210	Collector	Collector (Residential or Industrial)	400	4,000	1
			Southbound Northbound	2,230 7,170	, ,				,	1 1
50 Street	50 Avenue	Hwy 28/55	Southbound	6,810	13,980	Collector	Undivided Arterial 800		8,000	1
45 Street	54 Avenue	50 Avenue	Northbound	700 450	1,150	Collector	Local (Residential or Industrial)	100	1,000	1
45 Street	50 Avenue	43 Avenue	Southbound Northbound	960	2.420	Collector	Local (Residential or Industrial)	100	1,000	1 1
40 00000	30 Aveilue	45 Aveilue	Southbound	1,460	2,420	Collector	Local (Residential of Industrial)	for Road Classification (veh/hour/lane) 3 400 400 400 400 400 400 400 400 400 4	1,000	2
41 Street	54 Avenue	50 Avenue	Northbound Southbound	3,870 2.510	6,380	Collector	Collector (Residential or Industrial)	400	4,000	1 1
Future Arterial	75 Avenue	69 Avenue	Northbound	6,900	12.070	2-Lane Arterial	Undivided Arterial	800	8.000	1
1 didio 7 il tolidi	707110100	OO / WOULD	Southbound	5,170 6.510	12,010	2 Edito / titorida	Ondivided / trends	000	0,000	
Future Arterial	69 Avenue	54 Avenue	Northbound Southbound	5,510	12,080	2-Lane Arterial	Undivided Arterial	800	8,000	11
Future Arterial	54 Avenue	50 Avenue	Northbound	4,730	8,570	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
			Southbound Eastbound	3,840 12.400						1 2
Kingsway	59 Street	Glenwood	Westbound	8,220	20,620	2-Lane Arterial	Divided Arterial	1,000	10,000	1
Kingsway	Timberline	Glenwood	Eastbound Westbound	14,040 8.710	22,750	Collector	Divided Arterial	1,000	10,000	2
Kingsway	Queensway	Timberline	Eastbound	7,020	12.210	Collector	Undivided Arterial	900	8,000	i
rungsway	Queensway	Timberine	Westbound	5,190	12,210	Collector			0,000	
Kingsway	Tennis Court Road	Queensway	Eastbound Westbound	1,280 2,020	3,300	Collector	Collector (Residential or Industrial)	400	4,000	1 1
Kingsway	End of Road	Tennis Court Road	Eastbound Westbound	1,440 2.040	3,480	Collector	Collector (Residential or Industrial)	400	4,000	1
		121	Northbound	330		0.11.				N/A
Tennis Court Road	Queensway	Kingsway	Southbound	70	400	Collector	Lane	N/A	N/A	N/A
Queensway	Tennis Court Road	Kingsway	Northbound Southbound	3,130 2.440	5,570	Collector	Collector (Residential or Industrial)	400	4,000	1 1
Queensway	Kingsway	Hanger Ln	Northbound	4,100	5.010	Collector	Collector (Residential or Industrial)	400	4.000	2
Queensway	Niligaway	rianger En	Southbound	910	3,010	Collector	Collector (Residential or Industrial)	Tor Road Classification (veh/hour/lane) 3	4,000	
Timberline	Juniper Avenue	Kingsway	Northbound Southbound	1,920	3,320	Collector	Collector (Residential or Industrial)	400	4,000	1 1
Timberline	Kingsway	Athahasca Road	Northbound	4,170	6.480	Collector	Collector (Residential or Industrial)	400	4.000	2
Timbonino	rangonay	7 ti lubububu 1 tudu	Southbound Northbound	2,310 8.460	0,400	Collected	Colocial (residential of massing)	100	4,000	1 2
Glenwood Drive	Glenwood	Kingsway	Southbound	5,270	13,730	2-Lane Arterial	Undivided Arterial	800	8,000	1

### **FINAL REPORT**



**Appendix D - Transportation Study Cold Lake North - Parking Study** 

# **Technical Memorandum**

# **City of Cold Lake**

**Transportation Study Cold Lake North - Parking Study** 

#### **April 2011**



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#### City of Cold Lake

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# Introduction

The City of Cold Lake (City) is located 287 km northeast of the City of Edmonton in Alberta and was formed in 1996 by merging three municipalities, namely Grand Centre, Medley (Canadian Forces Base W4) and Cold Lake. Grand Centre was subsequently renamed Cold Lake South (CLS) and the original Cold Lake is now known as Cold Lake North (CLN).

The City has experienced noticeable growth in recent years. According to municipal census the City had a population of 11,991 in 2006 and 13,924 in 2009. This corresponds to a 5.4% linear growth annually. Current transportation improvements within the City have been based on the previous transportation study completed in 2000 and is no longer considered representative of the actual transportation network required to address current and future transportation needs.

In light of the continuing accelerated pace of development in the region and the need to rationalize and identify the transportation network requirements for the City, including surrounding rural municipalities and counties, the existing transportation plan requires a comprehensive update.

#### 1.1 PROJECT BACKGROUND

Associated Engineering (AE) was retained by the City to update the 2000 transportation study. The purpose of the transportation study is to provide a comprehensive long range plan that integrates the transportation infrastructure requirements of the existing and future land uses. The transportation study will provide the City with a blueprint on which to plan and implement specific transportation network improvement projects over the next 20 years in 5-year, 10-year, 15-year and 20-year planning horizons. The transportation study will consider municipal roads, traffic calming, parking, traffic safety, traffic signal coordination, school zones, transit, truck routes, traffic management, and transportation system operations.

One component of the transportation study was to complete parking studies for CLN and CLS. The parking studies for the two study areas were completed independently. This technical memorandum documents the result of the parking study completed within CLN for the Canada Day long weekend, which represents the peak parking demand in the area.

#### 1.2 STUDY AREA

The City indicated that CLN experiences parking problems on Canada Day due to festivities along 1 Avenue and at Kinosoo Beach. The existing parking condition along Lakeshore Drive, 1 Avenue, and the adjacent areas, were considered to be of interest.

The study area for the CLN Parking Study is presented in Figure 1.1. The study area is roughly bounded by 23 Street/10 Street to the west, Cold Lake to the north and east, and 8 Avenue/2 Avenue to the south.



#### 1.3 STUDY METHODOLOGY

The following tasks were completed for the CLN Parking Study:

- Project initiation meeting,
- Site reconnaissance,
- Information and data collection,
- Parking survey and analysis,
- Development of parking strategies,
- Review meeting, and
- Draft and final reports.



PROJECT No. 2010—3050

DATE: SEPT 2010

APPROVED: SCALE: N.T.S.

DWG. No.

Associated Engineering

CITY OF COLD LAKE
COLD LAKE NORTH PARKING STUDY

FIGURE 1.1 STUDY AREA

### **Data Collection**

#### 2.1 SITE RECONNAISSANCE

A site reconnaissance was completed on May 5, 2010 to observe existing parking conditions within the CLN study area. The prevailing weather condition was cloudy with light flurries. The following observations were made during the site visit:

- No parking zones indicated by painted yellow curbs are provided around intersection corners, at fire hydrants, at alley ways, and at the locations indicated in Figure 2.1.
- A 2-hour parking zone is provided for approximately 250 m along the west side of 23 Street, between Birch Avenue and 1 Avenue and is illustrated in Figure 2.1.

The number of parking stalls provided in the off-street parking lots located at the marina (marina lot) and along 1 Avenue (1 Avenue Lot) were counted. The marina lot contains 58 regular parking stalls and 5 handicap stalls. The 1 Avenue Lot contains 75 regular parking stalls and 11 long stalls intended for recreational vehicles (RVs) and trailers.

An additional off-street parking lot was observed at Kinosoo Beach, in the northeast corner of Birch Avenue. This parking lot is a gravel lot; therefore, the parking stalls are not marked and could not be counted during the site visit. This lot will be referred to as the gravel lot in this report.

#### 2.2 PARKING INVENTORY

On-street parking stalls in CLN are not marked with paint lines. The existing on-street parking inventory was estimated by AE using curb lengths from the City's base map and the following assumptions:

- No parking around corners and alley ways and at the locations indicated in Figure 2.1.
- No parking in front of residential driveways. Where front driveways were present, a standard driveway width of 6.0 m was subtracted from the curb length.
- A standard parking stall length of 7.0 m.

The on-street parking inventory can vary depending on the space left between two parked vehicles. The estimated parking inventory was adjusted based on the parking survey results and observed parking behaviour characteristics. Figure 2.2 presents the on-street parking inventory for CLN.

The off-street parking inventory for the marina lot and the 1 Avenue Lot is presented in Figure 2.3. The parking inventory for the Gravel lot was obtained from the parking survey and is also presented in Figure 2.3.



#### 2.3 PARKING SURVEY

AE conducted the parking survey over a three day period from July 1 to July 3, 2010 to capture the peak parking condition expected in CLN. The prevailing weather condition was sunny on July 1 and July 2, and cloudy on July 3. The purpose of the parking survey was to complete the following:

- Collect parking utilization, duration and turnover data for all on-street parking.
- Collect parking utilization, duration and turnover data for the three off-street parking lots.
- Conduct parking interviews.

Parking utilization is defined as the number of parked vehicles (per hour) divided by the number of available parking stalls.

Parking duration is defined as the length of time, in hours, that a vehicle is parked in one parking stall.

Parking turnover is defined at the number of different vehicles parked within the study period divided by the number of available parking stalls.

Figure 2.4 presents a photograph of the parallel parking observed along 1 Avenue during the Canada Day festivities.



2-Hour Parking Zone

No Parking Zone

PROJECT No. <u>2010-3050</u> DATE: SEPT 2010 APPROVED:

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CITY OF COLD LAKE COLD LAKE NORTH PARKING STUDY

FIGURE 2.1 OBSERVED PARKED RESTRICTIONS



Study Boundary

On-Street Parking Stalls

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CITY OF COLD LAKE
COLD LAKE NORTH PARKING STUDY

FIGURE 2.2
ON-STREET PARKING INVENTORY



Study Boundary

Off-Street Parking Lots

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CITY OF COLD LAKE
COLD LAKE NORTH PARKING STUDY

FIGURE 2.3 OFF-STREET PARKING INVENTORY



Figure 2.4
Parallel Parking along 1 Avenue on Canada Day

#### 2.4 PARKING UTILIZATION, DURATION AND TURNOVER

Parking utilization, duration and turnover information was collected through a license plate survey. Surveyors collected the full license plate number of parked vehicles in the study area. Each location was revisited every hour for a six hour study period, from 11:00 a.m. to 4:00 p.m.

The license plate information was processed to obtain the following information and is provided in Appendix A:

- Total number of available parking stalls,
- Total number of parked vehicles per hour,
- Duration of parked vehicles, and
- Turnover rates.



#### 2.5 PARKING INTERVIEWS

During the parking survey, commuters in the study area were approached and asked the following parking related questions:

- 1. Where do you live?
- 2. Where are you coming from (origin)?
- 3. Where are you going (destination)?
- 4. How far from your destination did you park?
- 5. How far are you willing to walk from where you park to your destination?
- 6. How long did you/will you be parked for?
- 7. What was your reason for parking today?

A total of 76 interviews were conducted over the course of the three days. The responses were compiled and provided in Appendix B.

# **Data Synthesis**

#### 3.1 ON-STREET PARKING

To analyze the existing on-street parking conditions, the study area was examined as a whole, as different parking zones, and as separate study corridors.

The entire study area was broken into the following five parking zones, presented in Figure 3.1:

- Zone 1 Lakeshore Drive
- Zone 2 Residential area west of Lakeshore Drive
- Zone 3 1 Avenue and Kinosoo Beach
- Zone 4 Residential area west of Kinosoo Beach
- Zone 5 Residential area south of 1 Avenue.

The existing parking condition was analyzed in detail for three study corridors, which include:

- Lakeshore Drive, from 1 Avenue to 7 Street
- 10 Street, from 1 Avenue to 8 Avenue
- 1 Avenue, from 23 Street to 10 Street.

The on-street parking utilization, duration and turnover rates for the study area, study zones and study corridors are presented in Appendix C.

#### 3.2 OFF-STREET PARKING

Off-street parking conditions were analyzed for each of the three off-street parking lots independently. The off-street parking utilization, duration, and turnover rates for each parking lot are presented in Appendix D.

#### 3.3 PARKING INTERVIEWS

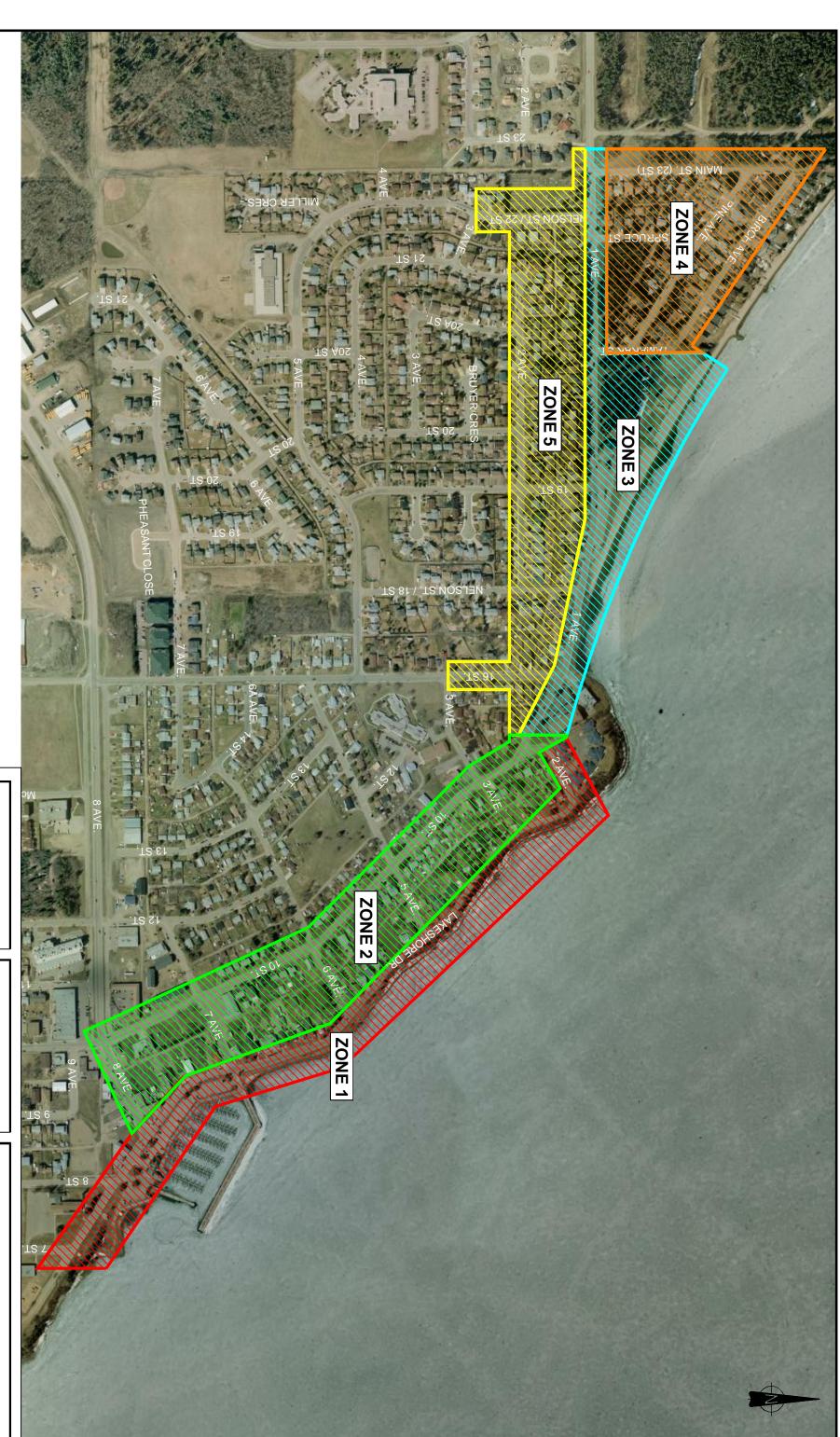
The responses from the parking interviews were summarized and included in Appendix E. The following summarizes the major findings from the Canada Day long weekend:

- The majority of commuters lived within the City 45% lived in CLN and 33% lived in CLS
- The majority of commuters started their trip from within the City 36% of the trips originated from CLN and 30% originated from CLS
- Half the commuters (50%) were destined for Kinosoo Beach and 14% were destined for the Waterfront/marina



#### City of Cold Lake

- The majority of commuters parked within 2 blocks of their destination 45% parked within less than 1 block, 14% parked within 1 block and 20% parked within 2 blocks
- Most commuters (43%) were willing to park further than 2 blocks from their destination
- Most commuters (28%) parked in their parking spot for 2 hours
- The predominant trip purpose for CLN was leisure (43%).



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FIGURE 3.1

PARKING STUDY ZONES

CITY OF COLD LAKE
COLD LAKE NORTH PARKING STUDY

# **On-Street Parking Data Analysis**

The existing on-street parking condition was analyzed for the entire study area and for the different parking zones and study corridors identified in previous section. The parking condition on July 1 represented the worst-case scenario and was analyzed separately from the parking condition on July 2 and July 3. The data for July 2 and July 3 were averaged and analyzed together to represent typical summer weekend parking conditions within CLN.

The on-street parking analysis was completed and provided in Appendix F. The following sections discuss the results from the analysis.

#### 4.1 ENTIRE STUDY AREA

#### 4.1.1 Parking Demand/Utilization

The existing on-street parking supply was able to accommodate the overall on-street parking demand in the entire study area. The parking demand on July 1 was higher than the parking demand on July 2 and July 3; the parking demand on July 1 was almost double the parking demand on July 2 and July 3. The highest overall parking demand was observed on July 1 at 1:00 p.m. when the parking utilization was 44%.

#### 4.1.2 Parking Duration

The majority of commuters parked for 2 hours or less. On July 1, 48% of commuters parked for 1 hour and 27% parked for 2 hours. On July 2 and July 3, an average of 45% of commuters parked for 1 hour and an average of 16% parked for 2 hours. It should be noted that the proportion of commuters who parked for 6 hours was significantly larger on July 2 and July 3 (22%) than on July 1 (7%).

#### 4.1.3 Parking Turnover

The turnover rate for the entire study area was 1.09 on July 1, 0.32 on July 2 and 0.41 on July 3. The turnover rate is much higher on July 1 (Canada Day) than on a typical day on the weekend.



#### 4.2 STUDY ZONES

#### 4.2.1 Parking Demand/Utilization

The existing on-street parking supply within each parking zone was able to accommodate the onstreet parking demand for the zone. Generally, the on-street parking demand observed on July 1 was higher than the parking demand observed on July 2 and July 3. The most significant difference in parking demand occurred in Zone 3, where the parking demand on July 1 was more than seven times the demand observed on July 2 and July 3.

#### 4.2.2 Parking Duration

#### Zone 1

The majority of commuters in Zone 1 parked for 2 hours or less. On July 1, 59% of commuters parked for 1 hour and 15% parked for 2 hours. On July 2 and July 3, an average of 65% of commuters parked for 1 hour and an average of 18% parked for 2 hours.

#### Zone 2

The majority of commuters in Zone 2 parked for 2 hours or less. On July 1, 50% of commuters parked for 1 hour and 20% parked for 2 hours. On July 2 and July 3, an average of 45% of commuters parked for 1 hour and an average of 16% parked for 2 hours. It should be noted that on July 2 and July 3, and average of 25% of commuters parked for 6 hours.

#### Zone 3

The majority of commuters in Zone 3 parked for 2 hours or less. On July 1, 53% of commuters parked for 1 hour and 28% parked for 2 hours. On July 2 and July 3, an average of 37% of commuters parked for 1 hour and an average of 18% parked for 2 hours. It should be noted that on July 2 and July 3, and average of 27% of commuters parked for 6 hours.

#### Zone 4

The majority of commuters in Zone 4 parked for 2 hours or less. On July 1, 40% of commuters parked for 1 hour and 25% parked for 2 hours. On July 2 and July 3, an average of 40% of commuters parked for 1 hour and an average of 16% parked for 2 hours. It should be noted that on July 2 and July 3, and average of 27% of commuters parked for 6 hours.

#### Zone 5

On July 1, the majority of commuters in Zone 5 parked for 2 hours or less, with 40% of commuters parked for 1 hour and 34% of commuters parked for 2 hours. On July 2 and July 3, most commuters parked for 6 hours (35%) or 1 hour (24%).

#### 4.2.3 Parking Turnover

The turnover rates observed on July 1 are higher than the average turnover rates observed on July 2 and July 3. On July 1, Zone 3 and Zone 5 experienced the highest turnover rates at 1.88 and 1.29 respectively. It should be noted that Zone 3 and Zone 5 have the lowest average turnover rates on July 2 and July 3.

#### 4.3 STUDY CORRIDORS

#### 4.3.1 Parking Demand/Utilization

The existing on-street parking supply within each study corridor was able to accommodate the onstreet parking demand for the corridor. The parking demand along Lakeshore Drive and 10 Street remained relatively consistent over the three day study period. The parking demand on 1 Avenue, however, was significantly higher on July 1 than on July 2 and July 3.

#### 4.3.2 Parking Duration

#### **Lakeshore Drive**

The majority of commuters along Lakeshore Drive parked for 2 hours or less. On July 1, 60% of commuters parked for 1 hour and 15% parked for 2 hours. On July 2 and July 3, an average of 66% of commuters parked for 1 hour and an average of 18% parked for 2 hours.

#### 10 Street

On July 1, the majority of commuters along 10 Street parked for 2 hours or less, with 39% of commuters parked for 1 hour and 27% of commuters parked for 2 hours. On July 2 and July 3, most commuters along 10 Street either parked for 6 hours (44%) or 1 hour (24%).

#### 1 Avenue

On July 1, the majority of commuters along 1 Avenue parked for 2 hours or less, with 53% of commuters parked for 1 hour and 28% of commuters parked for 2 hours. On July 2 and July 3, most commuters along 1 Avenue either parked for 1 hour (37%) or 6 hours (27%).

#### 4.3.3 Parking Turnover

The turnover rates observed on July 1 are higher than the average turnover rates observed on July 2 and July 3. On July 1, the 1 Avenue corridor experiences the highest turnover rates at 1.88. It should be noted that the 1 Avenue corridor has the lowest average turnover rates on July 2 and July 3.



#### 4.4 OTHER ON-STREET PARKING ISSUES

#### 4.4.1 Illegal Parking

Parking was observed at the following no parking zones during the three-day study period:

- East side of Lakeshore Drive, from 7 Street to 8 Street July 2 and July 3
- East side of Lakeshore Drive, from 8 Avenue to 8 Street July 1
- East side of Lakeshore Drive, from 6 Avenue to 7 Avenue July 1
- South side of 1 Avenue from 16 Street to 10 Street July 1.

The no parking zones are indicated by yellow paint on the side of the curb. Visitors unfamiliar with the area may not understand that the yellow curb paint indicates no parking zones. During the wintertime the yellow curb paint may also become obscured under the snow. AE recommends that no parking signs be installed at the no parking zones to reinforce the parking restriction. Enforcement of the no parking zones should also be increased to ensure that the parking restrictions are obeyed.

#### 4.4.2 Vehicle Type

The following vehicle types were observed during the parking survey, aside from passenger cars:

- Bicycles
- Motorcycles
- Recreational vehicles
- Trailer/boat trailers
- Farm vehicles
- Other.

Of particular interest was the number of RVs and trailers parked on-street in the study area. These vehicle types are longer and will require more than one on-street parking stall. RVs and trailers accounted for approximately 2% of on-street parked vehicles on July 1, 2010, approximately 6% of on-street parked vehicles on July 2, 2010, and 4% of on-street parked vehicles on July 3, 2010. The percentages quoted above are for the entire study area over the entire study period (11:00 a.m. to 4:00 p.m.) each day. It should be noted that Zone 1 (Lakeshore Drive) contained the highest proportion of RVs and trailers for all three days surveyed.

# **Off-Street Parking Data Analysis**

The existing off-street parking condition was analyzed for each parking lot independently and provided in Appendix G. The following sections discuss the results from the analysis.

#### 5.1 MARINA LOT

#### 5.1.1 Parking Demand/Utilization

The existing parking supply provided in the marina lot was unable to accommodate the parking in several time periods, over the three days observed. The parking demand exceeded the parking supply from 12:00 p.m. to 5:00 p.m. on July 1 and the average parking demand exceeded the parking supply from 1:00 p.m. to 3:00 p.m. on July 2 and July 3.

#### 5.1.2 Parking Duration

The majority of commuters in the marina lot parked for 2 hours or less. On July 1, 59% of commuters parked for 1 hour and 17% of commuters parked for 2 hours. On July 2 and July 3, an average of 62% of commuters parked for 1 hour and an average of 17% of commuters parked for 2 hours.

#### 5.1.3 Parking Turnover

The parking turnover rate for the marina lot was 3.27 on July 1, 2.56 on July 2 and 3.14 on July 3. The turnover rate for the marina lot was relatively consistent over the course of the 3 day study period and was high when compared to the turnover rates for on-street parking and for the other two off-street lots.

#### 5.2 1 AVENUE LOT

#### 5.2.1 Parking Demand/Utilization

The existing parking supply provided in the 1 Avenue Lot was able to accommodate the parking demand, for the entire study period. On July 1, parking in the 1 Avenue Lot was only available to vendors participating in the Canada Day parade. Public parking was not allowed.

#### 5.2.2 Parking Duration

On July 1, most of the commuters in the 1 Avenue Lot parked for 6 hours (36%). On July 2 and July 3, the majority of commuters parked for 2 hours or less, with an average of 63% of commuters parking for 1 hour and an average of 23% of commuters parking for 2 hours.



#### 5.2.3 Parking Turnover

The parking turnover rate for the 1 Avenue Lot was 0.63 on July 1, 0.62 on July 2 and 0.53 on July 3. The turnover rate for the 1 Avenue Lot was relatively consistent over the course of the 3 day study period and was low when compared to the marina lot.

#### 5.3 GRAVEL LOT

#### 5.3.1 Parking Demand/Utilization

The existing parking supply provided in the gravel lot was able to accommodate the parking demand, for the entire study period. It should be noted that the parking demand on July 1 is significantly higher than the average parking demand on July 2 and July 3.

#### **5.3.2 Parking Duration**

The majority of commuters in the gravel lot parked for 2 hours or less. On July 1, 38% of commuters parked for 1 hour and 28% of commuters parked for 2 hours. On July 2 and July 3, an average of 50% of commuters parked for 1 hour and an average of 30% of commuters parked for 2 hours.

#### 5.3.3 Parking Turnover

The parking turnover rate for the gravel lot was 2.14 on July 1, 0.19 on July 2 and 0.16 on July 3. The turnover rates indicate that the gravel lot was highly utilized on Canada Day, but was underutilized for the remainder of the study period.

#### 5.4 OTHER OFF-STREET PARKING ISSUES

#### 5.4.1 Illegal Parking

The utilization rate for the marina lot exceeded 100% every day for the three-day study period. This indicates that the number of vehicles parked in the marina lot exceeded the number of parking stalls available and provides evidence that people were making their own parking spots and parking illegally.

#### 5.4.2 Vehicle Type

The number of RVs and trailers parked in the off-street lots are very low during the study period. On July 1, 2010, only one RV was observed in the marina lot and one trailer was observed in the 1 Avenue Lot, over the course of the 6-hour study period. No RVs or trailers were observed in the off-street lots on July 2, 2010 or July 3, 2010. The 1 Avenue Lot currently contains 11 long stalls that are intended for RVs and trailers. The demand for RV and trailer parking can be easily accommodated by these stalls.



# **Lakeshore Redevelopment Plan Considerations**

The Lakeshore Redevelopment Plan (LRP) was finalized in March 2010 and provided a strategic direction for the revitalization of the Lakeshore Area to a vibrant "Urban Village" that would attract residents and tourists. The LRP considered various aspects of revitalization including changes to the character of the area, infrastructure upgrades, park and public space upgrades, parking improvements, and pedestrian network upgrades.

Section 3.3.1 of the Lakeshore Redevelopment Plan identified a need to reconstruct a portion of Lakeshore Drive between 7 Avenue and 8 Avenue. Four design options were developed for the reconstruction and are presented below and in Figure 6.1:

**Option 1:** Reconstruct Lakeshore Drive in its current configuration. The advantage of this option is that it maintains the existing level of on-street parking (28 parallel stalls) and two-way traffic flow, however it does not allow for an expanded pedestrian area. AE estimated 35 parallel stalls available on Lakeshore Drive between 7 Avenue and 8 Avenue.

**Option 2:** Reconstruct Lakeshore Drive as a one-way southbound. This configuration allows for 31 parking stalls set at a 45-degree angle, and an expansion of the sidewalk area to accommodate increased pedestrian use. Additionally, the one-way configuration may reduce the amount of vehicle traffic thereby increasing pedestrian safety.

**Option 3:** Reconstruct Lakeshore Drive as a one-way northbound. This option allows for 34 parking stalls set at a 45-degree angle and expanded pedestrian space. By allowing northbound traffic, this configuration allows traffic arriving via 8 Avenue (Highway 28) to disperse more efficiently.

**Option 4:** (Recommended option) Reconstruct Lakeshore Drive as a one-way northbound with parking on alternate sides of the driving lane. This configuration allows for 35 parking stalls set at a 45-degree angle while still accommodating an expanded pedestrian area. The northbound routing permits efficient vehicle circulation, while alternating parking from one side to the other creates a traffic-calming measure to reduce vehicle speeds.

AE completed a sensitivity analysis to determine the impact on parking as a result of the four design options. Table 6.1 presents the expected parking supply along Lakeshore Drive between 7 Avenue and 8 Avenue for each of the design options, and Table 6.2 summarizes the parking demand observed in the worst-case (Canada Day) and typical summer weekend scenarios.



Table 6.1 Expected Parking Supply for Design Options

Design Option	Parking Supply
Option 1	35
Option 2	31
Option 3	34
Option 4	35

Table 6.2
Observed Parking Demand

	Parking Demand (Vehicles)				
Time	July 1, 2010 (Worst-case Scenario)	July 2 & 3, 2010 (Typical Summer Weekend Scenario)			
11:00 a.m.	10	14			
12:00 p.m.	11	13			
1:00 p.m.	21	15			
2:00 p.m.	17	14			
3:00 p.m.	14	10			
4:00 p.m.	4	9			

A comparison of the expected parking supply versus the existing parking demand indicates that regardless of the design option implemented, the expected parking supply along Lakeshore Drive (between 7 Avenue and 8 Avenue) will accommodate the demand.



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FROM APPENDIX B, CITY OF COLD LAKE, LAKESHORE REDEVELOPMENT PLAN (MARCH 2010)

COLD LAKE NORTH PARKING STUDY CITY OF COLD LAKE

FIGURE 6.1 LAKESHORE REDEVELOPMENT PLAN DESIGN OPTIONS

N.I.S.

Associated Engineering



OPTION 3

# **Summary and Recommendations**

#### 7.1 SUMMARY OF ANALYSIS RESULTS

From the existing parking conditions analysis, the following conclusions can be made:

- Overall, the existing on-street parking supply is able to accommodate the on-street parking demand
- The existing off-street parking supply provided in the marina lot is unable to accommodate the offstreet parking demand
- The existing off-street parking supply provided in the 1 Avenue Lot is able to accommodate the offstreet parking demand
- The existing off-street parking supply provided in the gravel lot is able to accommodate the offstreet parking demand but approaches capacity on July 1
- For both on-street and off-street parking, the parking demand observed on July 1 (Canada Day) was higher than the parking demand on July 2 and 3, which represents a typical weekend period
- The difference in parking demand between Canada Day and a typical weekend period was most noticeable in the areas surrounding the 1 Avenue corridor and Kinosoo Beach. Zone 3, Zone 4 and Zone 5 were most affected by the increase in parking demand on Canada Day along with the 1 Avenue corridor and the gravel lot
- Parking in "no parking" zones was observed at various locations along Lakeshore Drive and 1
   Avenue
- Illegal parking in the off-street lot was observed in the marina lot.

#### 7.2 PARKING STRATEGIES

The following parking strategies were developed to improve the existing parking condition in CLN:

- Provide summer overflow parking for the marina lot
- Provide marked (painted) on-street parking stalls
- Enforce "no-parking" zones
- Pave and paint stalls in the gravel lot.

#### 7.2.1 Provide Summer Overflow Parking for the marina lot

The capacity of the marina lot was exceeded in both the worst-case (Canada Day) and the typical summer weekend scenarios. The peak parking utilization observed on Canada Day was 124% while the peak parking utilization observed on July 2 and July 3 was 106%.

The marina is open from mid-May to the end of September. The parking utilization rates observed during the parking survey can be assumed for the weekends in the summer months. During the off-season, the parking utilization is expected to be much lower.



The City indicated in the Lakeshore Redevelopment Plan that there are currently four lots being used for overflow parking for the marina. The LRP recommends that the City provide a landscaped parking lot on one of the four lots currently used. The landscaped parking lot would be reserved for the marina from mid-May to the end of September and would be available for the general public for the remainder of the year. The results from the parking study confirm the need for an additional parking lot to accommodate overflow parking from the marina and AE supports the recommendations presented in the LRP.

#### 7.2.2 Provide Marked (Painted) On-Street Parking Stalls

With unmarked on-street parking stalls, the available parking supply is dependent on local parking behaviour and the amount of space left between two parked vehicles. Larger spaces would result in less on-street parking stalls available.

The City should consider marking the pavement to indicate on-street parking stalls along 1 Avenue and Lakeshore Drive. Painted on-street parking should be provided along the 1 Avenue from 25 Street to Lakeshore Drive, and along Lakeshore Drive from 2 Avenue to 7 Street. Painted stalls will help to regulate and maximize parking stalls along these corridors and minimize the gaps left between two parked vehicles.

Additionally, the City could enhance the current parking zones by providing more delineation. Parking zones could be physically delineated with concrete bulbs or pavement markings to mark the start and end of the parking zone. The City should consider the concrete bulbs in conjunction with the traffic calming measures proposed in the In-Service Road Safety Reviews technical memorandum for 1 Avenue and Lakeshore Drive corridors.

#### 7.2.3 Enforce "No-Parking" Zones

Illegal parking was observed at several "no-parking" zones within CLN. "No parking" zones within the City are indicated by painted yellow curbs that might be unfamiliar to some visitors and may become obscured during the winter months. The City should consider the installation of Parking Control (RB-51, RB-52) signs to complement the painted curbs and enhance the parking restriction. Figure 7.1 presents examples of Parking Control (RB-51, RB-52) signs from the Manual of Uniform Traffic Control Devices (MUTCD).

Figure 7.1
Parking Control (RB-51, RB-52) Signs







The City should review the no-parking zones along 1 Avenue and Lakeshore Drive on a typical, non-summer, weekday and weekend and evaluate the extent of the no-parking violations and the impact on traffic flow. If the no-parking violations do not have adverse effects on traffic flow, the City should consider the removal of the no-parking zones. If there are adverse effects, the City should consider stricter enforcement. Consistent enforcement of the no-parking policy will reduce the no-parking violations.

The City indicated that complaints have been received regarding illegal parking at various locations throughout Cold Lake. AE recommends that the City monitor the parking conditions throughout the City and determine the effect on traffic operations. Areas which have been specifically identified to have illegal parking issues include: 16 Street, roadways adjacent to schools and playgrounds, and snow removal routes.

#### 7.2.4 Pave and Paint Stalls in the gravel lot

The City should consider paving the gravel lot and providing pavement markings to indicate the designated stalls, in accordance with the policies outlined in the Lakeshore Redevelopment Plan regarding parking. Painted parking stalls will help to regulate the available parking supply in the gravel lot and maximize the number of available stalls, alleviating parking supply problems during peak periods.

Opportunities exist to integrate streetscaping and landscaping in the gravel lot to coordinate with the beautification efforts for the Lakeshore commercial and beachfront areas. Landscaped islands with shrubs and trees can be provided between and at the end of aisles to improve the aesthetic of the parking lot. Additionally good illumination and a designated pathway between the parking lot and Kinosoo Beach could increase the utilization of the gravel lot.





# **Appendix A - Parking Survey Data**



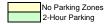
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2 Avenue         20 Street         19 Street         South         7         7         5         5         5         4         3         100%         71%         71%         71%         57%         43%         4         5         1         3         0         0         12         1.71           2 Avenue         19 Street         Nelson Street / 18 Street         North         12         10         6         7         6         3         3         83%         50%         58%         50%         25%         25%         11         3         6         0         0         0         19         1.58           2 Avenue         19 Street         Nelson Street / 18 Street         South         13         6         4         6         6         5         4         46%         31%         46%         46%         38%         31%         2         11         1         1         0         0         13         1.00	Ī								5			· ·														7	
2 Avenue 19 Street Nelson Street / 18 Street Nelson Street / 18 Street North 12 10 6 7 6 3 3 83% 50% 58% 50% 25% 25% 11 3 6 0 0 0 19 1.58 2 Avenue 19 Street Nelson Street / 18 Street South 13 6 4 6 6 5 4 46% 31% 46% 46% 38% 31% 2 11 1 1 0 0 0 13 1.00								7																		12	
				19 Street	Nelson Street / 18 Street	t North	12	10	6	7	6	3	3		50%							6				19	1.58
Zone #5 Total 180 85 78 89 80 75 68 51% 49% 54% 48% 45% 41% 90 76 27 23 0 10 212 1.29					Nelson Street / 18 Street	t South																					
				Zone #5 Total			180	85	78	89	80	75	68	51%	49%	54%	48%	45%	41%	90	76	27	23	0	10	212	1.29



### COLD LAKE NORTH - PARKING UTILIZATION - JULY 2, 2010

						Darking Stalla			PARKED VEHICLES				В	A DVINC LITH	LIZATION (%)					DADVING	DURATION			DARVING	TURNOVER
Zone	Type	Road	From	То	Side	Parking Stalls Available	11:00 AM	12:00 PM		3:00 PM	4:00 PM	11:00 AM		1:00 PM		3:00 PM	4:00 PM	1 Hour	2 Hours			5 Hours	6 Hours	Unique Veh.	Turnover Rate
		Lakeshore Drive	8 Street	7 Street	North	0	2 2	12.00 FW	5 5	5.00 FW	4.00 FW		12.00 FW	1.00 FW	2.00 F W	3.00 FW	4.00 FW	1 11001	0	0	0	3 110urs	1	6	-
		Lakeshore Drive	8 Street	7 Street	South	14	0	4	6 6	6	7	0%	29%	43%	43%	43%	50%	1	0	0	2	4	0	7	0.50
		Lakeshore Drive	8 Avenue	8 Street	North	0	0	0	0 0	0	0	-	-	-	-	-	-	0	0	0	0	0	0	0	-
		Lakeshore Drive	8 Avenue	8 Street	South	7	0	0	2 1	2	2	0%	0%	29%	14%	29%	29%	4	0	1	0	0	0	5	0.71
		Lakeshore Drive	7 Avenue	8 Avenue	East	19	3	5	9 4	0	4	16%	26%	47%	21%	0%	21%	23	1	0	0	0	0	24	1.26
		Lakeshore Drive	7 Avenue	8 Avenue	West	16	3	6	5 3	3	2	19%	38%	31%	19%	19%	13%	9	4	0	0	1	0	14	0.88
	0	Lakeshore Drive	6 Avenue	7 Avenue	East	0	0	0	0 0	0	0	-	-	-	-	-	-	0	0	0	0	0	0	0	-
1	On Street	Lakeshore Drive	6 Avenue	7 Avenue	West	8	1	3	4 6	5	4	13%	38%	50%	75%	63%	50%	6	2	1	1	0	1	11	1.38
		Lakeshore Drive	5 Avenue	6 Avenue	East	0	0	0	0 0	0	0	-	-	-	-	-	-	0	0	0	0	0	0	0	-
		Lakeshore Drive	5 Avenue	6 Avenue	West	14	0	0	1 0	0	0	0%	0%	7%	0%	0%	0%	1	0	0	0	0	0	1	0.07
		Lakeshore Drive	2 Avenue	5 Avenue	East	0	0	0	0 0	0	0	-	-	-	-	-	-	0	0	0	0	0	0	0	-
		Lakeshore Drive	2 Avenue	5 Avenue	West	30	1	1	1 2	2	1	3%	3%	3%	7%	7%	3%	2	0	0	0	0	1	3	0.10
		2 Avenue	Lakeshore Drive	10 Street	North	0	0	0	0 0	0	0	-	-	-	-	-	-	0	0	0	0	0	0	0	-
		2 Avenue	Lakeshore Drive	10 Street	South	9	3	1	2 2	2	1	33%	11%	22%	22%	22%	11%	2	0	1	0	0	1	4	0.44
	Off Street		Marina Parking Lo	ot		63	38	52	<b>64</b> 63	47	49	60%	83%	102%	100%	75%	78%	110	25	16	11	5	6	161	2.56
			Zone #1 Total			180	51	77	99 92	72	75	16%	25%	37%	33%	28%	28%	159	32	19	14	14	10	236	0.88
		10 Street	1 Avenue	3 Avenue	West	3	0	0	0 0	0	0	0%	0%	0%	0%	0%	0%	0	0	0	0	0	0	0	0.00
		10 Street	1 Avenue	3 Avenue	East	9	2	2	2 2	2	1	22%	22%	22%	22%	22%	11%	0	0	0	0	1	1	2	0.22
		10 Street	3 Avenue	5 Avenue	West	19	0	0	0 0	0	0	0%	0%	0%	0%	0%	0%	0	0	0	0	0	0	0	0.00
		10 Street	3 Avenue	5 Avenue	East	16	3	2	3 2	2	2	19%	13%	19%	13%	13%	13%	2	0	0	0	0	2	3	0.19
		10 Street	5 Avenue	6 Avenue	West	15	1	0	0 0	0	0	7%	0%	0%	0%	0%	0%	1	0	0	0	0	0	1	0.07
		10 Street	5 Avenue	6 Avenue	East	15	2	2	2 2	2	2	13%	13%	13%	13%	13%	13%	0	0	0	0	0	2	2	0.13
		10 Street	6 Avenue	7 Avenue	West	15	6	6	6 6	7	6	40%	40%	40%	40%	47%	40%	1	0	0	0	0	6	7	0.47
		10 Street	6 Avenue	7 Avenue	East	14	5	5	5 3	4	5	36%	36%	36%	21%	29%	36%	3	3	2	0	0	2	6	0.43
	0 . 0	10 Street	7 Avenue	8 Avenue	West	19	1	1	0 0	0	0	5%	5%	0%	0%	0%	0%	0	1	0	0	0	0	1	0.05
2	On Street	10 Street	7 Avenue	8 Avenue	East	14	0	1	2 2	2	2	0%	7%	14%	14%	14%	14%	1	0	0	2	0	0	3	0.21
		5 Avenue	Lakeshore Drive	10 Street	North	12	1	1	4 2	2	1	8%	8%	33%	17%	17%	8%	4	0	1	1	0	0	6	0.50
		5 Avenue	Lakeshore Drive	10 Street	South	12	5	7	4 4	4	4	42%	58%	33%	33%	33%	33%	2	1	0	0	0	4	7	0.58
		6 Avenue	Lakeshore Drive	10 Street	North	11	2	2	2 2	2	2	18%	18%	18%	18%	18%	18%	1	0	0	0	1	1	3	0.27
		6 Avenue	Lakeshore Drive	10 Street	South	14	2	2	1 2	2	2	14%	14%	7%	14%	14%	14%	0	1	1	0	0	1	2	0.14
		7 Avenue	Lakeshore Drive	10 Street	North	14	2	2	1 2	2	1	14%	14%	7%	14%	14%	7%	2	1	0	0	0	1	4	0.29
		7 Avenue	Lakeshore Drive	10 Street	South	13	4	3	5 3	2	2	31%	23%	38%	23%	15%	15%	6	2	1	0	0	1	10	0.77
		8 Avenue	Lakeshore Drive	10 Street	North	18	2	1	2 4	4	4	11%	6%	11%	22%	22%	22%	3	1	2	0	0	1	7	0.39
		8 Avenue	Lakeshore Drive	10 Street	South	9	2	1	3 2	3	3	22%	11%	33%	22%	33%	33%	7	2	1	0	0	0	9	1.00
			Zone #2 Total	10 011001	Coun	242	40	38	42 38	40	37	17%	16%	18%	16%	17%	16%	33	12	8	3	2	22	73	0.32
		1 Avenue	23 Street	Spruce Street	North	11	0	0	0 0	0	0	0%	0%	0%	0%	0%	0%	0	0	0	0	0	0	0	0.00
		1 Avenue	Spruce Street	Tamarak Street	North	22	0	0	0 0	0	0	0%	0%	0%	0%	0%	0%	0	0	0	0	0	0	0	0.00
		1 Avenue	Tamarak Street	Nelson Street / 18 Street	North	51	2	1	1 1	3	3	4%	2%	2%	2%	6%	6%	1	2	0	0	0	1	4	0.08
		1 Avenue	Nelson Street / 18 Street	16 Street	North	18	0	0	0 0	0	0	0%	0%	0%	0%	0%	0%	0	0	0	0	0	0	0	0.00
		1 Avenue	16 Street	10 Street	North	8	2	2	2 2	2	4	25%	25%	25%	25%	25%	50%	2	0	0	0	0	2	4	0.50
	On Street	1 Avenue	23 Street	Nelson Street / 22 Street	South	8	0	0	0 0	0	0	0%	0%	0%	0%	0%	0%	0	0	0	0	0	0	0	0.00
3		1 Avenue	Nelson Street / 22 Street	19 Street	South	41	5	6	7 4	8	7	12%	15%	17%	10%	20%	17%	6	4	1	2	0	2	14	0.34
		1 Avenue	19 Street	Nelson Street / 18 Street	South	13	0	0	0 0	0	0	0%	0%	0%	0%	0%	0%	0	0	0	0	0	0	0	0.00
		1 Avenue	Nelson Street / 18 Street	16 Street	South	9	2	2	2 2	2	2	22%	22%	22%	22%	22%	22%	0	0	0	0	0	2	2	0.22
		1 Avenue	16 Street	10 Street	South	0	0	0	0 0	0	0	-	-	-	-	-	-	0	0	0	0	0	0	0	-
		171701140	1 Avenue Parking I		Coun	86	5	7	20 30	19	5	6%	8%	23%	35%	22%	6%	29	16	7	1	0	0	53	0.62
	Off Street		Unmarked Parking			58	1	3	4 6	5	3	2%	5%	7%	10%	9%	5%	5	2	3	1	0	0	11	0.19
		•	Zone #3 Total			325	17	21	36 45	39	24	6%	7%	9%	9%	9%	10%	43	24	11	4	0	7	88	0.18
		23 Street	Birch Avenue	1 Avenue	West	42	2	2	1 2	1	3	5%	5%	2%	5%	2%	7%	5	0	0	0	0	1	5	0.12
		23 Street	Birch Avenue	Pine Avenue	East	6	2	1	2 2	1	1	33%	17%	33%	33%	17%	17%	1	1	0	0	0	1	3	0.50
		23 Street	Pine Avenue	1 Avenue	East	22	7	6	6 6	6	4	32%	27%	27%	27%	27%	18%	1	0	0	0	2	4	7	0.32
		Birch Avenue	23 Street	Tamarak Street	North	28	5	5	7 5	4	2	18%	18%	25%	18%	14%	7%	5	0	1	2	0	2	9	0.32
		Birch Avenue	23 Street	Tamarak Street	South	29	9	9	7 6	4	9	31%	31%	24%	21%	14%	31%	12	5	0	1	0	3	18	0.62
	0- 0:	Pine Avenue	23 Street	Tamarak Street	North	31	8	7	8 7	6	7	26%	23%	26%	23%	19%	23%	4	1	0	2	1	4	11	0.35
4	On Street	Pine Avenue	23 Street	Spruce Street	South	12	5	5	5 5	4	4	42%	42%	42%	42%	33%	33%	0	0	0	1	0	4	5	0.42
		Pine Avenue	Spruce Street	Tamarak Street	South	16	3	3	2 2	2	4	19%	19%	13%	13%	13%	25%	2	1	0	0	0	2	4	0.25
		Spruce Street	Pine Avenue	1 Avenue	West	12	0	0	0 0	0	0	0%	0%	0%	0%	0%	0%	0	0	0	0	0	0	0	0.00
		Spruce Street	Pine Avenue	1 Avenue	East	17	0	1	1 0	0	0	0%	6%	6%	0%	0%	0%	0	1	0	0	0	0	1	0.06
		Tamarak Street	Birch Avenue	1 Avenue	West	9	0	0	0 0	0	0	0%	0%	0%	0%	0%	0%	0	0	0	0	0	0	0	0.00
		Tamarak Street	Birch Avenue	1 Avenue	East	16	1	2	1 1	1	0	6%	13%	6%	6%	6%	0%	1	0	0	0	1	0	2	0.13
			Zone #4 Total			240	42	41	40 36	29	34	18%	17%	17%	16%	12%	13%	31	9	1	6	4	21	65	0.26
		Nelson Street / 22 Street	1 Avenue	3 Avenue	West	21	4	6	4 4	5	4	19%	29%	19%	19%	24%	19%	3	0	0	0	0	4	7	0.33
		Nelson Street / 22 Street	1 Avenue	3 Avenue	East	15	3	2	2 2	2	2	20%	13%	13%	13%	13%	13%	1	0	0	0	0	2	3	0.20
		19 Street	1 Avenue	2 Avenue	West	11	0	0	0 0	0	0	0%	0%	0%	0%	0%	0%	0	0	0	0	0	0	0	0.00
		19 Street	1 Avenue	2 Avenue	East	10	3	2	1 1	1	1	30%	20%	10%	10%	10%	10%	2	1	0	0	1	0	3	0.30
		Nelson Street / 18 Street	1 Avenue	2 Avenue	West	7	0	0	0 0	0	0	0%	0%	0%	0%	0%	0%	0	0	0	0	0	0	0	0.00
		Nelson Street / 18 Street	1 Avenue	2 Avenue	East	8	3	2	2 2	2	2	38%	25%	25%	25%	25%	25%	1	0	0	0	0	2	3	0.38
1 _ 1	0.6	16 Street	1 Avenue	3 Avenue	West	18	3	3	3 0	0	1	17%	17%	17%	0%	0%	6%	1	0	3	0	0	0	3	0.17
5	On Street	16 Street	1 Avenue	3 Avenue	East	13	5	5	4 4	3	3	38%	38%	31%	31%	23%	23%	0	1	0	1	0	3	5	0.38
		2 Avenue	21 Street	19 Street	North	26	5	3	2 5	6	3	19%	12%	8%	19%	23%	12%	3	5	0	0	1	1	9	0.35
		2 Avenue	21 Street	20A Street	South	6	1	2	2 2	2	1	17%	33%	33%	33%	33%	17%	0	0	0	0	2	0	2	0.33
		2 Avenue	20A Street	20 Street	South	13	3	3	3 2	2	2	23%	23%	23%	15%	15%	15%	0	1	1 1	1	0	1	4	0.31
		2 Avenue	20 Street	19 Street	South	7	1	1	1 1	1	1	14%	14%	14%	14%	14%	14%	0	0	2	0	0	0	2	0.29
		2 Avenue	19 Street	Nelson Street / 18 Street		12	1	1	1 0	0	0	8%	8%	8%	0%	0%	0%	0	0	1	0	0	0	1	0.08
		2 Avenue	19 Street	Nelson Street / 18 Street		13	1	1	1 1	1	1	8%	8%	8%	8%	8%	8%	0	0	2	0	0	0	2	0.15
			Route #5 Total		Coulii	180	33		26 24	25		18%	17%		13%			11	8	9		4	13	44	0.13
			. totalo no Total			100	33	31	20 24	20	Z1	10 /0	11 /0	13/0	13/0	13/0	12/0		0	3		-	10		0.20



#### COLD LAKE NORTH - PARKING UTILIZATION - JULY 3, 2010

						1				(=111A) = -														
Zone	Туре	Road	From	То	Side	Parking Stalls Available	11:00 AM	12:00 PM	PARKED \	2:00 PM	3:00 PM 4:00 P	M 11:00 AM	12:00 PM		LIZATION (%)	3-00 PM	4:00 PM	11:00 AM   12:00 PM	PARKING I		3-00 PM	4:00 PM	PARKING Unique Veh.	TURNOVER Turnover Rate
		Lakeshore Drive	8 Street	7 Street	North	0	6	6	5	2.00 F W	2 3	- 11.00 AW	- 12.00 F 191		2.00 F W			1 1	3	0	0	2	7	-
	Į.	Lakeshore Drive	8 Street	7 Street	South		4	2	2	4	4 4	29%	14%	14%	29%	29%	29%	2 0	2	0	0	2	6	0.43
		Lakeshore Drive Lakeshore Drive	8 Avenue 8 Avenue	8 Street 8 Street	North South	7	6	4	3	3	0 0	86%	57%	43%	43%	14%	14%	0 0 7 4	0	0	0	0	0 11	1.57
	ŀ	Lakeshore Drive	7 Avenue	8 Avenue	East	19	19	12	8	11	10 7	100%	63%	42%	58%	53%	37%	33 10	2	2	0	0	46	2.42
		Lakeshore Drive	7 Avenue	8 Avenue	West	16	3	2	7	9	6 4	19%	13%	44%	56%	38%	25%	15 4	1	0	1	0	21	1.31
, Or	n Street	Lakeshore Drive	6 Avenue	7 Avenue	East	0	0	0	0	0	0 0	-	-	-	-	-	-	0 0	0	0	0	0	0	-
1	-	Lakeshore Drive Lakeshore Drive	6 Avenue 5 Avenue	7 Avenue 6 Avenue	West East	8	0	0	0	0	0 0	38%	50%	50%	50%	50%	50%	5 4	0	0	0	0	10	1.25
	-	Lakeshore Drive	5 Avenue	6 Avenue	West	, ,	0	0	0	0	0 0	0%	0%	0%	0%	0%	0%	0 0	0	0	0	0	0	0.00
		Lakeshore Drive	2 Avenue	5 Avenue	East		0	0	0	0	0 0	-	-	-	-	-	-	0 0	0	0	0	0	0	-
	F	Lakeshore Drive	2 Avenue	5 Avenue	West	30	0	0	0	0	2 2	3%	3%	3%	3%	7%	7%	0 1	0	0	0	1	2	0.07
	ŀ	2 Avenue 2 Avenue	Lakeshore Drive Lakeshore Drive	10 Street 10 Street	North South	9	0	0	0	0	0 0	0%	0%	0%	0%	0%	0%	0 0	0	0	0	0	0	0.00
Off	f Street	271101100	Marina Parking Lo		Coun	63	61	64	67	67	64 47	97%	102%	106%	106%	102%	75%	122 38	15	9	11	6	198	3.14
			Zone #1 Total			180	103	95	97	101	93 72	41%	34%	34%	38%	32%	26%	185 62	24	12	12	12	301	1.13
	-	10 Street	1 Avenue	3 Avenue	West	3	1	0	0	0	0 0	0%	0%	0%	0%	0%	0%	0 0	0	0	0	0	0	0.00
	-	10 Street 10 Street	1 Avenue 3 Avenue	3 Avenue 5 Avenue	East West	9 19	0	0	0	0	0 0	11% 0%	11% 0%	11% 0%	11% 0%	11% 0%	11% 0%	0 0	0	0	0	0	0	0.11
		10 Street	3 Avenue	5 Avenue	East	16	2	2	2	2	2 2	13%	13%	13%	13%	13%	13%	0 0	0	0	0	2	2	0.13
		10 Street	5 Avenue	6 Avenue	West	15	1	1	1	1	1 1	7%	7%	7%	7%	7%	7%	0 0	0	0	0	1	1	0.07
	F	10 Street 10 Street	5 Avenue 6 Avenue	6 Avenue 7 Avenue	East West	15 15	5	7	3	4	2 1	13% 33%	20% 47%	20% 27%	13% 27%	13% 27%	7% 33%	0 1 3 2	0	1	0	3	3	0.20
	F	10 Street	6 Avenue	7 Avenue 7 Avenue	East		4	3	5	5	6 6	29%	21%	36%	36%	43%	43%	2 1	1	1	0	3	8	0.57
2 Or	n Street	10 Street	7 Avenue	8 Avenue	West	19	0	0	0	0	0 0	0%	0%	0%	0%	0%	0%	0 0	0	0	0	0	0	0.00
2 101	Olieel	10 Street	7 Avenue	8 Avenue	East	14	0	0	0	0	0 0	0%	0%	0%	0%	0%	0%	0 0	0	0	0	0	0	0.00
	F	5 Avenue 5 Avenue	Lakeshore Drive Lakeshore Drive	10 Street 10 Street	North South	12 12	4	3	4	5 4	3 3	33% 33%	25% 33%	33% 33%	42% 33%	25% 17%	25% 17%	0 0	0	2	0	2	5 4	0.42 0.33
	-	6 Avenue	Lakeshore Drive	10 Street	North	11	1	2	2	2	2 2	9%	18%	18%	18%	18%	18%	0 0	0	0	1	1	2	0.18
		6 Avenue	Lakeshore Drive	10 Street	South	14	3	2	1	1	1 1	21%	14%	7%	7%	7%	7%	1 1	0	0	0	1	3	0.21
	-	7 Avenue	Lakeshore Drive	10 Street	North	14	5	6	3	6	2 2	36%	43%	21%	43%	14%	14%	9 4	0	0	0	1	14	1.00
	ŀ	7 Avenue 8 Avenue	Lakeshore Drive Lakeshore Drive	10 Street 10 Street	South North	13 18	5 5	6	5 1	2	3 1	38% 28%	46% 17%	38% 6%	31% 11%	15% 17%	23% 6%	9 2	0	0	0	1	14 8	1.08 0.44
		8 Avenue	Lakeshore Drive	10 Street	South		7	4	2	2	1 1	78%	44%	22%	22%	11%	11%	11 3	0	0	0	0	14	1.56
			Route #2 Total			242	49	47	38	41	32 31	21%	20%	16%	17%	13%	13%	44 15	3	6	2	20	88	0.38
	-	1 Avenue 1 Avenue	23 Street Spruce Street	Spruce Street Tamarak Street	North North	11 22	0	0	0	0	0 0	0% 0%	0% 0%	0% 0%	0% 0%	9% 0%	0% 0%	1 0 0 0	0	0	0	0	0	0.09
	-	1 Avenue	Tamarak Street	Nelson Street / 18 Street	North	51	9	4	2	2	2 2	18%	8%	4%	4%	4%	4%	7 1	0	0	0	2	10	0.20
		1 Avenue	Nelson Street / 18 Street	16 Street	North	18	2	2	3	2	1 2	11%	11%	17%	11%	6%	11%	1 2	1	1	0	0	5	0.28
Or	n Street	1 Avenue	16 Street	10 Street	North	8	4	4	4	4	4 4	50%	50%	50%	50%	50%	50%	0 0	0	0	0	4	4	0.50
3	-	1 Avenue 1 Avenue	23 Street Nelson Street / 22 Street	Nelson Street / 22 Street 19 Street	South South	8 41	0 8	9	10	11	0 0	0% 20%	0% 22%	0% 24%	0% 27%	0% 22%	0% 22%	0 0	0	3	2	0 4	0 14	0.00
	F	1 Avenue	19 Street	Nelson Street / 18 Street	South	13	0	0	0	0	0 0	0%	0%	0%	0%	0%	0%	0 0	0	0	0	0	0	0.00
		1 Avenue	Nelson Street / 18 Street	16 Street	South	9	1	1	0	0	0 0	11%	11%	0%	0%	0%	0%	0 1	0	0	0	0	1	0.11
	-	1 Avenue	16 Street	10 Street	South	0	0	9	0	0	0 0	-	-	- 040/	100/	400/	470/	0 0	2	2	2	0	0 46	- 0.52
Of	ff Street		1 Avenue Parking L Unmarked Parking I			86 58	2	4	18 3	14 3	16 15 1 0	0% 3%	10% 7%	21% 5%	16% 5%	19% 2%	17% 0%	34 7 5 4	0	0	0	0	9	0.53 0.16
·			Route #3 Total			325	26	33	40	36	34 32	10%	11%	11%	10%	10%	9%	53 16	4	6	4	10	90	0.20
		23 Street	Birch Avenue	1 Avenue	West	42	3	2	1	4	3 1	7%	5%	2%	10%	7%	2%	2 3	0	0	0	1	6	0.14
	-	23 Street	Birch Avenue	Pine Avenue	East	6	1	2	2	1	1 1	17%	33%	33%	17%	17%	17%	0 1	0	0	0	1	2	0.33 0.18
	F	23 Street Birch Avenue	Pine Avenue 23 Street	1 Avenue Tamarak Street	East North	22 28	5	3	3	1	3 3	18% 18%	14% 14%	14% 11%	14% 4%	14% 4%	14% 7%	1 0 5 1	3	0	0	0	<u>4</u> 7	0.18
	F	Birch Avenue	23 Street	Tamarak Street	South	29	14	11	11	11	11 8	48%	38%	38%	38%	38%	28%	4 0	0	0	4	7	15	0.52
4 Or	n Street	Pine Avenue	23 Street	Tamarak Street	North	31	10	9	10	8	6 8	32%	29%	32%	26%	19%	26%	5 5	1	1	0	5	15	0.48
		Pine Avenue Pine Avenue	23 Street Spruce Street	Spruce Street Tamarak Street	South South		3 4	3	4	4	3 3	25% 25%	25% 25%	33% 25%	33% 25%	25% 25%	25% 31%	0 2	0	1	0	3	<u>6</u> 5	0.50 0.31
	F	Spruce Street	Pine Avenue	1 Avenue	West		1	1	1	1	2 1	8%	8%	8%	8%	17%	8%	5 1	0	0	0	0	6	0.50
		Spruce Street	Pine Avenue	1 Avenue	East	17	2	3	3	3	0 0	12%	18%	18%	18%	0%	0%	5 1	0	1	0	0	7	0.41
	F	Tamarak Street	Birch Avenue	1 Avenue	West	9	0	0	0	0	0 0	0%	0%	0%	0%	0%	0%	0 0	0	0	0	0	0	0.00
		Tamarak Street	Birch Avenue Route #4 Total	1 Avenue	East	16 240	0 47	0 42	0 42	40	0 0	0% 18%	0% 17%	0% 18%	0% 16%	0% 14%	0% 13%	0 0 28 15	0 4	7	0 4	20	0 73	0.00
T	T	Nelson Street / 22 Street	1 Avenue	3 Avenue	West		6	7	6	5	7 6	29%	33%	29%	16% 24%	33%	29%	4 0	1	0	0	5	10	0.48
	F	Nelson Street / 22 Street	1 Avenue	3 Avenue	East	15	2	2	2	2	3 3	13%	13%	13%	13%	20%	20%	0 1	0	0	0	2	3	0.20
	F	19 Street	1 Avenue	2 Avenue	West		0	0	0	2	2 2	0%	0%	0%	18%	18%	18%	0 0	2	0	0	0	2	0.18
	-	19 Street Nelson Street / 18 Street	1 Avenue 1 Avenue	2 Avenue 2 Avenue	East West	10 7	0	0	0	0	1 1 0	10% 0%	10% 0%	10% 0%	10% 0%	10% 0%	10% 0%	0 0	0	0	0	0	1	0.10 0.00
	F	Nelson Street / 18 Street	1 Avenue	2 Avenue	East	8	3	3	3	3	3 4	38%	38%	38%	38%	38%	50%	1 0	0	0	0	3	4	0.50
5 Or	n Street	16 Street	1 Avenue	3 Avenue	West	18	5	5	4	3	3 6	28%	28%	22%	17%	17%	33%	3 2	1	1	0	2	8	0.44
3 101	Olieel	16 Street	1 Avenue	3 Avenue	East		8	8	8	8	7 7	62%	62%	62%	62%	54%	54%	4 1	0	0	2	5	10	0.77
	F	2 Avenue 2 Avenue	21 Street 21 Street	19 Street 20A Street	North South	26 6	3	3	7 2	7 2	6 5	15% 50%	12% 50%	27% 33%	27% 33%	23% 33%	19% 33%	3 2 0 1	0	0	0	2	9	0.35 0.50
	F	2 Avenue	20A Street	20 Street	South		6	6	5	4	4 3	46%	46%	38%	31%	31%	23%	0 1	1	0	1	3	6	0.46
		2 Avenue	20 Street	19 Street	South	7	0	0	0	0	0 0	0%	0%	0%	0%	0%	0%	0 0	0	0	0	0	0	0.00
	-	2 Avenue	19 Street	Nelson Street / 18 Street			1	1	2	2	2 2	8%	8%	17%	17%	17%	17%	1 0	1	0	0	1	3	0.25 0.31
		2 Avenue	19 Street Route #5 Total	Nelson Street / 18 Street	South	13 180	43	3 42	42	41	2 2 42 43	31% 24%	23%	15% 22%	15% 22%	15% 22%	15% 23%	1 1 17 9	0	0	0 4	27	4 63	0.31
						100	+3	→Z	74	7	43	Z4 /0	23/0	ZZ /0	22 /0	ZZ /0	23/0	- 17		3	-			0.02



# **Appendix B - Parking Interview Data**



### Parking Survey Questionnaire Responses

Survey	Survey	Street Location	Time of Day	Driver/Passenger	1) Where do you	live? 2) Where are you coming from (origin)?	3) Where are you	going (destination)?	4) How far from	your destination did you park?		villing to walk from where you your destination?	6) How long did y	ou / will you be parked 7) What was yo	our reason for parking today?
	1	Lakeshore	11:00	Driver	Cold Lake North	Home-Cold Lake	Waterfront/Marina		1 Block		1 Block		2 Hours	Tourism	
L	2	10th Street	13:37	Driver	Cold Lake South	Residence	Waterfront/Marina		2 Blocks		2 Blocks		> 4 hours	Business	
L	3	10th Street	11:15	Driver	Cold Lake South	Cold Lake South	Other	Home CLS	Other		Other	Close as possible	4 Hours	Business	
Ļ	4	10th Street	11:20	Driver	Cold Lake North	Home	Other	Work	Other		> 2 Blocks	10 7	4 Hours	Business	
	5	10th Street	11:20	Driver	Out-of-Town (within AB)	Home	Waterfront/Marina		Other		Other	2 miles	2 Hours	Leisure	
<b> </b> -	7	Lakeshore 10th Street	14:00 13:20	Passenger No Response	Cold Lake South Cold Lake North	Home Home	Kinosoo Beach Waterfront/Marina		> 2 Blocks Other	Walked from home	< 1 Block No Response		2 Hours No Response	Leisure	
-	ν	5th Ave	14:58	Driver	Cold Lake North	Home	Other	Work Site	< 1 Block	Walked Holli Hollie	< 1 Block		> 4 hours	Leisure Business	
-	9	19th Street & 2nd Ave	16:10	Passenger	Out-of-Town (Outside AB)	BC BC	Kinosoo Beach	WOIK Site	2 Blocks		> 2 Blocks		4 Hours	Leisure	
F	10	19th Street & 2nd Ave	16:00	Driver	Out-of-Town (Outside AB)	BC	Kinosoo Beach		2 Blocks		> 2 Blocks		4 Hours	Leisure	
F	11	16th Street & 1st Ave	12:25	Driver	Out-of-Town (within AB)	Edmonton	Kinosoo Beach		1 Block		> 2 Blocks		2 Hours	Leisure	
F	12	10th Street & 1st Ave	12:05	Passenger	Cold Lake South	52nd Ave & 51st St	Kinosoo Beach		1 Block		2 Blocks		2 Hours	Leisure	i
F	13	10st Street & 1st Ave	12:00	Driver	Cold Lake South	52nd Ave & 51st St	Kinosoo Beach		1 Block		> 2 Blocks		1 Hour	Leisure	i
	14	16th Street & 3rd Ave	11:45	Driver	Medley	Martineav	Kinosoo Beach		2 Blocks		> 2 Blocks		2 Hours	Leisure	
	15	2nd Ave & 20 A Street	11:35	Driver	Cold Lake North	16th Street & 12th Ave	Kinosoo Beach		> 2 Blocks		> 2 Blocks		3 Hours	Leisure	
	16	1st Ave & 16th Street	11:00	Passenger	Medley	Base	Kinosoo Beach		1 Block		2 Blocks		2 Hours	Leisure	
	17	1st Ave & 16th Street	11:00	Driver	Cold Lake South	No Response	Kinosoo Beach		1 Block		> 2 Blocks		1 Hour	Leisure	
	18	16th Street & 1st Ave	12:29	Passenger	Out-of-Town (within AB)	Edmonton	Kinosoo Beach		1 Block		> 2 Blocks		2 Hours	Leisure	
	19	3rd Ave & 16th Street	15:45	Passenger	Cold Lake South	Main Ave 50th Ave & 45th St	Kinosoo Beach		2 Blocks		> 2 Blocks		4 Hours	Leisure	
L	20	1st Ave & Nelson Street	15:20	Passenger	Cold Lake North	11st near 8th Ave	Kinosoo Beach		2 Blocks		> 2 Blocks		2 Hours	Leisure	
L	21	2nd Ave & Nelson Street	15:30	Driver	Cold Lake South	52nd Ave	Kinosoo Beach		2 Blocks		> 2 Blocks		3 Hours	Leisure	
L-	22	3rd Ave & 16th Street	16:45	Passenger	Out-of-Town (within AB)	Red Deer	Kinosoo Beach		1 Block		> 2 Blocks		1 Hour	Leisure	
L	23	No Response	No Response	No Response	Cold Lake South	Cold Lake South	Waterfront/Marina		< 1 Block		> 2 Blocks		1 Hour	Other	Walk
ļ-	24	No Response	No Response	No Response	Cold Lake North	Cold Lake	Waterfront/Marina		> 2 Blocks		1 Block		3 Hours	Leisure	
F	25 26	Lakeshore Dr. & 5th Ave 8th St. & Lakeshore Drive	12:00 10:50	Driver No Response	Cold Lake North Cold Lake North	South Canada Day Parade	Waterfront/Marina Other	Home	2 Blocks 2 Blocks		1 Block > 2 Blocks		2 Hours > 4 Hours	Tourism Personal	
-	27														Loigura Canada Day
- ⊢	28	7 Ave & 5 Ave 609 Lakeshore Drive	11:00 12:00	No Response No Response	Cold Lake North Cold Lake North	No Response Home	Other Other	Parade No Where	2 Blocks Other	Homeowner	> 2 Blocks > 2 Blocks	+	4 Hours Other	Tourism Other	Leisure Canada Day
-	29	No Response	No Response	No Response	Cold Lake South	Local	Waterfront/Marina	Kinosoo Beach	> 2 Blocks	Homeowner	> 2 Blocks		2 Hours	Leisure	
F	30	No Response	No Response	No Response	Cold Lake North	Comox BC	Waterfront/Marina	Milosoo Deacii	> 2 Blocks		1 Block		< 30 Minutes	Tourism	
F	31	No Response	No Response	No Response	Cold Lake South	Cold Lake South - Home	Kinosoo Beach		2 Blocks		> 2 Blocks		2 Hours	Business	
1-Jul-10	32	No Response	No Response	No Response	Cold Lake North	Home	Kinosoo Beach		2 Blocks		1 Block		3 Hours	Leisure	
· · · · · · · · · · · ·	33	Lakeshore 500 Block	14:30	Driver	Cold Lake North	Beach	Other	Home	> 2 Blocks		> 2 Blocks		Other	Walking Other	
<u> </u>	34	Pine	11:15	Driver	Cold Lake North	Beach Ave	Kinosoo Beach	rionio	< 1 Block		> 2 Blocks		2 Hours	Leisure	
F	35	Birch Ave	11:25	Driver	Cold Lake North	Home	Kinosoo Beach		1 Block		2 Blocks		4 Hours	Leisure	i
	36	Spruce	11:35	Driver	Out-of-Town (within AB)	Lloydminister	Other	Family Visit	> 2 Blocks		2 Blocks		1 Hour	Tourism	
	37	Birch Ave	12:05	Driver	Cold Lake South	52 Ave	Other	Birch Ave	< 1 Block		1 Block		4 Hours	Business	
	38	Birch	13:03	Driver	Cold Lake North	21 Street	Kinosoo Beach		< 1 Block		< 1 Block		2 Hours	Leisure	
	39	Birch	13:07	Driver	Out-of-Town (within AB)	Red Deer	Other	Visiting friends/famil	<pre>/ &lt; 1 Block</pre>		> 2 Blocks		> 4 Hours	3 Days Personal	
L	40	Pine	13:18	Passenger	Cold Lake South	50th Ave	Kinosoo Beach		< 1 Block		2 Blocks		2 Hours	Leisure	
L	41	Pine	13:27	Driver	Cold Lake North	13 St	Kinosoo Beach		2 Blocks		> 2 Blocks		3 Hours	Leisure	
L	42	Pine	14:12	Driver	Cold Lake South	50 Ave	Other		> 2 Blocks		> 2 Blocks		> 4 hours	Leisure	
<u> </u>	43	Spruce	14:20	Driver	Cold Lake South	54 Ave	Kinosoo Beach		2 Blocks		> 2 Blocks		4 Hours	Leisure	
L	44	Pine	15:17	Passenger	Out-of-Town (within AB)	Dewberry	Kinosoo Beach		< 1 Block		2 Blocks		< 30 Minutes	Leisure	
-	45	Pine	15:21	Driver	Cold Lake North	11 Street	Kinosoo Beach		< 1 Block		2 Blocks		2 Hours	Leisure	
-	46	Birch	16:06	Driver	Cold Lake South	50th Street	Kinosoo Beach		< 1 Block		1 Block		< 30 Minutes	Leisure	
L	47 48	Pine Ave	16:19	Passenger	Cold Lake North	26th Street	Kinosoo Beach		< 1 Block		1 Block		3 Hours	Leisure	
F	48	Pine 1st	16:20 No Response	Passenger	Cold Lake South	Wildwood	Kinosoo Beach		< 1 Block 1 Block		2 Blocks < 1 Block		4 Hours < 30 Minutes	Leisure	Canada
-	50	No Response	No Response	No Response No Response	Cold Lake South Out-of-Town (within AB)	Home Edmonton	Kinosoo Beach Kinosoo Beach		< 1 Block		Other	Anytime	2 Hours	Other Other	Canada Canada Dav
<b>⊢</b>	51	No Response	No Response	No Response	Out-of-Town (Outside AB)		Kinosoo Beach		< 1 Block		> 2 Blocks	Anyume	> 4 hours	Other	Canada Day Canada Day
F	52	No Response	No Response	No Response	Cold Lake South	Beach	Other	Food	1 Block		> 2 Blocks	1	3 Hours	Other	Canada Day
⊢	53	No Response	No Response	No Response	Cold Lake South	Home	Kinosoo Beach	. 500	2 Blocks		< 1 Block	<del> </del>	3 Hours	Other	Canada Day
<b>⊢</b>	54	No Response	No Response	No Response	Out-of-Town (within AB)	Edmonton	Kinosoo Beach		< 1 Block		< 1 Block		2 Hours	Other	Canada Day
F	55	No Response	No Response	No Response	Cold Lake South	Beach	Other	Home	< 1 Block		> 2 Blocks		1 Hour	Other	Canada Day
F	56	No Response	No Response	No Response	Cold Lake North	Home	Kinosoo Beach		< 1 Block		> 2 Blocks		1 Hour	Other	Canada Day
F	57	No Response	No Response	No Response	Cold Lake North	Local Establishment	Kinosoo Beach		> 2 Blocks		< 1 Block		> 4 hours	Tourism	
	58	No Response	No Response	No Response	Cold Lake North	Beach	Other	Home	< 1 Block		1 Block		> 4 hours	Other	Canada Day
	59	No Response	No Response	No Response	Cold Lake North	Home	Kinosoo Beach		< 1 Block		Other	Anything	> 4 hours	Other	Canada Day
	60	No Response	No Response	No Response	Cold Lake North	Beach	Other	Home	< 1 Block		> 2 Blocks		2 Hours	Other	Canada Day
L	61	Lakeshore	15:36	No Response	Cold Lake North	Home	Waterfront/Marina	Kinosoo Beach	> 2 Blocks		> 2 Blocks		Other	Walked from home Tourism	Leisure
L	62	Lakeshore	15:00	Passenger	Cold Lake South	Home	Waterfront/Marina	Kinosoo Beach	< 1 Block		2 Blocks		2 Hours	Tourism	Leisure
	63	10 Street	14:20	Driver	Out-of-Town (within AB)	Home	Other	Walk	Other		Other		2 Hours	Business	Description
F	64	2nd Ave & Nelson Street	14:55	Driver	Cold Lake North	16th Ave & 12th Street	Kinosoo Beach	Home	< 1 Block		2 Blocks	<del></del>	3 Hours	Leisure	Personal
<b> </b>	65	23 Street	11:07	Driver	Cold Lake North	1 Ave	Other	Home	< 1 Block		> 2 Blocks	<del></del>	< 30 Minutes	Personal	<del></del>
2-Jul-10	66	10 Street 10 St & 3rd Ave	11:10 11:30	Driver Driver	Cold Lake South Cold Lake North	Residence Home	Other	Work Mail Pay	< 1 Block		< 1 Block < 1 Block		> 4 Hours < 30 Minutes	Business	Pickup Mail
⊢	68	10 St & 3rd Ave	11:30	Passenger	Cold Lake South	Work	Other	Mail Box Work	< 1 Block		< 1 Block	+	1 Hour	Personal Business	i-ickup iviail
⊢	69	10 Street & 7 Ave	13:15	Driver	Cold Lake South	Work	Other	Cold Lake South	< 1 Block		< 1 Block	+	3 Hours	Personal	
	70	10 Street & Ave	13:15	Driver	Cold Lake North	Work	Other	Working	< 1 Block		< 1 Block	+	1 Hour	Business	
<b>⊢</b>	70	Spruce Street	11:30	Driver		MD of Bonnyville No Response	Other	Garage Sale	< 1 Block		1 Block	+	< 30 Minutes	Personal	
F	72	Spruce	No Response	Driver	Out-of-Town (Outside AB)		Other	Yard Sale	< 1 Block	-	< 1 Block	+	< 30 Minutes	Personal	Yard Sale
3-Jul-10	73	23 Street	14:00	Driver	Cold Lake South	53 Ave	MD Campground	. ara Gaio	< 1 Block	-	1 Block	+	1 Hour	Personal	Tara Gaio
	74	No Response	No Response	No Response	Cold Lake North	Home	Kinosoo Beach	+	< 1 Block	-	> 2 Blocks	+	3 Hours	Other	Jogging & Swim
			No Response		Cold Lake North	Cold Lake South		+							
F	75	No Response	No Response	No Response	Cold Lake North	ICOID LAKE SOULT	Other	Home	< 1 Block		1 Block		Other	Home Other	Home



# **Appendix C - Data Synthesis: On-Street Parking**



## Appendix C – Data Synthesis: On-Street Parking Summary

## C.1 Entire Study Area

**Table C.1** through **Table C.3** present the parking utilization, duration, and turnover rates calculated for all the on-street parking in the entire study area. The rates were calculated from the information obtained in the parking survey. It should be noted that the average parking utilization and average turnover rates presented in the following tables were first calculated for each block and then averaged to calculate the parking utilization and turnover rate for the entire study area.

Table C.1
Parking Utilization - Entire Study Area

		July 1	, 2010	July 2	, 2010	July 3	, 2010	3-Day	Period
Time Period	Parking Stalls Available	Number of Parked Vehicles	Average Parking Utilization Rate	Number of Parked Vehicles	Average Parking Utilization Rate	Number of Parked Vehicles	Average Parking Utilization Rate	Number of Parked Vehicles	Average Parking Utilization Rate
11:00 AM		363	39%	137	14%	199	22%	233	25%
12:00 AM		338	36%	141	15%	176	19%	218	23%
1:00 PM	060	400	44%	150	17%	166	18%	239	26%
2:00 PM	960	395	43%	131	15%	173	19%	233	26%
3:00 PM		364	40%	129	15%	152	17%	215	24%
4:00 PM		318	34%	129	15%	145	16%	197	22%

Table C.2
Parking Duration - Entire Study Area

	July 1	, 2010	July 2	, 2010	July 3	3, 2010	3-Day	Period
Duration	Number of Vehicles	Percentage of Total	Number of Vehicles	Percentage of Total	Number of Vehicles	Percentage of Total	Number of Vehicles	Percentage of Total
1 Hour	501	48%	132	45%	165	44%	266	47%
2 Hours	277	27%	42	14%	67	18%	129	23%
3 Hours	103	10%	22	8%	23	6%	49	9%
4 Hours	72	7%	16	5%	23	6%	37	6%
5 Hours	15	1%	15	5%	13	3%	14	3%
6 Hours	76	7%	66	23%	81	22%	74	13%

Table C.3

Parking Turnover - Entire Study Area

	Parking Turnover R	ate (6-Hour Period)	
July 1, 2010	July 2, 2010	July 3, 2010	3-Day Period
1.09	0.32	0.41	0.61

## C.2 Study Zones

**Table C.4** through **Table C.6** present the parking utilization, duration, and turnover rates calculated for the on-street parking in each zone. The rates were calculated from the information obtained in the parking survey. It should be noted that the average parking utilization and average turnover rates presented in the following tables were first calculated for each block and then averaged to calculate the parking utilization and turnover rate for each study zone.

Table C.4
Parking Utilization - By Study Zone

			July 1	, 2010	July 2	2, 2010	July 3	, 2010	3-Day	Period
Zone	Time Period	Parking Stalls	Number of Parked Vehicles	Average Parking Utilizatio n Rate	Number of Parked Vehicles	Average Parking Utilizatio n Rate	Number of Parked Vehicles	Average Parking Utilizatio n Rate	Number of Parked Vehicles	Average Parking Utilizatio n Rate
	11:00 AM		19	17%	11	10%	36	34%	22	20%
	12:00 AM		19	16%	20	18%	25	25%	21	20%
7ana 1	1:00 PM	117	44	40%	30	29%	25	25%	33	31%
Zone 1	2:00 PM	117	43	40%	24	25%	32	30%	33	32%
	3:00 PM		39	39%	20	23%	27	24%	29	28%
	4:00 PM		25	28%	21	22%	22	20%	23	23%
	11:00 AM		48	23%	40	17%	49	21%	46	20%
	12:00 AM		39	20%	38	16%	47	20%	41	19%
Zone 2	1:00 PM	242	51	24%	42	18%	38	16%	44	20%
Zone z	2:00 PM	242	57	28%	38	16%	41	17%	45	21%
	3:00 PM		42	19%	40	17%	32	13%	38	16%
	4:00 PM		35	15%	37	16%	31	13%	34	15%
	11:00 AM		139	69%	11	7%	24	12%	58	29%
	12:00 AM		131	59%	11	7%	20	11%	54	26%
Zone 3	1:00 PM	181	132	59%	12	7%	19	11%	54	26%
Zone 3	2:00 PM	101	128	58%	9	7%	19	10%	52	25%
	3:00 PM		125	56%	15	8%	17	10%	52	25%
	4:00 PM		110	50%	16	11%	17	10%	48	24%
	11:00 AM		72	35%	42	18%	47	18%	54	23%
	12:00 AM		71	35%	41	17%	42	17%	51	23%
Zono 4	1:00 PM	240	84	41%	40	17%	42	18%	55	25%
Zone 4	2:00 PM	240	87	41%	36	16%	40	16%	54	24%
	3:00 PM		83	39%	29	12%	34	14%	49	22%
	4:00 PM		80	36%	34	13%	32	13%	49	21%

			July 1	, 2010	July 2	, 2010	July 3	, 2010	3-Day	Period
Zone	Time Period	Parking Stalls	Number of Parked Vehicles	Average Parking Utilizatio n Rate	Number of Parked Vehicles	Average Parking Utilizatio n Rate	Number of Parked Vehicles	Average Parking Utilizatio n Rate	Number of Parked Vehicles	Average Parking Utilizatio n Rate
	11:00 AM		85	51%	33	18%	43	24%	54	31%
	12:00 AM		78	49%	31	17%	42	23%	50	30%
Zone 5	1:00 PM	180	89	54%	26	15%	42	22%	52	30%
Zone 5	2:00 PM	100	80	48%	24	13%	41	22%	48	28%
	3:00 PM		75	45%	25	13%	42	22%	47	27%
	4:00 PM		68	41%	21	12%	43	23%	44	25%

Table C.5
Parking Duration - By Study Zone

		July 1	, 2010	July 2	2, 2010	July 3	, 2010	3-Day	Period
Zone	Duration	Number of Vehicles	Percentage of Total	Number of Vehicles	Percentage of Total	Number of Vehicles	Percentag e of Total	Number of Vehicles	Percentage of Total
	1 Hour	58	59%	48	70%	62	63%	56	63%
	2 Hours	15	15%	7	10%	23	23%	15	17%
Zone 1	3 Hours	13	13%	3	4%	6	6%	7	8%
Zone	4 Hours	8	8%	3	4%	3	3%	5	5%
	5 Hours	1	1%	5	7%	1	1%	2	3%
	6 Hours	4	4%	3	4%	4	4%	4	4%
	1 Hour	62	50%	33	41%	44	49%	46	47%
	2 Hours	25	20%	12	15%	15	17%	17	18%
Zone 2	3 Hours	13	10%	8	10%	3	3%	8	8%
Zone z	4 Hours	11	9%	3	4%	6	7%	7	7%
	5 Hours	2	2%	2	3%	2	2%	2	2%
	6 Hours	12	10%	22	28%	20	22%	18	18%
	1 Hour	216	53%	2	11%	14	38%	80	51%
	2 Hours	115	28%	6	33%	5	14%	42	27%
7 0	3 Hours	32	8%	1	6%	2	5%	12	7%
Zone 3	4 Hours	19	5%	2	11%	4	11%	8	5%
	5 Hours	8	2%	0	0%	2	5%	3	2%
	6 Hours	18	4%	7	39%	10	27%	12	7%

		July 1	, 2010	July 2	2, 2010	July 3	, 2010	3-Day	Period
Zone	Duration	Number of Vehicles	Percentage of Total	Number of Vehicles	Percentage of Total	Number of Vehicles	Percentag e of Total	Number of Vehicles	Percentage of Total
	1 Hour	75	40%	6	13%	28	36%	45	40%
	2 Hours	46	25%	9	19%	15	19%	23	21%
Zone 4	3 Hours	18	10%	1	2%	4	5%	8	7%
Zone 4	4 Hours	11	6%	6	13%	7	9%	8	7%
	5 Hours	4	2%	4	9%	4	5%	4	4%
	6 Hours	32	17%	21	45%	20	26%	24	22%
	1 Hour	90	40%	11	23%	17	25%	39	35%
	2 Hours	76	34%	8	17%	9	13%	31	27%
7000 F	3 Hours	27	12%	9	19%	8	12%	15	13%
Zone 5	4 Hours	23	10%	2	4%	3	4%	9	8%
	5 Hours	0	0%	4	9%	4	6%	3	2%
	6 Hours	10	4%	13	28%	27	40%	17	15%

Table C.6
Parking Turnover - By Study Zone

7		Parking Turnover R	ate (6-Hour Period)	
Zone	July 1, 2010	July 2, 2010	July 3, 2010	3-Day Period
Zone 1	0.87	0.67	0.88	0.81
Zone 2	0.58	0.32	0.38	0.43
Zone 3	1.88	0.13	0.17	0.72
Zone 4	0.86	0.26	0.30	0.47
Zone 5	1.29	0.23	0.32	0.62

## **C.3** Study Corridors

**Table C.7** through **Table C.9** present the parking utilization, duration, and turnover rates calculated for the on-street parking along the Lakeshore Drive, 10 Street, and 1 Avenue corridors. The rates were calculated from the information obtained in the parking survey. It should be noted that the average parking utilization and average turnover rates presented in the following tables were first calculated for each block and then averaged to calculate the parking utilization and turnover rate for each study corridor.

Table C.7
Parking Utilization - By Study Corridor

			July 1	l, 2010	July	2, 2010	July 3	3, 2010	3-Day	Period
Corridor	Time Period	Parkin g Stalls	Number of Parked Vehicles	Average Parking Utilization Rate	Number of Parked Vehicles	Average Parking Utilization Rate	Number of Parked Vehicles	Average Parking Utilizatio n Rate	Number of Parked Vehicles	Average Parking Utilization Rate
	11:00 AM		16	14%	8	7%	36	39%	20	20%
	12:00 AM		16	14%	19	19%	25	29%	20	20%
Lakeshore	1:00 PM	108	42	42%	28	30%	25	28%	32	34%
Drive	2:00 PM	100	42	45%	22	26%	32	34%	32	35%
	3:00 PM		38	43%	18	23%	27	27%	28	31%
	4:00 PM		24	30%	20	24%	22	23%	22	26%
	11:00 AM	_	20	19%	20	14%	15	11%	18	15%
	12:00 AM		17	18%	19	14%	17	12%	18	15%
10 Street	1:00 PM		17	18%	20	14%	16	11%	18	15%
10 Street	2:00 PM		18	20%	17	12%	15	11%	17	14%
	3:00 PM		17	13%	19	14%	16	11%	17	13%
	4:00 PM		13	10%	18	13%	16	11%	16	11%
	11:00 AM		139	69%	11	7%	24	12%	58	29%
	12:00 AM		131	59%	11	7%	20	11%	54	26%
1 Δυρουσ	1:00 PM	101	132	59%	12	7%	19	11%	55	26%
1 Avenue	2:00 PM	181	128	58%	9	7%	19	10%	52	25%
	3:00 PM		125	56%	15	8%	17	10%	52	25%
	4:00 PM		110	50%	16	11%	17	10%	48	24%

Table C.8
Parking Duration - By Study Corridor

		July 1	, 2010	July 2	, 2010	July 3, 2010		3-Day	Period
Corridor	Duration	Number of Vehicles	Percentage of Total	Number of Vehicles	Percentage of Total	Number of Vehicles	Percentage of Total	Number of Vehicles	Percentage of Total
	1 Hour	58	60%	46	71%	62	63%	55	64%
	2 Hours	14	15%	7	11%	23	23%	15	17%
Lakeshore Drive	3 Hours	12	13%	2	3%	6	6%	7	8%
Dilve	4 Hours	8	8%	3	5%	3	3%	5	5%
	5 Hours	1	1%	5	8%	1	1%	2	3%
	6 Hours	3	3%	2	3%	4	4%	3	3%
	1 Hour	16	39%	8	27%	5	21%	10	31%
	2 Hours	11	27%	4	13%	4	17%	6	20%
10 Street	3 Hours	4	10%	2	7%	1	4%	2	7%
10 Street	4 Hours	2	5%	2	7%	2	8%	2	6%
	5 Hours	2	5%	1	3%	1	4%	1	4%
	6 Hours	6	15%	13	43%	11	46%	10	32%
	1 Hour	216	53%	9	36%	14	38%	80	51%
	2 Hours	115	28%	6	24%	5	14%	42	27%
1 Avenue	3 Hours	32	8%	1	4%	2	5%	12	7%
i Avenue	4 Hours	19	5%	2	8%	4	11%	8	5%
	5 Hours	8	2%	0	0%	2	5%	3	2%
	6 Hours	18	4%	7	28%	10	27%	12	7%

Table C.9
Parking Turnover - By Study Corridor

Corridor	Parking Turnover Rate (6-Hour Period)						
Corridor	July 1, 2010	July 2, 2010	July 3, 2010	3-Day Period			
Lakeshore Drive	0.95	0.70	1.01	0.88			
10 Street	0.39	0.18	0.17	0.25			
1 Avenue	1.88	0.13	0.17	0.72			



# **Appendix D - Data Synthesis: Off-Street Parking**



## Appendix D – Data Synthesis: Off-Street Parking Summary

**Table D.1** through **Table D.3** present the parking utilization, duration, and turnover rates calculated for the off-street parking in the Marina Lot, the 1 Avenue Lot and the Gravel Lot.

Table D.1
Parking Utilization – By Parking Lot

			July 1	, 2010		, 2010		3, 2010	3-Day	Period
Parking Lot	Time Period	Parking Stalls	Number of Parked Vehicles	Average Parking Utilization Rate	Number of Parked Vehicles	Average Parking Utilization Rate	Number of Parked Vehicles	Average Parking Utilization Rate	Number of Parked Vehicles	Average Parking Utilization Rate
	11:00 AM		50	79%	38	60%	61	97%	50	79%
	12:00 AM		78	124%	52	83%	64	102%	65	103%
Marina	1:00 PM		76	121%	64	102%	67	106%	69	110%
Lot	2:00 PM	63	73	116%	63	100%	67	106%	67	107%
	3:00 PM		75	119%	47	75%	64	102%	62	98%
	4:00 PM		78	124%	49	78%	47	75%	58	92%
	11:00 AM	86	31	36%	5	6%	0	0%	12	14%
	12:00 AM		38	44%	7	8%	9	10%	18	21%
1 Avenue	1:00 PM		39	45%	20	23%	18	21%	26	30%
Lot	2:00 PM		39	45%	30	35%	14	16%	28	32%
	3:00 PM		34	40%	19	22%	16	19%	23	27%
	4:00 PM		34	40%	5	6%	15	17%	18	21%
	11:00 AM		32	55%	1	2%	2	3%	12	20%
	12:00 AM		38	66%	3	5%	4	7%	15	26%
Gravel	1:00 PM	58	47	81%	4	7%	3	5%	18	31%
Lot	2:00 PM		58	100%	6	10%	3	5%	22	39%
	3:00 PM		57	98%	5	9%	1	2%	21	36%
	4:00 PM		50	86%	3	5%	0	0%	18	30%

Table D.2

Parking Duration – By Parking Lot

			July 1, 2010		July 2, 2010		July 3, 2010		3-Day Period	
Parking Lot	Duration	Number of Vehicles	Percentage of Total	Number of Vehicles	Percentage of Total	Number of Vehicles	Percentage of Total	Number of Vehicles	Percentage of Total	
	1 Hour	130	59%	110	64%	122	61%	121	61%	
	2 Hours	37	17%	25	14%	38	19%	33	17%	
	3 Hours	19	9%	16	9%	15	7%	17	8%	
Marina Lot	4 Hours	15	7%	11	6%	9	4%	12	6%	
	5 Hours	10	5%	5	3%	11	5%	9	4%	
	6 Hours	10	5%	6	3%	6	3%	7	4%	
	1 Hour	9	16%	29	55%	34	72%	24	46%	
	2 Hours	13	23%	16	30%	7	15%	12	23%	
1 Avenue	3 Hours	2	4%	7	13%	2	4%	4	7%	
Lot	4 Hours	6	11%	1	2%	2	4%	3	6%	
	5 Hours	6	11%	0	0%	2	4%	3	5%	
	6 Hours	20	36%	0	0%	0	0%	7	13%	
	1 Hour	50	38%	5	45%	5	56%	20	40%	
	2 Hours	36	28%	2	18%	4	44%	14	28%	
	3 Hours	26	20%	3	27%	0	0%	10	19%	
Gravel Lot	4 Hours	10	8%	1	9%	0	0%	4	7%	
	5 Hours	6	5%	0	0%	0	0%	2	4%	
	6 Hours	2	2%	0	0%	0	0%	1	1%	

Table D.3
Parking Turnover – By Parking Lot

	Parking Turnover Rate (6-Hour Period)						
Parking Lot	July 1, 2010	July 2, 2010	July 3, 2010	3-Day Period			
Marina Lot	3.27	2.56	3.14	2.99			
1 Avenue Lot	0.63	0.62	0.53	0.59			
Gravel Lot	2.14	0.19	0.16	0.83			



# **Appendix E - Data Synthesis: Parking Interviews**



### Appendix E – Data Synthesis: Parking Interview Summary

**Table E.1** through **Table E.7** summarizes the responses obtained from the parking interviews.

Table E.1 Where do you live?

	Response	Percentage of Total
Cold Lake North	34	45%
Cold Lake South	25	33%
Medley	2	3%
Out-of-Town (within AB)	10	13%
Out-of-Town (Outside AB)	5	7%
No Response	0	0%
Total	76	100%

The majority of commuters interviewed lived within the City, with 45% living in Cold Lake North and 33% living in Cold Lake South.

Table E.2
Where are you coming from (origin)?

	Response	Percentage of Total
Cold Lake North	27	36%
Cold Lake South	23	30%
Medley	2	3%
Out-of-Town (within AB)	11	14%
Out-of-Town (Outside AB)	5	7%
Unknown	8	11%
Total	76	100%

The majority of commuters interviewed started their trip from within the City, with 36% originating from Cold Lake North and 30% originating from Cold Lake South.

Table E.3 Where are you going (destination)?

	Response	Percentage of Total
Waterfront/Marina	11	14%
Kinosoo Beach	38	50%
MD Campground	1	1%
Cold Lake North	8	11%
Cold Lake South	3	4%
Unknown	15	20%
No Response	0	0%
Total	76	100%

Half the commuters (50%) interviewed were destined for Kinosoo Beach and 14% were destined for the Waterfront/Marina.

Table E.4
How far from your destination did you park?

		<u> </u>
	Response	Percentage of Total
< 1 Block	34	45%
1 Block	11	14%
2 Blocks	15	20%
> 2 Blocks	10	13%
Other	6	8%
No Response	0	0%
Total	76	100%

The majority of commuters interviewed parked within 2 blocks of their destination, with 45% parked within less than 1 block, 14% parked within 1 block and 20% parked within 2 blocks.

Table E.5

How far are you willing to walk from where you park to your destination?

.g .c	you paint to your accum
Response	Percentage of Total
13	17%
13	17%
11	14%
33	43%
5	7%
1	1%
76	100%
	Response  13 13 11 11 33 5 1

Most commuters (43%) interviewed are willing to park further than 2 blocks from their destination.

Table E.6
How long did you / will you be parked for?

	Response	Percentage of Total
< 30 Minutes	8	11%
1 Hour	10	13%
2 Hours	21	28%
3 Hours	11	14%
4 Hours	10	13%
> 4 hours	10	13%
Other	5	7%
No Response	1	1%
Total	76	100%

Most commuters (28%) interviewed will be parked in their parking spot for a duration of 2 hours.

Table E.7
What was your reason for parking today?

		<u> </u>		
	Response	Percentage of Total		
Business	10	13%		
Tourism	8	11%		
Shopping	0	0%		
Leisure	33	43%		
Personal	9	12%		
Other	16	21%		
No Response	0	0%		
Total	76	100%		

The predominant trip purpose for most commuters interviewed in Cold Lake North was leisure (43%).



# **Appendix F - On-Street Parking Analysis**



### Appendix F - On Street Parking Analysis

## F.1 Entire Study Area

### F.1.1 Parking Demand/Utilization

**Figure F.1** presents the overall on-street parking supply and demand plotted for the entire study area.

1000 900 800 700 600 500 400 395 364 300 318 200 168 159 158 152 141 100 137 0 11:00 AM 4:00 PM 12:00 PM 1:00 PM 2:00 PM 3:00 PM Parking Demand - July 1 Average Parking Demand - July 2 & 3 Parking Supply

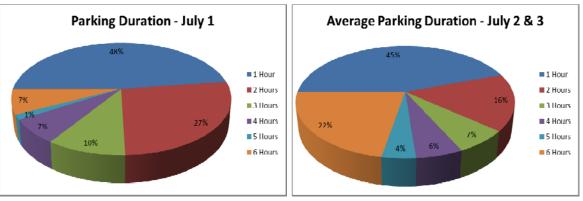
Figure F.1
On-Street Parking Supply vs. Demand – Entire Study Area

The existing on-street parking supply was able to accommodate the overall on-street parking demand in the entire study area. The parking demand on July 1 was higher than the parking demand on July 2 and July 3; the parking demand on July 1 was almost double the parking demand on July 2 and July 3. The highest overall parking demand was observed on July 1 at 1:00 pm.

### F.1.2 Parking Duration

**Figure F.2** presents the parking duration observed on July 1 and the average parking duration observed on July 2 and July 3, for the entire study area.

Figure F.2
Parking Duration – Entire Study Area



The majority of commuters parked for a duration of 2 hours or less. On July 1, 48% of commuters parked for 1 hour and 27% parked for 2 hours. On July 2 and July 3, an average of 45% of commuters parked for 1 hour and an average of 16% parked for 2 hours. It should be noted that the proportion of commuters who parked for 6 hours was significantly larger on July 2 and July 3 (22%) than on July 1 (7%).

### F.1.3 Parking Turnover

The turnover rate for the entire study area was 1.09 on July 1, 0.32 on July 2 and 0.41 on July 3. The turnover rate is much higher on July 1 (Canada Day) than on a typical day on the weekend.

### F.2 Study Zones

## F.2.1 Parking Demand/Utilization

**Figure F.3** through **Figure F.7** present the on-street parking supply and demand plotted for the different parking zones.

Figure F.3
On-Street Parking Supply vs. Demand – Study Zone 1

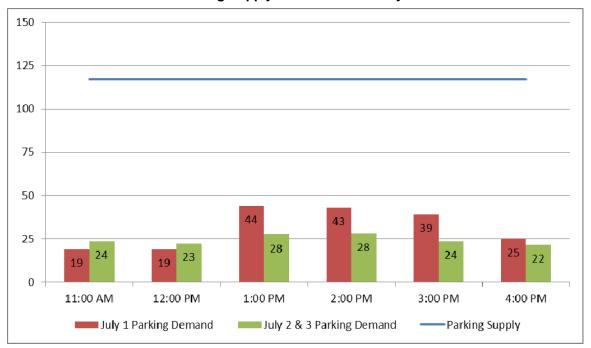


Figure F.4
On-Street Parking Supply vs. Demand – Study Zone 2

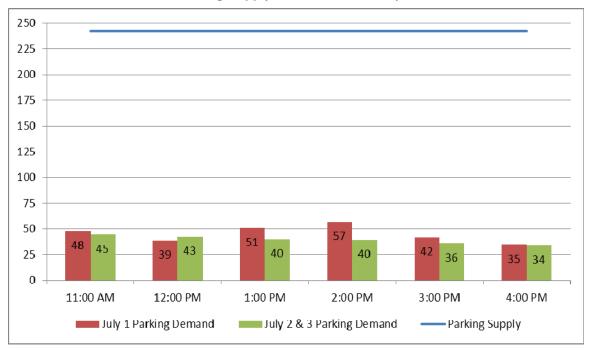


Figure F.5
On-Street Parking Supply vs. Demand – Study Zone 3

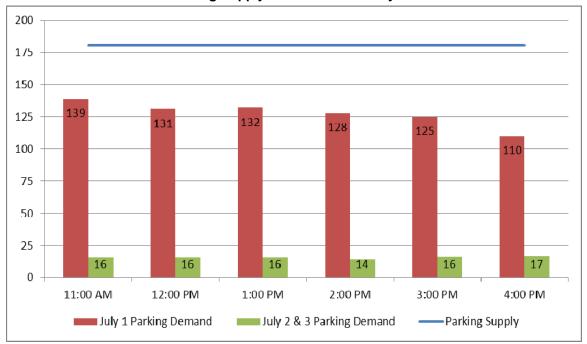
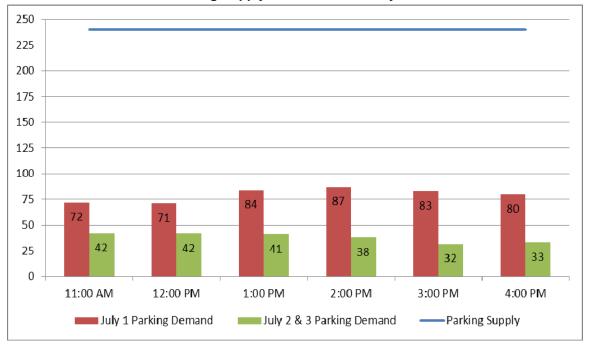


Figure F.6
On-Street Parking Supply vs. Demand – Study Zone 4



200 175 150 125 100 89 75 85 80 78 68 50 38 25 37 34 34 33 32 0 11:00 AM 12:00 PM 1:00 PM 2:00 PM 3:00 PM 4:00 PM July 1 Parking Demand July 2 & 3 Parking Demand -Parking Supply

Figure F.7
On-Street Parking Supply vs. Demand – Study Zone 5

The graphs presented in **Figure F.3** through **Figure F.7** illustrate that the existing on-street parking supply within each parking zone was able to accommodate the on-street parking demand for the zone.

Generally, the on-street parking demand observed on July 1 was higher than the parking demand observed on July 2 and July 3. The most significant difference in parking demand occurred in Zone 3, where the parking demand on July 1 was more than seven times the demand observed on July 2 and July 3.

## F.2.2 Parking Duration

**Figure F.8** through **Figure F.12** present the parking duration observed on July 1 and the average parking duration observed on July 2 and July 3, for each of the parking zones.

Figure F.8
Parking Duration – Zone 1

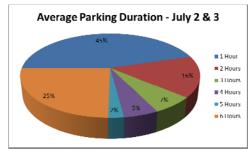




The majority of commuters in Zone 1 parked for a duration of 2 hours or less. On July 1, 59% of commuters parked for 1 hour and 15% parked for 2 hours. On July 2 and July 3, an average of 65% of commuters parked for 1 hour and an average of 18% parked for 2 hours.

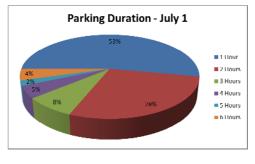
Figure F.9
Parking Duration – Zone 2

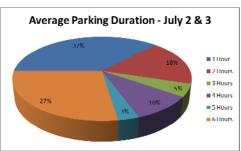




The majority of commuters in Zone 2 parked for a duration of 2 hours or less. On July 1, 50% of commuters parked for 1 hour and 20% parked for 2 hours. On July 2 and July 3, an average of 45% of commuters parked for 1 hour and an average of 16% parked for 2 hours. It should be noted that on July 2 and July 3, and average of 25% of commuters parked for 6 hours.

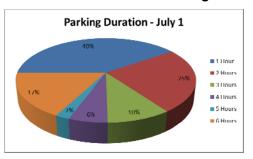
Figure F.10 Parking Duration – Zone 3

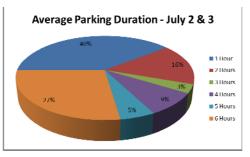




The majority of commuters in Zone 3 parked for a duration of 2 hours or less. On July 1, 53% of commuters parked for 1 hour and 28% parked for 2 hours. On July 2 and July 3, an average of 37% of commuters parked for 1 hour and an average of 18% parked for 2 hours. It should be noted that on July 2 and July 3, and average of 27% of commuters parked for 6 hours.

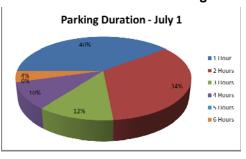
Figure F.11
Parking Duration – Zone 4

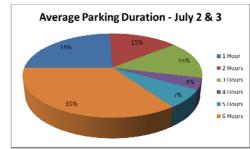




The majority of commuters in Zone 4 parked for a duration of 2 hours or less. On July 1, 40% of commuters parked for 1 hour and 25% parked for 2 hours. On July 2 and July 3, an average of 40% of commuters parked for 1 hour and an average of 16% parked for 2 hours. It should be noted that on July 2 and July 3, and average of 27% of commuters parked for 6 hours.

Figure F.12
Parking Duration – Zone 5





On July 1, the majority of commuters in Zone 5 parked for a duration of 2 hours or less, with 40% of commuters parked for 1 hour and 34% of commuters parked for 2 hours. On July 2 and July 3, most commuters in Zone 5 either parked for 6 hours (35%) or 1 hour (24%).

## F.2.3 Parking Turnover

The parking turnover rates presented in **Table C.6** (in **Appendix C**) for each parking zone are resummarized in **Table F.1** to present the turnover rates observed on July 1 and the average turnover rates observed on July 2 and July 3.

Table F.1
Parking Turnover – Study Zones

	Parking Turnover Rate (6-Hour Period)			
Zone	July 1, 2010	July 2 & 3, 2010 Average		
Zone 1	0.87	0.77		
Zone 2	0.58	0.35		
Zone 3	1.88	0.15		
Zone 4	0.86	0.28		
Zone 5	1.29	0.28		

The turnover rates observed on July 1 are higher than the average turnover rates observed on July 2 and July 3. On July 1, Zone 3 and Zone 5 experienced the highest turnover rates at 1.88 and 1.29 respectively. It should be noted that Zone 3 and Zone 5 have the lowest average turnover rates on July 2 and July 3.

# F.3 Study Corridors

## F.3.1 Parking Demand/Utilization

**Figure F.13** through **Figure F.15** present the on-street parking supply and demand plotted for the individual study corridors.

Figure F.13
On-Street Parking Supply vs. Demand – Lakeshore Drive

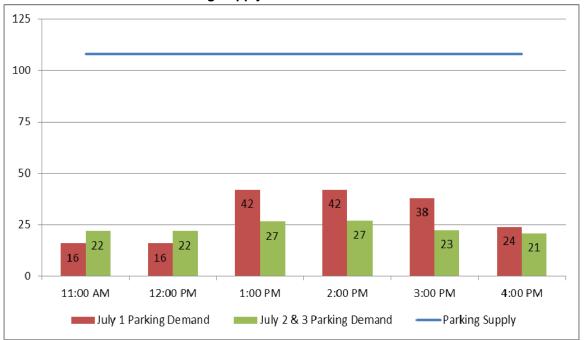


Figure F.14
On-Street Parking Supply vs. Demand – 10 Street

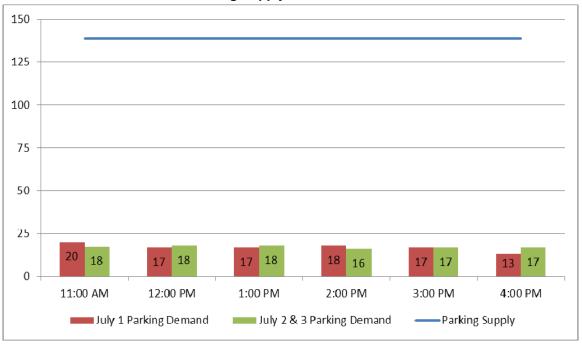


Table F.15
On-Street Parking Supply vs. Demand – 1 Avenue

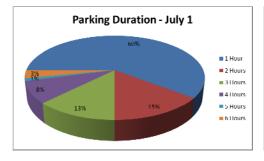


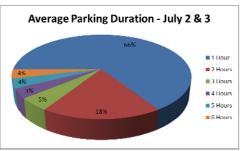
The graphs presented in **Figure F.13** through **Figure F.15** illustrate that the existing on-street parking supply within each study corridor was able to accommodate the on-street parking demand for the corridor. The parking demand along Lakeshore Drive and 10 Street remained relatively consistent over the three day study period. The parking demand on 1 Avenue, however, was significantly higher on July 1 than on July 2 and July 3.

### F.3.2 Parking Duration

**Figure F.16** through **Figure F.18** present the parking duration observed on July 1 and the average parking duration observed on July 2 and 3, for each of the study corridors.

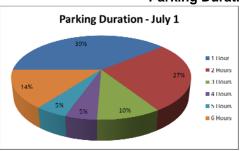
Figure F.16
Parking Duration – Lakeshore Drive

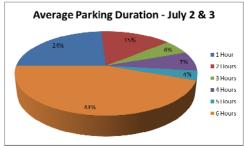




The majority of commuters along Lakeshore Drive parked for a duration of 2 hours or less. On July 1, 60% of commuters parked for 1 hour and 15% parked for 2 hours. On July 2 and July 3, an average of 66% of commuters parked for 1 hour and an average of 18% parked for 2 hours.

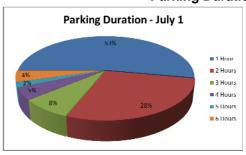
Figure F.17
Parking Duration – 10 Street

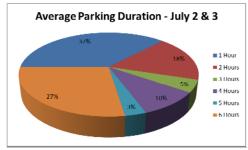




On July 1, the majority of commuters along 10 Street parked for a duration of 2 hours or less, with 39% of commuters parked for 1 hour and 27% of commuters parked for 2 hours. On July 2 and July 3, most commuters along 10 Street either parked for 6 hours (44%) or 1 hour (24%).

Figure F.18
Parking Duration – 1 Avenue





On July 1, the majority of commuters along 1 Avenue parked for a duration of 2 hours or less, with 53% of commuters parked for 1 hour and 28% of commuters parked for 2 hours. On July 2 and July 3, most commuters along 1 Avenue either parked for 1 hour (37%) or 6 hours (27%).

### F.3.3 Parking Turnover

The parking turnover rates presented in **Table C.9** (in **Appendix C**) for each study corridor are re-summarized in **Table F.2** to present the turnover rates observed on July 1 and the average turnover rates observed on July 2 and July 3.

Table F.2
Parking Turnover – Study Corridors

	Parking Turnover Rate (6-Hour Period)				
Zone	July 1, 2010	July 2 & 3 2010 - Average			
Lakeshore Drive	0.95	0.85			

	Parking Turnover R	ate (6-Hour Period)
Zone	July 1, 2010	July 2 & 3 2010 - Average
10 Street	0.39	0.17
1 Avenue	1.88	0.15

The turnover rates observed on July 1 are higher than the average turnover rates observed on July 2 and July 3. On July 1, the 1 Avenue corridor experiences the highest turnover rates at 1.88. It should be noted that the 1 Avenue corridor has the lowest average turnover rates on July 2 and July 3.

## F.4 Other On-Street Parking Issues

#### F.4.1 Illegal Parking

No parking zones are provided at the following on-street locations:

- East side of Lakeshore Drive, from 7 Street to 8 Avenue
- East side of Lakeshore Drive, from 2 Avenue to 7 Avenue
- West side of Lakeshore Drive, from 6 Avenue to midblock to 7 Avenue
- North side of 2 Avenue, from 10 Street to Lakeshore Drive
- South side of 1 Avenue, from 16 Street to 10 Street
- North side of 1 Avenue, at the 16 Street intersection

Despite the parking restriction, parking was observed at the following locations during the three-day study period:

- East side of Lakeshore Drive, from 7 Street to 8 Street July 2 and July 3
- East side of Lakeshore Drive, from 8 Avenue to 8 Street July 1
- East side of Lakeshore Drive, from 6 Avenue to 7 Avenue July 1
- South side of 1 Avenue from 16 Street to 10 Street July 1

The no parking zones listed above are indicated by yellow paint on the side of the curb. Visitors unfamiliar with the area may not understand that the yellow curb paint indicate no parking zones. During the wintertime the yellow curb paint may also become obscured under the snow. AE recommends that no parking signs be installed at the no parking zones to reinforce the parking restriction. Enforcement of the no parking zones should also be increased to ensure that the parking restrictions are obeyed.

#### F.4.2 Vehicle Type

The following vehicle types were observed during the parking survey, aside from passenger cars:

- Bicycles
- Motorcycles
- Recreational vehicles
- Trailer/boat trailers
- Farm vehicles
- Other

**Table F.3** through **Table F.5** present the breakdown of the vehicle type for on-street parking observed July 1, July 2, and July 3 respectively.

Table F.3
On-Street Vehicle Type – July 1, 2010

	Vehicle Type							
Zone	Passenger Car	Bicycle	Motorcycle	Recreational Vehicle	Trailer	Farm Vehicle	Other	
1	82	6	0	0	10	0	0	
2	113	2	0	1	4	0	0	
3	383	0	10	0	0	2	0	
4	169	0	0	2	5	0	2	
5	212	0	0	0	0	0	0	
Entire Study Area	959	8	10	3	19	2	2	

Table F.4
On-Street Vehicle Type – July 2, 2010

	Vehicle Type								
Zone	Passenger Car	Bicycle	Motorcycle	Recreational Vehicle	Trailer	Farm Vehicle	Other		
1	62	1	0	1	5	0	0		
2	69	0	0	0	4	0	0		
3	24	0	0	0	0	0	0		
4	59	0	0	0	4	0	2		
5	43	0	0	1	0	0	0		

Entire Study Area	257	1	0	2	13	0	2	
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Table F.5
On-Street Vehicle Type – July 3, 2010

l	Vehicle Type							
Zone	Passenger Car	Bicycle	Motorcycle	Recreational Vehicle	Trailer	Farm Vehicle	Other	
1	87	5	0	0	4	0	0	
2	83	0	0	2	3	0	0	
3	35	0	0	0	0	0	0	
4	66	0	0	0	3	0	4	
5	63	0	0	0	0	0	0	
Entire Study Area	334	5	0	2	10	0	4	

Of particular interest was the number of RVs and trailers parked on-street in the study area. These vehicle types are longer and will require more than one on-street parking stall. RVs and trailers accounted for approximately 2% of on-street parked vehicles on July 1, 2010, approximately 6% of on-street parked vehicles on July 2, 2010, and 4% of on-street parked vehicles on July 3, 2010. The percentages quoted above are for the entire study area over the entire study period (11:00 am to 4:00 pm) each day. It should be noted that Zone 1 (Lakeshore Drive) contained the highest proportion of RVs and trailers for all three days surveyed.

### **REPORT**



# **Appendix G - Off-Street Parking Analysis**



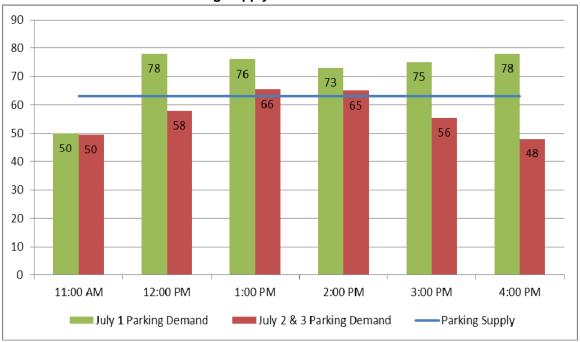
#### Appendix G - Off Street Parking Analysis

#### G.1 Marina Lot

#### **G.1.1 Parking Demand/Utilization**

Figure G.1 presents the off-street parking supply and demand plotted for the Marina Lot.

Figure G.1
On-Street Parking Supply vs. Demand – Marina Lot



The existing parking supply provided in the Marina Lot was unable to accommodate the parking in several time periods, over the three days observed. The parking demand exceeded the parking supply from 12:00 pm to 5:00 pm on July 1 and the average parking demand exceeded the parking supply from 1:00 pm to 3:00 pm on July 2 and July 3.

#### **G.1.2** Parking Duration

**Figure G.2** presents the parking duration observed on July 1 and the average parking duration observed on July 2 and July 3, for the Marina Lot.

Figure G.2
Parking Duration – Marina Lot





The majority of commuters in the Marina Lot parked for a duration of 2 hours or less. On July 1, 59% of commuters parked for 1 hour and 17% of commuters parked for 2 hours. On July 2 and July 3, an average of 62% of commuters parked for 1 hour and an average of 17% of commuters parked for 2 hours.

#### **G.1.3 Parking Turnover**

The parking turnover rate for the Marina Lot was 3.27 on July 1, 2.56 on July 2 and 3.14 on July 3. The turnover rate for the Marina Lot was relatively consistent over the course of the 3 day study period and was high when compared to the turnover rates for on-street parking and for the other two off-street lots.

#### G.2 1 Avenue Lot

#### **G.2.1 Parking Demand/Utilization**

Figure G.3 presents the off-street parking supply and demand plotted for the 1 Avenue Lot.

On-Street Parking Supply vs. Demand – 1 Avenue Lot 39 39

2:00 PM

July 2 & 3 Parking Demand

34

18

Parking Supply

3:00 PM

34

10

4:00 PM

Figure G.3

The existing parking supply provided in the 1 Avenue Lot was able to accommodate the parking demand, for the entire study period. On July 1, parking in the 1 Avenue Lot was only available to vendors participating in the Canada Day parade. Public parking was not allowed.

19

1:00 PM

38

12:00 PM

July 1 Parking Demand

#### **G.2.2 Parking Duration**

30

20

10

0

31

3

11:00 AM

Figure G.4 presents the parking duration observed on July 1 and the average parking duration observed on July 2 and July 3, for the 1 Avenue Lot.



Figure G.4 Parking Duration – 1 Avenue Lot

On July 1, most of the commuters in the 1 Avenue Lot parked for 6 hours (36%). On July 2 and July 3, the majority of commuters parked for a duration of 2 hours or less, with an average of 63% of commuters parked for 1 hour and an average of 23% of commuters parked for 2 hours.

#### **G.2.3 Parking Turnover**

The parking turnover rate for the 1 Avenue Lot was 0.63 on July 1, 0.62 on July 2 and 0.53 on July 3. The turnover rate for the 1 Avenue Lot was relatively consistent over the course of the 3 day study period and was low when compared to the Marina Lot.

#### G.3 Gravel Lot

#### **G.3.1 Parking Demand/Utilization**

**Figure G.5** presents the off-street parking supply and demand plotted for the Gravel Lot.

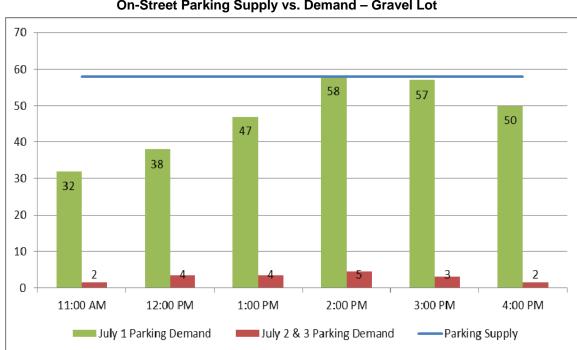


Figure G.5
On-Street Parking Supply vs. Demand – Gravel Lot

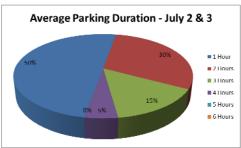
The existing parking supply provided in the Gravel Lot was able to accommodate the parking demand, for the entire study period. It should be noted that the parking demand on July 1 is significantly higher than the average parking demand on July 2 and July 3.

#### **G.3.2 Parking Duration**

**Figure G.6** presents the parking duration observed on July 1 and the average parking duration observed on July 2 and July 3, for the Gravel Lot.

Figure G.6
Parking Duration – Gravel Lot





The majority of commuters in the Gravel Lot parked for a duration of 2 hours or less. On July 1, 38% of commuters parked for 1 hour and 28% of commuters parked for 2 hours. On July 2 and July 3, an average of 50% of commuters parked for 1 hour and an average of 30% of commuters parked for 2 hours.

#### **G.3.3 Parking Turnover**

The parking turnover rate for the Gravel Lot was 2.14 on July 1, 0.19 on July 2 and 0.16 on July 3. The turnover rates indicate that the Gravel Lot was highly utilized on Canada Day, but was underutilized for the remainder of the study period.

#### G.4 Other Off-Street Parking Issues

#### **G.4.1** Illegal Parking

The utilization rate for the Marina Lot exceeded 100% every day for the three-day study period. This indicates that the number of vehicles parked in the Marina Lot exceeded the number of parking stalls available and provides evidence that people were making their own parking spots and parking illegally.

#### G.4.2 Vehicle Type

**Table G.1** through **Table G.3** presents the breakdown of the vehicle type for off-street parking on July 1, July 2, and July 3 respectively.

Table G.1
Off-Street Vehicle Type – July 1, 2010

	Vehicle Type								
Zone	Passenger Bicycle Moto		Motorcycle	Recreational Vehicle	Trailer	Boat	Farm Vehicle		
Marina Lot	204	1	0	1	0	0	0		
1 Avenue Lot	53	0	0	0	1	0	0		
Gravel Lot	123	1	0	0	0	0	0		

Table G.2
Off-Street Vehicle Type – July 2, 2010

	Vehicle Type								
Zone	Passenger Car	Bicycle	Motorcycle	Recreational Vehicle	Trailer	Boat	Farm Vehicle		
Marina Lot	161	0	0	0	0	0	0		
1 Avenue Lot	52	0	1	0	0	0	0		
Gravel Lot	11	0	0	0	0	0	0		

Table G.3 Off-Street Vehicle Type – July 3, 2010

	Vehicle Type								
Zone	Passenger Car	Bicycle	Motorcycle Recreationa Vehicle		Trailer	Boat	Farm Vehicle		
Marina Lot	197	1	0	0	0	0	0		
1 Avenue Lot	45	1	0	0	0	0	0		
Gravel Lot	9	0	0	0	0	0	0		

The number of RVs and trailers parked in the off-street lots are very low during the study period. On July 1, 2010, only one RV was observed in the Marina Lot and one trailer was observed in the 1 Avenue Lot, over the course of the 6-hour study period. No RVs or trailers were observed in the off-street lots on July 2, 2010 or July 3, 2010. The 1 Avenue Lot currently contains 11 long stalls that are intended for RVs and trailers. The demand for RV and trailer parking can be easily accommodated by these stalls.

## **FINAL REPORT**



**Appendix E - Transportation Study Cold Lake South - Parking Study** 

# **Technical Memorandum**



## **City of Cold Lake**

**Transportation Study Cold Lake South - Parking Study** 

### **April 2011**



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## **TECHNICAL MEMORANDUM**

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## Introduction

Associated Engineering (AE) was retained by the City of Cold Lake (City) to update the existing transportation study. The purpose of the transportation study is to provide the City with a master plan on which to plan and implement specific transportation network improvement projects over the next 20 years in 5-year, 10-year, 15-year and 20-year planning horizons. The transportation study will consider municipal roads, traffic calming, parking management, traffic safety, traffic signal coordination, school zones, transit, truck routes, traffic management, and transportation system operations.

One component of the transportation study is to complete parking studies for the business zones within Cold Lake North (CLN) and Cold Lake South (CLS). The parking studies for the two study areas were completed independently. This technical memorandum documents the result of the parking study completed for the business zone within CLS.

#### 1.1 STUDY BACKGROUND

A comprehensive parking study (1985 parking study) was completed for the downtown core within CLS (formerly the Town of Grand Centre) in August 1985. The study was completed by the Alberta Municipal Affairs Planning Branch at the request of the Town of Grand Centre and the results were published in the Parking Study, Town of Grand Centre report. The report has been included in Appendix A.

The City advised AE that the parking condition and parking policies within CLS has not significantly changed since the previous parking study. Downtown land uses have remained essentially the same since 1985, with the exception of some businesses that have relocated out of the downtown core. For this reason, the current parking demand in the downtown area is expected to be similar to or less than the parking demands observed in 1985.

#### 1.2 STUDY OBJECTIVE

The objective of the CLS parking study was to verify the City's assumption that the current parking condition within CLS has not significantly changed since the previous parking study (1985 parking study).

#### 1.3 STUDY METHODOLOGY

The CLS parking study was completed using the following methodology:

- Attend project initiation meeting and obtain relevant data
- Review previous parking study report
- Conduct verification study including parking survey and analysis
- Produce technical memorandum.



## **Project Initiation Meeting & Data Gathering**

A project initiation meeting was held on May 4, 2010 in the City to complete the following:

- Confirm the scope of the CLS parking study
- Obtain the parking study, Town of Grand Centre report.



## 1985 Parking Study Review

The 1985 parking study was completed in response to a perceived parking shortage within the downtown and involved analysis of both long and short term parking characteristics. The study area encompassed the downtown core which is bounded by 51 Avenue to the north, 49 Street to the east and Highway 28/55 to the south and west. Both on-street and off-street parking was analyzed including laneways, major and minor parking lots, vacant lots and select private lots.

The results and recommendations from the 1985 parking study are presented below:

#### Results

- The City did not have an overall shortage of parking spaces in the downtown. Only 37% of the City's available parking spaces were occupied at peak demand.
- The perceived parking shortage resulted from heavy demand for parking in a 2-block area of downtown east and west of the 50 Avenue/51 Street intersection and poorly-defined parking spaces.

#### Recommendations

- Parking spaces should be measured and clearly marked in the downtown block faces
- Handi-bus stops should be moved slightly so that high-demand parking spaces were not lost.



## **Verification Study**

#### 4.1 METHODOLOGY

A verification study was conducted on October 20, 2010 to validate the assumption that current parking demands are similar to the 1985 parking conditions. The verification study was completed using a license plate survey at select on-street locations. Surveyors collected the license plate of parked vehicles over a one hour period between 2:30 pm and 3:30 pm on October 20, 2010.

Figure 4.1 presents the on-street locations surveyed for the verification study.

One block from each of the four on-street parking areas examined previously was selected for the verification study. The information collected in the verification study has been included in Appendix B.

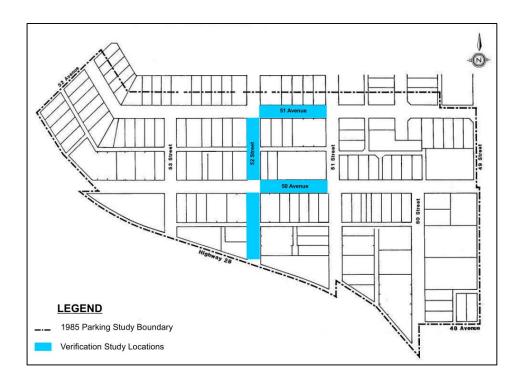


Figure 4.1
On-Street Locations for Verification Study



#### 4.2 ANALYSIS AND RESULTS

Table 4.1 presents the parking utilization for the select study locations during the 1985 and existing (2010) horizons. The table compares the parking utilization observed in the same time period of 2:30 pm.

Table 4.1
Parking Utilization (1985 Parking Study vs 2010 Verification Study)

ROAD	SIDE OF		UTILIZATION 30 PM)	PARKING UTILIZATION	ANNUAL
	ROAD	1985 Parking Study	2010 Verification Study	GROWTH (1985 - 2010)	GROWTH
	North	15%	23%	8%	0.3%
51 Avenue (51 Street to 52 Street)	South	27%	0%	-27%	-1.1%
(01 011 001 10 02 011 001)	Both Sides	21%	12%	-9%	-0.4%
	North	50%	62%	12%	0.5%
50 Avenue (51 Street to 52 Street)	South	52%	73%	21%	0.8%
(6.1 5.1 55.1 15 52 5.1 55.)	Both Sides	51%	67%	16%	0.6%
	East	11%	55%	44%	1.7%
52 Street (51 Avenue to 50 Avenue)	West	22%	54%	32%	1.3%
(0.7.110.1100.10007.110.1100)	Both Sides	17%	54%	38%	1.5%
_	East	89%	80%	-9%	-0.4%
52 Street (50 Avenue to Highway 28/55)	West	63%	40%	-23%	-0.9%
(11 11 11 11 11 11 11 11 11 11 11 11 11	Both Sides	76%	60%	-16%	-0.6%

The results in Table 4.1 indicate that the parking utilization along 51 Avenue and 52 Street (south of 50 Avenue) has decreased since 1985 while the parking utilization along 50 Avenue and 52 Street (north of 50 Avenue) has increased since 1985. The annual growth observed along 50 Avenue and 52 Street (north of 50 Avenue) is less than 2%. Even if the parking demand continues to grow at an annual growth rate of 2%, the current parking supply should be able to accommodate the parking demand for the next 30 years.

#### TECHNICAL MEMORANDUM

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## **Conclusion**

AE was retained by the City to undertake a parking study for the downtown in CLS. The City advised that the parking condition and parking policies within CLS has not significantly changed since the previous parking study conducted in 1985. For this reason, the current parking demand in the downtown is expected to be similar to or less than the parking demand observed in 1985.

AE reviewed the 1985 parking study and conducted a verification study to determine if current parking utilization rates at select locations are similar to those observed in the 1985 parking study. A parking survey was conducted on October 20, 2010 to obtain current parking utilization rates at select locations.

The verification study indicated that the parking utilization has not significantly changed in the downtown. Parking utilization along 51 Avenue and 52 Street (south of 50 Avenue) has decreased since 1985 while the parking utilization along 50 Avenue and 52 Street (north of 50 Avenue) has increased since 1985. The annual growth observed along 50 Avenue and 52 Street (north of 50 Avenue) is less than 2%. Even if the parking demand continues to grow at an annual growth rate of 2%, the current parking supply should be able to accommodate the parking demand for the next 30 years.

The City has experienced a trend of business relocation from the downtown to the commercial area along Highway 28, south of 43 Avenue, in recent years. The trend is expected to continue as the commercial area develops and continues to draw more businesses. With the relocation of businesses outside downtown, an annual parking growth rate of 2% for the downtown may not be achieved. The actual growth in parking demand will be dependent on the future land use changes within the downtown.

The City should monitor the land uses and parking condition periodically within the downtown and consider a detailed parking study if there is significant land use changes that would attract more trips into downtown CLS.



## **TECHNICAL MEMORANDUM**



# **Appendix A - 1985 Parking Study Grand Centre**



PARKING STUDY TOWN OF GRAND CENTRE AUGUST, 1985

Prepared By: Planning Branch Lakeland Unit

Alberta Municipal Affairs K.D. Kelly, W. Steblyk Planning Project Personnel

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#### EXECUTIVE SUMMARY

In May of 1985 at the request of the town, Alberta Municipal Affairs Planning Branch staff conducted a study of parking characteristics in the Town of Grand Centre. The study was in response to a perceived shortage of prime time parking in the Town's downtown core, and involved analysis of long and short term parking characteristics of residents and visitors. Lane, on-street, and parking lot areas were observed over regular intervals for 3 days so that occupancy rates and length of parking space use could be determined.

The results of the study indicate that the Town does not have an overall shortage of parking spaces in the downtown, but is heavily reliant on on-street parking to supplement a shortage of Bylaw-required off-street parking. A heavy demand for parking in a 2-block area of downtown east and west of the IGA intersection, coupled with poorly-defined parking spaces overall creates the impression of a parking shortage.

The following briefly summarizes many of the major findings of the study:

- 1. 6% of parkers are visitors to the Town.
- Of the visitor parkers: 82% are from Saskatchewan
   7% are from B.C.
   11% are from the rest of Canada and U.S.A.
- 3. Out-of-province vehicles consisted of: cars 65% trucks - 33% vans - 2%
- 4. Grand Centre's parked vehicles mixture is 28% trucks; 62% cars.
- 5. 41% of all parkers are long-term parkers.
- 6. 61% of long-term parkers park in lanes and off-street lots suitable for the purpose; 18% of long-term parkers park on-street in high-demand areas; while the remaining 21% of long-term parkers park on-street in areas of lesser demand.
- 7. The I.G.A. parking lot; Fields/Macleods block face; and the Value Drug Mart block face have the greatest parking demand and parking space turnover.
- 8. Only 37% of the Town's available parking spaces are occupied at peak demand.
- 9. Theoretical required parking in the study area is 1143 spaces (note-this is to be off-street parking).

- 10. The effective (actual) supply is 1241 spaces (note: this includes on-street and off-street parking).
- 11. If vacant lots currently used for parking were developed, about 100 spaces would be lost from effective supply.
- 12. 18% of total effective parking is lane parking, and all lanes except the Esso block are heavily used.
- 13. 96% of parkers park for less than 1 hour (most for less than 25 minutes) in the high-demand peak hour locations.
- 14. Peak hour demand locations are the Value Drug Mart; Fields; Select Furniture; I.G.A.; and Bordeleau Building Supply block faces.
- 15. Peak hours are approximately 11:00 AM to 3:00 PM with Thursday marginally generating greatest demand.
- 16. Parking violations are negligible with an occasional infraction noted whereby vehicles park over the 51 Street sidewalk in the I.G.A. lot.
- 17. In a 3 hour observation period, 27% of parked vehicles had CFB Cold Lake passes. Most were parked on-street in high parking demand areas.

#### Recommendations

- Parking spaces should be measured and clearly marked in the downtown block faces.
- 2. Handi-Bus stops should be moved slightly so that high-demand parking spaces are not lost.

#### Observations

- 1. Parking meters may not be appropriate for the town.
- 2. Public off-street parking lots could be made more attractive to users with clearly defined access to the core.
- Current parking controls and enforcement are adequate and effective.
- 4. Conversion of further block faces from parallel to angle parking is not advised.

#### 1.0 INTRODUCTION

#### 1.1 Background

In the "Fall" of 1984, the business community and the Town of Grand Centre became involved in a series of discussions which examined the amount, location, use, availability and effectiveness of parking in the downtown core. The discussions appeared to peak during the Christmas season when private lands were made available to citizens for municipal parking purposes.

There was a perception that insufficient parking was available in the Town and that some form of remedial action was required. It was also recognized that in order to understand the problems more clearly, and develop workable solutions, more hard data on actual parking useage in the downtown was needed.

Consequently, in February of 1985 the Planning Branch, Alberta Municipal Affairs was requested by the Town to undertake a detailed parking study of the Town's commercial core. A study outline and work program designed to examine the matter in detail were devised by Branch planning staff. The former was reviewed and accepted by the Municipal Planning Commission of the Town of Grand Centre as the Terms of Reference. Simultaneously the M.P.C. defined a study area (Figure 1) and a time frame of three typical peak demand days in the month of May during which data was subsequently gathered from actual field observations of parked vehicles. The analysis of that data forms the substance of this report.

#### 1.2 Purpose

This study provides specific data on parking in downtown Grand Centre. The data, conclusions and recommendations drawn from it should provide the Town with a clearer understanding of parking characteristics and habits in the Town's core. However, it should be remembered that this study represents a "snapshot in time". It identifies the parking pattern as it occurred during a 3 day period in May. Nevertheless it is considered that given the methodology used, the results obtained provide a useful indication of basic parking habits and characteristics in the Town.

Parking problems are however, as much perceptual as they are factual. This study cannot of itself determine whether the parking situation is satisfactory to the community. That is Council's and ultimately the Community's decision. We hope this study will assist in the decision making process.

#### 2.0 STUDY AREA

After careful consideration by the Town and its Municipal Planning Commission, the boundaries of the Study area were confirmed to be as shown on Figure 1. This was determined to be the extent of the Town's commercial downtown core area, and included the furthest outer limits from which it was expected people

would walk to the core. Subsequent data gathering during the course of the study confirmed this to be the case, and further confirmed the appropriateness of the study area and its limits. The survey included all on-street, lane, and private and public parking lots within those limits.

#### 3.0 APPROACH

#### 3.1 Timing

The Town considered May 16, 17, 18, 1985 an appropriate time to collect parking data. This coincided with a usual payroll period, as well as representing a usual mid-to-weekend shopping pattern. Although this also coincided with the first long-weekend of the year, it was felt that residents who might leave town for the weekend would still shop Wednesday or Thursday, and be "replaced" by tourists coming to the town on Saturday.

#### 3.2 Technique

The Grand Centre Study used two techniques to gather data on actual parking habits. The first involved a licence plate survey, the second was an "in-out" study of length of stay in selected locations.

#### 3.2.1 Licence Plate Survey

This involved walking a predetermined route on an hourly basis over 3 days (Thursday, Friday, Saturday) and identifying and recording occupied and vacant parking spaces (Figure 2). This was conducted on Thursday (9:30am - 8:30pm), Friday (8:30am - 12:15pm), Saturday (8:45am - 4:45pm).

(Note: an in-out survey was conducted Friday from (1:00pm - 3:00pm).

In order to determine whether the same vehicle was parked in that space on each hourly round, the licence plate number of the vehicle in the space was recorded. The plate numbers were not used for any other purpose. If the same licence (vehicle) was observed to be in the same space over a series of recorded intervals, this was noted in the record sheet and thus the duration and location of long-term parkers could be determined. In addition, the percentage of vacant spaces, their location, as well as peak demand hours and locations could also be determined.

In addition to the information which could be gleaned from analysis of the usual licence plate survey, it was felt by the Town that other useful data could be recorded simultaneously. Information regarding the numbers and types of out-of-province vehicles as well as types of vehicles occupying spaces (eg. truck, recreational vehicle, or car) was also recorded.

# PARKING STUDY - TOWN OF GRAND CENTRE

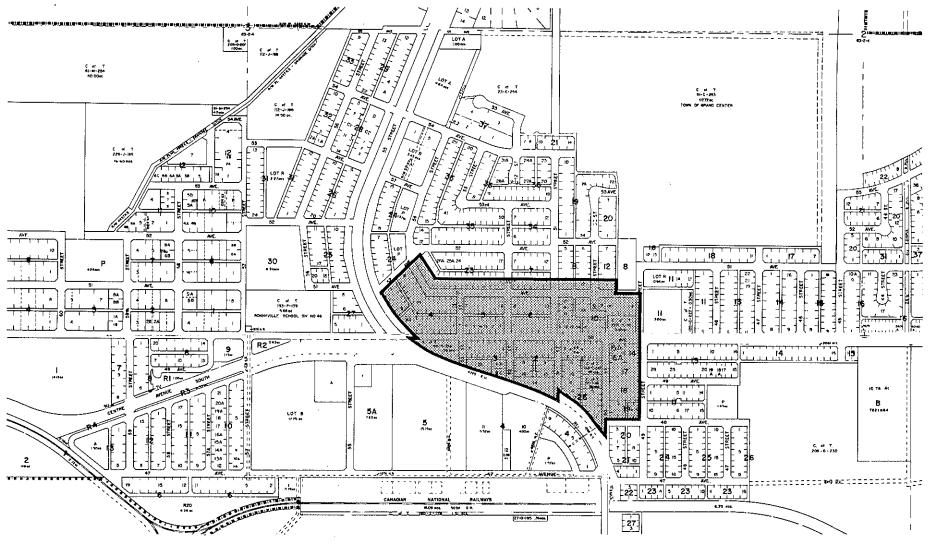
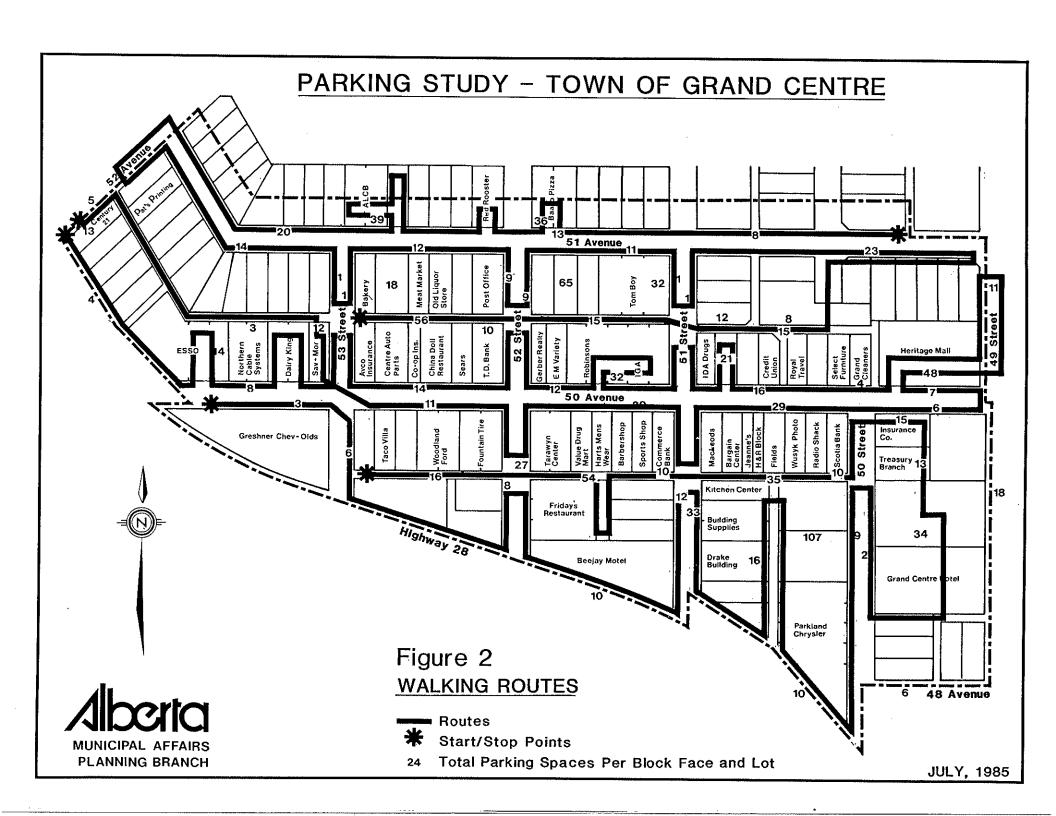




Figure 1 STUDY AREA



Study Area



#### 3.2.2 <u>In-Out\_Survey</u>

This second survey (conducted Friday 2:00pm - 5:00pm) was designed to identify the frequency of turnover of parking spaces in high-demand areas. This survey involved observing blocks of parking spaces (about 25-30) (Figure 3) over several hours and precisely recording the length of time each parked vehicle occupied that space. An indication of the intensity of use, car volumes, and space availability can be determined. The locations selected for this study were the Fields and Value Drug Mart block faces, and the Robinsons parking lot since data indicated a high parking use in these locations.

#### 3.3 Research Focus

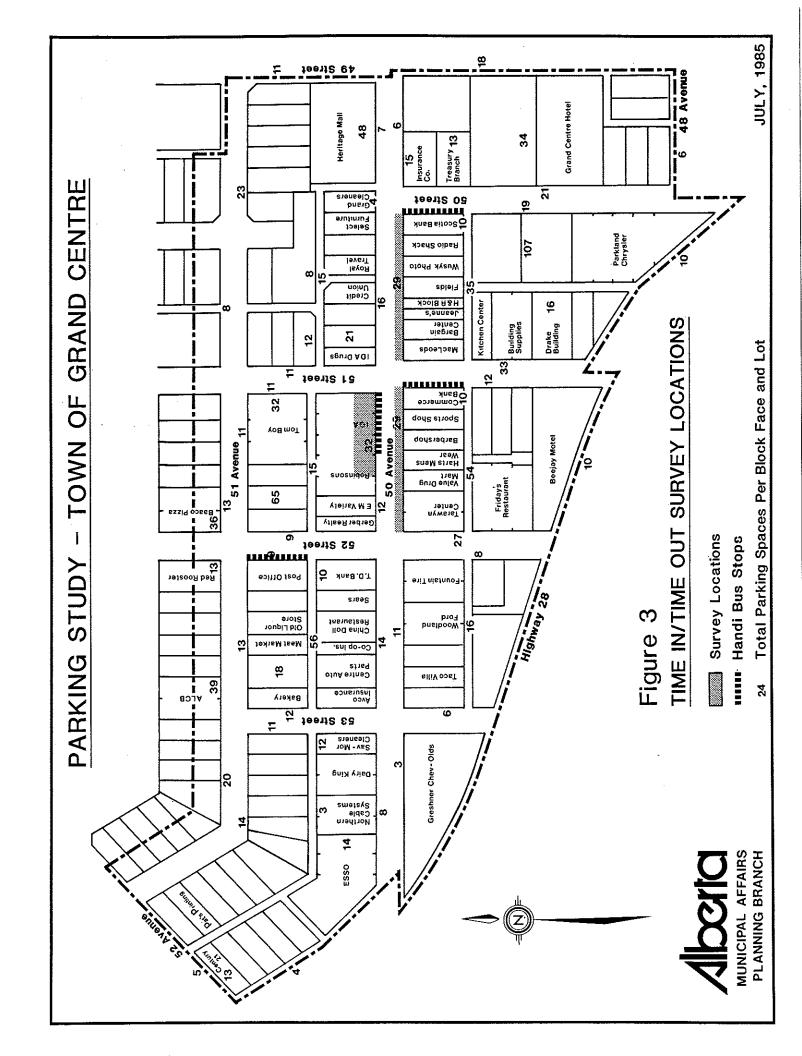
Given the techniques to be applied to the analysis of the Town's parking status, it was considered relevant to gather data which would address the following questions:

- 1. What percent of parkers are visitors?
- 2. What percent of parkers are from Saskatchewan/B.C./Other?
- 3. What percent of visitors drive R.V.'s/Cars/Trucks?
- 4. What percent of all parkers drive trucks/cars?
- 5. What percent of parkers are long term parkers?
- 6. Where do long term parkers park?
- 7. What location has the greatest parking space turnover?
- 8. What is the occupancy rate of each defined area?
- 9. What is the occupancy rate of the town as a whole?
- 10. What is the theoretical required parking?
- 11. What is the actual supply?
- 12. What parking will be required/lost if vacant lots are developed?
- 13. What percent of total parking is lane parking and what lanes are most heavily used?
- 14. What percentage of long term parkers use suitable long-term parking areas?
- 15. What is the occupancy rate of on-street vs off-street parking?
- 16. What are the peak hour demand locations?
- 17. What are the peak hours/days?
- 18. Where are the parking violations?
- 19. What percentage of vehicles observed have CFB Cold Lake Base passes?

#### 3.4 Parking Supply

#### 3.4.1 Actual Supply

An inventory of available parking spaces within the study area was undertaken. Where stall spaces were marked (as in some parking lots), they were counted. Where stalls were not marked (largely curbside and some large lots) the space was paced off allowing approximately 22 feet for a parallel stall and 12 feet for an angle stall. Laneways, access points, and other obstacles were taken into consideration.



Unmarked parking lots, where largely unorganized parking occurred, were counted by designing a parking lot grid on surveyed plans of the land. These spaces were then identified in the field by pacing them off in areas of the lot where vehicles tended to park.

Lane parking was estimated on the basis of observed parking practices with allowances for loading bays, garbage bins and other obstacles.

Using the above techniques therefore, the Town's actual supply is:

50 Ave (on-street)	118
Streets north of 50 Ave (on-street)	74
Streets south of 50 Ave (on-street)	144
51 Ave (on-street)	102
Laneways	225
Major parking lots	208
Minor parking lots	84
Vacant lots (currently used for parking)	210
Remaining streets and avenues	43
Remaining lots	_33_
TOTAL	1241 spaces

#### 3.4.2 Theoretical Demand

Theoretical parking demand can be calculated by assuming that 80% of any developed lot is leasable retail floor area. Calculations indicate that 80% of current commercially developed land in the study area totals approximately 63,895 square metres. Grand Centre's Land Use Bylaw stipulates a variety of parking requirements, including 1 space for every 65 sq. metres of leasable floor area. Given however that patron-intensive commercial uses such as restaurants and taverns are required to provide more parking based on seating and numbers of employees, then this figure could be adjusted to 55 sq metres in order to calculate theoretical demand. Therefore, given the above parameters, theoretical demand is estimated to be 1143 spaces. Thus there is a current excess of supply over theoretical demand of about 79 spaces. (see 3.4.3 below for an adjustment of this figure)

However, should vacant lots now used for parking be developed, up to 179 spaces would be deleted from current supply (net) resulting in a shortage of approximately 100 spaces.

This figure is described as a "net" amount since each new development would be required to supply parking as per bylaw regulations.

More significantly however is the fact that "theoretical parking demand" relates to required off-street parking. In Grand Centre, 481 spaces or 39% of the Town's actual supply is on-street parking. While this may indicate a possible shortcoming in the Town's Land Use Bylaw parking requirements, it should be noted that Grand Centre's status is similar to other Alberta towns (see 5.1).

#### 3.4.3 Handi-Bus

Since this study was completed and calculations finalized, the Town instituted a Handi-bus system. Five bus stops in the core have eliminated about 15 on-street parking spots. (see Figure 3) Therefore the figures noted in this report regarding parking supply could vary slightly from actual current circumstances as a result.

#### 4.0 Rural Factors

The Town of Grand Centre is a regional agricultural and resource-based centre serving as a watershed for large rural populations who regularly visit for shopping, business, and recreational purposes. This regional demand is superimposed on the internal parking demands. Thus, Grand Centre's parking characteristics are not just a product of resident activity. Furthermore, rural demand tends to be a peak demand in that late night shopping is an attractive time to "go to town". The downtown area is attractive to rural users as it offers them an opportunity to satisfy all their demands in one central area. This may account in part for the large number of light trucks observed in the core.

In order for communities to continue to function as focal points for rural areas, adequate parking must be available. To accommodate local and rural parking, some excess parking capacity will exist during off-peak times of the week. This excess capacity is then attractive to the all-day parker who may monopolize these spots continuously.

#### 5.0 ANALYSIS

#### 5.1 Comparative Characteristics

As noted above, an estimate of requirements for parking based on the current land use bylaw indicates the downtown requires about 1143 spaces. Grand Centre has an actual supply of approximately 1241 spaces or 388 spaces per 1000 population (3195 population 1984). Even when the 1985 population of 3506 persons in Grand Centre is used, the supply is still 327 stalls per 1000 population — significantly greater than other similar towns. The following data identifies the results of a recent study of parking in comparable Alberta towns:

#### Existing Parking Supply Characteristics

Community	On-street Number (%)		Off-street (Number (%)		<u>Total</u>	Population (1984)	Supply/1000 Population	
Fairview	265	(33)	549	(67)	814	3234	254	
Grand Centre	496	(40)	745	(60)	1241	3195	388	
Grimshaw	389	(64)	215	(36)	604	2488	243	
High Level	279	(40)	386	(60)	665	2806	237	
Pincher Creek	315	(38)	514	(62)	829	3712	224	
Vermilion	376	(61)	232	(39)	608	3769	161	

Source: Modified from Mackenzie Regional Planning Commission - "Municipal Parking Research Project" February 1985

This data suggests that there is a sufficient overall supply of parking in the downtown. However, parking problems usually arise not because of an overall shortage of parking spaces, but because there are not enough spaces in the right place. This is the case in Grand Centre as this study will demonstrate. The focus for parking activity is the Credit Union, Robinsons, Tarawyn Centre, Fields block faces, including 51 street north and south of 50 Avenue. The congestion in this area appears to result from a reluctance to park elsewhere and walk to this district.

Research reveals that distances people are willing to walk after parking their vehicles becomes larger as population increases. In smaller urban communities, patrons have a tendency to reduce their walking lengths by parking as close to an establishment as possible, and then moving their vehicles from store to store as opposed to walking. In most cases, this is due to the general availability of stalls within a given area.

This phenomenon was observed and recorded during the Grand Centre study, particularly where persons shopping with small children were involved.

Moreover, studies have shown that based upon duration, parkers tend to accept greater walking distances as their parking durations increase. On the contrary patrons who are short-time parkers seek out parking facilities within 200-500 feet of their destination. This was confirmed in the Grand Centre study where it was noted the highest parking space turnover rate was in the Fields to Robinsons blocks on both sides of 50 Avenue.

Equally important is the development of convenient and attractive pedestrian access from parking lots to commercial street frontage. This is often difficult to establish since off-street lots are either remote from the demand, or access is blocked due to existing development. Grand Centre experiences this problem most acutely in the high demand areas.

#### 5.2 Parking Demand

Other parking studies have shown that irrespective of the amount of curb parking that is available, curb facilities serve a larger proportion of the total parking demand than they constitute of the total supply. Whereas curb spaces usually constitute about 45% of the total supply (39% in Grand Centre), they serve about 65-75% of the total demand based upon previous studies. Analysis of the data indicates this is the case in Grand Centre.

A review of parking studies in other towns indicated that:

- at peak periods of the day, some key block faces reach capacity, however, parking areas immediately adjacent (on and off-street) were almost totally ignored.
- large off-street parking lots located in close proximity to the core areas were generally under-utilized.

- currently a general deficiency of off-street spaces with respect to bylaw requirements exists.
- employees of downtown businesses were found to be parked in on-street or parking lot stalls intended for customer useage. (Robinsons and Heritage Mall lots experienced this problem).
- rear lanes were generally unusable and thus failed to attract non-employee parkers.

All of the above were found to also be factors in Grand Centre. Based upon these general observations noted in other parking studies and outlined above, the following is apparent with respect to parking in Grand Centre's Core:

- the high rate of utilization of on-street parking spaces in a two block area of the core does not reflect a shortage but seems to indicate a reluctance on the part of patrons to park in off-street lots or laneways and walk a short distance to complete their business. On the other hand, a lack of strategically placed and readily accessible off-street lots, as well as good access to lane parking, may exacerbate the situation.
- this high rate of utilization may also be the result of an inappropriate use of on-street parking facilities. In Grand Centre it is commonplace for vehicles, especially light trucks to occupy space where two vehicles could park. When a buffer in front and rear of the vehicle is added (usually 3-4 feet at each end), the limited space devoted to parking is poorly or inefficiently used.
- on-street parking currently compensates for a general deficiency in parking required under bylaw practices.

#### 5.3 Occupancy Rates

The Grand Centre licence plate survey provided data on the overall parking occupancy rates of each block in the downtown. As a general rule, a block that is 90% occupied can be considered effectively full, a block or lot that is between 75 and 90% occupied suggests that the capacity of the block is being reached.

#### 5.3.1 On-Street Parking: 50 Avenue (Table #1)

50 Avenue (Main Street) as the principal business strip of Grand Centre generates the greatest demand for parking. Except for the Heritage mall block face, all other block faces on 50 Avenue approached capacity simultaneously on several occasions. It is noteworthy that this occurred from 4:00pm to 7:30pm on Thursday; lunchtime on Friday; 10:00am to 3:00pm on Saturday. The busiest block faces appeared from the data and observation to be the on-street parking in front of the I.G.A. and Credit Union, and the south side of 50th Avenue between 50 and 55 Street, each being completely full on 4 of the 17 occassions (90-100%). The remaining block faces along 50

Avenue were full or approaching full on a significant number of occasions. These block faces generate the greatest demand and seem to be the focus for downtown parking activity. Even the side streets north and south of these locations to the relevant laneways, are in demand for parking. (Figure 4)

#### 5.3.2 On-Street: 51 Avenue (Table #2)

In contrast with 50 Avenue, 51 Avenue is largely underutilized. Only the block face in front of the Post Office approached capacity twice or was full on one occasion. The remaining block faces along 51 Avenue never reached more than 55% of capacity on any occasion.

#### 5.3.3 On-Street: Streets North of 50 Avenue (Table #3)

Only two of these block faces ever approached capacity; these being both sides of 51 Street, and only on one occassion each. The remaining blocks rarely surpassed 50% occupancy indicating a significant degree of underutilization. According to the data and field observations, utilization of these side streets tended to occur mainly between 50 Avenue and the laneways.

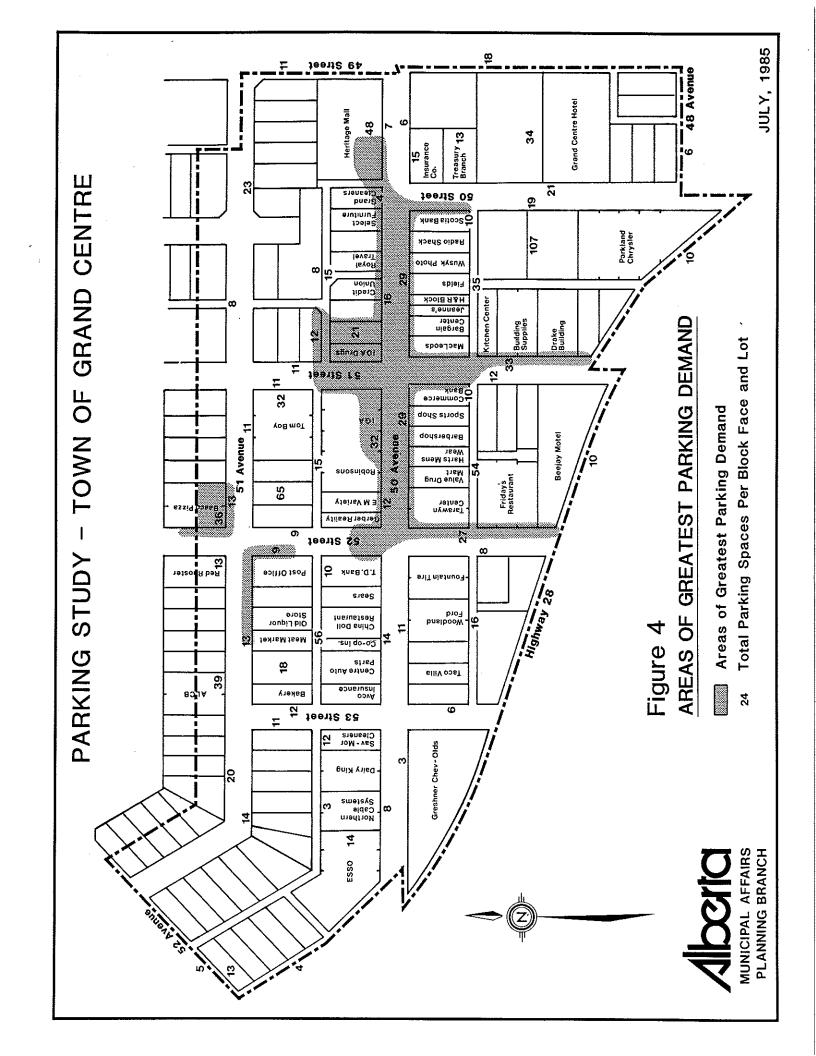
#### 5.3.4 On-Street: Streets South of 50 Avenue (Table #4)

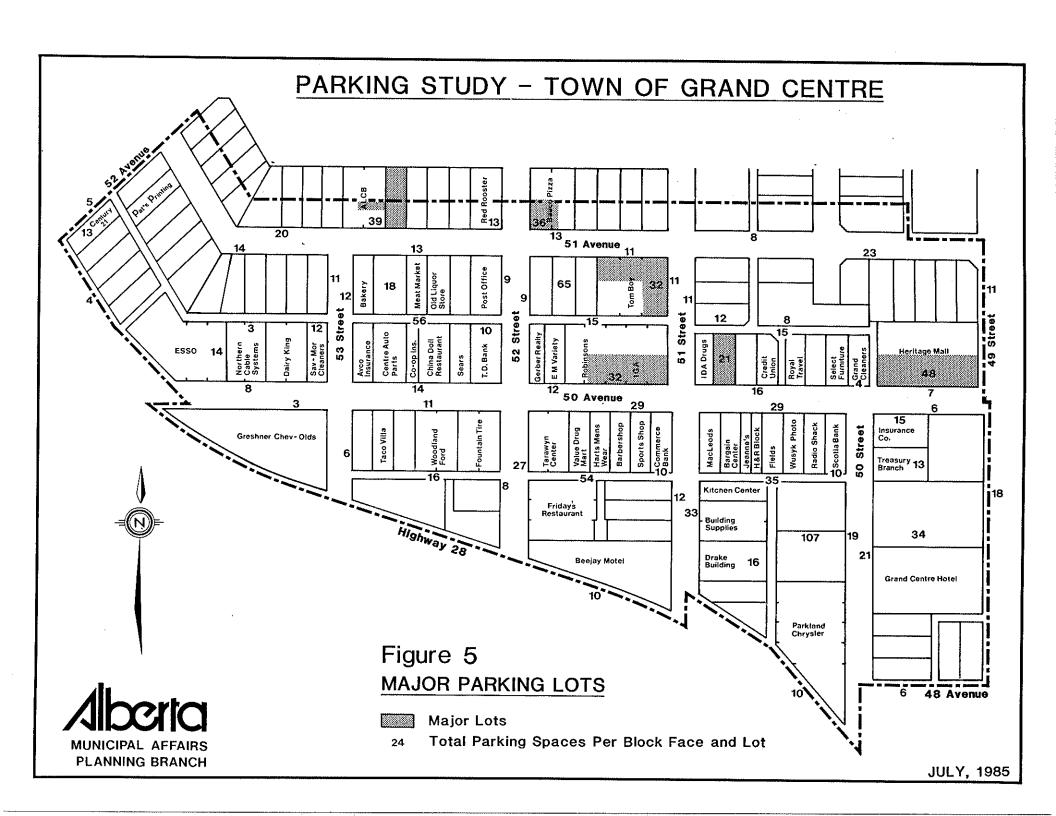
Two block faces were noted as areas of high demand; these being both sides of 52 Street by the Tarawyn Centre and Friday's Restaurant building. The west side of 51 Street adjacent to the Commerce Bank and the Beejay Motel also reached capacity on Saturday. All other block faces on side streets south of 50 Avenue rarely approached capacity, again indicating a high degree of underutilization for sidestreets off 50 Avenue.

#### 5.3.5 Laneways (Table #5)

In Grand Centre 18% of all parking spaces in the downtown study area are lane spaces (225). However only an average of 37% of all lane spaces are used for long-term parking (4 hours or more), while 43% of persons who park in lanes are long-term parkers.

Laneways, unlike streets, are considered well-utilized at 50% capacity since they do not demand a high turnover. They are intended primarily for long term employee parking and some customer use. Grand Centre's laneways are somewhat under-utilized with an average occupancy of 37% noted for the 3 days of the study. Only the I.G.A. block approached capacity in the laneway on one occasion, while all of the other lanes were largely under-utilized in terms of long-term parking and general use. Section 5.5 provides a discussion of long-term parking characteristics in the Town.





#### 5.3.6 Major Parking Lots (Table #6) (Figure 5)

The most intensively used parking lot is that of Robinsons and I.G.A. This lot was full or approaching capacity on all but 3 of the 17 occasions, it was observed. All other major lots (A.L.C.B., BAACO, Tomboy, I.D.A., Heritage Mall) appeared to be well utilized (approaching or reaching capacity on a number of occassions) with the exception of the A.L.C.B. parking lot which never exceeded 36% of capacity on any of the 17 observations.

#### 5.3.7 Minor Parking Lots (Table #7) (Figure 6)

Minor parking lots are those adjacent to banks, insurance agencies and convenience stores. Of the lots observed, the Scotia Bank was the most intensely used. The Toronto Dominion and Commerce Bank lots approached capacity on not more than 2 occassions each, with both being underutilized on all other occasions. The other lots (Red Rooster, Treasury Branch, Century 21, and the Insurance Co. lot east of Scotia Bank) also were under-utilized throughout the study period. The Kentucky Fried Chicken and Dairy King lots were rarely used, with quick pick-up and drive-through service being the most common useage. At the most two or three vehicles were noted at lunch and dinner time in these lots, the rest of the time they were vacant.

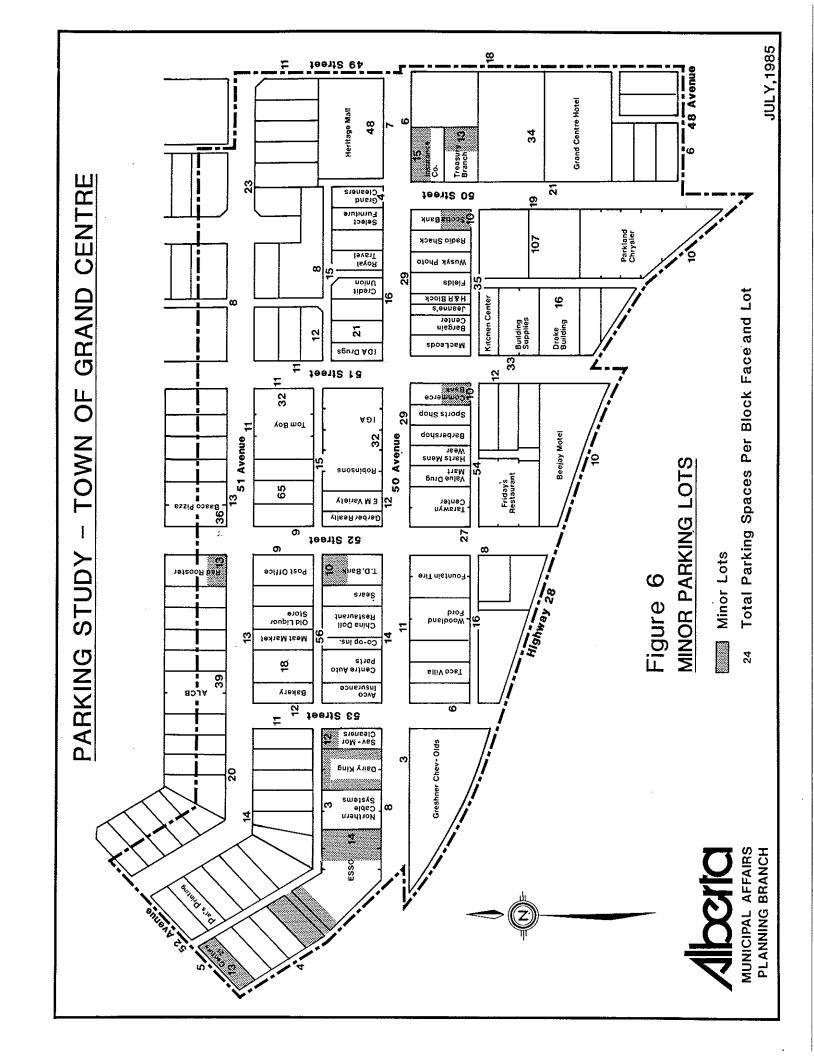
#### 5.3.8 Vacant Lots Currently Used for Parking (Table #8) (Figure 7)

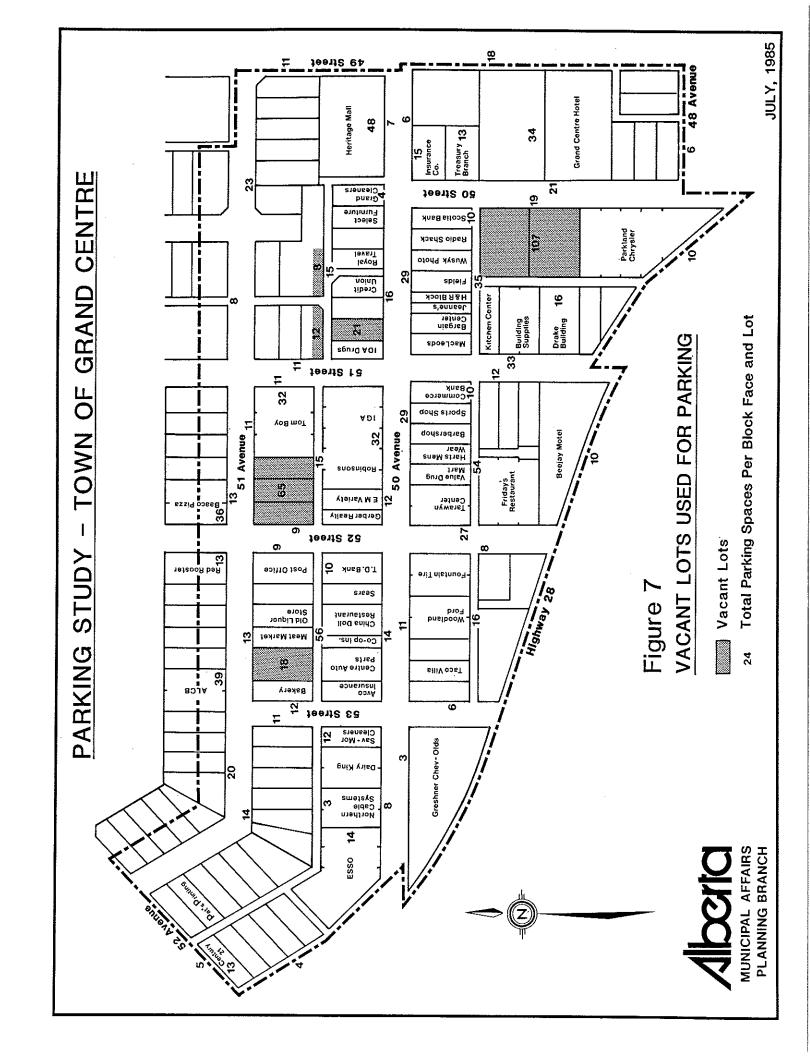
Of the lots in this category, only two ever approached a significant degree of use. The busiest was the lot located north of the laneway from the I.D.A. store. Its 12 spaces were full or approaching capacity on 12 of the 17 observations. The second, located behind the Credit Union has 8 spots and approached capacity only once.

The Kowalski lot and the vacant lot across from the Grand Centre Hotel each have the capability of accommodating 65 and 107 vehicles respectively, but are vastly underutilized. The Kowalski lot has topographical constraints, while the vacant lot across from the Grand Centre Hotel lacks identification as a parking area, as well as convenient access to 50 Avenue. It does not have a wet weather surface.

#### 5.3.9 Peripheral Streets and Avenues (Table #9)

Of these 6 locations which are peripheral to the core, only the portion of Highway 28 fronting Parkland Chrysler demonstrated high demand. The demand however appeared to arise from customers and employees of Parkland. The remaining streets were significantly underutilized possibly due to their remoteness from the core.





#### 5.3.10 Selected Private Parking Lots (Table #10)

These include the Esso Station, rear of Northern Cable, rear of Sav-Mor Cleaners and Grand Cleaners. Of these, the busiest was Grand Cleaners on 50 Street and 50 Avenue which was filled or approaching capacity on 4 of 17 occassions. While the Esso lot was well used, the Sav-Mor lot was not.

#### 5.4 General Occupancy Observations

As a whole, the occupancy rate for the town for all three days was 37% of capacity. The breakdown between on-street and off-street parking is as follows:

On-Street	Thursday Friday Saturday	39% 38% 38%
	Average	38%
Off-Street	Thursday Friday Saturday	38% 37% <u>32%</u>
	Average	35%

These figures support the observation that there is no overall shortage of available parking in the downtown area of Grand Centre. If there is a perceived shortage, it is that a majority of the parkers attempt to concentrate within a small portion of the downtown, on 50 Avenue between 50 and 53 Street and the I.G.A. parking lot. Table 11 indicates that even when the side street parking is considered with 50 Avenue block face parking as a unit, there is still only a 50-60% occupancy rate for this high demand area. Neither is there a significant problem with long-term parking in the core area since fewer than 6% of observed on-street parking was long-term.

The problem apparently lies mainly in the possible unwillingness of patrons to park elsewhere and walk to and through the core. In addition, the lack of <u>clearly defined</u> parking spaces in the high-demand areas (and elsewhere) leads to unintentional abuses whereby parkers use 2 or more spaces. Light trucks (which comprised 28% of all observed vehicles) because of their length and width are the most frequent abusers, although many cars were also observed in this practice. This practice has the effect of reducing the total number of available spaces in high demand areas.

It is felt that should all spaces in the town be clearly measured and marked, the effective supply would likely increase.

#### 5.5 Long Term Parking

Long term parkers are those parkers observed to occupy a space for 4 hours or more. Areas suitable for long-term parking are considered to be laneways, most vacant lots, major off-street controlled parking lots, as well as some peripheral on-street locations. In Grand Centre, 620 stalls, or 50% of the 1241 total are considered suitable for this purpose. (see 3.4.1) The data indicates that approximately 41% of all parkers in the study area are long-term parkers.

The percentage of total long-term parkers (LTP) who parked in long term parking areas (LTA) was calculated as follows:

	Total LTP	LTP in LTA	% of Total
Thursday	247	156	63%
Friday	240	140	58%
Saturday	<u>151</u>	92	61%
Total	638	388	61%

On average, 61% of all long-term parkers parked in areas suitable for that purpose. Although this is a significant majority, 39% are parking in areas not intended for long-term parking. The data indicates that (Tables 12 and 13) only 18% of long-term parkers parked on 50th Avenue in the high demand parking areas. Thus, further analysis indicates 51, 52 and 53 Streets north and south of 50 Avenue, as well as 51 Avenue itself are key locations for the remaining 21% of long-term parkers. It can be concluded therefore that long-term parkers are not entirely responsible for the peak hour congestion in the core focus of the town.

#### 5.6 Turnover

Turnover is the rate at which stalls are filled, vacated, and re-occupied. If the turnover rate is high this means that spaces are quickly vacated and quickly filled. If the turnover rate is low this means that vehicles park for longer periods of time or that stalls are not quickly re-occupied. The turnover rate is measured using the time in/time out method, as described previously. In the case of Grand Centre, this survey was conducted on Friday, May 17/85 between the hours of 1:00 and 3:00pm.

The town perceived a problem with turnover rate. It was believed that vehicles were parking too long along the key block faces of downtown. The time in/time out survey provided an analysis of this situation. Using the license plate survey, the areas indicating the greatest turnover over the balance of the day were determined.

These were the two southern sides of 50 ave between 50 and 52 Street (angle parking), and the I.G.A. parking lot.

During the study period, the southern block face between 50 and 51 Street accommodated 163 vehicles. 96.3% of these parked for less than one hour, 3.1% for between one and two hours, and 0.6% for more than 2 hours. This data can be analyzed in the following manner:

	Minutes	<u>Vehicles</u>	% of Total Vehicles
Parking Duration	0-14	107	65.6%
50 Avenue	15-29	36	22.1%
(50-51 Street)	30-44	12	7.4%
	45-59	2	1.2%
	60-74	2	1.2%
	75-89	1	0.6%
	90-104	1	0.6%
	105-119	1	0.6%
	120+	1	0.6%
TOTAL		163	100%

For the Southern block face between 51 and 52 Street, a total of 121 vehicle movements were recorded with 90.1% of these parked for less than one hour, 8.3% between one and two hours, and 1.7% for more than two hours.

	Minutes	<u>Vehicles</u>	% of Total Vehicles
Parking Duration	0-14	56	46.3%
50 Avenue	15-29	34	28.1%
(51-52 Streets)	30-44	13	10.7%
	45-59	6	5.0%
	60-74	2	1.7%
	75-89	5	4.1%
	90-104	3	2.5%
	105-119	0	0%
	120+	2	1.7%
TOTAL		121	100%

In the I.G.A. parking lot, a total of 123 vehicle movements were recorded. 94.4% of these parked for less than one hour, 2.4% between one and two hours, and 3.3% for more than two hours.

	Minutes	<u>Vehicles</u>	% of Total Vehicles
Parking Duration	0-14	67	54.5%
50 Avenue	15-29	31	25.2%
(I.G.A. Parking Lot)	30-44	14	11.4%
	45-59	4	3.3%
	60-74	2	1.6%
	75-89	1	0.8%
	90-104	0	0%
	105-119	0	0%
	120+	<u>4</u>	3.3%
TOTAL		123	100%

For the southern portion of 50 Avenue between 50 and 51 Street the average number of cars per space was 5.6 every two hours with an average occupancy time of 16.2 minutes for each vehicle. For the portion between 51 and 52 Street, the average number of cars per space was 4.2 every two hours with an average time of 24.6 minutes. For the I.G.A. parking lot the average was 3.8 cars per stall over two hours with an average time of 20.6 minutes.

Since only 7 of 407 vehicles observed parked for more than 2 hours it appears that the turnover rate within the busiest sections of downtown Grand Center is adequate to handle the volume of cars which use the downtown. A reduction in parking duration from the currently permitted 2 hours to 30 minutes may increase parking turnover, but this may not be acceptable or desirable.

#### 5.7 Trucks/Vans/Recreational Vehicles

Trucks, vans and R.V.'s are larger, less maneouverable vehicles which can create particular parking problems, such as occupying 2-3 spaces at one time. It is important when planning parking strategies, to determine the proportion of such vehicles to all parkers, and plan for their needs.

The proportions of trucks, vans and R.V.'s to the total number of vehicles surveyed in Grand Centre were as follows:

Thursday 27% Friday 29% Saturday 28%

All three days 28% (average)

Researchers noted a consistent pattern of truck parking along the Macleods/Bordeleau Building Supplies/51 Street block face; both sides of 50 Street of 50 Avenue; and along the 50 Avenue Credit Union block face. Light trucks also tended to locate in the Heritage Mall, Baaco Pizza, and I.G.A. parking lots. Elsewhere within the study area, trucks were fairly evenly distributed. These observations may be the result of the Town's policy of banning tall or large vehicles from parking along the Value Drug Mart and Fields block faces. At least one vehicle in violation of this By-law was noted to have been ticketed.

#### 5.8 Out-Of-Province Vehicles

Table 14 lists results of data collected regarding out-of-province parkers. It also identifies the proportion of visitor vehicles according to province of origin (ie Saskatchewan, B.C., other). This information was gathered since it was considered to be of value and/or interest to the business community.

Of the 227 out-of-province vehicle observations, 82% were from Saskatchewan, 7% from B.C., and the remainder from Ontario, Manitoba, Quebec, Nova Scotia, Montana, and California. The Saskatchewan vehicles accounted for 3.9% of all the parked vehicle observations in downtown Grand Centre for the study duration.

The most popular day for visitors was Saturday during which 6% of all downtown parkers were out-of-province. Perhaps the long weekend was a factor.

Out-of-province vehicle observations were as follows:

	Cars (%)	Trucks (%)	Vans (%)	R.V.'s (%)	Total
Thursday	67 (65)	35 (34)	1 (1)	0	103
Friday	14 (52)	13 (44)	1 (4)	0	27
Saturday	67 (69)	27 (28)	3 (3)	0	<u>97</u>
TOTAL	148 (65)	74 (33)	5 (2)	0	227

#### 5.9 Base Access Vehicles

During the data collection on Saturday, researchers noted a high proportion of vehicles with armed forces base stickers. In one casual observation, 27 of 33 vehicles along the Tarawyn Centre block face displayed the sticker. It was decided to identify those vehicles on subsequent licence plate number recording rounds for information purposes only.

Of the 1059 vehicle observations recorded during these subsequent rounds 285 or 27% carried base stickers. These vehicles were noted mainly in the high demand areas on 50 Avenue. This may seem to indicate a noticeable influence of the base, and those with access to it, on the parking requirements of the town.

#### 5.10 Handi-Bus Impact

As noted in Section 3.4.3 at least 15 parking spaces have been lost to the Town's new Handi-Bus. Unfortunately these are critical, high-demand spaces in the busiest part of the core (compare Figures 3 and 4). While the bus is intended among other things to reduce the need to travel downtown by car, it is unlikely to achieve a significant impact in this regard. The likely affect is to increase the demand for whatever spaces are left in this area and thereby increase the perceived shortage of core area parking.

#### 5.11 Peak Hours

The peak demand for parking is Thursday between 1:30 and 4:00pm, with Friday from 11:00am to 2:00pm and Saturday from 1:00 to 2:15pm close behind. The busiest days were also in the order noted above.

#### 5.12 General Observations

According to the data there is no shortage of parking spaces in the defined core of the Town. Congestion occurs in a small area along both sides of 50 Avenue between 50 and 52 Street since this location seems to be the focus of business activity. Patrons appear unwilling or unable to park further away and walk to this area even though the data indicates parking is readily available at remote locations (relatively speaking).

Consequently "cruising" this area searching for a parking spot in close proximity to a desired business, creates congestion at this focus. This is compounded by the fact that parking stalls at this location experience high turnover with most parkers staying for less than 15 minutes and many for less than 5. congestion is most critical on 51 Street at the entrance to the I.G.A. lot. Since parking spaces are frequently available in this area of the downtown, patrons have come to expect to find one there. This conditioning of expectations is difficult to change and undoubtedly has resulted in certain expressed frustrations when spaces are unavailable at this location, leading to the conclusion that there is a parking "shortage" downtown.

Researchers also noted that almost all parking spaces in the study area are undefined. As a consequence patrons park wherever they find it convenient. All too frequently the location chosen effectively uses two spaces. In addition, at unmarked locations (especially along block faces) patrons tend to leave inordinate amounts of room in front of and to the rear of their vehicles. This compounds the problem and effectively reduces the parking "carrying capacity" of the block face.

Moreover, in studies that it conducted the American Automobile Association found that it takes almost 43% less time to park in marked spaces. Undefined parking is also a problem in many off-street parking lots (eg Kowalski, Bordeleau, Grand Centre Hotel) since patrons park wherever convenient, and frequently maximum use of the lot is precluded.

Long-term parking in short-term parking areas is also not a significant problem overall. However it was noted that at least 5 of 32 spaces in the Robinsons/I.G.A. parking lot appeared to be used for employee parking. This may be undesirable in such a high demand area.

#### 6.0 Recommendations

#### 6.1 Stall Marking

It is evident that parking spaces should be clearly defined in the Town. This may have the desired effect of not only regulating and allocating parking, but also of increasing the actual supply. Painting the stalls may not be sufficient since they are covered by ice and snow for long periods. Allowances will have to be made for longer vehicles such as trucks and vans. Perhaps a designated "Trucks Only" parking area should be created to accommodate their unique needs.

#### 6.2 Handi Bus Stops

Since the designated stops for the Handi Bus are located in locations having peak demand, and this may tend to compound an already critical shortage of parking spaces there, other locations for bus stops could be found.

In the case of the 50 and 51 Street Handi-Bus stops, they could be moved 1/2 block south without any detrimental impact so that parking spaces less in demand could be used. Similarly the IGA stop could be shifted west on 50 Street to the front of Gerber Realty. The Post Office stop could relocate west of its current 52 Street location to 51 Street in front of the Bakery.

These alterantive locations would restore 15 spaces of much needed parking in high demand areas, without loss of Handi-bus efficiency or significant inconvenience to bus patrons.

#### 7.0 Summary Observations/Alternatives

#### 7.1 Parking Meters

Parking meters and posts clearly define parking spaces in all weather and regulate length of use, yet they may not be necessary or acceptable to the Town as a whole. The purpose of parking meters generally is to increase the turnover and availability of stalls and eliminate long-term parking. In the United States the effectiveness of meters was evaluated by the U.S. Highway Research Board. Their study found that meters resulted in 35% higher turnover; 45% fewer vehicles parked overtime; and one-third to one-fourth the time formerly needed to patrol areas with sign-dependent parking restrictions.

In Grand Centre however, parking stall turnover is not a problem as the data has demonstrated.

The initial costs of meters is relatively high (about \$500 - \$600 each) although it is generally believed that they pay for themselves over the longer term. In high turnover areas however, this may not be the case, particularly if a "grace period" is built into the meter system.

Medicine Hat and Swift Current (Saskatchewan) have experience with meters providing up to 15 minutes free parking before a parking fee is required. These meters apparently do not pay for themselves.

In spite of certain advantages, parking meters rarely generate favourable public response. Individuals feel inconvenienced because they must always fumble for change; or they do not want to put money in the meter if they intend to stop for a brief moment. Merchants feel that charging people to park discourages customers from doing business downtown. Councils must be careful how they approach the idea of parking meters and be quite sure that the benefits outweigh the costs.

#### 7.2 Off-Street Parking Lots

Other alternatives lie in the development of regulated off-street parking lots close to the focus of activity in the core (ie 50 Avenue). Grading, paving (or gravelling) and clearly defining parking stalls, back-up laneways, and points of access/egress are essential for maximum efficiency. The design should accommodate winter snow clearing and other related conditions. In addition pedestrian access to and from the parking lots and core area must be reasonably direct, safe, and attractive. Parker habits must be changed through effective information programs to encourage more use of off-street parking.

#### 7.3 Parking Controls

Parking controls such as the two hour limit presently used in much of the Town's core seem to be effective in encouraging rapid parking space turnover and in discouraging long-term parkers from occupying key parking locations. Illegal parking (other than exceeding the 2 hour limit) is generally not a problem in the Town. It was observed in the I.G.A. parking lot where occasionally vehicles would park next to the phone booth over the 51 Street sidewalk. It was also noted in the laneway east of Grand Cleaners. Elsewhere it was not noted to be a concern.

#### 7.4 Conversion from Parallel to Angle Parking

The purpose of such a conversion is to theoretically double the supply of on-street parking. There are however, a number of drawbacks to this proposal. First, angle parking is generally opposed by engineers and police as it results in substantial conflict with through traffic. In the case of Grand Centre, the street of highest demand already uses angle parking on two of its block faces. Angle parking on the opposite sides will aggravate an already acute problem. Secondly, a previous study conducted in the Town of Wainwright suggests that if the conversion was made, it would likely only serve to draw those vehicles, generally owned by employees and parked for most of the day, from adjacent streets and laneways to the street. Therefore, one would expect to find a high occupancy rate on 50 Avenue but with a lower turnover rate and greater proportion of long term parkers. The objective of increasing street front parking supply for shoppers would not be greatly enhanced by this method.

#### 8.0 Conclusions

This study provides data on the perceived parking habits of Grand Centre patrons at a point in time. Certain generalizations and comparisons (perhaps even with other Communities) can be made but certain discretions should be exercised in doing so.

In order to have a more definitive image of the Town's parking characteristics, several such studies should be conducted at various times of the year and the results generalized and compared. There are of course practical and financial limitations to such a process.

While the current study may lack the broader scope envisaged above, it nevertheless provides some excellent data for discussion and contemplation. It is our hope that it will be entirely useful as a basis for broader debate leading to resolution of the Town's current discussions on the matter.

APPENDICES

TABLE #1

## OCCUPANCY RATE OF PARKING STALLS 50 AVENUE ON-STREET (FIGURES SHOW PERCENT OCCUPIED)

LOCATION	North Side 52-53 St.	On Street Robinson Block	North Side IDA to Cleaners	On-Street Heritage Mall	South Side 50-51 St.	South Side 50-51 St.	South Side 52-53 St.
NUMBER OF SPACES	14	12	16	7	29	29	11
THURSDAY							
9:30	29	17	56	14	55	83	64
10:45	50	67	44	14	59	55	64
1:30	43	58	50	29	83	86	64
2:30	79	50	19	14	62	52	73
4:00	50	100	69	14	69	93	100
5:30	79	92	63	0	<b>45</b> .	59	82
6:30	86	76	44	0	66	72	64
7:30	57	67	69	0	52	83	82
FRIDAY							
8:30	14	8	6	29	10	23	45
9:45	36	33	69	29	62	77	64
11:15	57	58	63	29	48	65	100
SATURDAY							
8:45	43	42	56	14	28	41	64
10:00	43	58	63	43	48	90	73
11:15	93	75	75	0	69	100	91
1:00	71	75	75	0	83	100	82
2:15	79	92	63	29	83	83	64
3:15	36	83	69	43	69	76	64

TABLE #2

# OCCUPANCY RATE OF PARKING STALLS 51 AVENUE ON-STREET (Figures Show Percent Occupied)

LOCATION	North Side 50-51 St.	(Baaco) North Side 51-52 St.	(ALCB) North Side 52 St-52 Ave	South Side 52 Ave-53 St	(Post Office South Side 53-52 St.	e) South Side 52-51 St.	(Tom Boy) South Side 51-49 St.
NUMBER OF SPACES	8	13	20	14	13	11	23
THURSDAY							
9:30	0	8	15	43	69	0	0
10:45	0	8	20	36	38	0	22
1:30	0	31	30	21	38	36	13
2:30	0	15	30	21	62	27	22
4:00	0	23	30	21	92	. 55	30
5:30	0	8	10	21	69	18	35
6:30	0	38	0	7	15	0	9
7:30	0	38	5	7	23	0	17
FRIDAY							
8:30	0	15	20	21	38	27	0
9:45	0	23	30	29	77	27	0
11:45	0	0	25	21	54	18	0
SATURDAY							
8:45	0	23	30	21	46	9	0
10:00	0	23	55	21	77	18	9
11:15	0	23	55	21	69	18	13
1:00	0	23	50	14	69	9	0
2:15	0	38	40	21	46	9	0
3:15	0	15	40	14	23	9	0

TABLE #3

#### OCCUPANCY RATE OF PARKING STALLS STREETS NORTH OF 50 AVENUE ON-STREET (FIGURES SHOW PERCENT OCCUPIED)

LOCATION 53 St. W 52 St. W 52 St. E 51 St. E 49 St. W NUMBER OF SPACES THURSDAY 9:30 10:45 1:30 2:30 4:00 5:30 6:30 7:30 FRIDAY 8:30 9:45 11:45 SATURDAY 8:45 10:00 11:15 1:00 Ð 2:15 3:15 

TABLE #4

# OCCUPANCY RATE OF PARKING STALLS STREETS SOUTH OF 50 AVENUE ON-STREET (FIGURES SHOW PERCENT OCCUPIED)

LOCATION	53St. E	52 St. W	52 St. E	51 St. W	51 St. E	50 St. W	50 St. E	49 St. W
NUMBER OF SPACES	6	9	27	12	33	19	21	18
THURSDAY								
9:30	0	100	63	33	48	37	48	0
10:45	17	88	63	58	55	47	71	0
1:30	50	75	70	33	48	21	43	0
2:30	67	63	89	42	52	26	52	0
4:00	33	63	81	50	58	26	38	0
5:30	33	- 50	52	25	27	58	33	0
6:30	33	38	37	25	12	37	24	0
7:30	33	63	63	33	6	11	24	0
FRIDAY								
8:30	17	75	67	42	27	16	48	0
9:45	33	75	81	42	55	37	67	0
11:45	33	75	78	33	36	37	52	0
SATURDAY								
8:45	17	25	4	25	18	21	33	0
10:00	17	38	15	50	30	21	24	0
11:15	33	50	30	25	42	26	43	0
1:00	33	38	41	58	45	42	19	0
2:15	17	38	48	92	70	26	29	0
3:15	0	25	48	67	58	32	29	0

TABLE #5

## OCCUPANCY RATE OF LANEWAY PARKING EACH BLOCK IDENTIFIED BY A PROMINENT BUILDING (FIGURES SHOW PERCENT OCCUPIED)

BLOCK	Sears	I.G.A.	Select Furniture	Taco Villa	Tarawyn Centre	Fields	Grand Hotel
NUMBER OF SPACES	56	15	15	16	54	34	. 34
<u>JPACEJ</u>							
THURSDAY							
9:30	25	67	40	63	39	57	47
10:45	39	80	60	69	43	60	38
1:30	55	93	53	56	54	60	29
2:30	50	73	60	69	63	57	32
4:00	55	60	40	63	54	49	44
5:30	27	. 47	27	25	41	34	59
6:30	27	40	13	6	33	20	47
7:30	25	40	13	13	35	23	47
FRIDAY							
8:30	41	33	. 20	50	33	54	29
9:45	46	47	33	44	52	57	32
11:15	48	53	33	56	54	63	38
SATURDAY							
8:45	25	20	20	25	33	29	35
10:00	21	20	33	31	52	34	35
11:15	30	33	33	6	41	37	32
1:00	20	40	27	19	46	26	41
2:15	18	27	27	19	46	37	32
3:15	14	20	<u>_6</u>	<u>38</u>	<u>35</u>	<u>37</u>	<u>41</u>
AVERAGE	33	47	32	38	44	43	39
OCCUPANCY	Ý						

TABLE #6

#### OCCUPANCY RATE OF MAJOR PARKING LOTS (FIGURES SHOW PERCENT OCCUPIED)

LOCATION	ALCB	ВААСО	Tomboy	IGA	IDA	Heritage Mall
NUMBER OF						
SPACES	39	36	32	32	21	48
THURSDAY						
9:30	18	22	22	78	48	73
10:45	28	22	44	100	52	65
1:30	18	53	28	89	71	85
2:30	8	33	47	81	76	60
4:00	23	50	25	85	67	67
5:30	38	50	81	89	67	67
6:30	10	86	41	78	29	52
7:30	18	92	50	96	48	67
FRIDAY						
8:30	0	19	6	25	47	42
9:45	8	17	28	59	79	60
11:15	22	31	31	94	68	60
SATURDAY						
8:45	5	22	6	44	33	35
10:00	10	39	38	81	43	46
11:15	44	53	56	81	43	69
1:00	36	64	69	97	57	63
2:15	41	64	78	94	67	79
3:14	26	39	75	91	57	60

TABLE #7

#### OCCUPANCY RATE OF MINOR PARKING LOTS (FIGURES SHOW PERCENT OCCUPIED)

LOCATION	Red Rooster	T.D. Bank	Insurance	Treasury Branch	Scotia Bank	Commerce Bank	Century 21
NUMBER OF	10	10					
SPACES	13	10	15	13	10	10	13
THURSDAY							
9:30	23	30	40	69	100	50	23
10:45	38	20	53	62	100	70	23
1:30	23	10	60	62	90	50	31
2:30	46	70	40	69	100	50	31
4:00	15	80	33	38	90	40	31
5:30	23 .	30	0	8	80	20	31
6:30	23	0	0	0	0	0	15
7:30	31	0	0	0	0	0	0
FRIDAY							
8:30	23	20	47	54	60	40	8
9:45	15	80	60	54	80	90	15
11:15	15	70	53	46	80	60	8
SATURDAY							
8:45	36	0	0	0	0	0	31
10:00	50	0	0	0	0	20	46
11:15	43	0	0	0	0	30	54
1:00	7	10	0	0	0	30	38
2:15	21	30	0	0	0	20	38
3:15	7	40	0	0	0	30	0

TABLE #8

#### OCCUPANCY RATE OF VACANT LOTS CURRENTLY USED FOR PARKING (FIGURES SHOW PERCENT OCCUPIED)

LOCATION	Next to Bakery (51 Ave)	Kowalski Lot	Behind I.D.A.	Behind Credit Union	North of Parkland Chrysler
NUMBER OF SPACES	18	65	12	8	
<u> </u>		03	12	8	107
THURSDAY					
9:30	11	3	75	50	4
10:45	22	6	75	63	6
1:30	22	15	83	63	5
2:30	28	6	92	75	3
4:00	22	18	92	38	5
5:30	17	14	30	25	7
6:30	0	9	8	38	7
7:30	0	3	50	38	4
FRIDAY					
8:30	6	8	75	25	3
9:45	22	14	83	13	3
11:15	22	14	92	13	5
SATURDAY					
8:45	0	3	42	13	0
10:00	6	3	. 75	13	2
11:15	11	6	75	13	4
1:00	6	5	100	0	5
2:15	6	14	92	0	3
3:15	6	11	67	13	3

TABLE #9

#### OCCUPANCY RATE OF PERIPHERAL STREETS AND AVENUES (FIGURES SHOW PERCENT OCCUPIED)

LOCATION NUMBER OF	52 Avenue 55 St - 51 Ave	55 Street 52-50 Ave	50 Avenue 55-53 St	Highway 28 (Beejay Motel)	Highway 28 51-50 St.	48 Avenue 50-49 St
SPACES	5	4	8	10	10	6
THURSDAY 9:30	20	50	30			
10:45	20	50	13	20	40	0
1:30			25	40	50	0
	0	0	38	60	60	0
2:30	0	0	68	50	80	0 .
4:00	0	0	25	20	40	0
5:30	0	50	13	60	20	0
6:30	0	75	13	60	20	0
7:30	0	0	13	60	10	0
FRIDAY						
8:30	0	0	38	30	60	0
9:45	20	50	25	20	80	0
11:15	0	50	25	30	90	0
SATURDAY						
8:45	0	0	13	40	10	0
10:00	0	0	0	10	0	0
11:15	0	25	0	20	30	0
1:00	0	0	50	40	20	0
2:15	0	0	25	20	10	0
3:15	0	0	38	10	0	o

TABLE #10

#### OCCUPANCY RATE OF SELECTED PRIVATE PARKING LOTS (FIGURES SHOW PERCENT OCCUPIED)

LOCATION NUMBER OF	Esso Parking Lot	Behind Northern Cable	Behind Sav-Mor	Grand Cleaners
SPACES	14	3	12	4
THURSDAY				
9:30	64	33	58	100
10:45	71	33	58	50
1:30	64	33	67	100
2:30	64	33	17	25
4:00	57	33	17	25
5:30	50	67	17	25
6:30	50	67	17	0
7:30	0	67	17	. 25
FRIDAY				
8:30	71	67	50	0
9:45	71	100	42	100
11:15	64	100	50	25
SATURDAY				
8:45	71	33	25	25
10:00	64	67	17	75
11:15	64	67	25	50
1:00	71	0	42	50
2:15	64	0	25	0
3:15	86	0	17	50

TABLE 11

# OCCUPANCY RATE OF PARKING STALLS 50 AVENUE ON-STREET INCLUDING ADJACENT SIDE STREETS (FIGURES SHOW PERCENT OCCUPIED)

LOCATION	China Doll	Robinsons	Select	Fields	Tarawyn	₩∞dland
	Block	Block	Furniture Block	Block	Center Block	Ford Block
NUMBER OF SPACES	35	32	45	81	68	25
THURSDAY						
9:30	23	9	29	48	66	60
10:45	46	44	31	54	59	60
1:30	34	41	38	54	71	64
2:30	46	28	24	49	65	68
4:00	37	53	40	54	82	72
5:30	46	56	22	41	50	60
6:30	43	44	22	37	50	48
7:30	31	53	33	23	66	64
FRIDAY						
8:30	34	31	36	19	44	48
9:45	46	53	51	53	72	60
11:45	54	63	51	41	65	76
SATURDAY						
8:45	31	34	27	22	24	40
10:00	40	53	36	35	53	48
11:15	63	66	36	48	59	64
1:00	57	63	47	58	69	56
2:15	46	66	44	64	71	44
3:15	26	56	42	56	63	36

TABLE #12

## LONG TERM PARKERS AS PERCENTAGE OF TOTAL OBSERVATIONS (Parked at least 4 hours at one location)

LOCATION	N 50 Ave. 53-52 St	N 50 Ave 52-51 St	N 50 Ave 51-50 St	I.G.A. Lot	S 50 Ave 50-51 St	S 50 Ave 51-52 St	S 50 Ave 52-53 St
THURSDAY							
TOTAL	57	52	61	151	129	151	53
LTP	0	2	0	5	1	2	1
<b>%</b>	0% .	4%	0%	3%	0.8%	1.3%	2%
FRIDAY						•	
TOTAL.	16	11	21	45	33	45	20
LTP	0	0	0	2	0	1	1
*	0%	0%	0%	4%	0%	2%	5%
SATURDAY							
TOTAL	44	46	51	132	96	144	34
LTP	0	1	1	5	3	0	2
<b>%</b>	0%	2%	2%	4%	3%	0%	6%

TABLE #13

## LONG TERM PARKERS AS PERCENT OF TOTAL SPACES (parked at least 4 hours at one location)

LOCATION	N 50 Ave 53-52 St	N. 50 Ave 52-51 St	N 50 Ave 51-50 St	I.G.A. Lot	S 50 Ave 50-51 St	S. 50 Ave 51-52 St	S 50 Ave 52-53 St
THURSDAY							
SPACES	14	12	16	32	29	29	11
LTP	0	2	0	5	1	2	1
*	0%	0%	0%	3%	9%	7%	9%
FRIDAY							
SPACES	14	12	16	32	29	29	11
LTP	0	0	0	2	0	1	1
<b>*</b>	0%	0%	0%	6%	0%	3%	9%
SATURDAY							
SPACES	14	12	16	32	29	29	11
LTP	0	1	1	5	3	0	2
ŧ	0%	8%	6%	16%	10%	0%	18%

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TABLE #14

PROFILE OF OUT-OF-PROVINCE PARKERS

DAY	TOTAL #		OUT OF I	OUT OF PROVINCE						
	OF PARKERS	SASK	B.C.	OTHER	TOTAL					
THURSDAY	2287	88	8	7	103					
		3.8%	0.8%	0.3%	4.5%					
FRIDAY	845	23	1	3	27					
		2.7%	0.1%	0.4%	3.2%					
SATURDAY	1608	75	7	15	97					
		4.7%	0.4%	0.9%	6.0%					
		V	<u> </u>		and the same of th					
ALL DAYS	4740	186	16	25	227					
		3.9%	0.3%	0.5%	4.8%					

#### **TECHNICAL MEMORANDUM**



### **Appendix B - Verification Study Data**



#### **COLD LAKE SOUTH CONFIRMATION STUDY - PARKING DEMAND**

Password: Locked

ROAD	FROM	то	SIDE OF STREET	PARKING SUPPLY	PARKED VEHICLES		PARKING UTILIZATION	
					1 <sup>st</sup> Pass	2 <sup>nd</sup> Pass	1 <sup>st</sup> Pass	2 <sup>nd</sup> Pass
			North	13	3	2	23%	15%
51 Avenue 51 Street	52 Street	South	11	0	1	0%	9%	
			Total	24	3	3	12%	12%
		52 Street	North	13	8	8	62%	62%
50 Avenue	51 Street		South	22	16	20	73%	91%
			Total	35	24	28	67%	76%
			East	11	6	7	55%	64%
52 Street	51 Avenue	50 Avenue	West	13	7	6	54%	46%
			Total	24	13	13	54%	55%
			East	20	16	12	80%	60%
52 Street	50 Avenue	Highway 28/55	West	10	4	3	40%	30%
			Total	30	20	15	60%	45%

#### COLD LAKE SOUTH CONFIRMATION STUDY - 51 AVENUE PARKING SURVEY DATA SHEET -

**DATE:** October 20, 2010

WEATHER: Sunny SURVEYOR: M.R.

Password: Locked

ROAD	FROM	то	SIDE OF STREET	PARKING SUPPLY	PARKING	PARKED VEHICLES		COMMENTS
KOAD	FROIVI	10	SIDE OF STREET	PARKING SUPPLI	RESTRICTIONS	2:45 PM	3:45 PM	(ie: parking restrictions)
				2 Hour Parking	CWA706	JET679	Devellal Devision	
			North	13	9:00 AM - 6:00 PM	ZAV885	ZAV885	Parallel Parking Lots of empty spaces
51 Avenue	51 Street	51 Street 52 Street			Monday - Saturday	EPL189		Lots of empty spaces
31 Avenue	51 Avenue 51 Street	32 Sireet		11	2 Hour Parking		YRP689	Parallel Parking
			South		9:00 AM - 6:00 PM			All empty at 2:45 pm
					Monday - Saturday			All ellipty at 2:45 pill

#### COLD LAKE SOUTH CONFIRMATION STUDY - 50 AVENUE PARKING SURVEY DATA SHEET -

 DATE:
 October 20, 2010

 WEATHER:
 Sunny

 SURVEYOR:
 M.R.

Password: Locked

ROAD	FROM	то	SIDE OF STREET	PARKING SUPPLY	PARKING	PARKED '	VEHICLES	COMMENTS
ROAD	FRUIVI	10	SIDE OF STREET	PARKING SUPPLY	RESTRICTIONS	2:30 PM	3:30 PM	(ie: parking restrictions)
					Handicap			
					Handicap			
							MEU927	
						KEF878	KEF878	
						CJE983	CJE983	
						ZVP394		
			North	13	2 Hour Parking	921HBW	EYZ564	Parallel Parking
				9:00 AM - 6:00 PM	WGY000		ļ	
				Monday - Saturday	HJE726	HJE726	<u> </u>	
							0)0/4=0	ļ.
							GXY158	<del> </del>
						FFW000	GXU047	
						EFW200	ELW771	
						ZVP413		ł
						207413		
		52 Street				H74502	H74502	
						BBJ7325	BBJ7325	
50 Avenue	51 Street					TUW398	TUW398	
						EWU071	EWU071	
						VKA85	VKA85	
						WZC821	STY352	
						DZU756	EZE583	
					2 Hour Parking	NDH808	YGN384	
			South	22	9:00 AM - 6:00 PM	567TZR	ZVP567	Angle parking
					Monday - Saturday	DUU176	DUU176	
						LEN540	ZLM368	
						EZV334	HFY158	
						RAC959	964HID	
							60356	
							ZZT757	1
						71/0=16	YYX194	ļ
						ZVP542	MCU530	
						WYS766	RWU353	
						HUB692		
							303HPG	

 COLD LAKE SOUTH CONFIRMATION STUDY - 52 STREET PARKING SURVEY DATA SHEET - DATE:

 DATE:
 October 20, 2010

 WEATHER:
 Sunny

 SURVEYOR:
 B.T.

 Password:
 Locked

ROAD	FROM	то	SIDE OF STREET	PARKING SUPPLY	PARKING	PARKED	VEHICLES	COMMENTS
KOAD	FROW	10	SIDE OF STREET	FARRING SUPPLY	RESTRICTIONS	2:30 PM	3:30 PM	(ie: parking restrictions)
						CAM670	CAM670	
						LLY026		
		nue 50 Avenue			2 Hour Parking	LYC661	LYC661	
	51 Avenue		West	13	9:00 AM - 6:00 PM			Parallel Parking
					Monday - Saturday	ZRM327	ZSX680	
			PXA001					
						STY555	TBU932	
						MUB794	MUB794	
							ZWZ021	
						500075	500075	
						ESS675	ESS675	
						MAN025	MAN025	
					2 Hour Parking		Parked trailor	
	50 Avenue	Highway 28/55	West	10	9:00 AM - 6:00 PM	YYB502		Parallel Parking
					Monday - Saturday			-
						SYB084	SYB084	
						KYU100	KYU100	Angle Parking
						SYS324	KYU100	
						JET502	JET502	
52 Street					2 Hour Parking 9:00 AM - 6:00 PM	JL 1302	KYW917	
				20		ZAS773	ZAS773	
						FZE582	FZE582	
						JET920	JET920	
						021020	FUS224	
						ZPX006	1 00224	
	50 Avenue	Highway 28/55	East			ZHS906		
					Monday - Saturday	VRJ223	VRJ223	
						BBVX130		
						FAR203	YMU000	
						EHB376		
						RWV443	RWV443	
						LYL962	SYX709	
						YNB502		
						LEN300		
							MCV770	
						CBY080	CBY080	
					2 Hour Parking			
	51 Avenue	50 Avenue	East	11	9:00 AM - 6:00 PM		ZZT722	Parallel Parking
					Monday - Saturday	FUS373	FUS373	
1						ZVP530	ZVP530	
			RAX383	RAX383				
						]		
						77J787	ZSX568	

#### **FINAL REPORT**



# **Appendix F - Cold Lake Transportation Study In-Service Road Safety Reviews**

# **Technical Memorandum**

# **City of Cold Lake**

**Cold Lake Transportation Study In-Service Road Safety Reviews** 

#### **April 2011**



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### **TECHNICAL MEMORANDUM**

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#### City of Cold Lake

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#### Introduction

Associated Engineering (AE) was retained by the City of Cold Lake (City) to update its existing transportation study. The purpose of the transportation study is to provide a comprehensive long range plan that integrates transportation infrastructure requirements with existing and future land uses. The transportation study will provide the City with a master plan on which to plan and implement specific transportation network improvement projects over the next 20 years in 5-year, 10-year, 15-year and 20-year planning horizons. The transportation study will consider municipal roads, traffic calming, parking, traffic safety, traffic signal coordination, school zones, transit, truck routes, traffic management, and transportation system operations.

#### 1.1 STUDY BACKGROUND

One component of the transportation study was to complete in-service road safety reviews at select locations within the City. At project initiation, it was established that the following four study corridors would be reviewed:

- 1 Avenue, from the Municipal District (MD) Campground at 23 Street to 2 Avenue/10 Street
- 10 Street, from 2 Avenue to 8 Avenue
- Lakeshore Drive, from 1 Avenue/10 Street to 8 Avenue
- 50 Avenue, from Highway 28 to 49 Street.

This technical memorandum presents the analyses and results of the in-service road safety reviews completed for the four study corridors identified above. This technical memorandum also covers traffic calming, which was considered for each study corridor as part of the potential improvement options.

#### 1.2 STUDY OBJECTIVE

The purpose of the in-service road safety reviews was to identify potential safety issues along the corridors and propose improvement options that will reduce or eliminate identified issues. The in-service road safety reviews were conducted based on the procedures outlined in the Transportation Association of Canada (TAC) Canadian Guide to In-Service Road Safety Reviews (TAC safety guideline).

#### 1.3 STUDY AREA

Figure 1.1 through Figure 1.4 present the four study corridors being studied for the in-service road safety reviews.



#### 1.4 STUDY METHODOLOGY

The scope of an in-service road safety review can range from a high level review based on site observations to an in-depth review involving collision analysis, geometric analysis, traffic operational analysis, traffic conflict analysis, and human factors analysis. For the Cold Lake in-service road safety reviews, a high level approach was chosen and the following methodology was followed:

- Initiate project meeting
- Conduct site reconnaissance of existing conditions
- Review operational conditions at key intersections
- Identify safety issues
- Present improvement options
- Produce draft and final reports.

Various intersections along the four study corridors were analyzed as part of the existing (2010) traffic operational analysis completed for the transportation study update. The results of the operational analysis were reviewed as part of the in-service road safety reviews to identify any operational issues. Collision data for the four study corridors were unavailable at the time of this report.



DWG NO:	SCALE:	APPROVED:	DATE:	PROJECT NO: 2010-3050
	NTS		JANUARY 2011	2010-3050

NELSON ST / 22 ST SPRUCE ST TE NARAMAT

# CITY OF COLD LAKE TRANSPORTATION STUDY

FIGURE1.1 COLD LAKE NORTH 1 AVENUE CORRIDOR - STUDY AREA





PROJECT NO: 2010-3050
DATE: 2010-3050
JANUARY 2011

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# CITY OF COLD LAKE TRANSPORTATION STUDY

FIGURE 1.2 COLD LAKE NORTH 10 STREET CORRIDOR - STUDY AREA





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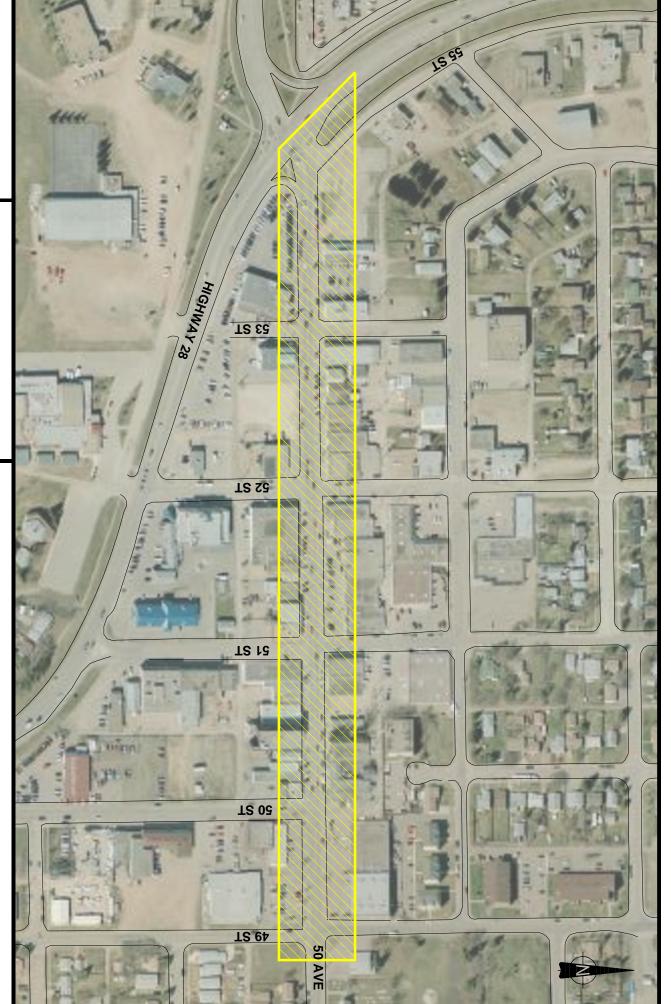
NTS

# CITY OF COLD LAKE TRANSPORTATION STUDY

FIGURE 1.3 COLD LAKE NORTH LAKESHORE DRIVE CORRIDOR - STUDY AREA



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# TRANSPORTATION STUDY

FIGURE 1.4 COLD LAKE SOUTH 50 AVENUE CORRIDOR - STUDY AREA 2

# **Project Initiation Meeting**

A project initiation meeting was held on May 4, 2010 in Cold Lake to complete the following:

- Confirm the scope of the in-service road safety review for each corridor
- Obtain the City's input regarding potential issues along each corridor.



3

# **Site Reconnaissance of Existing Conditions**

A daytime site reconnaissance was completed by the project team on May 5, 2010 to collect information about the existing conditions along each corridor and to identify potential safety issues. The prevailing weather condition was cloudy with light flurries.

The following sections describe the existing conditions information collected during the site visit.

#### 3.1 1 AVENUE CORRIDOR

#### 3.1.1 Physical Characteristics

1 Avenue is an east-west, two-lane, undivided collector which services several tourist attractions in Cold Lake North, including the MD campground and Kinosoo Beach. The section of interest presently is a 520 m stretch located between 23 Street and 2 Avenue/ 10 Street. The study section is relatively straight with no vertical curves.

There are eight intersections along 1 Avenue within the study limits. All the intersections are stop controlled on the northbound and southbound approaches, except for the intersection at 2 Avenue/10 Street. The southbound approach at 2 Avenue/10 Street is yield controlled. The eight intersecting roadways along 1 Avenue are two-lane undivided roadways and are listed below from west to east:

- 23 Street
- Nelson Street/22 Street
- Spruce Street
- Tamarak Street
- 19 Street
- Nelson Street/18 Street
- 16 Street
- 2 Avenue/10 Street.

The predominant land uses along 1 Avenue are residential and recreational, and are summarized below:

- Single family residential developments along the south side of 1 Avenue between 23 Street and 10 Street and along the north side of 1 Avenue between 23 Street and Tamarak Street
- Multi-family residential development in the northwest corner of 1 Avenue and 2 Avenue/10
   Street
- City's water treatment facility located immediately west of the multi-family development
- MD campground located along the north side of 1 Avenue west of 23 Street



 Kinosoo Beach located along the north side of 1 Avenue between Tamarak Street and the City's water treatment facility.

The traverse cross-section along 1 Avenue ranges from 13.1 m wide on the west end of the corridor to 10.6 m wide on the east end. Parking lanes and curb and gutter are provided on both sides of the corridor. Sidewalks are only provided on the north side of 1 Avenue, from 23 Street to just west of 10 Street. There are multiple residential driveways along the south side of 1 Avenue and several driveways along the north side for Kinosoo Beach and the water treatment plant.

The posted speed limit along 1 Avenue is 50 km/h, except for the stretch along Kinosoo Beach between Tamarak Street and the water treatment facility. The speed limit along this portion is 30 km/h. All of the cross streets have a posted speed limit of 50 km/h.

The existing pavement condition is poor; 1 Avenue contains many cracks and potholes. A pedestrian activated crosswalk with overhead signage and warning flashers is provided along 1 Avenue, at 19 Street.

#### 3.1.2 Traffic Characteristics

Traffic volume data were provided for various intersections along 1 Avenue to analyze the existing traffic conditions. The traffic data was analyzed to determine the existing (2010) traffic volumes as part of the existing (2010) traffic operational analysis completed for the transportation study update. The existing (2010) p.m. peak hour traffic volumes along 1 Avenue are presented in Figure 3.1.

#### 3.2 10 STREET CORRIDOR

#### 3.2.1 Physical Characteristics

10 Street is a north-south, two-lane, undivided collector which serves as a link between Highway 28 and Kinosoo Beach. The section of interest presently is an 850 m stretch located between 1 Avenue and 8 Avenue. The study section is relatively straight with no vertical curves, except at 3 Avenue where there is a vertical crest curve.



DATE:
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FIGURE 3.1 COLD LAKE NORTH 1 AVENUE CORRIDOR - 2010 TRAFFIC VOLUMES

JANUARY 2011



There are six intersections along 10 Street within the study limits. All the intersections are stop controlled on the eastbound and westbound approaches. The intersecting roadways along 10 Street are two-lane undivided roadways and are listed below from north to south:

- 1 Avenue/2 Avenue
- 3 Avenue
- 5 Avenue
- 6 Avenue
- 7 Avenue
- 8 Avenue.

The predominant land uses along 10 Street are residential and institutional, and are summarized below:

- Single family residential developments along both sides of 10 Street
- Multi-family residential developments in the northwest corner of the 10 Street and 1
   Avenue/2 Avenue intersection and the northwest corner of the 10 Street and 8 Avenue intersection
- Commercial and institutional (fire hall) land uses located in the northeast corner of the
   Street and 8 Avenue intersection
- Cenotaph Park located in the southeast corner of the 10 Street and 7 Avenue intersection
- Institutional land uses (churches) located in the southeast corner of the 10 Street and
   3 Avenue intersection and the southeast corner of the 10 Street and 7 Avenue intersection.

The traverse cross-section along 10 Street ranges from 13.0 m wide on the north end of the corridor to 15.2 m wide on the south end. Parking lanes and curb and gutter are provided on both sides of the corridor. Sidewalks are provided on the east side of 10 Street along the entire corridor. Multiple driveways for the residential, commercial and institutional land uses are located on both sides of 10 Street.

No speed limit signs were posted along 10 Street. For the purpose of this study, it was assumed that the posted speed limit along 10 Street is 50 km/h.

The existing pavement condition is poor with numerous cracks and potholes.

#### 3.2.2 Traffic Characteristics

The existing (2010) p.m. peak hour traffic volumes along 10 Street from the existing (2010) traffic operational analysis are presented in Figure 3.2.



#### 3.3 LAKESHORE DRIVE CORRIDOR

#### 3.3.1 Physical Characteristics

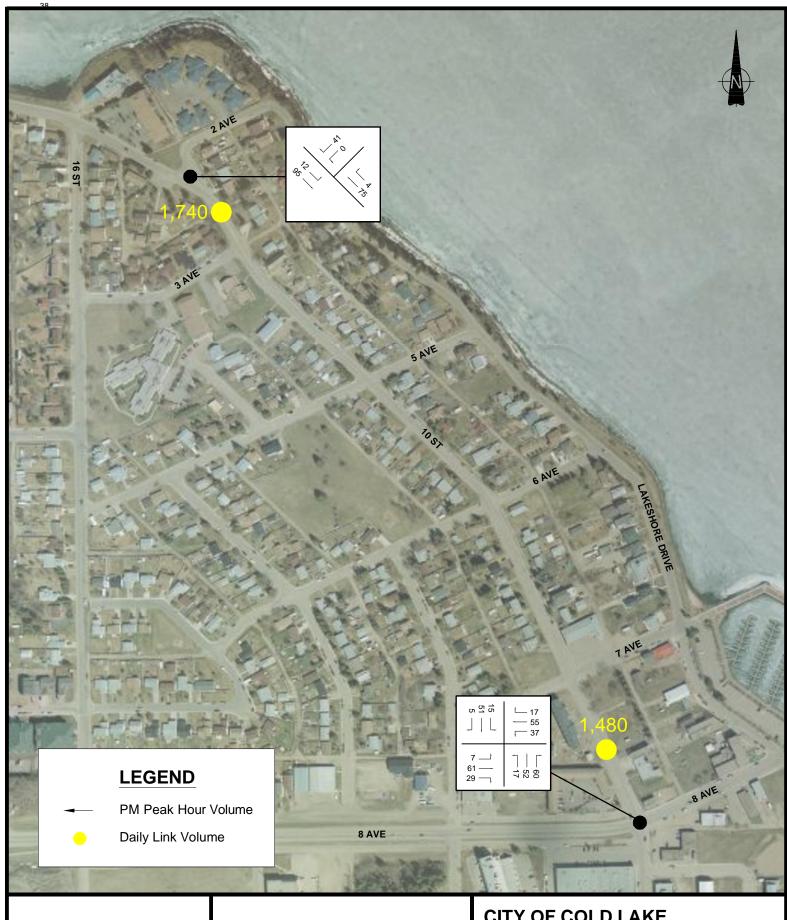
Lakeshore Drive is a north-south, two-lane, undivided local roadway which provides access to the Cold Lake Marina and the tourist district situated south of Cold Lake. The section of interest presently is a 1.0 km stretch located between 1 Avenue and 8 Avenue. On the north end, Lakeshore Drive transitions into 2 Avenue through a horizontal curve and intersects with 1 Avenue/10 Street. Lakeshore Drive follows the alignment of the lake; and contains many horizontal curves and slopes upwards on the north end.

There are five intersections along Lakeshore Drive within the study limits. All the intersections along Lakeshore Drive are stop controlled on the eastbound approaches, except for the intersection at 1 Avenue/10 Street. The southbound approach at 1 Avenue/ 10 Street is yield controlled. The intersecting roadways along Lakeshore Drive are two-lane undivided roadways and are listed below from north to south:

- 1 Avenue/10 Street
- 5 Avenue
- 6 Avenue
- 7 Avenue
- 8 Avenue.

The predominant land uses along Lakeshore Drive are residential and commercial, and are summarized below:

- Recreational land use in the form of Cold Lake and Cold Lake Marina along the east side of Lakeshore Drive;
- Single family residential developments along the west side of Lakeshore Drive, from 2
   Avenue to 6 Avenue; and
- Commercial land uses in the form of bed and breakfasts and restaurants along the west side of Lakeshore Drive, from 7 Avenue to 8 Avenue.





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# CITY OF COLD LAKE TRANSPORTATION STUDY

FIGURE 3.2 COLD LAKE NORTH 10 STREET CORRIDOR - 2010 TRAFFIC VOLUMES The traverse cross-section along Lakeshore Drive ranges from 7.0 m wide on the north end of the corridor to 13.4 m wide on the south end; the road widens at 6 Avenue. A parking lane is provided on the west side of Lakeshore Drive between 7 Avenue and 2 Avenue and along the south side of 2 Avenue between 1 Avenue and Lakeshore Drive. South of 7 Avenue, parking lanes are provided on both sides of Lakeshore Drive along with curb extensions and marked crosswalks at 7 Avenue, 8 Avenue, and midblock between 7 Avenue and 8 Avenue. Curb and gutter is provided along both sides of the roadway for the entire study corridor.

From north to south, sidewalks and crosswalks are provided along the study corridor in the following manner:

- Sidewalk provided on south side of 2 Avenue, from 1 Avenue to Lakeshore Drive
- Marked crosswalk provided at the horizontal curve where 2 Avenue transitions to Lakeshore Drive
- Sidewalk provided on east side of Lakeshore Drive, from 2 Avenue to 6 Avenue
- Sidewalk provided along both sides of Lakeshore Drive, from 6 Avenue to 7 Avenue
- Marked crosswalks provided at 7 Avenue, 8 Avenue, and midblock with curb extensions
- Sidewalk provided along west side of Lakeshore Drive, from 7 Avenue to 8 Avenue.

Multiple driveways for the residential and commercial land uses are located on the west side of Lakeshore Drive.

The posted speed limit along Lakeshore Drive is 30 km/h from 1 Avenue/10 Street to 7 Avenue. South of 7 Avenue, the posted speed limit is 50 km/h. All the cross-streets have a posted speed limit of 50 km/h.

The existing pavement condition is poor and Lakeshore Drive contains many cracks and potholes.

#### 3.3.2 Traffic Characteristics

The existing (2010) p.m. peak hour traffic volumes along Lakeshore Drive from the existing (2010) traffic operational analysis are presented in Figure 3.3.



#### 3.4 50 AVENUE CORRIDOR

#### 3.4.1 Physical Characteristics

50 Avenue is an east-west, two-lane, undivided arterial which services the Central Business District of Cold Lake South (CLS). The section of interest presently is a 650 m stretch located between Highway 28 and 49 Street. The study section of 50 Avenue does not have any horizontal or vertical curves.

There are seven intersections along 50 Avenue within the study limits. The study intersections, from west to east, along with a description of the traffic control and intersection configuration are listed below:

#### **Highway 28**

- Four-legged intersection with traffic signals
- Northbound approach: two through lanes with separate left turn and channelized right turn lanes
- Southbound approach: two through lanes with separate left turn and channelized right turn lanes
- Eastbound approach: one through lane with separate left turn and channelized right turn lanes
- Westbound approach: one shared left turn and through lane and channelized right turn lane.

#### 55 Street

- Three-legged intersection with stop control on the southbound approach
- Single shared left turn, through and right turn lane on all approaches.

#### 53 Street

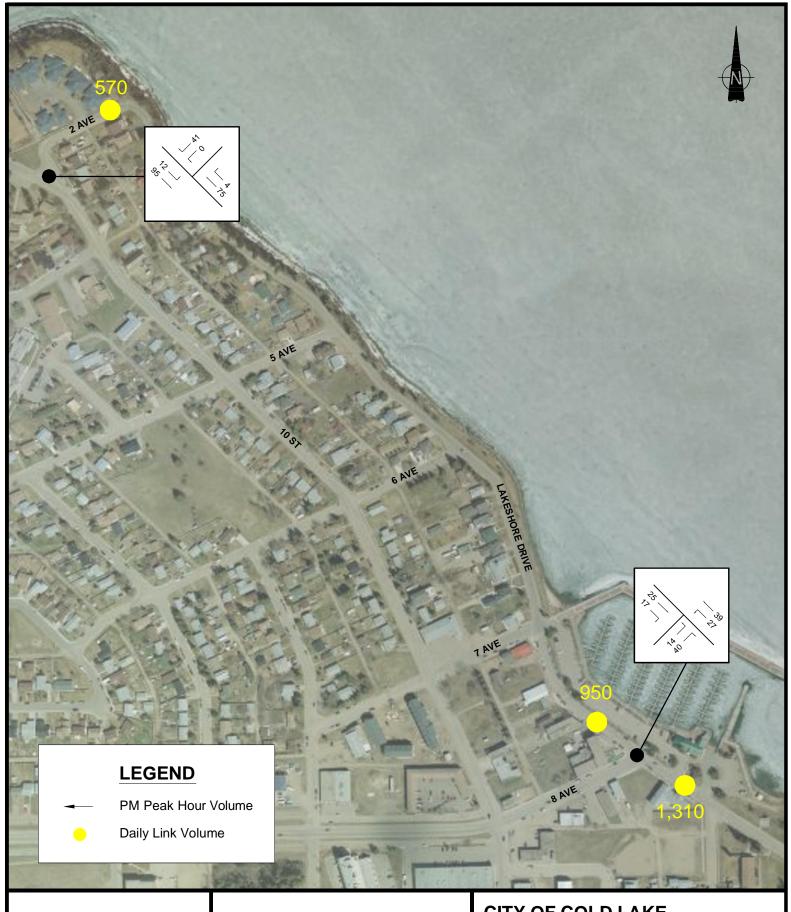
- Four-legged intersection with 2-way stop control on the northbound and southbound approaches
- Single shared left turn, through and right turn lane on all approaches

#### 52 Street

- Four-legged intersection with 4-way stop control
- Single shared left turn, through and right turn lane on all approaches.

#### 51 Street

- Four-legged intersection with 4-way stop control
- Single shared left turn, through and right turn lane on all approaches.





PROJECT NO: 2010-3050 DATE: APPROVED:

JANUARY 2011

SCALE: NTS DWG NO:

### **CITY OF COLD LAKE** TRANSPORTATION STUDY

FIGURE 3.3 COLD LAKE NORTH LAKESHORE DRIVE CORRIDOR - 2010 TRAFFIC **VOLUMES** 

#### 50 Street

- Four-legged intersection with 3-way stop control
- Single shared left turn, through and right turn lane on all approaches
- Southbound approach is the entrance to a private driveway and does not have any traffic control.

#### 49 Street

- Four-legged intersection with 2-way stop control on the northbound and southbound approaches
- Single shared left turn, through and right turn lane on all approaches.

The predominant land use along 50 Avenue is commercial.

The traverse cross-section along 50 Avenue ranges from 17.0 m to 19.0 m. Parking lanes and curb and gutter are provided on both sides of the corridor. Parking along 50 Avenue is restricted to two-hours from Monday to Saturday (between 9:00 a.m. and 6:00 p.m.) and is provided in the following manner from west to east:

- From 55 Street to 53 Street: angle and parallel parking on the north side, parallel parking on the south sides
- From 53 Street to 52 Street: angle parking on the north side, parallel parking on the south side.
- From 52 Street to 49 Street: parallel parking on the north side, angle parking on the south side.

Sidewalks are provided along both sides of 50 Avenue and multiple driveways are located along the corridor. Marked crosswalks have been provided at all the intersections except for 55 Street and a midblock crosswalk has been provided between 50 Street and 51 Street. Pedestrian crossing (RA-4) signs have been provided for the midblock crosswalk.

The posted speed limit along 50 Avenue is 30km/h between Highway 28 and 55 Street. East of 55 Street, the posted speed limit is 50 km/h. All the cross-streets have a posted speed limit of 50 km/h except for Highway 28 which has a posted speed limit of 30 km/h at 50 Avenue.

The existing pavement condition is fair. A single yellow centerline is provided along 50 Avenue to indicate the separation between the eastbound and westbound travel lanes. Pavement markings have been provided on 50 Avenue to indicate both the parallel and angle parking stalls. The centerline, painted stall lines, and crosswalk markings are worn and need to be repainted.



#### 3.4.2 Traffic Characteristics

The existing (2010) p.m. peak hour traffic volumes along 50 Avenue from the Existing (2010) Traffic Operational Analysis are presented in Figure 3.4.

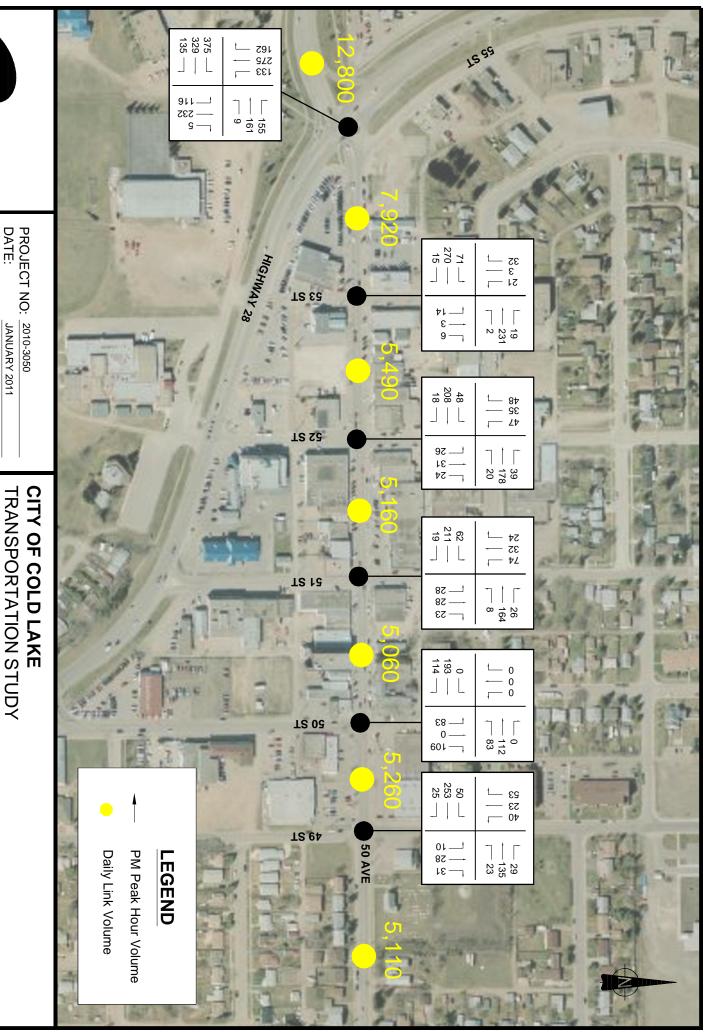


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FIGURE 3.4

COLD LAKE SOUTH
50 AVENUE CORRIDOR - 2010 TRAFFIC VOLUMES

JANUARY 2011



4

## **Operational Analysis**

Operational analyses were completed for the intersections along the study corridors as part of the existing (2010) traffic operational analysis for the transportation study update. The following section provides the overall intersection results from the traffic analysis. Information regarding the methodology and assumptions used for the traffic analysis can be found in the technical memorandum titled Existing (2010) Traffic Operational Analysis and the detailed intersection analyses results have been included Appendix A.

#### 4.1 1 AVENUE CORRIDOR

Only three of the eight intersections along 1 Avenue were analyzed for the existing (2010) horizon analysis. Table 4.1 presents the overall intersection capacity results for the study intersections analyzed along 1 Avenue.

Table 4.1
Overall Intersection Capacity Results - 1 Avenue Corridor

Intersection	Maximum V/C Ratio	Delay (s)	LOS
Nelson Street/22 Street	0.11	1.3	А
16 Street	0.09	1.9	А
2 Avenue/10 Street	0.05	2.1	А

All the study intersections are currently operating well with an overall intersection LOS A and maximum v/c ratio of 0.11 or less. All individual movements are also operating at LOS B or better.

#### 4.2 10 STREET CORRIDOR

Only two of the six intersections along 10 Street were analyzed for the existing (2010) traffic operational analysis. Table 4.2 presents the overall intersection capacity results for the study intersections analyzed along 10 Street.



Table 4.2
Overall Intersection Capacity Results - 10 Street Corridor

Intersection	Maximum V/C Ratio	Delay (sec)	LOS
1 Avenue/2 Avenue	0.05	2.1	А
8 Avenue	0.21	6.6	А

All the study intersections are currently operating well with an overall intersection LOS A and maximum v/c ratio of 0.21 or less. All individual movements are also operating at LOS B or better.

#### 4.3 LAKESHORE DRIVE CORRIDOR

Only two of the five intersections along Lakeshore Drive were analyzed for the existing (2010) traffic operational analysis. Table 4.3 presents the overall intersection capacity results for the study intersections analyzed along Lakeshore Drive.

Table 4.3
Overall Intersection Capacity Results - Lakeshore Drive Corridor

Intersection	Maximum V/C Ratio	Delay (sec)	LOS
1 Avenue/10 Street	0.05	2.1	Α
8 Avenue	0.07	4.3	А

All the study intersections are currently operating well with an overall intersection LOS A and maximum v/c ratio of 0.07 or less. All individual movements are also operating at LOS A.

#### 4.4 50 AVENUE CORRIDOR

Only six of the seven intersections along 50 Avenue were analyzed for the existing (2010) traffic operational analysis. Table 4.4 presents the overall intersection capacity results for the study intersections analyzed along 50 Avenue.

Table 4.4
Overall Intersection Capacity Results – 50 Avenue Corridor

Intersection	Maximum V/C Ratio	Delay (sec)	LOS
Highway 28	0.76	14.3	В
53 Street	0.16	3.1	Α
52 Street	0.45	11.1	В
51 Street	0.47	11.2	В
50 Street	0.46	10.9	В
49 Street	0.32	5.5	А

All the study intersections are currently operating well with an overall intersection LOS B or better and maximum v/c ratio of 0.76 or less. All individual movements are also operating at LOS C or better



5

# **Safety Issues**

Potential safety issues along each study corridor were identified based on observations during site reconnaissance. The results from the operational analysis at the various intersections indicated that no improvements are required from an operational standpoint. The safety issues are discussed in the following section for each study corridor.

#### 5.1 1 AVENUE CORRIDOR

#### 5.1.1 1 Avenue/2 Avenue/10 Street Intersection Configuration

Figure 5.1 and Figure 5.2 present photographs of the intersection configuration at 1 Avenue and 2 Avenue/10 Street.

Figure 5.1

1 Avenue/2 Avenue/10 Street Intersection - Southbound on 2 Avenue



Figure 5.2
1 Avenue/2 Avenue/10 Street Intersection - Northbound on 10 Street



1 Avenue transitions into 10 Street at the intersection and free flow is provided for the eastbound/westbound movements along 1 Avenue and 10 Street. Yield control is provided for the southbound movements along 2 Avenue. Due to the alignment and the geometry of the intersection, the through movements along 1 Avenue and 10 Street have to turn slightly to stay on the travel path while movements between 10 Street and 2 Avenue can travel straight. Typically through movements are provided with straight travel paths through an intersection. The travel paths at 1 Avenue/2 Avenue/10 Street may result in driver confusion, particularly at night and for visitors.

Figure 5.3 presents a photograph of the study intersection from the southbound approach.





Figure 5.3
1 Avenue/2 Avenue/10 Street Intersection - Southbound Approach

Visibility of the study intersection from 2 Avenue is poor because of the horizontal and vertical curve provided on this approach. The yield sign provided for this approach is also partially obscured by the streetlight. As a result, the need to stop at the intersection immediately after the curve may not be expected by drivers.

#### 5.1.2 Speeding Problem

The City indicated that there is a speeding problem in the summer months along 1 Avenue, near Kinosoo Beach. Two site visits were conducted by AE, one in May 2010 and one in July 2010 on the Canada Day long weekend. Both site visits were not representative of a typical summer weekend; May is not a summer month and driver behaviour on the long weekend were affected by the Canada Day parade. Therefore, representative travel speed along 1 Avenue could not be observed.

A speed study should be conducted along 1 Avenue on a typical summer weekend to confirm the speeding problem.

#### **5.1.3** Poor Pavement Conditions

The existing pavement along 1 Avenue is in poor condition and contains many cracks and potholes. According to the TAC safety guideline, poor pavement surfaces will likely reduce the available friction with vehicle tires; causing longer braking distances and a higher risk of rear-end collisions.

#### 5.1.4 19 Street Pedestrian Crosswalk

The only pedestrian crosswalk along 1 Avenue is provided at 19 Street. Figure 5.4 presents a photograph of the pedestrian crosswalk.



Figure 5.4
Pedestrian Crosswalk at 19 Street

The crosswalk is equipped with overhead warning flashers for both direction of travel along 1 Avenue. No pavement markings are provided to delineate the actual crosswalk and no pedestrian crossing (RA-4) signs are provided along 1 Avenue in advance of the actual crosswalk. Crosswalk lines should be provided to clearly mark the crosswalk location for pedestrians and approaching traffic along 1 Avenue. Pedestrian crosswalk (RA-4) signs should be provided at all crosswalks except for at signalized intersections or stop signs, in accordance with Section A6 of the Manual of Uniform Traffic Control Devices (MUTCD).

#### 5.2 10 STREET CORRIDOR

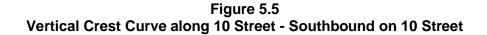
#### 5.2.1 1 Avenue/2 Avenue/ 10 Street Intersection

This is covered in Section 5.1.1.

#### 5.2.2 Vertical Crest Curve at 3 Avenue

There is a vertical crest curve along 10 Street at 3 Avenue. Figure 5.5 presents a photograph of the vertical crest curve.







The vertical curve limits driver sight distance in both directions along 10 Street. Limited sight distances increase the risk of head-on, off-road, rear-end and hidden-intersection collisions. Visibility of the 3 Avenue intersection is poor from 1 Avenue at the 1 Avenue/2 Avenue/10 Street intersection because of the intersection alignment, the vertical curve, and the trees along the west side.

#### 5.2.3 Poor Pavement Conditions

The existing pavement along 10 Street is in poor condition and contains many cracks. According to the TAC safety guideline, poor pavement surfaces will likely reduce the available friction with vehicle tires; causing longer braking distances and a higher risk of rear-end collisions.

#### 5.3 LAKESHORE DRIVE CORRIDOR

#### 5.3.1 1 Avenue/2 Avenue/10 Street Intersection

This is covered in Section 5.1.1.

#### 5.3.2 5.3.2 Road Width and Alignment

Figure 5.6 presents a photograph of Lakeshore Drive north of 6 Avenue.



Figure 5.6 Lakeshore Drive - Northbound.

The pavement width of Lakeshore Drive is narrow between 2 Avenue and 6 Avenue. Two-way traffic with parking on the west side is provided along this section. During the site visit, AE observed that simultaneous northbound and southbound movement along Lakeshore Drive is difficult with parked vehicles on the west side. Vehicles need to slow down and yield to one another.

The alignment of Lakeshore Drive follows the shoreline of the City. As a result, there are numerous horizontal curves along Lakeshore Drive. The horizontal curves limit the sight distance along the corridor. Limited sight distances increase the risk of head-on, off-road, rear-end and hidden-intersection collisions. The sight distance along Lakeshore Drive is further reduced by the natural vegetation along the east side.

#### **5.3.3** Poor Pavement Conditions

The existing pavement along Lakeshore Drive is worn and contains many cracks, ruts, and potholes. According to the TAC safety guideline, poor pavement surfaces will likely reduce the available friction with vehicle tires; causing longer braking distances and a higher risk of rear-end collisions.

## 5.3.4 Pedestrian Crosswalks

There are a total of four crosswalks along Lakeshore Drive, at the following locations:

- 8 Avenue
- Midblock between 8 Avenue and 7 Avenue
- 7 Avenue
- At the horizontal curve along Lakeshore Drive where it transitions to 2 Avenue.

Due to the traffic calming measures, curb extensions have been provided at the crosswalks between 8 Avenue and 7 Avenue. Pavement markings have also been provided at these crosswalks, however the existing pavement markings are faded.

Figure 5.7 presents a photograph of a typical crosswalk provided between 8 Avenue and 7 Avenue.



Figure 5.7 Crosswalk at 7 Avenue

The pedestrian crosswalk at 2 Avenue is located on the horizontal curve and has faded pavement markings. No pedestrian crosswalk (RA-4) signs have been provided along Lakeshore Drive or 2 Avenue to warn motorist in advance of the crosswalk. Pedestrian crosswalk signs should be provided, especially at this location, to compensate for the limited sight distances in both directions.

The locations of the sidewalks along Lakeshore Drive vary along the corridor in the following manner:

- South side of 2 Avenue, from 1 Avenue to Lakeshore Drive
- East side of Lakeshore Drive, from 2 Avenue to 6 Avenue
- Both sides of Lakeshore Drive, from 6 Avenue to 7 Avenue
- West side of Lakeshore Drive, from 7 Avenue to 8 Avenue.

At 6 Avenue, the sidewalk switches from one side of Lakeshore Drive to the other side. Pedestrians travelling along Lakeshore Drive would need to cross the road in order to walk safely on the sidewalk. A pedestrian crosswalk should be provided at 6 Avenue to provide a safe crossing location.

#### 5.4 50 AVENUE CORRIDOR

## 5.4.1 Highway 28 Intersection

Figure 5.8 presents a photograph of the 50 Avenue and Highway 28 intersection.



Figure 5.8 50 Avenue/Highway 28 Intersection - Westbound on 50 Avenue

The intersection is located on horizontal curves along Highway 28 and Centre Avenue/Kingsway. Visibility of the intersection along the northbound, southbound and eastbound approaches is reduced as a result of the horizontal curves; however sufficient stopping sight distance is provided for the posted speed of 30 km/h.



A channelized right turn lane, with a yield control, is currently provided for the southbound to eastbound movement. Due to the geometry of the intersection, the angle between the channelized right-turn lane and 50 Avenue is approximately 20 degrees. This is an extremely acute angle that falls well below the 60 degrees stipulated by Section 2.3.6 of the TAC Geometric Design Guide for Canadian Roads (TAC geometric guidelines). Acute angles are difficult for merging drivers, as they require the driver to look back at very large angles to check for gaps in traffic. Not only is this physically difficult but it also detracts the driver's attention from objects ahead of them for a longer period of time.

As per Section 1.3.4 of the TAC geometric guidelines, a minimum intersection spacing of 200 m is required for an arterial roadway. The intersection spacing between the Highway 28/50 Avenue and 55 Street/50 Avenue intersections is 30 m. There could be potential queue problems that result from eastbound traffic waiting to turn left onto 55 Street that could spillback into the Highway 28/50 Avenue intersection, due to the short separation distance.

The westbound approach has one shared left turn and through lane and a channelized right turn lane. However the westbound approach can accommodate a designated left turn lane, a through lane and a channelized right turn lane. The wide through lane may result in driver confusion regarding the designated lane usage.

#### 5.4.2 Angle Parking

Angle parking along one side with parallel parking on other side is provided along 50 Avenue between 53 Street and 49 Street. Angle parking along 50 Avenue increases the Downtown parking supply and serves as a traffic calming measure as traffic along 50 Avenue is cautious of vehicles backing out of the angle parking stalls and travel at slower speeds.

Despite the above mentioned benefits to angle parking, there are also safety concerns since parked vehicles must back into traffic on 50 Avenue. Driver visibility is significantly reduced when backing out, especially with a large vehicle parked beside it. Also, traffic on 50 Avenue is delayed by vehicles backing out of the angle parking stall. The delay was captured by Figure 5.9.



Figure 5.9
Delay along 50 Avenue due to Angle Parking

## 5.4.3 Faded Pavement Markings

The pavement markings provided along 50 Avenue including the centerline, stop lines, on-street parking stalls and crosswalks are worn. Worn pavement markings are not visible to drivers, especially at night, and will not effectively convey the message intended.

#### 5.4.4 Multiple Driveway Accesses

Multiple driveway accesses are provided along 50 Avenue for private businesses. AE observed that some driveways were closed off using concrete barriers (as an example, the driveway at Tire Country). The City should determine the land ownership of all the driveways along 50 Avenue and identify whether the driveways are currently being used. Unnecessary driveways should be closed to reduce the traffic conflict points along the corridor. The sight distance onto 50 Avenue from the driveways are reduced by vehicles in the parking lane, particularly the angle parking lane.

#### 5.4.5 Pedestrian Crosswalks

With the exception of the midblock crosswalk between 50 Street and 51 Street, pedestrian crossing (RA-4) signs have not been provided in advance of the crosswalks along the corridor. According to Section A6 in the MUTCD, pedestrian crosswalk (RA-4) signs should be provided at all crosswalks except for at signalized intersections or stop signs.

Pedestrians waiting to cross at crosswalks may not be visible to the approaching motorist due to the vehicles parked on-street, especially in the angle parking stalls. Figure 5.10 presents the midblock crossing provided between 50 Street and 51 Street. Pedestrians waiting to cross 50



Avenue from the south side would not be visible to approaching traffic unless they step off the sidewalk and move beyond the parked vehicle in the angle parking stall.

Figure 5.10
Midblock Crossing between 50 Street and 51 Street





## **Improvement Options**

Improvement options have been developed to address each of the potential safety issues along the study corridors. The improvement options are discussed in detail below.

#### 6.1 1 AVENUE CORRIDOR

#### 6.1.1 Improve Intersection Configuration at 1 Avenue/2 Avenue/10 Street Intersection

The geometric layout of the intersection of 1 Avenue and 2 Avenue/10 Street requires improvement to provide clear designated travel path. The option of providing a roundabout at this location was presented by the City at the project initiation meeting. A roundabout with a raised centre island would be beneficial at this intersection for the following two reasons:

- The roundabout would provide clarity regarding the designated travel paths
- The roundabout would act as a traffic calming measure during busy summer months and reduce speeds in the adjacent area.

A preliminary assessment determined that a roundabout with an inscribed circle diameter (ICD) of 30 m can be accommodated with minimal impact to the residential lots adjacent to the intersection. A conceptual design for the roundabout should be developed to determine the ultimate configuration including approach legs, splitter islands and access to the existing cul-de-sac. The land ownership and land acquisition will be required once the roundabout detailed design is finalized.

#### 6.1.2 Conduct Speed Study and Provide Traffic Calming

A speed study should be conducted in front of Kinosoo Beach on a typical summer weekend to confirm the excessive speeding problem. If excessive speeding is observed, traffic calming should be considered as an option to reduce the speeding problem.

Traffic calming should be provided along the beachfront between 16 Street and 25 Street. The following traffic calming measures should be considered along 1 Avenue:

- Narrow traffic lanes by enhancing the parking lane with painted stall lines
- Curb extensions with concrete curbs or concrete planters at intersections
- Speed humps or raised pedestrian crossings at intersections.

To reduce travel speeds along 1 Avenue, the City could also install chicanes along the corridor to create a curvilinear travel path. Chicanes are S-shaped curves in the vehicle driving path that are used to slow cars. Most traffic calming chicanes are created by building curb extensions that alternate from one side of the street to the other. Chicanes can also be created by taking



advantage of on-street parking lanes. On-street parking lanes can be alternated from one side of the street to the other to create a chicane-like effect. The parking lanes can be parallel or angled, and the chicane effect can be created through the use of pavement markings or curb extensions and landscaping to screen and define the parking areas.

Some of the traffic calming measures above will result in higher maintenance costs associated with snow removal and street cleaning. Prior to the implementation of these traffic calming measures, consideration should be given to the impact on the maintenance budget.

## 6.1.3 Repave Corridor

1 Avenue should be repaved to remove the cracks and potholes and to improve the pavement surface. After the corridor is repaved, pedestrian crosswalks and on-street parking stalls should be delineated.

### 6.1.4 Provide Pavement Marking and Signage at 19 Street Crosswalk

The 19 Street crosswalk should be delineated to clearly identify the crosswalk, in accordance with Section A6.3 of the MUTCD. Additionally pedestrian crossing (RA-4) signs should be provided in both directions along 1 Avenue to provide advance warning to approaching vehicles.

#### 6.2 10 STREET CORRIDOR

## 6.2.1 Provide Signage

To reduce the safety concerns associated with the limited sight distance due to the vertical crest curve, warning signs should be installed to communicate the hidden intersection. A concealed road (WA-13) sign should be installed in both directions along 1 Avenue to warn the motorists of the upcoming intersection at 3 Avenue. Figure 6.1 presents the concealed road (WA-13) sign.

Figure 6.1 Concealed Road (WA-13) Sign



## 6.2.2 Repave Corridor

10 Street should be repayed to remove the cracks and to improve the payement surface.

#### 6.3 LAKESHORE DRIVE CORRIDOR

### 6.3.1 Change Lane Configuration along Corridor

Simultaneous two-way travel along Lakeshore Drive, north of 6 Avenue, is difficult with parking allowed on the west side. The following two options are available to the City:

- Remove the parking lane and provide two-way traffic along Lakeshore Drive
- Maintain the parking lane and provide one-way traffic along Lakeshore Drive.

The parking lane on the west side of Lakeshore Drive, north of 6 Avenue, can be removed since residents have off-street parking in the form of garages, parking pads and driveways on their property. This option would have no impact on the existing traffic pattern since two-way traffic will be maintained on Lakeshore Drive.

Converting Lakeshore Drive to allow for one-way traffic was considered in the Lakeshore Redevelopment Plan (LRP) that was finalized in March 2010. Section 3.3.1 of the LRP identified the need to reconstruct a portion of Lakeshore Drive between 7 Avenue and 8 Avenue. The LRP considered the conversion of this portion to allow for one-way traffic in either the northbound or southbound direction.

The City should consider extending the scope of the reconstruction to the entire length of Lakeshore Drive from the 1 Avenue/10 Street intersection to 8 Avenue. This option would have significant impact on the traffic patterns along Lakeshore Drive and the adjacent roadways (i.e. 10 Street, 5 Avenue, 6 Avenue, 7 Avenue and 8 Avenue). A detailed traffic analysis should be completed to evaluate the impact of converting Lakeshore Drive to one way northbound or one-way southbound. The traffic analysis should include the intersection at 1 Avenue/2 Avenue/10 Street to coordinate with the improvements required at this intersection and the possible implementation of a roundabout.

Figure 6.2 through Figure 6.4 present the traffic flow along Lakeshore Drive and the adjacent roadways under the following scenarios:

- Two-way traffic on Lakeshore Drive
- One-way northbound traffic on Lakeshore Drive
- One-way southbound traffic on Lakeshore Drive.



## 6.3.2 Repave Corridor

Lakeshore Drive should be repaved to remove the cracks, ruts, and potholes and to improve the pavement surface.

## 6.3.3 Improve Pedestrian Crosswalks

The following improvements are required along Lakeshore Drive to improve pedestrian safety along the corridor:

- Repaint crosswalk lines to enhance visibility at all locations
- Provide pedestrian crosswalk (RA-4) signs for all crosswalk locations except for at stop signs
- Provide a pedestrian crosswalk at 6 Avenue with curb extensions and the proper pavement markings and signage in accordance with the MUTCD.

#### 6.4 50 AVENUE CORRIDOR

## 6.4.1 Review Highway 28 and 50 Avenue Intersection

The Cold Lake South Arena that was previously located in the southwest corner of Highway 28 and 50 Avenue has moved. In light of the developmental changes adjacent to the study intersection, the City indicated an opportunity to review the intersection for improvements to geometry and lane configuration. AE recommends that a detailed intersection analysis be completed for Highway 28 and 50 Avenue to address the safety concerns discussed in Section 5.4.1.

## 6.4.2 Close 55 Street Intersection and Re-configure Westbound Approach

The City should consider closing the intersection of 50 Avenue and 55 Street to remove the short separation distance currently provided between 55 Street and Highway 28. This will reduce the number of conflict points that currently exists on the westbound approach as a result of traffic to and from 55 Street.

#### 6.4.3 Conduct Main Street Analysis

50 Avenue is currently designated as an arterial roadway since it is the only east-west route available through Cold Lake South. However, 50 Avenue runs through the central business district and as such, may require a different designation (i.e. collector roadway) to reduce travel speeds and address access requirements for businesses located along the corridor. The City has identified a need to review the designation of 50 Avenue to address the conflicting service requirements and has committed to undertaking a 'Main Street Analysis' along 50 Avenue. The Main Street Analysis will review the corridor function and the possible conversion from an arterial roadway to an urban boulevard.





PROJECT NO: 2010-3050
DATE: 2010-3050
JANUARY 2011

APPROVED: SCALE: DWG NO:

NTS

# CITY OF COLD LAKE TRANSPORTATION STUDY

FIGURE 6.2 - TRAFFIC FLOW TWO-WAY TRAFFIC ON LAKESHORE DRIVE





PROJECT NO: 2010-3050 DATE: APPROVED:

SCALE: DWG NO: JANUARY 2011 NTS

## **CITY OF COLD LAKE** TRANSPORTATION STUDY

FIGURE 6.3 - TRAFFIC FLOW ONE-WAY NORTHBOUND TRAFFIC ON LAKESHORE DRIVE





PROJECT NO: 2010-3050 DATE:

APPROVED:

SCALE: DWG NO: JANUARY 2011 NTS

## **CITY OF COLD LAKE** TRANSPORTATION STUDY

FIGURE 6.4 - TRAFFIC FLOW ONE-WAY SOUTHBOUND TRAFFIC ON LAKESHORE DRIVE

## 6.4.4 Provide Back-in Angle or Parallel Parking Stalls

The City should consider replacing the existing angle parking stalls to "back-in" angle parking stalls. Back-in angle parking stalls require drivers to back into the parking stall and park their vehicles with the rear bumper against the curb. Figure 6.5 presents an example of a back-in angle parking stall.



Figure 6.5
Back-in Angle Parking Stalls

Back-in angle parking stalls provide the following benefits:

- Better visibility of traffic, cyclists and pedestrians when leaving a parking stall.
- Naturally guides pedestrians to the sidewalk. The open doors of a parked vehicle encourage pedestrians to use the sidewalk by blocking access to the travel lane.
- Allow commuters to load their trunks from the curb, rather than in the street with traffic.

To facilitate back-in angle parking, the existing pavement markings would need to be changed and the public would need to be educated on the proper procedure for entering/exiting a back-in angle parking stall.

The City could also replace the existing angle parking along 50 Avenue with parallel parking. The



parking supply within the central business district would decrease with the conversion to parallel parking; however, it should be able to accommodate the parking demand. The CLS parking study verified that parking is underutilized in the central business district, with a peak utilization of approximately 37%. A reduction in the parking supply should not have an adverse effect on the parking condition.

### 6.4.5 Repaint Pavement Markings

The pavement markings along 50 Avenue should be repainted to improve visibility. The City should delineate the pavement markings including the centerline, stop lines, crosswalk lines and parking stall lines.

#### 6.4.6 Close Unnecessary Driveway Accesses

If 50 Avenue is maintained as an arterial roadway, following the Main Street Analysis, the City should determine the land ownership of all the driveways along 50 Avenue and identify whether the driveways are currently being used. Unnecessary driveways should be closed to reduce the traffic conflict points along the corridor.

#### 6.4.7 Provide Curb Extensions and Signage at Crosswalks

The following improvements are required along 50 Avenue to improve pedestrian safety along the corridor:

- Provide pedestrian crosswalk (RA-4) signs at 49 Street and 53 Street, in accordance with the MUTCD
- Provide curb extensions at all crosswalk locations to improve driver visibility of pedestrians waiting to cross.

7

## **Conclusion**

AE was retained by the City to undertake in-service road safety reviews along the following four corridors:

- 1 Avenue, from the MD Campground (23 Street) to 2 Avenue/10 Street
- 10 Street, from 2 Avenue to 8 Avenue
- Lakeshore Drive, from 1 Avenue/10 Street to 8 Avenue
- 50 Avenue, from Highway 28 to 49 Street.

The purpose of the in-service road safety reviews was to identify potential safety issues along the corridors and propose improvement options that will reduce/eliminate the safety issues. The in-service road safety reviews were conducted based on the procedures outlined in the TAC safety guideline. Potential safety issues were identified based on observations during the site reconnaissance and the results from the operational analysis for the existing (2010) traffic operational analysis.

No operational issues were identified as far as traffic flow and intersection capacity is concerned.

Table 7.1 summarizes the safety issues identified for each corridor and the improvement options developed to address each of the potential safety issues.

Table 7.1
Summary of Safety Issues and Improvement Options

Study Corridor	Safety Issues	Improvement Options
	1 Avenue/2 Avenue/10 Street intersection configuration	Improve intersection configuration at 1 Avenue/ 2 Avenue/10 Street intersection. Roundabout option should be considered but requires further conceptual design.
1 Avenue	Speeding problem	Conduct speed study to confirm speeding problem. Provide traffic calming measures.
	Poor pavement conditions	Repave corridor.
	19 Street pedestrian crosswalk	Provide pavement marking and signage at 19 Street crosswalk.
10 Street	Vertical crest curve at 3 Avenue	Provide signage.



Study Corridor	Safety Issues	Improvement Options	
	Poor pavement conditions	Repave corridor.	
Lakeshore Drive	Road width and alignment	Change lane configuration along corridor. Detailed traffic analysis is required to determine the impact of lane changes prior to implementation.	
	Poor pavement conditions	Repave corridor.	
	Pedestrian crosswalks	Improve pedestrian crosswalks.	
	Highway 28 intersection	Complete detailed intersection analysis to review intersection geometry and lane configuration.  Close 55 Street intersection.	
		Conduct Main Street Analysis	
50 Avenue	Angle parking	Provide back-in angle or parallel parking stalls.	
	Faded pavement markings	Repaint pavement markings.	
	Multiple driveway accesses	If 50 Avenue remains an arterial roadway, close unnecessary driveway accesses.	
	Pedestrian crosswalks	Provide curb extensions and signage at crosswalks.	

Figure 7.1 and Figure 7.2 summarize the recommendations for the study corridors in Cold Lake North and Cold Lake South respectively.

## 7.1 COORDINATION OF IMPROVEMENTS ALONG 1 AVENUE AND LAKESHORE DRIVE

The Lakeshore Redevelopment Plan (LRP) was finalized in March 2010 and provided a strategic direction for the revitalization of the Lakeshore Commercial District to a vibrant "urban village" that would attract residents and tourists. The LRP identified the need to improve the aesthetics of the Lakeshore Commercial District, given its beautiful setting and prominent location at the end of Highway 28. The opportunity exists to expand the scope of the LRP to include the 1 Avenue/beachfront area, to take advantage of the attractiveness of the area in the summer months.

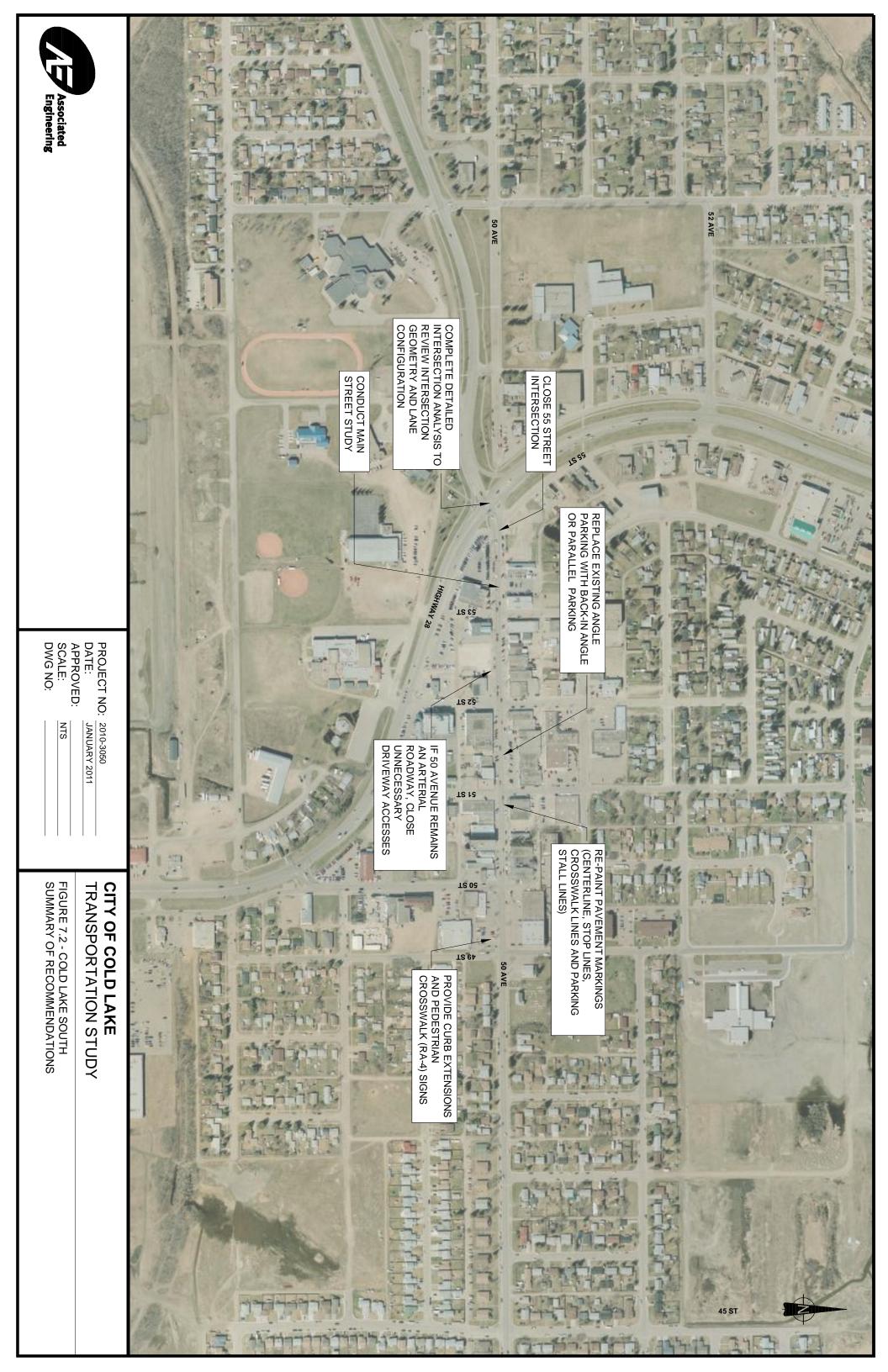
1 Avenue and Lakeshore Drive were analyzed as part of the Cold Lake North Parking Study. The parking study recommended the following strategies for the area:

- Provide overflow parking for the Marina Lot
- Provide marked (painted) parking stalls for on-street parking along 1 Avenue and Lakeshore Drive
- Enforce "no-parking" zones
- Pave and delineate parking stalls in the gravel lot located in the northeast corner of Birch Avenue

The improvements in Table 7.1 for the 1 Avenue and Lakeshore Drive corridors should be coordinated with the visions presented in the LRP and the improvements recommended from the parking study. Integration of the recommendations from the various studies will provide for cost effectiveness and a unified vision for the revitalization/beautification of the area.







## **TECHNICAL MEMORANDUM**



## **Appendix A - Operational Analysis Results**



**Project: Cold Lake Transportation Study** 

Project No: 2010-3050

Date Revised: September 16, 2010

Node #	Intersection	Traffic Control	Approach	V/C Ratio	Delay (s)	LOS	95th Queue (m)
			Overall Intersection	0.11	1.3	Α	
			EB	0.11	0.0	Α	0.0
				0.11	0.0	Α	0.0
				0.01	0.1	Α	0.2
	1 Avenue &	Unsignalized	WB	0.01	0.6	Α	0.2
103	Nelson Street	Stop Control - NB					
	Neison Street	Approach		0.05	10.3	В	1.4
			NB				
				0.05	10.3	В	1.4
			SB				
			Overall Intersection	0.09	1.9	Α	
	1 Avenue & 16 Street	Unsignalized	EB	0.09	0.0	Α	0.0
				0.09	0.0	Α	0.0
			WB	0.01	0.0	Α	0.1
				0.01	0.5	Α	0.1
104		Stop Control - NB					
		Approach	NB	0.07	10.2	В	1.9
				0.07	10.2	В	1.9
			SB				
			Overall Intersection	0.05	2.1	Α	
				0.01	0.1	Α	0.2
			EB	0.01	0.9	Α	0.2
105	1 Avenue / 2	Unsignalized	WB				
	Avenue & 10	Yield Control - SB					
	Street	Approach					
		7.pp.odo	NB	0.05	0.0	Α	0.0
				0.05	0.0	Α	0.0
				-	-	-	-
			SB				
				0.05	9.2	Α	1.3

**Project: Cold Lake Transportation Study** 

Project No: 2010-3050

Date Revised: September 16, 2010

Node #	Intersection	Traffic Control	Approach	V/C Ratio	Delay (s)	LOS	95th Queue (m)
			Overall Intersection	0.05	2.1	Α	
				0.01	0.1	Α	0.2
			EB	0.01	0.9	Α	0.2
105	1 Avenue / 2 Avenue & 10	Unsignalized Yield Control - SB	WB				
	Street	Approach					
			NB	0.05	0.0	Α	0.0
				0.05	0.0	Α	0.0
			SB	-	-	-	-
				0.05	9.2	Α	1.3
			Overall Intersection	0.21	6.6	Α	
			EB	0.01	0.0	Α	0.1
		Unsignalized		0.01	0.6	Α	0.1
				0.01	0.6	Α	0.1
				0.03	0.2	Α	0.7
	8 Avenue & 10		WB	0.03	2.7	Α	0.7
107	Street	Stop Control - NB/SB		0.03	2.7	Α	0.7
	Street	Approaches		0.21	11.4	В	6.3
			NB	0.21	11.4	В	6.3
				0.21	11.4	В	6.3
				0.14	12.2	В	4.0
			SB	0.14	12.2	В	4.0
				0.14	12.2	В	4.0

**Project: Cold Lake Transportation Study** 

Project No: 2010-3050

Date Revised: September 16, 2010

Node #	Intersection	Traffic Control	Approach	V/C Ratio	Delay (s)	LOS	95th Queue (m)
			Overall Intersection	0.05	2.1	Α	
105				0.01	0.1	Α	0.2
			EB	0.01	0.9	A	0.2
	1 Avenue / 2 Avenue & 10	Unsignalized Yield Control - SB	WB				
	Street	Approach					
			NB	0.05	0.0	Α	0.0
				0.05	0.0	Α	0.0
			SB	-	-	-	-
				0.05	9.2	Α	1.3
			Overall Intersection	0.07	4.3	Α	
				0.07	9.1	Α	1.7
	8 Avenue &	Unsignalized	EB	0.07	9.1	Α	1.7
			WB				
106	Lakeshore Drive	Stop Control - EB		0.00	0.0	^	0.5
		Approach	NB	0.02	0.2 3.1	A A	0.5 0.5
			IND	0.02	ა. I	А	0.5
			SB	0.03	0.0	Α	0.0
				0.03	0.0	Α	0.0

Project: Cold Lake Transportation Study Project No: 2010-3050 Date Revised: September 16, 2010

Node #	Intersection	Traffic Control	Approach	V/C Ratio	Delay (s)	LOS	95th Queue (m)	
			Overall Intersection	0.76	14.3	В		
				0.76	22.5	С	76.5	
			EB	0.42	11.4	В	48.0	
				0.19	2.3	Α	7.1	
			WB	- 0.22	-	-	- 24.4	
303	Highway 28 / 55	0:	VVD	0.22	9.3 2.2	A A	24.4	
303	& 50 Avenue	Signalized <sup>2</sup>		0.21 0.40	22.9	C	7.5 29.3	
			NB	0.40	17.3	В	22.8	
			110	0.01	10.8	В	2.3	
				0.44	23.5	C	33.1	
			SB	0.28	17.6	В	26.7	
				0.30	4.7	Α	11.4	
			Overall Intersection	0.16	3.1	Α		
				0.07	0.7	Α	1.8	
			EB	0.07	2.2	Α	1.8	
				0.07	2.2	Α	1.8	
				0.00	0.0	Α	0.0	
	50 Avenue & 53	Unsignalized	WB	0.00	0.1	Α	0.0	
316	Street	Stop Control - NB/SB		0.00	0.1	Α	0.0	
	Otroot	Approach		0.09	18.2	С	2.3	
			NB	0.09	18.2	С	2.3	
				0.09	18.2	С	2.3	
			OD	0.16	15.9	С	4.7	
			SB	0.16	15.9	С	4.7	
			Overell Interesetion	0.16	15.9	С	4.7	
			Overall Intersection	0.45	11.1	В		
			EB	0.45	12.0	B B	-	
			EB	0.45 0.45	12.0 12.0	В	-	
				0.45	11.1	В	-	
	50 Avenue & 52 Street	Unsignalized	WB	0.38	11.1	В	-	
317		Stop Control - All	****	0.38	11.1	В	-	
317		Approaches		0.15	9.6	A	_	
			NB	0.15	9.6	A	_	
				0.15	9.6	A	-	
			SB	0.23	10.1	В	-	
				0.23	10.1	В	-	
				0.23	10.1	В	-	
			Overall Intersection	0.47	11.2	В		
			EB	0.47	12.5	В	-	
				0.47	12.5	В	-	
				0.47	12.5	В	-	
				0.33	10.6	В	-	
	50 Avenue & 51	Unsignalized	WB	0.33	10.6	В	-	
318	Street	Stop Control - All		0.33	10.6	В	-	
		Approaches		0.14	9.5	A	-	
			NB	0.14	9.5	A	-	
					0.14	9.5	A B	-
			SB	0.23	10.3	В	-	
			56	0.23 0.23	10.3 10.3	В	-	
			Overall Intersection	0.46	10.9	В	-	
			C 7010II IIII III III III	-	- 10.9	-	-	
			EB	0.46	11.4	В	-	
			_ [	0.46	11.4	В	-	
		Unsignalized		0.32	10.4	В	-	
	F0 A 0.55	Stop Control -	WB	0.32	10.4	В	-	
319	50 Avenue & 50	EB/WB/NB		-	-	-	-	
	Street	Approaches Assumed Yield Control		0.31	10.4	В	-	
		- SB Approach	NB	-	-	-	-	
		- OD Appidacii		0.31	10.4	В	-	
				-	-	-	-	
			SB	1	-	-	-	
				-	-	-	-	
			Overall Intersection	0.32	5.5	Α		
				0.04	0.4	Α	1.1	
			EB	0.04	1.5	A	1.1	
				0.04	1.5	A	1.1	
		11- 1 - 2 - 1	WD	0.02	0.2	A	0.5	
200	50 Avenue & 49	Unsignalized	WB	0.02	1.2	A	0.5	
320	Street	Stop Control - NB/SB		0.02	1.2	A	0.5	
		Approaches	NB	0.19	15.5	С	5.5	
			IND	0.19	15.5	C C	5.5	
				0.19 0.32	15.5 17.6	C	5.5 10.9	
			SB	0.32	17.6 17.6	C	10.9	
			SD	0.32	17.6	C	10.9	
		l	l .	0.32	17.0	U	10.8	

## **FINAL REPORT**



# **Appendix G - Cold Lake Transportation Study Highway 28 Functional Review**

# **Technical Memorandum**

## **City of Cold Lake**

**Cold Lake Transportation Study Highway 28 Functional Review** 

## **April 2011**



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## **TECHNICAL MEMORANDUM**

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## TECHNICAL MEMORANDUM

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## Introduction

#### 1.1 STUDY BACKGROUND

Associated Engineering (AE) was retained by the City of Cold Lake (City) to update the existing transportation study. The purpose of the transportation study is to provide a comprehensive long-range plan that integrates the transportation infrastructure with requirements of the existing and future land uses. The transportation study will provide the City with a blueprint for planning and implementing specific transportation network improvement projects over the next 20 years in 5-year, 10-year, 15-year and 20-year planning horizons.

One component of the transportation study was to undertake a review of the Highway 28 Functional Plan, from 52 Avenue to the south city limits. The purpose for the review was to provide the City with recommendations on intersection requirements and improvements along the Highway 28 corridor. After an in-depth discussion with the City, the scope of the Highway 28 Functional Review was revised to include the following:

- Capacity analysis of links along the Highway 28 corridor
- Conduct a capacity analysis of one intersection along the study corridor under 20-year horizon
- Develop a template for an intersection that the City could use in the future to determine right-of-way requirements.

The capacity analysis of the links along the Highway 28 corridor was completed as part of the Traffic Volume Forecast and Analysis component of the transportation study update. Overall, the existing four-lane arterial provided along Highway 28 (between 52 Avenue to the south City limit) is sufficient to accommodate both the existing (2010) traffic volumes and the future (2030) forecasted traffic volumes. However, in the 20-year planning horizon, portions of Highway 28 will need to be reclassified as an Expressway classification and portions of the highway will need to be widen to provide a centre median. The traffic volume forecast and analysis technical memorandum should be referenced for further information and details.

## 1.2 STUDY OBJECTIVE

The objectives of the Highway 28 Functional Review were:

- Select and analyze the traffic operations at a representative intersection under the 20-year planning horizon to determine the required traffic control and intersection configuration
- Develop an intersection configuration template with designated turn lanes, storage lengths, and channelization, if required.

The intersection at Highway 28 and 43 Avenue was selected as the representative intersection as it currently experiences high traffic volumes and is expected to experience higher traffic volumes in the 20-year planning horizon. The afternoon (p.m.) traffic volumes were analyzed to determine the required intersection configuration.

## 1.3 STUDY AREA

Figure 1.1 shows the location of the Highway 28 and 43 Avenue intersection within the City.





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TRANSPORTATION STUDY

FIGURE 1.1 HIGHWAY 28 / 43 AVENUE INTERSECTION

## **TECHNICAL MEMORANDUM**

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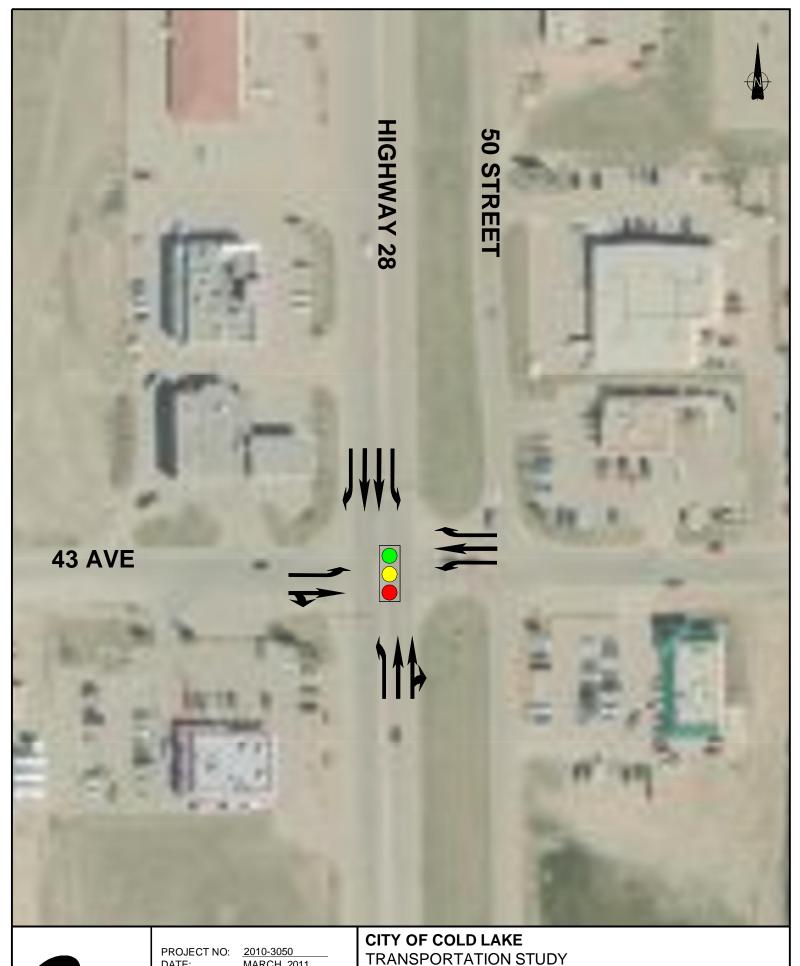
## **Existing Condition**

## 2.1 INTERSECTION CONFIGURATION

Highway 28 and 43 Avenue is currently a signalized intersection. The existing lane configuration is presented in Figure 2.1.

## 2.2 TRAFFIC VOLUME

Figure 2.2 presents the existing (2010) traffic volumes at the study intersection, in the p.m. peak hour.





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FIGURE 2.1 - HIGHWAY 28 / 43 AVENUE EXISTING (2010) INTERSECTION CONFIGURATION





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FIGURE 2.2 - HIGHWAY 28 / 43 AVENUE EXISTING (2010) PM PEAK HOUR TRAFFIC VOLUMES 3

## **Future Condition**

#### 3.1 INTERSECTION CONFIGURATION

For the transportation study update, the intersection of Highway 28 and 43 Avenue was analyzed as part of the Existing (2010) Traffic Operational Analysis technical memorandum. The analysis indicated that the intersection is currently operating well, at an overall intersection LOS A and delays of 9.9 seconds. All the intersection movements are operating at LOS B or better, and the maximum v/c ratio observed was 0.38. For further details, please refer to the Existing (2010) Traffic Operational Analysis technical memorandum.

No intersection improvements will be required to accommodate the existing (2010) traffic volumes; therefore, the current intersection configuration was used to conduct the capacity analysis in the 20-year planning horizon.

#### 3.2 20-YEAR (2030) TRAFFIC VOLUME

20-year traffic volumes were forecasted as part of the future traffic volume forecasts for the transportation study update. A spreadsheet model was used to forecast the future traffic volumes within the City. To complete the spreadsheet model, a skeletal road network was established to represent the anticipated road network and future traffic volumes were assigned onto the skeletal road network with consideration given to the logical routes (based on impedance and travel time). The trip assignment process was simplified by using select intersections to represent the different traffic zones within the City and future traffic volumes were assumed to enter/exit the zones from those intersections. For more details about the methodology and assumptions used to generate the future traffic volume forecasts, the Traffic Volume Forecast and Analysis technical memorandum should be referenced.

Developments are anticipated on both sides of Highway 28, near 43 Avenue in the future horizons. On the west side of Highway 28, two residential developments (Iron Horse and Fischer Estates) are expected between 46 Avenue and 34 Avenue. On the east side of Highway 28, the Grand Centre Southeast development is expected between 46 Avenue/50 Avenue and 34 Avenue. The Grand Centre Southeast development will contain commercial, industrial, and residential land uses, and part of the proposed commercial and industrial land uses have already been developed.

For the future traffic volume forecasted, the Highway 28 and 43 Avenue intersections served as a major access point for two traffic zones. As a result, this intersection is expected to experience a significant increase in traffic volumes, especially the movements into and out of 43 Avenue to the north. Figure 3.1 presents the traffic volumes forecasted for the Highway 28 and 43 Avenue intersection, in the 20-year planning horizon.





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FIGURE 3.1 - HIGHWAY 28 / 43 AVENUE 20-YEAR (2030) PM PEAK HOUR TRAFFIC VOLUME FORECASTS

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## **Intersection Capacity Analysis**

#### 4.1 METHODOLOGY

The Synchro 7.0 traffic analysis software based on the Highway Capacity Manual (HCM) was used to complete the capacity analysis of the study intersection. Synchro 7.0 applies the methodology established by the HCM to output a level of service (LOS) for study intersections, given the lane configuration, vehicular volumes, heavy vehicle percentages and signal timing.

A design criteria was developed by AE at project initiation to set various parameters for the capacity analysis and is included in Appendix A. Changes to the design criteria were made as the study progressed and additional information was provided. The Peak Hour Factor (PHF) was revised to 0.86 from a default value of 1.00; a PHF of 0.86 reflects the average calculated PHF from the traffic counts conducted by the City in June 2010. Additionally, default signal timing parameters such as minimum green time and pedestrian walk/clearance time were revised to reflect those proposed as part of the Highway 28 twinning project.

The operational capability of the study intersections were assessed using capacity, which is a measure of the sustainable flow rate at which vehicles can be expected to transverse a point. The critical measures used in the assessment were:

**Volume to capacity (v/c) ratio** provides the amount of congestion for each turning movement and for each lane group for signalized intersections. A v/c value over 1.00 indicates that the movement or lane group is at capacity.

Control delay is the amount of delay a vehicle experiences in seconds.

**LOS** is a qualitative measure describing operational conditions within a traffic stream and is based on service measures such as delay and congestion.

For an intersection to be considered operating at the acceptable level, LOS C or above is required for the overall intersection as well as for the individual intersection approaches. The LOS definitions for unsignalized and signalized intersections are included in the design criteria in Appendix A. In the 20-year horizon, traffic volumes are expected to increase such that LOS C may not be achievable at the study intersection; therefore, LOS D was targeted for both the overall intersection as well as the individual approach movements in the 20-year planning horizon.

#### 4.2 RESULTS

Table 4.1 presents the intersection capacity analysis results based on the existing lane configuration and traffic control. Existing signal timing for Highway 28 and 43 Avenue were not available; therefore, Highway 28 and 50 Avenue signal timing plans were used to analyze the traffic operations. Table 4.1 presents the capacity result after the signal timing was optimized to improve the traffic operations.

Table 4.1 Capacity Analysis Results - 20 Year (2030) Forecasted p.m. Traffic Volumes

Intersection	Traffic Control	Approach	Movement	Laning	Volume (veh)	V/C Ratio	Delay (s)	LOS	95th Queue (m)
			Overall Inte	ersection		1.10	50.1	D	-
			Left	L	334	1.05	98.4	F	#109.5
		EB	Through	TR	64	0.43	39.2	D	41.6
	Signalized		Right	IK	54	-	-	ı	-
		WB	Left	L	171	0.56	38.8	D	56.4
			Through	Т	106	0.39	48.9	D	44.7
Highway 28 / 43 Avenue			Right	R	225	0.55	10.1	В	19.7
			Left	L	33	0.22	34.0	С	16.0
		NB	Through	TTR	893	1.02	73.0	E	#188.5
			Right	IIK	107	-	-	-	
			Left	L	452	1.10	106.2	F	#186.7
		SB	Through	2T	884	0.49	14.6	В	82.4
			Right	R	419	0.44	2.2	Α	10.3

Note: The # footnote indicates that the volume for the 95th percentile cycle exceeds capacity. If the reported v/c < 1 for this movement, the methods used represent a valid method for estimating the 95th percentile queue.

The existing intersection configuration cannot accommodate the traffic volumes anticipated at the study intersection in the 20-year planning horizon. With the existing lane configuration, the intersection is expected to operate at an overall intersection LOS D with delays of 50.1 seconds. Three intersection movements are expected to operate at LOS E or worst. These movements include the eastbound and southbound left turns, and the northbound through lane. These movements are expected to operate above capacity with v/c ratios of 1.05, 1.10 and 1.02.

In order for the intersection to operate above capacity (LOS D), geometric improvements such as left/right turn lanes and channelization were introduced. The required intersection configuration is presented in Figure 4.1.

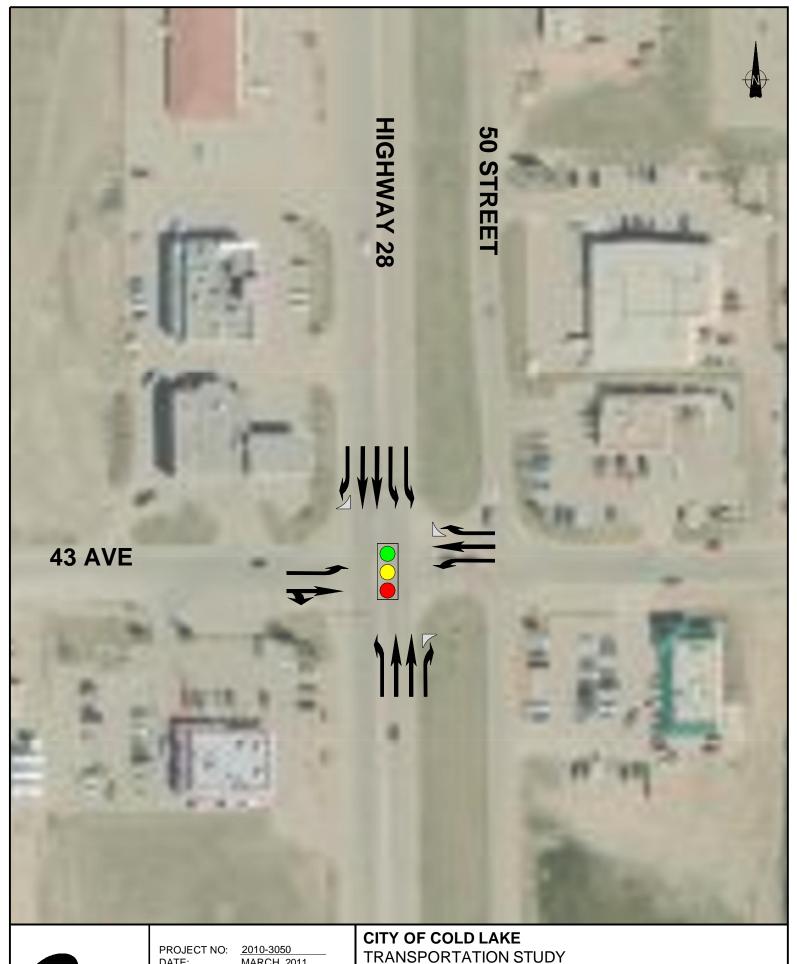
Table 4.2 presents the intersection capacity results based on the improved intersection configuration.

Table 4.2
Capacity Analysis Results - 20 Year (2030) Forecasted p.m. Traffic Volumes - Required Intersection Configuration

Intersection	Traffic Control	Approach	Movement	Laning	Volume (veh)	V/C Ratio	Delay (s)	LOS	95th Queue (m)
			Overall Inte	ersection		0.91	29.7	С	-
			Left	L	334	0.89	48.6	D	#87.7
		EB	Through	1	64	0.42	25.4	С	28.6
	Signalized		Right	TR	54	-	-	-	-
		WB	Left	L	•	0.48	24.0	С	38.4
			Through	Т	106	0.45	37.6	D	33.3
Highway 28 / 43 Avenue			Right	R [C]	225	0.58	9.9	Α	16.8
		NB	Left	L	33	0.19	37.4	D	15.2
			Through	2T	893	0.91	40.5	D	#131.4
			Right	R [C]	107	0.21	6.9	Α	12.9
			Left	2L	452	0.85	48.4	D	#73.0
		SB	Through	2T	884	0.65	22.4	С	107.3
			Right	R [C]	419	0.51	4.4	Α	18.1

Note: The # footnote indicates that the volume for the 95th percentile cycle exceeds capacity. If the reported v/c < 1 for this movement, the methods used represent a valid method for estimating the 95th percentile queue.

[C] - Channelization





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FIGURE 4.1 - HIGHWAY 28 / 43 AVENUE 20-YEAR (2030) REQUIRED INTERSECTION CONFIGUATION

## **TECHNICAL MEMORANDUM**

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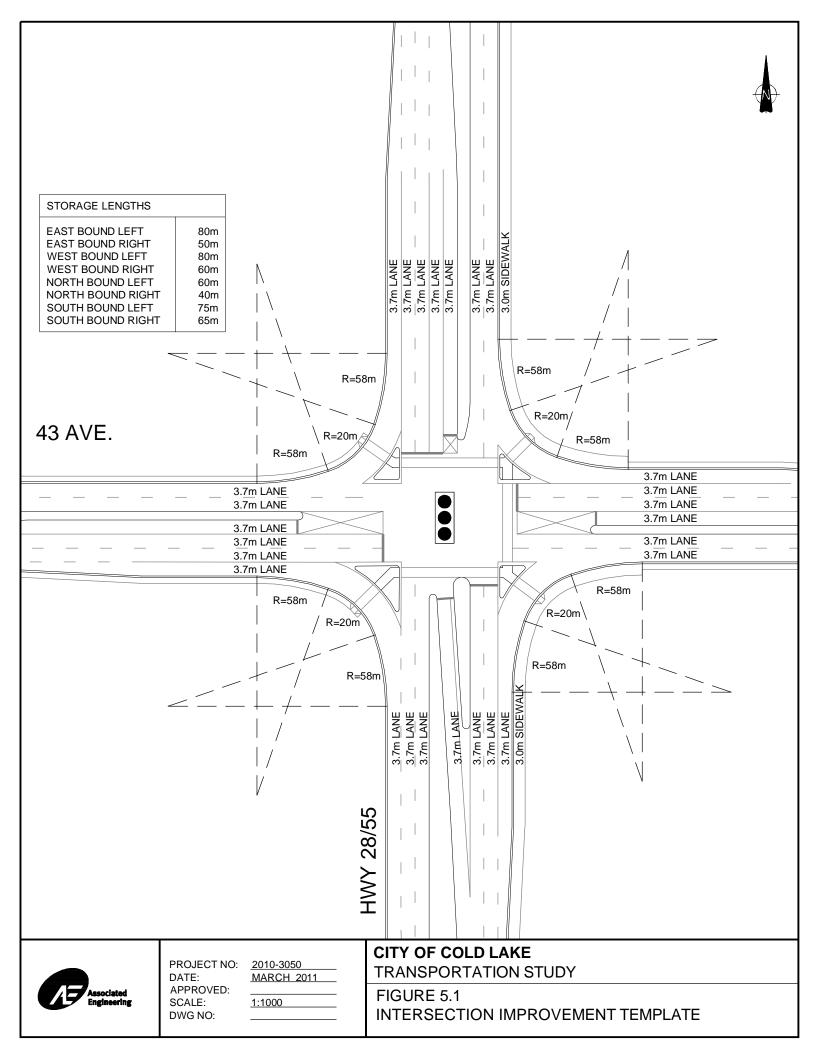
## **Intersection Template**

The City requested that a standard intersection template be developed with designated turn lanes and channelization. The template will be used by the City to determine and reserve the necessary right-of-way to accommodate future intersection improvements along Highway 28.

As per the City's requirements, a future standard intersection template was developed based on the capacity analysis results for Highway 28 and 43 Avenue. The City's design standards as outlined in the Municipal Engineering Servicing Standards and Standard Construction Specifications (2008) and the Transportation Association of Canada's Geometric Design Guide for Canadian Roads were referenced in the development of the template presented in Figure 5.1.

The intersection template presented in Figure 5.1 was designed to accommodate the traffic volumes assumed for Highway 28 and 43 Avenue. While additional through lanes were not required on the eastbound and westbound approaches from the capacity analysis, two through lanes were provided to reflect the four-lane collector road classification anticipated for 43 Avenue in the 20-year planning horizon.

For application at other intersection locations along Highway 28, between 50 Avenue and 43 Avenue, the City should complete an intersection capacity analysis to determine the designated turn lanes and storage lengths required, as well as geometric design to determine the actual land requirements. The template should be modified to reflect the requirements of the specific intersection.



## TECHNICAL MEMORANDUM



## **Conclusion**

The scope of the Highway 28 Functional Review was revised to:

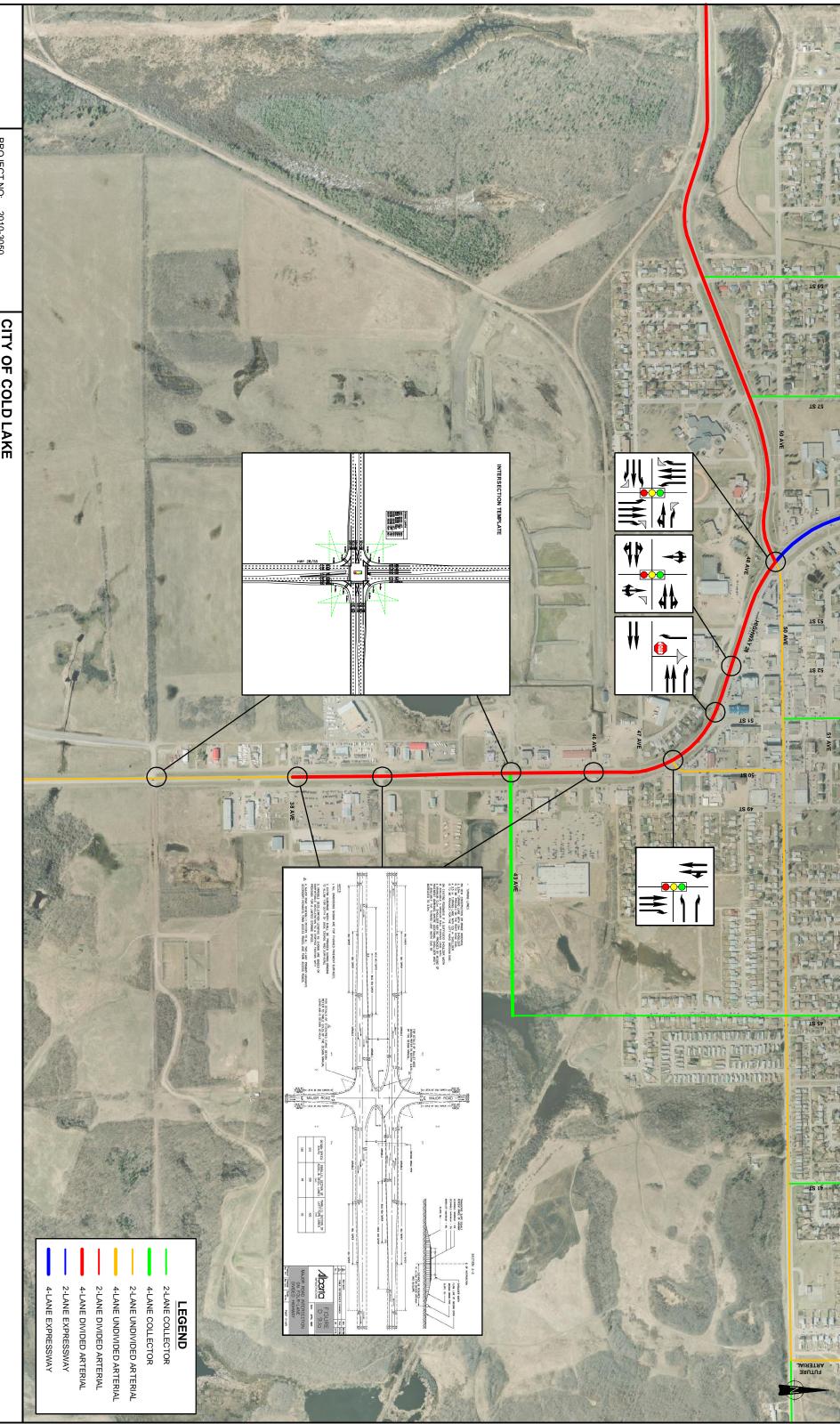
- Select and analyze the traffic operations at a representative intersection under the 20-year planning horizon to determine the required traffic control and intersection configuration
- Develop a standard intersection template with designated turn lanes, storage lengths, and channelization, if required.

The intersection of Highway 28 and 43 Avenue currently experiences high traffic volumes and is expected to experience higher traffic volumes in the 20-year planning horizon; therefore, it was selected as the representative intersection. The afternoon (p.m.) traffic volumes, from the 20-year planning horizon, were analyzed to determine the required traffic control and intersection configuration. Figure 4.1 presents the intersection configuration required to accommodate the future traffic volumes anticipated at Highway 28 and 43 Avenue.

As per the City's requirements, a future standard intersection template was developed based on the capacity analysis results for Highway 28 and 43 Avenue. The City's design standards as outlined in the Municipal Engineering Servicing Standards and Standard Construction Specifications (2008) and the Transportation Association of Canada's Geometric Design Guide for Canadian Roads were referenced in the development of the template presented in Figure 5.1.

The intersection template presented in Figure 5.1 was designed to accommodate the traffic volumes assumed for Highway 28 and 43 Avenue. For application at other intersection locations, the City should complete an intersection operational analysis to determine the designated turn lanes and storage lengths required, as well as geometric design to determine the actual land requirements. The template should be modified to reflect the requirements of the specific intersection.

Figure 6.1 summarizes the results from the Highway 28 Functional Review. The figure presents the 20-year road network classification along Highway 28 (between 50 Avenue and 34 Avenue), potential intersections where the intersection template developed could be applied, and the intersection configuration required at various intersections in the existing (2010) conditions.





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TRANSPORTATION STUDY

FIGURE 6.1 HIGHWAY 28 FUNCTIONAL REVIEW - SUMMARY

## **TECHNICAL MEMORANDUM**



## **Appendix A - Design Criteria**

City of Cold Lake Transportation Study Project No: 2010-3050 Date: July 5, 2010

#### TRAFFIC ANALYSIS CRITERIA

A micro model using Synchro/SimTraffic 7.0 will be developed to identify and review intersection capacity needs. Level of service will be used as the common reference in terms of average delay times categorized into six general grades. Table 1.1 defines the LOS criteria for signalized intersections and unsignalized intersections.

**Table 1.1 Level of Service Definitions** 

Level of Service (LOS)	Overall Average Delay at Unsignalized Intersection	Overall Average Delay at Signalized Intersection
А	≤ 10 seconds	≤ 10 seconds
В	> 10 and ≤ 15 seconds	> 10 and ≤ 20 seconds
С	> 15 and ≤ 25 seconds	> 20 and ≤ 35 seconds
D	> 25 and ≤ 35 seconds	> 35 and ≤ 55 seconds
E	> 35 and ≤ 50 seconds	> 55 and ≤ 80 seconds
F	> 50 seconds	> 80 seconds

The minimum LOS criteria recommended by Associated Engineering (AE) is LOS C for the overall intersection. Additionally, each specific movement is targeted to achieve a LOS C or better in all cases. To achieve improved levels of service, the following criteria are proposed where applicable in the traffic network model:

- Right turn channelization (yield condition) provided when turning movements exceed 60 vehicles per hour.
- Right turn bays provided to satisfy LOS E or queuing issues in right or through movements.
- Left and right turn bay lengths provided based on 95<sup>th</sup> queue lengths from Synchro with a minimum storage length of 60 meters.
- Double left turn lanes provided when turning volumes significantly exceed 300 vehicles per hour and LOS or v/c ratios are above the stated minimums.

Table 2.2 presents the recommended traffic analysis assumptions that will be used in the Synchro model. The table also presents assumptions used by four different municipalities within Alberta including the Regional Municipality of Wood Buffalo (RMWB), the City of Calgary, the City of Lethbridge and the City of Medicine Hat. The assumptions used by the RMWB were developed by AE for a specific project.

**Table 2.2 Traffic Analysis Assumptions for Synchro** 

Traffic Analysis Parameters							
Parameter	RMWB*	City of Calgary	City of Lethbridge	City of Medicine Hat	Recommended		
Link Speed		Existing poste	ed speed limits		Existing posted speed limits		
Lane Widths		3.7	7m		3.7m		
Storage Length		Minimu	ım 60m		Minimum 60m		
Adjacent Parking Lanes		Apply data wl	nere available		Apply data where available		
Lane Window							
Ideal Saturation Flow (vphpl)	1900	1850	1750	1850 (through) 1650 (turning)	1850		
Lost Time	-	Default	Default	Default	Default		
Leading Detector	2m (turning) 10m (through)	8m (left turn) 4m (through)	Default	-	Default		
Trailing Detector	0	2m	Default	-	Default		
Turning Speed	-	Default	Default	Default	Default		
Lane Utilization	-	Default	Default	Default	Default		
Right Turn Factor	-	Default	Default	Default	Default		
Left Turn Factor (protected)	-	Default	Default	Default	Default		
Saturated Flow Rate (protected)	-	Default	Default	Default	Default		

Left Turn Factor (permitted)	-	Default	Default	Default	Default
Saturated Flow Rate (permitted)	-	Default	Default	Default	Default
Saturated Flow Rate (RTOR)	-	Default	Default	Default	Default
Headway Factor	-	Default	Default	Default	Default
		Volume	Window		
Conflicting Pedestrian #	-	Apply data where available	Apply data where available	Apply data where available. Minimum = 5.	Apply data where available. Minimum = 5.
Conflicting Bikes #	-	Apply data where available	Apply data where available	Apply data where available. Minimum = 5.	Apply data where available. Minimum = 5.
Peak Hour Factor	1.00	1.00	0.88 1.00 (15 min data used)	0.95 – Congested Urban Conditions 0.92 – Current / Base Case Urban Conditions 0.88 – Current / Base Case Undeveloped areas 0.85 – Forecast Case, Local and Collector Roads 0.93 – Forecast Case, Congested Collectors and Minor Arterial Roads 0.95 – Forecast Case, Principal Arterials	0.86
Growth Factor	1.0	1.0	1.0	1.0	1.0

	1	1	ı		
Heavy Vehicle (%)	5	Apply data where available. Default 5% (main street), 2% (side street) and 7.5% or greater in industrial areas.	Apply data where available. Default 5% (main street), 2% (side street) and 10% in industrial areas.	Apply data where available. Default 7.5% or greater in industrial areas.	Apply data where available. Default 5% (main street), 2% (side street) and 7.5% or greater in industrial areas.
Bus Blockage (#/hour)	0	Apply data where available	Apply data where available	Apply data where available	Apply data where available
Traffic from Mid- Block (%)	None	Apply data where available	Apply data where available	Apply data where available	Apply data where available
Link OD Volumes	-	Alterations must be documented in detail	Alterations must be documented in detail	Alterations must be documented in detail	Default
Lane Group Flow	-	Default	Default	Default	Default
Vehicle Clearance / Existing Timings	-	Contact City of Calgary - Traffic Signals	Contact City of Lethbridge – Traffic Operations	Minimum Green = 7 seconds on left turns, 10 seconds for through Maximum Time = 20 - 30 seconds on main road	Use existing signal timing where available
		Timing '	Window		
Main Street Minimum Initial	-	20 seconds or pedestrian time, whichever is greater	20 seconds or pedestrian interval, whichever is greater	10 seconds or pedestrian time, whichever is greater	15 seconds or pedestrian interval, whichever is greater
Side Street Minimum Initial	-	10 seconds	10 seconds or minimum pedestrian interval, whichever is greater	10 seconds	12 seconds
Minimum Initial Arrows	-	5 seconds	5 seconds	7 seconds	7 seconds
Minimum Initial Split	-	Default	-	Default	Default

Recall	-	Main Street – Ped. / min. unless on fixed (pretimed) mode. Fixed mode generally used in Downtown / Beltline areas. Minor Street or Turns – No recall.	Main Street – Ped. / min. unless on fixed (pretimed) mode. Minor Street or Turns – No recall.	Main Street – Ped. / min. unless on fixed (pretimed) mode. Fixed mode generally used in Downtown area. Minor Street or Turns – No recall.	Main Street – Ped. / min. unless on fixed (pretimed) mode. Minor Street or Turns – No recall.
		Phasing	Window		
Pedestrian Walk Time	8 seconds	Minimum 8 seconds	Minimum 6 seconds	20 seconds	7 seconds
Pedestrian Clearance Time (Don't Walk)	11 seconds	Contact City of Calgary – Traffic Signals	Minimum value derived from actual crossing distance (m) divided by walking speed of 1.2 m/s. In areas with high senior citizens, walking speed of 1.0 m/s should be used.	Pedestrian walk time plus 7 seconds (27 seconds)	17 seconds
Pedestrian Calls (#/hr)	5	Apply data where available	Apply data where available	Apply data where available. Minimum = 5.	Apply data where available. Minimum = 5.
Minimum Splits for Arrows	-	10 seconds plus clearance. In extreme cases 8 seconds plus clearance for prot / perm arrows, 9 seconds plus clearance for prot only arrows.	10 seconds plus clearance. In extreme cases 8 seconds plus clearance for prot / perm arrows, 9 seconds plus clearance for prot only arrows.	10 seconds plus vehicle clearance	10 seconds plus vehicle clearance
Dual Entry	Yes	Yes	Yes	Yes	Yes

Inhibit Max	Yes	Contact City of Calgary – Traffic Signals	Default	No. Contact City of Medicine Hat – Municipal Engineering.	Yes
-------------	-----	---	---------	--	-----

<sup>\*</sup>Project specific to the West Loop Road Project

All other factors will be set at the default or calculated values.

#### General comments:

- If an arrow (protected) phase is found to be needed in one peak period, it will be included in the signal phasing in the analysis of all peak hours.
- Summary sheets will include v/c ratios, level of service values and 95<sup>th</sup> queue lengths.



## **Appendix H - School Zone Safety Analysis**

## **Technical Memorandum**

## **City of Cold Lake**

**School Zone Safety Analysis** 

## **April 2011**



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## **TECHNICAL MEMORANDUM**

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## Introduction

The City of Cold Lake (City) is located 287 km northeast of the City of Edmonton in Alberta and was formed in 1996 by merging three municipalities, namely Grand Centre, Medley (Canadian Forces Base W4) and Cold Lake. Grand Centre was renamed Cold Lake South (CLS) and the original Cold Lake is known as Cold Lake North (CLN).

The City has experienced noticeable growth in recent years. According to municipal census, the City had a population of 11,991 in 2006 and 13,924 in 2009. This corresponds to a 5.4% linear growth annually. Current transportation improvements within the City have been based on the previous transportation study completed in 2000 and is no longer considered representative of the actual transportation network required to address the current and future transportation needs.

In light of the continuing accelerated pace of development in the region and the need to rationalize and identify the transportation network requirements for the City, including surrounding rural municipalities and counties, the existing transportation plan requires a comprehensive update.

#### 1.1 PROJECT BACKGROUND

Associated Engineering (AE) was retained by the City to update the existing transportation study. The purpose of the transportation study is to provide a comprehensive long range plan that integrates the transportation infrastructure requirements of the existing and future land uses. The transportation study will provide the City with a master plan on which to plan and implement specific transportation network improvement projects over the next 20 years in 5-year, 10-year, 15 year and 20-year planning horizons. The transportation study will consider municipal roads, traffic calming, parking, traffic safety, traffic signal coordination, school zones, transit, truck routes, traffic management, and transportation system operations.

This technical memorandum documents the results of the school zone safety analysis completed for the schools with the City of Cold Lake.

#### 1.2 STUDY OBJECTIVE

The objective of the school zone safety analysis was to evaluate the safety at school sites, with respect to the following:

- Traffic operation during the morning drop-off and afternoon pick-up period (one site only)
- Proper school area/zone designation for roadways abutting the school sites
- Proper playground area/zone designation for roadways abutting the playground sites.



#### 1.3 STUDY AREA

The City provided AE with a comprehensive list of the existing schools within the City. The following schools were studied as part of the school zone safety analysis:

## **Cold Lake North**

- Nelson Heights Middle School
- St. Dominic Elementary School
- Lakesland Christian Academy
- Cold Lake Elementary School.

#### **Cold Lake South**

- Ecole Voyageur
- Trinity Christian School
- Grand Centre Middle School
- Grand Centre Elementary School
- Assumption School
- Holy Cross Elementary School.

## Medley

- MacKenzie School
- RA Reynolds School
- Ecole Voyager.

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## **Study Methodology**

The Transportation Association of Canada (TAC) published the School and Playground Areas and Zones: Guidelines for Application and Implementation (TAC guideline) in October 2006. The TAC guideline was developed to "provide a set of uniform guidelines towards the establishment and the signing and marking of school and playground areas and zones in both rural and urban environments", and was developed after a review of the Manual of Uniform Traffic Control Devices (MUTCD) and public consultation with road authorities and stakeholders across Canada. The TAC guideline provides worksheets that can be used to systematically establish school and playground areas and zones, and signing and marking plans that can be implemented at the school and playground areas and zones.

Alberta Transportation (AT) also released a new version of their Guidelines for School and Playground Zones and Areas (AT guideline) in December 2007. The material covered in the AT guideline is similar to the TAC guideline and the school zone and playground zone worksheets are identical. There are some minor variation in the signing and marking plans between the two guidelines. The City currently uses the AT guideline for the establishment of their school and playground areas and zones.

Some municipalities use only school/playground zones and do not differentiate between a school area or school zone, and a playground area or playground zone. It is the City's discretion whether or not to implement a similar standard.

For the purpose of this school zone safety analysis, the school zone and playground zone worksheets from the TAC guideline were used. The following methodology was used to complete the school zone safety analysis:

- Initiate project meeting
- Conduct site reconnaissance and assemble data
- Observe morning and afternoon operation at a representative school site for safety
- Complete school/playground area and zone worksheet analysis
- Provide guidance on signage and pavement markings
- Provide advice on school zones along Highway 28 and Centre Avenue/Kingsway
- Produce draft and final reports.



## 3

## **Study Process and Analysis**

#### 3.1 PROJECT INITIATION MEETING

A project initiation meeting was held on May 4, 2010 in Cold Lake to complete the following:

- Confirm the scope of the school zone safety analysis
- Obtain the complete list of schools within the City
- Determine the representative school site to observe the morning drop-off and afternoon pick-up operations.

#### 3.2 DATA COLLECTION, SITE RECONNAISSANCE AND OBSERVATIONS

#### 3.2.1 Data Collection

Site visits were conducted at each school site to collect photographs and relevant information required for input into the analysis worksheets discussed in a later section. The site visits were completed over a 2-day period on May 5 and May 6, 2010.

#### 3.2.2 Site Reconnaissance and Observations

As mentioned, a representative school site was selected to observe during the drop-off and pick-up operations from a safety standpoint. Grand Centre Middle School was selected as the representative site because of the following reasons:

- Location: Grand Centre Middle School is centrally located between two major roadways,
   Highway 28 to the east and Centre Avenue/Kingsway to the south. School zones have been provided on both these roadways.
- Playground: Grand Centre Middle School shares a common field with Grand Centre Elementary. This means that the roadway adjacent to the schools would be more congested during the morning and afternoon periods as a result of drop-off and pick-up at both schools.
- Pedestrian Crosswalk: A pedestrian activated crosswalk signal is also provided across
  Highway 28 at 51/52 Avenue for safe crossing of students who attend Grand Centre Middle
  School and Grand Centre Elementary School.

The site reconnaissance was completed on May 5, 2010 from 3:30 p.m. to 4:30 p.m. to observe the afternoon peak hour pick-up operations, and on May 6, 2010 from 7:30 a.m. to 8:30 a.m. to observe the morning peak hour drop-off operations. A one-hour time period was originally chosen for the afternoon observations; however, the time period was shortened since all of the students were discharged and picked up by 4:00 p.m.



The existing site conditions are presented in Figure 3.1 and the observations from the site reconnaissance are summarized below:

- The school bus dropped off and picked up students at the school entrance, in front of the main doors.
- Five crosswalks are currently provided in the vicinity of the school at the following locations: across 52 Avenue at 56 Street, across 56 Street at 52 Avenue, across 56 Street immediately before the horizontal curve which transitions 56 Street to 51 Avenue, across 51 Avenue at the service road, and across the service road at 51 Avenue.
- No crossing guards were present at any of the marked crosswalks.
- The marked crosswalk provided across 56 Street immediately before the horizontal curve was not used by students. Students who were dropped off by parents on the non-school side were typically dropped off at the main door and crossed 56 Street at that point.
- The placement of the pedestrian activated crosswalk across Highway 28 directed students to the school main entrance at 51 Avenue and reduced pedestrian-vehicle conflicts.

No issues were observed during the morning drop-off and afternoon pick-up periods as far as school zone safety was concerned. The AE project team did not observe anything that was considered unsafe.

In the 2010/2011 school year, Grand Centre Middle School was relocated south of Centre Avenue, to the building previously occupied by Grand Centre High School. The high school, in turn, was relocated north of Cold Lake South to the Energy Centre. The old location for Grand Centre Middle School, on 56 Street, currently is vacant and there are no immediate plans for redevelopment of the building. With the relocation of Grand Centre Middle School, student traffic across Highway 28 will shift from the pedestrian activated crosswalk at 51 Avenue to the intersection of Highway 28 and 50 Avenue. The pedestrian crosswalks at this intersection should be maintained to ensure maximum visibility.

Grand Centre Elementary School is still located on the previously shared lot with Grand Centre Middle School. To improve safety for the elementary school, the crosswalk located on the horizontal curve along 56 Street/51 Avenue should be realigned to provide a north-south crosswalk across 51 Avenue. This would provide a safe crosswalk location for students walking along the east side of 56 Street. If the crosswalk is relocated, the City should install a warning sign along the west side of 56 Street to warn southbound motorists of the crosswalk ahead. When the crosswalk is moved across 51 Avenue, visibility of the crosswalk will be partially obscured by the horizontal curve. A warning sign would help to promote driver awareness and improve crosswalk safety.



Figure 3.1 Existing Site Conditions

## 3.3 SCHOOL/PLAYGROUND AREA AND ZONE WORKSHEET ANALYSIS

#### 3.3.1 School Area and School Zone

According to the TAC guideline, school areas and school zones differ in the following manner:

 School areas are a section of roadway adjacent to a school that is denoted by school area signing only while school zones are a section of roadway adjacent to a school that is denoted by school area signing and reduced speed limit signs.



The worksheet provided in the TAC guideline determines whether a given roadway should be established as a school area or a school zone using a point-based system based on the following criteria:

- School type
- Road classification
- Fencing
- Property line separation
- School entrance
- Sidewalks.

Detailed information about each criterion is provided in Section 2.3 of the TAC guideline. A sample of the TAC school zone input worksheet has been provided in Appendix A along with the TAC school zone results matrix used to assign the required designation.

School zone input worksheets were completed, in accordance with the TAC guideline, for each school site and the results are summarized in Table 3.1. The detailed worksheets are included in Appendix A.

Table 3.1 TAC School Zone Worksheet Results

School Site	Adjacent Road	Warranted Designation
Nelson Heights Middle School	5 Avenue	School Area
St. Dominic Elementary School	Lakeshore Drive	School Area or School Zone
	7 Street	School Zone
Lakesland Christian Academy	10 Street	School Area or School Zone
	14 Avenue	School Zone
Cold Lake Elementary	16 Avenue	School Area
	8 Street	School Area or School Zone
Ecole Voyageur	49 Street	School Zone
Trinity Christian School	51 Avenue	School Area or School Zone
	61 Street	School Area or School Zone

School Site	Adjacent Road	Warranted Designation
Grand Centre Middle School	57 Street	Nothing
	52 Avenue	Nothing
	56 Street	School Area
Grand Centre Elementary School	57 Street	School Area
	50 Avenue	School Area
	51 Avenue	School Zone
Grand Centre High School	48 Avenue	School Area
	57 Street	School Area
	47 Avenue	School Area
Assumption School	48 Avenue	School Area or School Zone
	47 Avenue	School Area or School Zone
Holy Cross Elementary	49 Street	School Zone
MacKenzie School	Hickory Street	School Zone
RA Reynolds School	Queensway	School Area or School Zone
	Spruce Crescent	School Area or School Zone
Ecole Voyager	Birch Avenue	School Zone

## 3.3.2 Playground Area and Playground Zone

Playground areas and playground zones differ in the same manner as school areas and school zones:

 Playground areas are a section of roadway adjacent to a playground that is denoted by playground area signing only while playground zones are a section of roadway adjacent to a playground that is denoted by playground area signing and reduced speed limit signs.

For the playground zone analysis, only the playgrounds adjacent to existing school sites were analyzed. Stand-alone playground areas within the City were not analyzed. Additionally, playgrounds completely fenced within a school site were not analyzed.



The worksheet provided in the TAC guideline determines whether a given roadway should be established as a playground area or playground zone using a point-based system based on the following criteria:

- Playground type
- Road classification
- Fencing
- Property line separation
- Playground entrance
- Sidewalks.

Detailed information about each criterion is provided in Section 2.5 of the TAC guideline. A sample of the TAC playground zone input worksheet has been provided in Appendix B along with the TAC playground zone results matrix used to assign the required designation.

Playground zone input worksheets were completed, in accordance with the TAC guideline, for the selected playground sites and the results are summarized in Table 3.2. The detailed worksheets are included in Appendix B.

Table 3.2
TAC Playground Zone Worksheet Results

Playground Site	Adjacent Road	Warranted Designation
St. Dominic Elementary School	7 Street	Playground Area
Grand Centre Elementary School	57 Street	Playground Area
	50 Avenue	Nothing
MacKenzie School	Oak Drive	Playground Area
RA Reynolds School	Queensway	Playground Area
	Spruce Crescent	Playground Area

#### 3.4 SIGNAGE AND PAVEMENT MARKINGS

The signing and marking plans recommended by both TAC and AT for school and playground areas and zones are provided in Appendix C. The existing signage and pavement markings at the school sites should be reviewed against these plans to ensure compliance with the standards. The results presented in Table 3.1 and Table 3.2 indicates that some school and playground sites need to be converted from an

existing school/playground area to a school/playground zone. At these locations, care should be made to comply with the signage and marking plans for the areas/zones.

#### 3.5 SCHOOL ZONES ALONG HIGHWAY 28 AND CENTRE AVENUE/KINGSWAY

Additional school zones currently exist along the following roadways even though there are no school sites located immediately adjacent to them:

- Highway 28, from 52 Avenue to 46 Avenue
- Service road parallel and west of Highway 28, from 52 Avenue to 50 Avenue
- Centre Avenue, from 57A Street to service road west of Highway 28.

The combination of the above school zones have resulted in a large school zone area centred around the intersection of Highway 28 and Centre Avenue/50 Street. The City has expressed concerns over the presence of the long school zone and potential driver frustration resulting from the prolonged 30 km/h speed zone.

While AE is unfamiliar with the history behind the establishment of the above school zones, it is inferred that the school zones were established in an attempt to improve pedestrian safety in crossing these roadways. According to Section 2.0 of the TAC guideline, school and playground areas and zones should be used sparingly and should not be provided in an attempt to increase the safety of crossing the roadway. Other devices should be developed and implemented for these purposes.

AE recommends the removal of these school zones since excessive and prolonged school zones can lead to driver frustration and non-compliance. Before the removal of these school zones, the City should undertake a speed study to determine the current level of compliance within these school zones and review the collision history. If the majority of drivers are currently exceeding the posted 30 km/h limit, the compliance level is already low and the school zone should be removed immediately. If the driver compliance level is high, the City may choose to delay the removal of school zone to such a time when driver compliance is low. On the other hand, if the collision history indicates the presence of pedestrian related collisions on Highway 28 and Centre Avenue/ 50 Street, the City may choose to maintain the 30 km/h zones.

If the removal of the above school zones is not feasible, AE recommends re-designating the school zone to a school area. With a school area, motorists are warned to be cautious of the nearby school and the associated student traffic, but are not required to reduce their travel speeds.



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## **Summary and Recommendations**

AE was retained by the City to undertake a school zone safety analysis and complete the following tasks:

- Observe existing operations at a representative school site for safety
- Verify the existing school and playground areas and zones
- Determine the need to establish school/playground zone policy and guideline, and recommend the principles and best practices for establishing the guideline for the City.

Roadways adjacent to existing schools within Cold Lake were analyzed to verify the current school/playground areas and zones. Worksheets provided in TAC's School and Playground Areas and Zones: Guidelines for Application and Implementation were used for the verification and the results are summarized in Table 3.1 and Table 3.2. It should be noted that the worksheets from TAC are identical to the worksheets published in AT's guidelines for school and playground zones and areas, which is currently used by the City.

Figure 4.1 presents the results from the TAC school and playground zone worksheets for Cold Lake.

The existing signage and pavement markings at the school sites should be compared against the TAC or AT signage and pavement marking plans provided in Appendix C to ensure compliance with the standards.

Grand Centre Middle School was selected as the representative school site to observe the morning drop-off and afternoon pick-up periods. The school site was observed to identify any safety issues related to traffic operations and pedestrian movement. Overall, the morning and afternoon discharge periods operated well. AE did not observe anything that was considered unsafe.

In the 2010/2011 school year, Grand Centre Middle School was relocated south of Centre Avenue, to the building previously occupied by Grand Centre High School. The old location for Grand Centre Middle School, on 56 Street, currently is vacant and there are no immediate plans for redevelopment of the building. With the relocation of Grand Centre Middle School, student traffic across Highway 28 will shift from the pedestrian activated crosswalk at 51 Avenue to the intersection of Highway 28 and 50 Avenue. The pedestrian crosswalks at this intersection should be maintained to ensure maximum visibility

Grand Centre Elementary School is still located on the previously shared lot with Grand Centre Middle School. To improve safety for the elementary school, AE recommends that the crosswalk currently provided at the horizontal curve transition between 56 Street and 51 Avenue be realigned to provide a north-south crosswalk across 51 Avenue. Signage should be provided on the west side of 51 Avenue to warn southbound drivers of the crosswalk, if it is relocated.







DATE: APPROVED: APRIL 2011 SCALE: NTS DWG NO:

# TRANSPORTATION STUDY

FIGURE 4.1 - COLD LAKE NORTH TAC SCHOOL/PLAYGROUND ZONE WORKSHEET RESULTS

The City should consider the removal of the existing school zones at following locations:

- Highway 28, from 52 Avenue to 46 Avenue
- Service road parallel and west of Highway 28, from 52 Avenue to 50 Avenue
- Centre Avenue, from 57A Street to service road west of Highway 28.

There are no schools located on these roadways and unwarranted school zones can lead to driver frustration and non-compliance. Prior to the removal of these school zones, the City should undertake a speed study to determine the level of driver compliance within these school zones and review the collision history. If current driver compliance level is low, the City should remove the school zones immediately. If current driver compliance level is high, the City can delay the removal of the school zones. On the other hand, if the collision history indicates the presence of pedestrian related collisions on Highway 28 and Centre Avenue/50 Street, the City may choose to maintain the 30 km/h zones. If the removal of the above school zones is not feasible, AE recommends re-designating the school zone to a school area. With a school area, motorists are warned to be cautious of the nearby school and the associated student traffic, but are not required to reduce their travel speeds.

AE recommends that the City continue to follow the policies outlined in the AT guideline. An established guideline would help to promote uniformity in the establishment and signing and marking of schools and playground areas and zones within the City. It should be noted that the methodology established in the AT guideline is similar to the methodology established in the TAC guideline.



### **TECHNICAL MEMORANDUM**



### **Appendix A - School Zone Worksheets**



Table A.1 presents the School Zone Input Worksheet from the TAC Guideline

Table A.1
TAC School Zone Input Worksheet

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCRI	DESCRIPTION		WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)	SCORE (MPV * WF)	
		Eleme	ntary	1.00			
School <u>T</u> ype	40	Middle / Ju	ınior High	0.40		T=	
Genoor Type	40	High School		0.20		'-	
		Post-Seconda Unive	ry / College / ersity	0.00			
		Urban Land Use	Rural Land Use				
		Local		1.00			
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50		C=	
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00			
		Fully Traversable		1.00			
<u>F</u> encing	20	Partially Traversable		0.50		F=	
		Non-Trav	versable	0.10			
		Abuts Ro	oadway	1.00			
Property <u>L</u> ine Separation	10	Within 50	) metres	0.50		L=	
		Further than	50 metres	0.00			
		Main En Multiple Second		1.00			
School Entrance	5	Secondary	Entrance	0.60		E=	
		No	ne	0.00			
<u>S</u> idewalks	F	None or Non-	-School Site	1.00		S=	
<u>S</u> iucwaiks	5	Schoo	Side	0.60		<b>5=</b>	

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCRIPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)	SCORE (MPV * WF)
		Both Sides	0.00		
	то	TAL SCORE (sum of T,C,F,L,E and S)			

The appropriate school area or school zone designation is determined by comparing the total score from the worksheet against the School Zone Results Matrix, provided below in **Table A.2**.

Table A.2
TAC School Zone Results Matrix

TOTAL SCORE	AREA OR ZONE
0 – 40	Nothing
41 – 64	School Area
65 – 80	School Area or School Zone*
81 – 100	School Zone

<sup>\*</sup> Local conditions must be considered in detail in order to determine the appropriate treatment. Wherever possible, mitigation measures should be explored that would reduce the score so that marginal school zones can be avoided. The reasons for the final decision should always be documented.

#### **Table 2.1 School Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

**Nelson Heights Middle School - 5 Avenue** 

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)	SCC (MPV	
		Eleme	entary	1.00			
School <u>T</u> ype	40	Middle / J	unior High	0.40	0.40		
осноог <u>т</u> уре	40	High S	School	0.20			
		Post Secondary / 0	College / University	0.00		T=	16
		Urban Land Use	Rural Land Use				
		Local		1.00			
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50	0.50		
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00		C=	10
		Fully Tra	versable	1.00	1.00		
<u>F</u> encing	20	Partially Traversable		0.50			
		Non-Tra	versable	0.10		F=	20
		Abuts R	oadway	1.00			
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50	0.50		
		Further than	n 50 metres	0.00		L=	5
		Main Er Multiple Secon		1.00	1.00		
School Entrance	5	•	/ Entrance	0.60			
		No	ne	0.00		E=	5
		None or Non	-School Side	1.00			
<u>S</u> idewalks	5	School	ol Side	0.60			
		Both	Sides	0.00	0.00	S=	0
	TO	TAL SCORE (sum of	T,C,F,L,E and S)			5	6
						Schoo	l Area

#### Table 2.1 School Zone Input Worksheet

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

#### St. Dominic Elementary School - Lakeshore Drive

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		ORE ' * WF)
		Eleme	entary	1.00	1.00		
Sahaal Turna	40	Middle / J	unior High	0.40			
School <u>T</u> ype	40	High School		0.20			
		Post Secondary / 0	College / University	0.00		T=	40
		Urban Land Use	Rural Land Use				
		Local		1.00	1.00		
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50			
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00		C=	20
		Fully Tra	versable	1.00			
<u>F</u> encing 20	20	Partially Traversable		0.50			
		Non-Tra	versable	0.10	0.10	F=	2
		Abuts R	loadway	1.00			
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50	0.50		
		Further that	n 50 metres	0.00		L=	5
		Main Er Multiple Secon	ntrance / dary Entrances	1.00			
School Entrance	5	Secondary	/ Entrance	0.60	0.60		
		No	one	0.00		E=	3
		None or Non	-School Side	1.00			
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
		Both	Sides	0.00	0.00	S=	0
	T	OTAL SCORE (sum	of T,C,F,L,E and S)				70
							Area or ol Zone

#### Table 2.1 School Zone Input Worksheet

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

#### St. Dominic Elementary School - 7 Street

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		ORE * WF)
		Eleme	entary	1.00	1.00		
Cohool Time	40	Middle / J	unior High	0.40			
School <u>T</u> ype	40	High School		0.20			
		Post Secondary / 0	College / University	0.00		T=	40
		Urban Land Use	Rural Land Use				
		Local		1.00	1.00		
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50			
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00		C=	20
			versable	1.00			
<u>F</u> encing	20	Partially Traversable		0.50	0.50		
		Non-Tra	versable	0.10		F=	10
		Abuts R	loadway	1.00	1.00		
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50			
		Further that	n 50 metres	0.00		L=	10
		Main Er Multiple Secon	ntrance / dary Entrances	1.00	1.00		
School <u>E</u> ntrance	5	-	/ Entrance	0.60			
		No	ne	0.00		E=	5
		None or Non	-School Side	1.00			
<u>S</u> idewalks	5	None or Non	-School Side	0.60			
		Both Sides		0.00	0.00	S=	0
	T(	OTAL SCORE (sum	of T,C,F,L,E and S)		,	8	5

#### Table 2.1 School Zone Input Worksheet

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

Lakeland Christian Academy - 10 Street

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		ORE / * WF)
		Eleme	entary	1.00	1.00		
School <u>T</u> ype	40	Middle / J	unior High	0.40			
School <u>T</u> ype	40	High S	School	0.20			
		Post Secondary / 0	College / University	0.00		T=	40
		Urban Land Use	Rural Land Use				
		Local		1.00			
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50	0.50		
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00		C=	10
		Fully Traversable		1.00	1.00		
<u>F</u> encing	20	Partially Traversable		0.50			
		Non-Tra	versable	0.10		F=	20
		Abuts R	loadway	1.00	1.00		
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50			
		Further that	n 50 metres	0.00		L=	10
		Main Er Multiple Secon	ntrance / dary Entrances	1.00			
School <u>E</u> ntrance	5	Secondary	/ Entrance	0.60			
		No	ne	0.00	0.00	E=	0
		None or Non	-School Side	1.00			
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
		Both	Sides	0.00	0.00	S=	0
	T	OTAL SCORE (sum	of T,C,F,L,E and S)				80
							l Area or ol Zone

#### Table 2.1 School Zone Input Worksheet

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

#### Lakeland Christian Academy - 14 Avenue

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	RIPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		ORE * WF)
		Elem	entary	1.00	1.00		
Cohool Timo	40	Middle / J	unior High	0.40			
School <u>T</u> ype	40	High S	School	0.20			
		Post Secondary / 0	College / University	0.00		T=	40
		Urban Land Use	Rural Land Use				
		Local		1.00	1.00		
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50			
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00		C=	20
			aversable	1.00	1.00		
<u>F</u> encing	20	Partially Traversable		0.50			
		Non-Tra	versable	0.10		F=	20
		Abuts R	Roadway	1.00	1.00		
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50			
		Further tha	n 50 metres	0.00		L=	10
		Main Er Multiple Secon	ntrance / dary Entrances	1.00	1.00		
School <u>E</u> ntrance	5		y Entrance	0.60			
		No	one	0.00		E=	5
		None or Non	-School Side	1.00			
<u>S</u> idewalks	5	School	ol Side	0.60			
		Both	Sides	0.00	0.00	S=	0
	T	OTAL SCORE (sum	of T,C,F,L,E and S)	<u> </u>		9	95
		OTAL SCORE (SUM	or i,c,r,L,E and S)				ol Zone

#### Table 2.1 School Zone Input Worksheet

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

#### **Cold Lake Elementary - 16 Avenue**

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		ORE * WF)
		Eleme	entary	1.00	1.00		
School <u>T</u> ype	40	Middle / Junior High		0.40			
301001 <u>T</u> ype	40	High S	School	0.20			
		Post Secondary / 0	College / University	0.00		T=	40
		Urban Land Use	Rural Land Use				
		Local		1.00			
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50	0.50		
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00		C=	10
		Fully Traversable		1.00			
<u>F</u> encing	20	Partially T	raversable	0.50			
		Non-Tra	versable	0.10	0.10	F=	2
		Abuts R	oadway	1.00			
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50	0.50		
		Further than	n 50 metres	0.00		C= 1  F= 2  L= !	5
		Main En Multiple Secon	itrance / dary Entrances	1.00	1.00		
School <u>E</u> ntrance	5	Secondary	/ Entrance	0.60			
		No	ne	0.00		E=	5
		None or Non	-School Side	1.00			
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
		Both	Sides	0.00	0.00	S=	0
	T	OTAL SCORE (sum	of T,C,F,L,E and S)			6	2
						Schoo	ol Area

#### Table 2.1 School Zone Input Worksheet

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

#### **Cold Lake Elementary School - 8 Street**

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	EIPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		ORE * WF)
		Eleme	entary	1.00	1.00		
0.1.1.	40	Middle / J	unior High	0.40			
School <u>T</u> ype	40	High S	School	0.20			
		Post Secondary / 0	College / University	0.00		C= 2	40
		Urban Land Use	Rural Land Use				
		Local		1.00	1.00		
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50			
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00		C=	20
	Fully Traver		versable	1.00			
<u>F</u> encing	20	Partially T	Partially Traversable				
		Non-Tra	versable	0.10	0.10	F=	2
		Abuts R	loadway	1.00			
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50	0.50		
		Further that	n 50 metres	0.00		L=	5
		Main Er Multiple Secon	ntrance / dary Entrances	1.00			
School Entrance	5		y Entrance	0.60	1.00		
		No	one	0.00		E=	5
		None or Non	-School Side	1.00	1.00	-	
<u>S</u> idewalks	5	School Side		0.60			
		Both	Sides	0.00		S=	5
		TOTAL SCORE (SI	um of T,C,F,L,E and	S)			7
				_		School School	Area or I Zone

#### **Table 2.1 School Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

#### École Voyageur - 49 Street

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)	SCC (MPV	ORE * WF)
		Eleme	entary	1.00	1.00		
School <u>T</u> ype	40	Middle / J	unior High	0.40			
301001 <u>1</u> ype	40	High S	School	0.20			
		Post Secondary / 0	College / University	0.00		T=	40
		Urban Land Use	Rural Land Use			-	
		Local		1.00	1.00		
Road <u>C</u> lassification	20	Minor Collector	Local	0.75			
		Collector Major Collector Minor Arterial	Collector Arterial	0.50 0.25			
		Major Arterial Expressway	Freeway	0.00		C=	20
		Fully Traversable		1.00	1.00		
<u>F</u> encing	20	Partially Traversable		0.50			
		Non-Tra	versable	0.10		F=	20
		Abuts R	loadway	1.00			
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50			
		Further than	n 50 metres	0.00	0.00	L=	0
		Main Er Multiple Secon		1.00	1.00		
School Entrance	5	Secondary	/ Entrance	0.60			
		No	ne	0.00		E=	5
		None or Non	-School Side	1.00	_		
<u>S</u> idewalks	5	Schoo	ol Side	0.60	0.60		
		Both	Sides	0.00		S=	3
	ТО	TAL SCORE (sum of	T,C,F,L,E and S)			8	8
						Schoo	l Zone

#### **Table 2.1 School Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

**Trinity Christian School - 51 Avenue** 

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)	SCC (MPV	
		Eleme	entary	1.00	1.00		
School <u>T</u> ype	40	Middle / Junior High		0.40			
осноог <u>т</u> уре	40	High S	School	0.20			
		Post Secondary / 0	College / University	0.00		T=	40
		Urban Land Use	Rural Land Use				
		Local		1.00	1.00		
Road <u>C</u> lassification	20	Minor Collector	Local	0.75			
_		Collector Major Collector Minor Arterial	Collector Arterial	0.50 0.25			
		Major Arterial Expressway	Freeway	0.00		C=	20
		Fully Traversable		1.00			
<u>F</u> encing	20	Partially Traversable		0.50			
		Non-Tra	versable	0.10	0.10	F=	2
		Abuts R	Roadway	1.00	1.00		
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50			
		Further that	n 50 metres	0.00		L=	10
			ntrance / dary Entrances	1.00	1.00		
School <u>E</u> ntrance	5	Secondary	y Entrance	0.60			
		No	one	0.00		E=	5
		None or Non	-School Side	1.00			
<u>S</u> idewalks	5	School Side		0.60	0.60		
		Both	Sides	0.00		S=	3
	ТО	TAL SCORE (sum of	T,C,F,L,E and S)			8	0
						School Schoo	

#### **Table 2.1 School Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

#### **Trinity Christian School - 61 Street**

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)	SCC (MPV	ORE * WF)
		Eleme	entary	1.00	1.00		
School <u>T</u> ype	40	Middle / J	unior High	0.40			
осноог <u>т</u> уре	40	High S	School	0.20		C= 2	
		Post Secondary / 0	College / University	0.00		T=	40
		Urban Land Use	Rural Land Use			·	
		Local		1.00	1.00		
Road <u>C</u> lassification	20	Minor Collector	Local	0.75			
		Collector Major Collector Minor Arterial	Collector Arterial	0.50 0.25			
		Major Arterial Expressway	Freeway	0.00		C=	20
			versable	1.00			
<u>F</u> encing	20	Partially T	raversable	0.50			
		Non-Tra	versable	0.10	0.10	F=	2
		Abuts R	loadway	1.00			
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50	0.50		
		Further that	n 50 metres	0.00		L=	5
		Main Er Multiple Secon		1.00		•	
School <u>E</u> ntrance	5	Secondary	/ Entrance	0.60	0.60		
		No	ne	0.00		E=	3
		None or Non	-School Side	1.00			
<u>S</u> idewalks	5	Schoo	ol Side	0.60	0.60		
		Both	Sides	0.00		S=	3
	ТО	TAL SCORE (sum of	T,C,F,L,E and S)			7	3
						School Schoo	

#### **Table 2.1 School Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

#### **Grand Centre Middle School - 57 Street**

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)	SCC (MPV	
		Eleme	entary	1.00			
School <u>T</u> ype	40	Middle / J	unior High	0.40	0.40		
- Conoor <u>- 1</u> ypc	40	High School		0.20		C= 1	
		Post Secondary / 0	College / University	0.00		T=	16
		Urban Land Use	Rural Land Use			•	
		Local		1.00			
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50	0.50		
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00		C=	10
			versable	1.00			
<u>F</u> encing	20	Partially T	raversable	0.50			
		Non-Tra	versable	0.10	0.10	F=	2
		Abuts R	loadway	1.00			
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50			
		Further that	n 50 metres	0.00	0.00	L=	0
		Main Er Multiple Secon	trance / dary Entrances	1.00			
School <u>E</u> ntrance	5	Secondary	/ Entrance	0.60	0.60		
		No	ne	0.00		E=	3
		None or Non	-School Side	1.00	1.00		
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
		Both	Sides	0.00		S=	5
	то	TAL SCORE (sum of	T,C,F,L,E and S)			3	6
						Noth	ning

#### **Table 2.1 School Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

#### **Grand Centre Middle School - 52 Avenue**

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)	SCC (MPV	
		Eleme	entary	1.00			
School <u>T</u> ype	40	Middle / J	unior High	0.40	0.40		
301001 <u>1</u> ype	40	High S	School	0.20			
		Post Secondary / 0	College / University	0.00		T=	16
		Urban Land Use	Rural Land Use			<u>-</u>	
		Local		1.00			
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50	0.50		
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00		C=	10
		Fully Tra	versable	1.00		•	
<u>F</u> encing	20	Partially T	raversable	0.50			
		Non-Tra	versable	0.10	0.10	F=	2
		Abuts R	loadway	1.00		•	
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50			
		Further than	n 50 metres	0.00	0.00	L=	0
		Main Er Multiple Secon	trance / dary Entrances	1.00			
School Entrance	5	Secondary	/ Entrance	0.60		_	
		No	ne	0.00	0.00	E=	0
		None or Non	-School Side	1.00	1.00		
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
		Both	Sides	0.00		S=	5
	ТО	TAL SCORE (sum of	T,C,F,L,E and S)			3	3
						Noth	ing

#### **Table 2.1 School Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

#### **Grand Centre Middle School - 56 Street**

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		ORE / * WF)
		Eleme	entary	1.00			
School <u>T</u> ype	40	Middle / J	unior High	0.40	0.40		
301001 <u>1</u> ype	40	High S	School	0.20			
		Post Secondary / 0	College / University	0.00		C= 2	16
		Urban Land Use	Rural Land Use				
		Local		1.00	1.00		
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50			
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00		C=	20
		Fully Tra	versable	1.00			
<u>F</u> encing	20	Partially T	raversable	0.50	0.50		
		Non-Tra	versable	0.10		F=	10
		Abuts R	loadway	1.00	1.00		
Property <u>Line</u> Separation	10	Within 5	0 metres	0.50			
		Further than	n 50 metres	0.00		L=	10
		Main Er Multiple Secon	trance / dary Entrances	1.00	1.00		
School Entrance	5	Secondary	/ Entrance	0.60			
		No	one	0.00		E=	5
		None or Non	-School Side	1.00			
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
		Both	Sides	0.00	0.00	S=	0
	TO	TAL SCORE (sum of	T,C,F,L,E and S)				61
						Scho	ol Area

Cold Lake Transportation Study 2010-3050 June 14, 2010 Project Name: Project No: Date:

**Table 2.1 School Zone Input Worksheet**Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

**Grand Centre Elementary School - 57 Street** 

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		ORE ' * WF)
		Eleme	entary	1.00	1.00		
School <u>T</u> ype	40	Middle / J	unior High	0.40			
301001 <u>1</u> ype	40	High S	School	0.20			
		Post Secondary / College / University		0.00		C= 1	40
		Urban Land Use	Rural Land Use				
		Local		1.00			
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50	0.50		
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00		C=	10
			versable	1.00			
<u>F</u> encing	20	Partially T	raversable	0.50			
		Non-Tra	versable	0.10	0.10	F=	2
		Abuts R	oadway	1.00			
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50			
		Further than	n 50 metres	0.00	0.00	L=	0
		Main Er Multiple Secon	trance / dary Entrances	1.00			
School Entrance	5	Secondary	/ Entrance	0.60	0.60		
		No	ne	0.00		E=	3
		None or Non	-School Side	1.00	1.00		
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
		Both	Sides	0.00		S=	5
	то	TAL SCORE (sum of	T,C,F,L,E and S)			(	60
						School	ol Area

#### **Table 2.1 School Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

**Grand Centre Elementary School - 50 Avenue** 

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)	SCC (MPV	
		Eleme	entary	1.00	1.00		
School <u>T</u> ype	40	Middle / J	unior High	0.40			
301001 <u>T</u> ype	40	High S	School	0.20		C= 1	
		Post Secondary / 0	College / University	0.00		T=	40
		Urban Land Use	Rural Land Use				
		Local		1.00			
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50	0.50		
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00		C=	10
			versable	1.00			
<u>F</u> encing	20	Partially T	raversable	0.50			
		Non-Tra	versable	0.10	0.10	F=	2
		Abuts R	loadway	1.00			
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50	0.50		
		Further than	n 50 metres	0.00		L=	5
		Main Er Multiple Secon	trance / dary Entrances	1.00	1.00		
School Entrance	5	•	/ Entrance	0.60			
		No	one	0.00		E=	5
		None or Non	-School Side	1.00			
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
		Both	Sides	0.00	0.00	S=	0
	то	TAL SCORE (sum of	T,C,F,L,E and S)		,	6	2
						Schoo	l Area

#### **Table 2.1 School Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

**Grand Centre Elementary School - 51 Avenue** 

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)	SCC (MPV	
		Eleme	entary	1.00	1.00		
School <u>T</u> ype	40	Middle / J	unior High	0.40			
3c11001 <u>1</u> ype	40	High S	School	0.20			
		Post Secondary / 0	College / University	0.00		C= 2	40
		Urban Land Use	Rural Land Use			-	
		Local		1.00	1.00		
Road <u>C</u> lassification	20	Minor Collector	Local	0.75			
		Collector Major Collector Minor Arterial	Collector Arterial	0.50 0.25			
		Major Arterial Expressway	Freeway	0.00		C=	20
		Fully Tra	versable	1.00			
<u>F</u> encing	20	Partially T	raversable	0.50	0.50		
		Non-Tra	versable	0.10		F=	10
		Abuts R	loadway	1.00	1.00		
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50			
		Further that	n 50 metres	0.00		L=	10
		Main Er Multiple Secon	trance / dary Entrances	1.00	1.00		
School Entrance	5	Secondary	/ Entrance	0.60			
		No	ne	0.00		E=	5
		None or Non	-School Side	1.00			
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
		Both	Sides	0.00	0.00	S=	0
	TO	TAL SCORE (sum of	T,C,F,L,E and S)			8	5
						Schoo	l Zone

#### **Table 2.1 School Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

**Grand Centre High School - 48 Avenue** 

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		ORE * WF)
		Eleme	entary	1.00			
School <u>T</u> ype	40	Middle / J	unior High	0.40			
осноог <u>т</u> уре	40	High S	School	0.20	0.20	T=	
		Post Secondary / 0	College / University	0.00		T=	8
		Urban Land Use	Rural Land Use				
		Local		1.00	1.00		
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50			
		Major Collector Minor Arterial	Arterial	0.30			
		Major Arterial Expressway	Freeway	0.00		C=	20
		Fully Tra	versable	1.00	1.00		
<u>F</u> encing	20	Partially T	raversable	0.50			
		Non-Tra	versable	0.10		F=	20
		Abuts R	oadway	1.00			
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50			
		Further than	n 50 metres	0.00	0.00	L=	0
		Main Er Multiple Secon		1.00	1.00		
School Entrance	5	Secondary	/ Entrance	0.60			
		No	ne	0.00		E=	5
		None or Non	-School Side	1.00	1.00		
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
		Both	Sides	0.00		S=	5
	TO	TAL SCORE (sum of	T,C,F,L,E and S)			5	8
						Schoo	l Area

#### **Table 2.1 School Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

**Grand Centre High School - 57 Street** 

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		ORE ' * WF)
		Eleme	entary	1.00			
School <u>T</u> ype	40	Middle / J	unior High	0.40		C= 2	
301001 <u>1</u> ype	40	High S	School	0.20	0.20		
		Post Secondary / 0	College / University	0.00			8
		Urban Land Use	Rural Land Use				
		Local		1.00	1.00		
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50			
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00		C=	20
			versable	1.00	1.00		
<u>F</u> encing	20	Partially T	raversable	0.50			
		Non-Tra	versable	0.10		F=	20
		Abuts R	loadway	1.00			
Property <u>Line</u> Separation	10	Within 5	0 metres	0.50	0.50		
		Further that	n 50 metres	0.00		L=	5
		Main Er Multiple Secon	trance / dary Entrances	1.00	1.00		
School Entrance	5	Secondary	/ Entrance	0.60			
		No	one	0.00		E=	5
		None or Non	-School Side	1.00	1.00		
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
		Both	Sides	0.00		S=	5
	TO	TAL SCORE (sum of	T,C,F,L,E and S)			(	63
						School	ol Area

#### **Table 2.1 School Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

**Grand Centre High School - 47 Avenue** 

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		ORE * WF)
		Eleme	entary	1.00			
School <u>T</u> ype	40	Middle / J	unior High	0.40			
осноог <u>т</u> уре	40	High S	School	0.20	0.20	C= 2	
		Post Secondary / 0	College / University	0.00		T=	8
		Urban Land Use	Rural Land Use				
		Local		1.00	1.00		
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50			
		Major Collector Minor Arterial	Arterial	0.30			
		Major Arterial Expressway	Freeway	0.00		C=	20
		Fully Tra	versable	1.00	1.00		
<u>F</u> encing	20	Partially T	raversable	0.50			
		Non-Tra	versable	0.10		F=	20
		Abuts R	oadway	1.00			
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50			
		Further that	n 50 metres	0.00	0.00	L=	0
		Main Er Multiple Secon		1.00	1.00		
School Entrance	5	Secondary	/ Entrance	0.60			
		No	ne	0.00		E=	5
		None or Non	-School Side	1.00	1.00		
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
		Both	Sides	0.00		S=	5
	то	TAL SCORE (sum of	T,C,F,L,E and S)			5	8
						Schoo	l Area

#### **Table 2.1 School Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

**Assumption School - 48 Avenue** 

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)	SCC (MPV	ORE * WF)
		Eleme	entary	1.00			
School <u>T</u> ype	40	Middle / J	unior High	0.40	0.40		
	40	High S	School	0.20			
		Post Secondary / 0	College / University	0.00		C= 2	16
		Urban Land Use	Rural Land Use			<u>-</u>	
		Local		1.00	1.00		
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50			
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00		C=	20
		Fully Tra	versable	1.00	1.00		
<u>F</u> encing	20	Partially T	raversable	0.50			
		Non-Tra	versable	0.10		F=	20
		Abuts R	oadway	1.00			
Property <u>Line</u> Separation	10	Within 5	0 metres	0.50	0.50		
		Further than	n 50 metres	0.00		L=	5
		Main En Multiple Secon		1.00	1.00		
School <u>E</u> ntrance	5	Secondary	/ Entrance	0.60			
		No	ne	0.00		E=	5
		None or Non	-School Side	1.00			
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
		Both	Sides	0.00	0.00	S=	0
	TO	TAL SCORE (sum of	T,C,F,L,E and S)			6	6
						School Schoo	

#### **Table 2.1 School Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

**Assumption School - 47 Avenue** 

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	RIPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		ORE ' * WF)
		Eleme	entary	1.00			
School <u>T</u> ype	40	Middle / J	unior High	0.40	0.40		
Зспоот <u>т</u> уре	40	High S	School	0.20			
		Post Secondary / 0	College / University	0.00		T=	16
		Urban Land Use	Rural Land Use				
		Local		1.00	1.00		
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50			
		Major Collector Minor Arterial	Arterial	0.25		_	
		Major Arterial Expressway	Freeway	0.00		C=	20
		Fully Tra	aversable	1.00	1.00		
<u>F</u> encing	20	Partially T	raversable	0.50			
		Non-Tra	versable	0.10		F=	20
		Abuts R	Roadway	1.00			
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50			
		Further that	n 50 metres	0.00	0.00	L=	0
			ntrance / dary Entrances	1.00	1.00		
School Entrance	5		y Entrance	0.60			
		No	one	0.00		E=	5
		None or Non	-School Side	1.00	1.00		
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
		Both	Sides	0.00		S=	5
	TO	TAL SCORE (sum of	T,C,F,L,E and S)			(	66
							Area or ol Zone

#### **Table 2.1 School Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

#### **Holy Cross Elementary School - 49 Street**

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)	SCC (MPV	ORE * WF)
		Eleme	entary	1.00	1.00		
School <u>T</u> ype	40	Middle / J	unior High	0.40			
осноог <u>т</u> уре	40	High S	School	0.20			
		Post Secondary / 0	College / University	0.00		T=	40
		Urban Land Use	Rural Land Use				
		Local		1.00	1.00		
Road <u>C</u> lassification	20	Minor Collector	Local	0.75			
_		Collector Major Collector Minor Arterial	Collector Arterial	0.50 0.25			
		Major Arterial Expressway	Freeway	0.00		C=	20
		Fully Traversable		1.00	1.00		
<u>F</u> encing	20	Partially Traversable		0.50			
		Non-Tra	versable	0.10		F=	20
		Abuts R	loadway	1.00			
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50	0.50	T=	
		Further that	n 50 metres	0.00			5
		Main Er Multiple Secon		1.00	1.00		
School Entrance	5	Secondary	/ Entrance	0.60		_	
		No	ne	0.00		C=  E=  S=	5
	_	None or Non	-School Side	1.00			
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
		Both	Sides	0.00	0.00	S=	0
	TO	TAL SCORE (sum of	T,C,F,L,E and S)			9	0
						Schoo	l Zone

#### **Table 2.1 School Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

#### **MacKenzie School - Hickory Street**

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)	SCC (MPV	
		Eleme	entary	1.00	1.00		
School <u>T</u> ype	40	Middle / Junior High High School		0.40			
Зспоот <u>т</u> уре	40			0.20			
		Post Secondary / 0	College / University	0.00		T=	40
		Urban Land Use	Rural Land Use			-	
		Local		1.00	1.00		
Road <u>C</u> lassification	20	Minor Collector	Local	0.75			
-		Collector Major Collector Minor Arterial	Collector Arterial	0.50 0.25			
		Major Arterial Expressway	Freeway	0.00		C=	20
		Fully Traversable		1.00	1.00		
<u>F</u> encing	20	Partially Traversable		0.50			
		Non-Tra	versable	0.10		F=	20
		Abuts R	oadway	1.00	1.00		
Property <u>L</u> ine Separation	20   Partially Traversable   0.50     Non-Traversable   0.10     Abuts Roadway   1.00     ne						
		Further than	n 50 metres	0.00		L=	10
		Main Er Multiple Secon	trance / dary Entrances	1.00	1.00		
School Entrance	5	Secondary	/ Entrance	0.60			
		No	ne	0.00		E=	5
		None or Non	-School Side	1.00			
<u>S</u> idewalks	5	Schoo	ol Side	0.60	0.60		
		Both	Sides	0.00		S=	3
	TO	TAL SCORE (sum of	T,C,F,L,E and S)			9	8
						Schoo	Zone

#### Table 2.1 School Zone Input Worksheet

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

#### **RA Reynolds School - Queensway**

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		ORE * WF)
		Eleme	entary	1.00	1.00		
Cohool Tymo	40	Middle / J	unior High	0.40			
School <u>T</u> ype	40	High S	School	0.20			
		Post Secondary / 0	College / University	0.00		T=	40
		Urban Land Use	Rural Land Use				
		Local		1.00			
Road <u>C</u> lassification	20	Minor Collector Collector	Collector tor Arterial	0.75 0.50	0.50		
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00		C=	10
		Fully Traversable		1.00			
<u>F</u> encing	20	Partially T	raversable	0.50			
		Non-Tra	versable	0.10	0.10	F=	2
		Abuts R	Roadway	1.00	1.00		
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50		(MPV *   T=	
		Further that	n 50 metres	0.00			10
		Main Er Multiple Secon	ntrance / dary Entrances	1.00	1.00	<u> </u>	
School Entrance	5		y Entrance	0.60		T=  C=  L=  S=	
		No	one	0.00		E=	5
		None or Non	-School Side	1.00	1.00		
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
		Both	Sides	0.00		S=	5
	T	OTAL SCORE (sum	of T,C,F,L,E and S)			7	<b>'</b> 2
					•		Area or I Zone

#### Table 2.1 School Zone Input Worksheet

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

#### **RA Reynolds School - Spruce Crescent**

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		ORE / * WF)
		Eleme	entary	1.00	1.00		
Cohool Time	40	Middle / Junior High		0.40			
School <u>T</u> ype	40	High S	High School  Post Secondary / College / University				
		Post Secondary / 0				T=	40
		Urban Land Use	Rural Land Use				
		Local		1.00	1.00		
oad <u>C</u> lassification 20	20	Minor Collector Collector	Local Collector	0.75 0.50			
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00		C=	20
		Fully Traversable		1.00			
<u>F</u> encing	20	Partially Traversable		0.50			
		Non-Tra	versable	0.10	0.10	F=	2
		Abuts R	loadway	1.00	1.00		
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50		T=  C=  L=  E=  S=	
		Further that	n 50 metres	0.00			10
			ntrance / dary Entrances	1.00			
School Entrance	5	Secondary	/ Entrance	0.60	0.60		
		No	ne	0.00		E=	3
		None or Non	-School Side	1.00	1.00		
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
	_	Both	Sides	0.00		S=	5
_	T	OTAL SCORE (sum	of T,C,F,L,E and S)				30
							l Area or ol Zone

#### **Table 2.1 School Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

#### **Ecole Voyager - Birch Avenue**

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		ORE * WF)
		Eleme	entary	1.00	1.00		
Cobool Tymo	40	Middle / J	unior High	0.40			
School <u>T</u> ype	40	High S	School	0.20			
		Post Secondary / 0	College / University	0.00		T=	40
		Urban Land Use	Rural Land Use				
		Local		1.00	1.00		
Road <u>C</u> lassification	20	Minor Collector Collector	Local Collector	0.75 0.50			
		Major Collector Minor Arterial	Arterial	0.25			
		Major Arterial Expressway	Freeway	0.00		C=	20
		Fully Traversable		1.00	1.00	'	
<u>F</u> encing	20	Partially T	raversable	0.50			
		Non-Tra	versable	0.10		F=	20
		Abuts R	oadway	1.00			
Property <u>L</u> ine Separation	10	Within 5	0 metres	0.50	0.50		
·		Further that	n 50 metres	0.00		(MPV   T=   T=   T=   T=   T=   T=   T=   T	5
		Main Er Multiple Secon	ntrance / dary Entrances	1.00	1.00		
School <u>E</u> ntrance	5	Secondary	y Entrance	0.60			
		No	one	0.00		E=	5
		None or Non	-School Side	1.00	1.00		
<u>S</u> idewalks	5	Schoo	ol Side	0.60			
		Both	Sides	0.00		S=	5
	T	OTAL SCORE (sum	of T,C,F,L,E and S)			9	5
						Schoo	l Zone

### **TECHNICAL MEMORANDUM**

# В

## **Appendix B - Playground Zone Worksheets**



 Table B.1 presents the Playground Zone Input Worksheet from the TAC Guideline

Table B.1
TAC Playground Zone Input Worksheet

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCR	IPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)	SCORE (MPV * WF)	
		Frontage	Playground Capacity (number of children)	n/a			
			16 or more	1.00			
Playground <u>T</u> ype	ad 20	40		5 to 15	0.75		T=
Flayground Type	40	≥ 50 m	1 to 4	0.40		1=	
			No play equipment: sport field or open field only	0.20			
		< 50 m	Any Facilities	0.20			
	20	Urban Land Use	Rural Land Use				
		Local		1.00			
Road		Minor Collector	Local	0.75		C=	
<u>C</u> lassification	20	Collector	Collector	0.50		<b>C</b> =	
		Major Collector/ Minor Arterial	Arterial	0.25			
		Major Arterial/ Expressway	Freeway	0.00			
		Fully Tra	versable	1.00			
<u>F</u> encing	20	Partially Tr	aversable	0.50		F=	
		Non-Traversable	e/Indoor Facility	0.10			
		Abuts Ro	oadway	1.00			
Property <u>L</u> ine Separation	10	Within 50	) metres	0.50		L=	
		Further than	50 metres	0.00			
Playground <u>E</u> ntrance	5	Main Entrance / M Entra	ultiple Secondary nces	1.00		E=	

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DESCRIPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)	SCORE (MPV * WF)
		Secondary Entrance	0.60		
		None	0.00		
		None (or Non-Playground Side)	1.00		
<u>S</u> idewalks	5	Playground Side	0.40		S=
		Both Sides	0.00		
	то	TAL SCORE (sum of T,C,F,L,E and S)			

The appropriate playground area or playground zone designation is determined by comparing the total score calculated in the above mentioned worksheet against the Playground Zone Results Matrix, provided below in **Table B.2**.

Table B.2
TAC Playground Zone Results Matrix

TOTAL SCORE	AREA OR ZONE				
0 – 40	Nothing				
41 – 80	Playground Area				
81 – 100	Playground Zone				

#### **Table 2.3 Playground Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

#### St. Dominic Elementary School - 7 Street

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DES	SCRIPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)				
		Frontage	Playground Capacity (number of children)						
					16 or more	1.00			
Playground <b>T</b> ype	40	≥ 50 m	5 to 15	0.75					
Playground <u>T</u> ype	40	2 50 III	1 to 4	0.40		(MPV * WI			
			No play equipment: sport field or open field only	0.20					
		< 50 m	Any Facilities	0.20	0.20		8		
		Urban Land Use	Rural Land Use						
		Local		1.00	1.00				
Road Classification	20	Minor Collector	Local	0.75					
Road <u>C</u> lassifcation 20	20	Collector	Collector	0.50					
		Major Collector/ Minor Arterial	Arterial	0.25					
		Major Arterial/ Expressway	Freeway	0.00		C=	20		
		Fully Traversable		1.00					
<u>F</u> encing	20	Partial	ly Traversable	0.50					
		Non-Travers	sable/Indoor Facility	0.10	0.10	F=	2		
		Abu	ts Roadway	1.00	1.00				
Property <u>L</u> ine Separation	10	With	in 50 metres	0.50		T=			
		Further	than 50 metres	0.00			10		
		Main Entrance / Mu	Itiple Secondary Entrances	1.00					
Playground <u>E</u> ntrance	5	Secon	dary Entrance	0.60					
			None	0.00	0.00	E=	0		
		None (or No	n-Playground Side)	1.00	1.00				
<u>S</u> idewalks	5	Play	ground Side	0.40					
		Both Sides		0.00		S=	5		
		TOTAL SCORE (S	sum of T,C,F,L,E and S)			4	5		

#### **Table 2.3 Playground Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

#### **Grand Centre Elementary School - 57 Street**

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DES	SCRIPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		ORE ' * WF)
		Frontage	Playground Capacity (number of children)				
			16 or more	1.00	1.00		
Diagrams and Torse	40	> 50	5 to 15	0.75			
Playground <u>T</u> ype	40	≥ 50 m	1 to 4	0.40		C=	
			No play equipment: sport field or open field only	0.20			
		< 50 m	Any Facilities	0.20		T=	40
		Urban Land Use	Rural Land Use			C=	
Road <u>C</u> lassifcation 20		Local		1.00			
	20	Minor Collector	Local	0.75			
Road <u>C</u> lassification	20	Collector	Collector	0.50	0.50		
		Major Collector/ Minor Arterial	Arterial	0.25			
		Major Arterial/ Expressway	Freeway	0.00		C=	10
			Traversable	1.00			
<u>F</u> encing	20	Partial	ly Traversable	0.50			
		Non-Travers	sable/Indoor Facility	0.10	0.10	F=	2
		Abu	ts Roadway	1.00			
Property <u>L</u> ine Separation	10	Withi	in 50 metres	0.50		C= 1	
		Further	than 50 metres	0.00	0.00		0
		Main Entrance / Mu	Itiple Secondary Entrances	1.00			
Playground <u>E</u> ntrance	5	Secon	dary Entrance	0.60			
			None	0.00	0.00	E=	0
		None (or No	n-Playground Side)	1.00	1.00		
<u>S</u> idewalks	5	Playground Side		0.40			
		В	oth Sides	0.00		S=	5
		TOTAL SCORE (S	um of T,C,F,L,E and S)				57

Project Name: Cold Lake Transportation Study
Project No: 2010-3050
Date: June 14, 2010

## **Table 2.3 Playground Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

## **Grand Centre Elementary School - 50 Avenue**

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DES	SCRIPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		ORE * WF)
		Frontage	Playground Capacity (number of children)  16 or more  5 to 15  1 to 4  No play equipment: sport field or open field only Any Facilities  Rural Land Use  Local  Collector  Arterial  Freeway  Traversable  ly Traversable sable/Indoor Facility ts Roadway in 50 metres than 50 metres ltiple Secondary Entrances dary Entrance None In-Playground Side)				
				1.00			
Dlayground Type	40	≥ 50 m	5 to 15	0.75			
Playground <u>I</u> ype	40	2 50 III	1 to 4	0.40			
				0.20			
		< 50 m	Any Facilities	0.20	0.20	T=	8
		Urban Land Use	Rural Land Use				
	Fencing 20  roperty Line Separation 10	Local		1.00			
Road <u>C</u> lassifcation	20	Minor Collector	Local	0.75			
	20	Collector	Collector	0.50	0.50		
	Urban Land Use	Arterial	0.25		_		
			ial Arterial 0.25 ial/ Freeway 0.00			C=	10
		Fully	Traversable	1.00			
<u>F</u> encing	20	Partial	ly Traversable	0.50			
		Non-Travers	sable/Indoor Facility	0.10	0.10	F=	2
		Abu	ts Roadway	1.00	1.00		
	10	With	in 50 metres	0.50			
		Further	than 50 metres	0.00		L=	10
		Main Entrance / Mu	Itiple Secondary Entrances	1.00	1.00		
Playground <u>E</u> ntrance	5	Secon	dary Entrance	0.60		_	
			None	0.00		E=	5
		None (or No	n-Playground Side)	1.00			
<u>S</u> idewalks	5	Play	ground Side	0.40			
		В	oth Sides	0.00	0.00	S=	0
		TOTAL SCORE (S	um of T,C,F,L,E and S)			3	5

Project Name: Project No: Date: Cold Lake Transportation Study 2010-3050 June 14, 2010

**Table 2.3 Playground Zone Input Worksheet**Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

## MacKenzie School - Oak Drive

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DES		WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)	(MPV * W		
		Frontage	Playground Capacity (number of children)					
			16 or more	1.00				
Discouración d' Tropa	40	> 50	5 to 15	0.75				
Playground <u>I</u> ype	40	≥ 50 III	1 to 4	0.40				
			No play equipment: sport field or open field only	0.20				
		< 50 m	Any Facilities	0.20	0.20	T=	8	
		Urban Land Use	Rural Land Use					
	Point Value (MPV)   Point Value (MPV)   Point Value (MPV)   Frontage   Playground Capar (number of children of children of partial		1.00	1.00				
Dood Classifestian	20	Minor Collector	Local	0.75				
Road <u>C</u> lassification	layground Type  40 ≥ 50 m    Viban Land Use	Collector	Collector	0.50				
		Arterial	0.25					
			Freeway	0.00		C=	20	
			Traversable	1.00				
<u>F</u> encing	20	Partial	ly Traversable	0.50	0.50			
		Non-Travers	sable/Indoor Facility	0.10		F=	10	
		Abu	ts Roadway	1.00	1.00			
Property <u>L</u> ine Separation	10	With	in 50 metres	0.50				
		Further	than 50 metres	0.00		L=	10	
		Main Entrance / Mu	Itiple Secondary Entrances	1.00				
Playground <u>E</u> ntrance	5	Secon	dary Entrance	0.60				
			None	0.00	0.00	E=	0	
		None (or No	on-Playground Side)	1.00				
<u>S</u> idewalks	5	Play	ground Side	0.40	0.40			
		В	oth Sides	0.00		S=	2	
		TOTAL SCORE (S	sum of T,C,F,L,E and S)				50	

Project Name: Cold Lake Transportation Study
Project No: 2010-3050
Date: June 14, 2010

## **Table 2.3 Playground Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

## **RA Reynolds School - Queensway**

INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DES	SCRIPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)		
		Frontage	Playground Capacity (number of children)			-	
			16 or more	1.00	1.00	T=  C=  L=  S=  70	
Dlayground Type	40	> E0 m	5 to 15	0.75			
Playground <u>T</u> ype	40	2 50 III	1 to 4	0.40			
			No play equipment: sport field or open field only	0.20			
		< 50 m	Any Facilities	0.20		T=	40
		Urban Land Use	Rural Land Use				
		Local		1.00			
Dood Classification	POINT VALUE (MPV)   POINT VALUE (MPV)   Frontage	Minor Collector	Local	0.75			
Road <u>C</u> lassification		Collector	0.50	0.50			
			Arterial	0.25		### Comparison of Care Comparison of Care Comparison of Care Care Comparison of Care Care Care Care Care Care Care Care	
			Freeway	0.00		C=	10
			Traversable	1.00			
<u>F</u> encing	20	Partial	ly Traversable	0.50			
		Non-Travers	sable/Indoor Facility	0.10	0.10	F=	2
		Abu	ts Roadway	1.00	1.00		
Property <u>L</u> ine Separation	10	With	in 50 metres	0.50			
		Further	than 50 metres	0.00		L=	10
		Main Entrance / Mu	Itiple Secondary Entrances	1.00			
Playground <u>E</u> ntrance	5	Secon	dary Entrance	0.60	0.60		
			None	0.00		E=	3
		None (or No	n-Playground Side)	1.00	1.00		-
<u>S</u> idewalks	5	Play	ground Side	0.40			
		В	oth Sides	0.00		S=	5
		TOTAL SCORE (S	sum of T,C,F,L,E and S)			7	0
						Playgrou	ınd Area

Project Name: Cold Lake Transportation Study
Project No: 2010-3050
Date: June 14, 2010

## **Table 2.3 Playground Zone Input Worksheet**

Source: TAC School & Playground Areas & Zones: Guidelines for Application and Implementation

## RA Reynolds School - Spruce Cres.

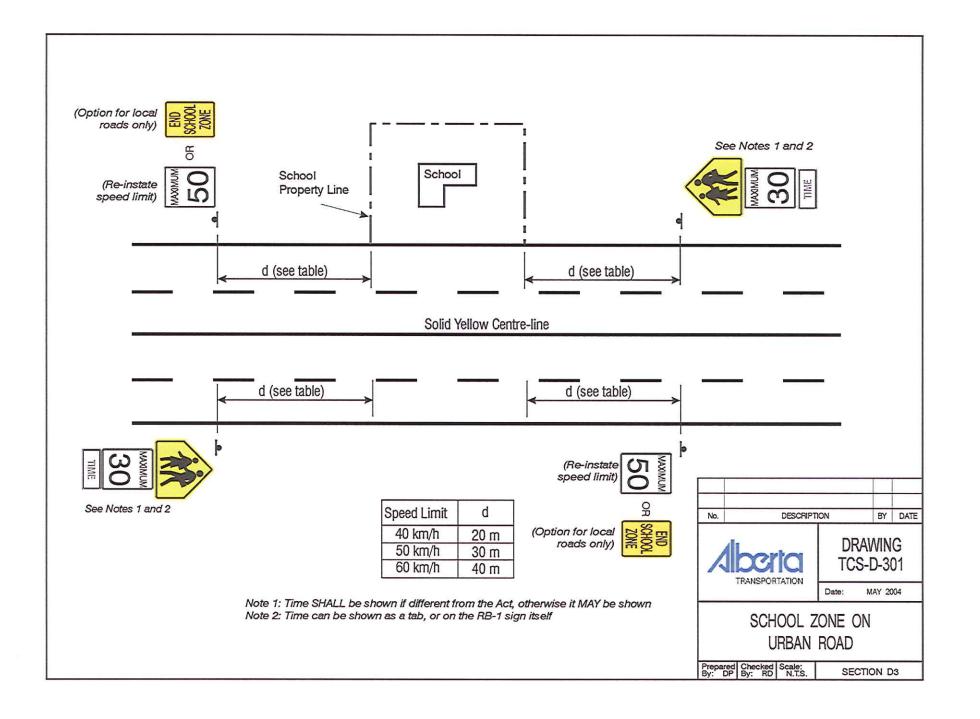
INSTALLATION CRITERION	MAXIMUM POINT VALUE (MPV)	DES	SCRIPTION	WEIGHTING FACTOR (WF)	WEIGHTING FACTOR FOR STUDY SITE (Pick one from category on left)	SCORE (MPV * WF)			
		Frontage	Playground Capacity (number of children)			<u>-</u>			
			16 or more	1.00					
Dlayground Type	40	> E0 m	5 to 15	0.75					
Playground <u>T</u> ype	40	2 50 111	1 to 4	0.40					
			No play equipment: sport field or open field only	0.20					
		< 50 m	Any Facilities	0.20	0.20	T=	8		
		Urban Land Use	Rural Land Use						
		Profitage   (number   1   1   1   1   1   1   1   1   1		1.00	1.00				
Dood Classifestian	Urban Land Use	Minor Collector	Local	0.75					
Road <u>C</u> lassification		Collector	0.50						
			Playground Capacity (number of children)  16 or more  1 to 4  No play equipment: sport field or open field only Any Facilities  Rural Land Use  1 Local Collector Arterial Freeway  raversable 1 Traversable ble/Indoor Facility Roadway 1 50 metres an 50 metres ple Secondary Entrances ary Entrance None Playground Side h Sides  1 Collector	0.25					
		Major Arterial/	Freeway	0.00		C=	20		
		ĺ	Traversable	1.00					
<u>F</u> encing	20	Partial	ly Traversable	0.50					
		Non-Travers	sable/Indoor Facility	0.10	0.10	F=	2		
		Abu	ts Roadway	1.00	1.00				
Property <u>L</u> ine Separation	10	With	in 50 metres	0.50					
·		Further	than 50 metres	0.00		L=	10		
		Main Entrance / Mu	Itiple Secondary Entrances	1.00					
Playground <u>E</u> ntrance	5	Secon	dary Entrance	0.60	0.60				
			None	0.00		E=	3		
		None (or No	n-Playground Side)	1.00	1.00				
<u>S</u> idewalks	5	Play	ground Side	0.40					
		В	oth Sides	0.00		S=	5		
		TOTAL SCORE (S	sum of T,C,F,L,E and S)			4	8		
	<u> </u>					Playgrou	ınd Area		

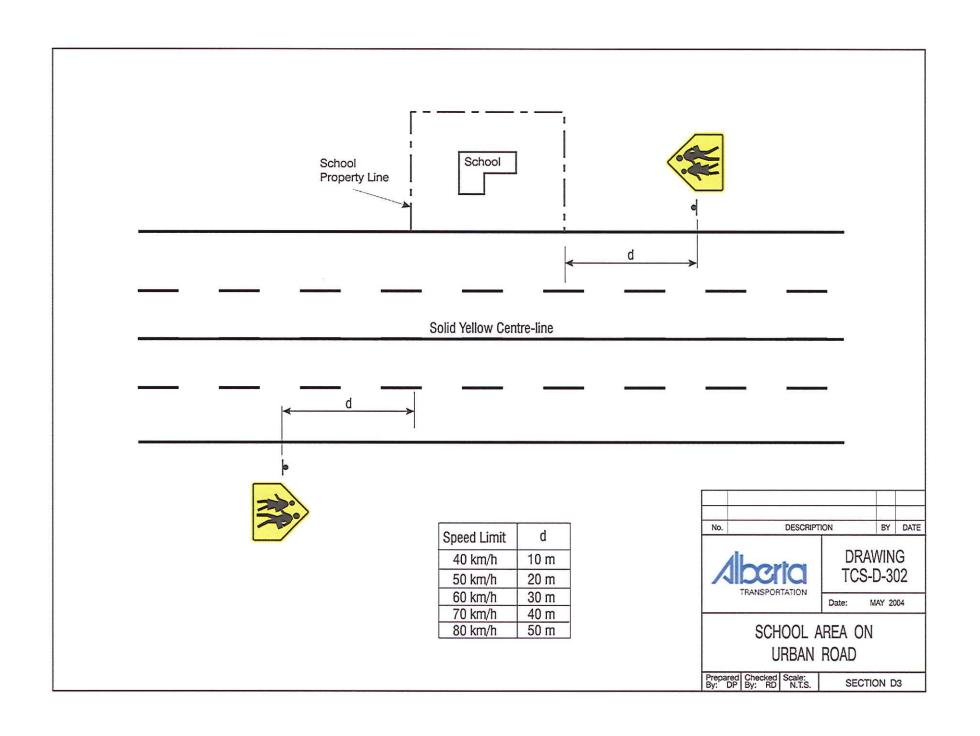
## **TECHNICAL MEMORANDUM**

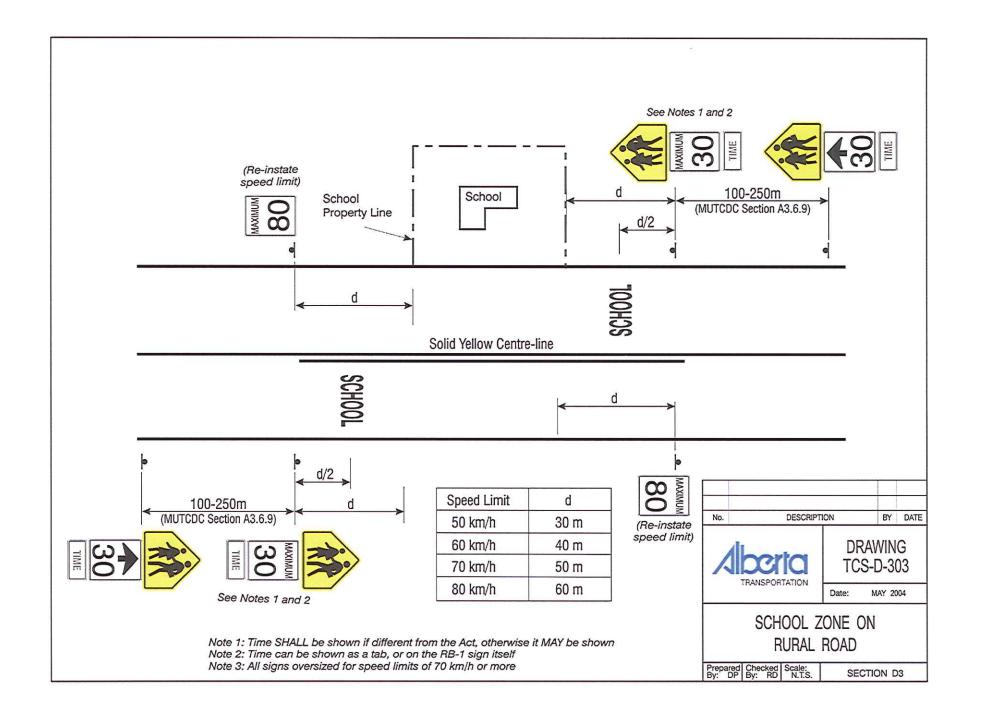


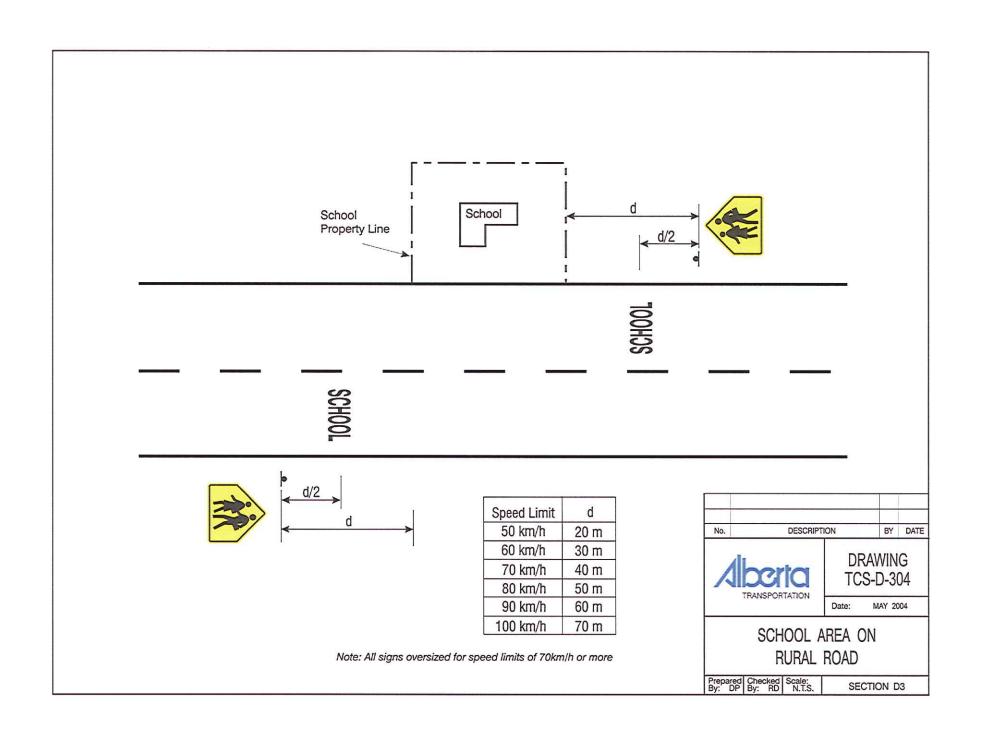
# **Appendix C - Signage and Pavement Marking Plans**

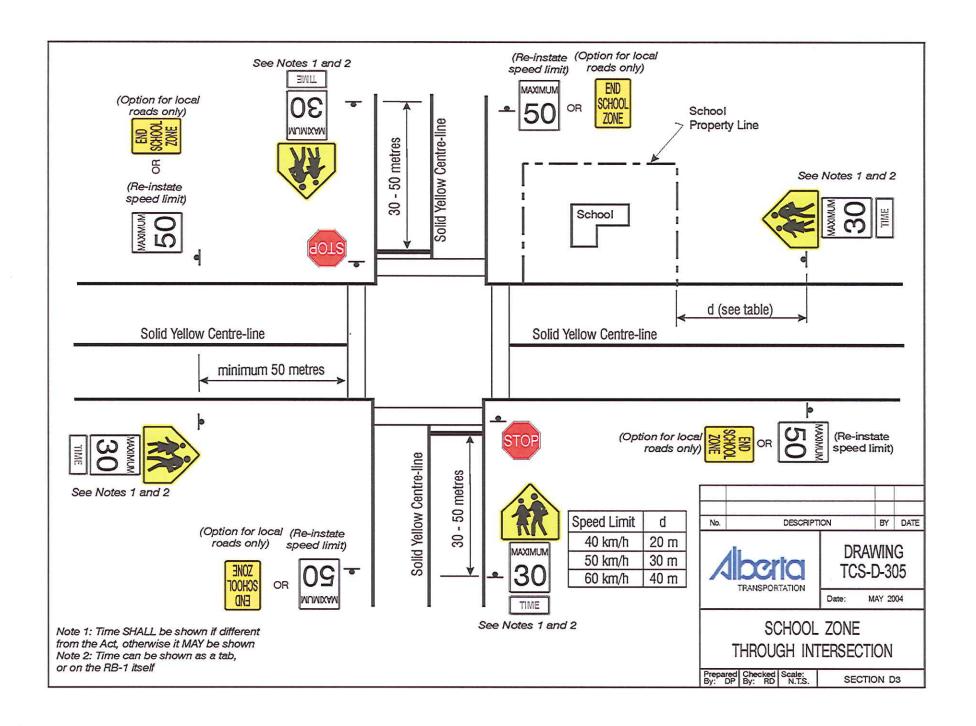


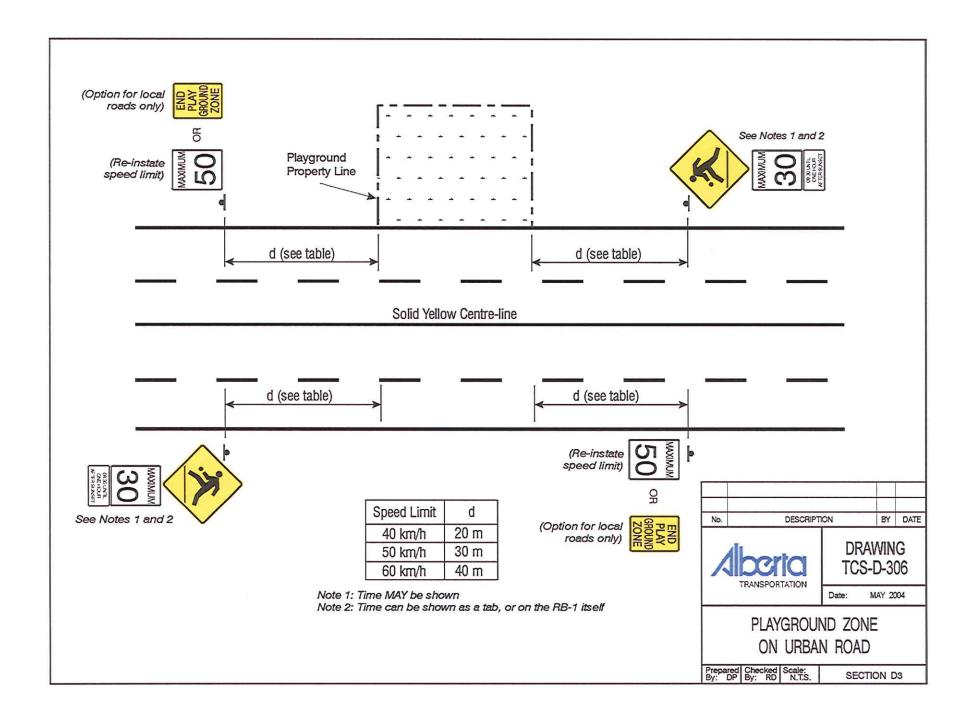


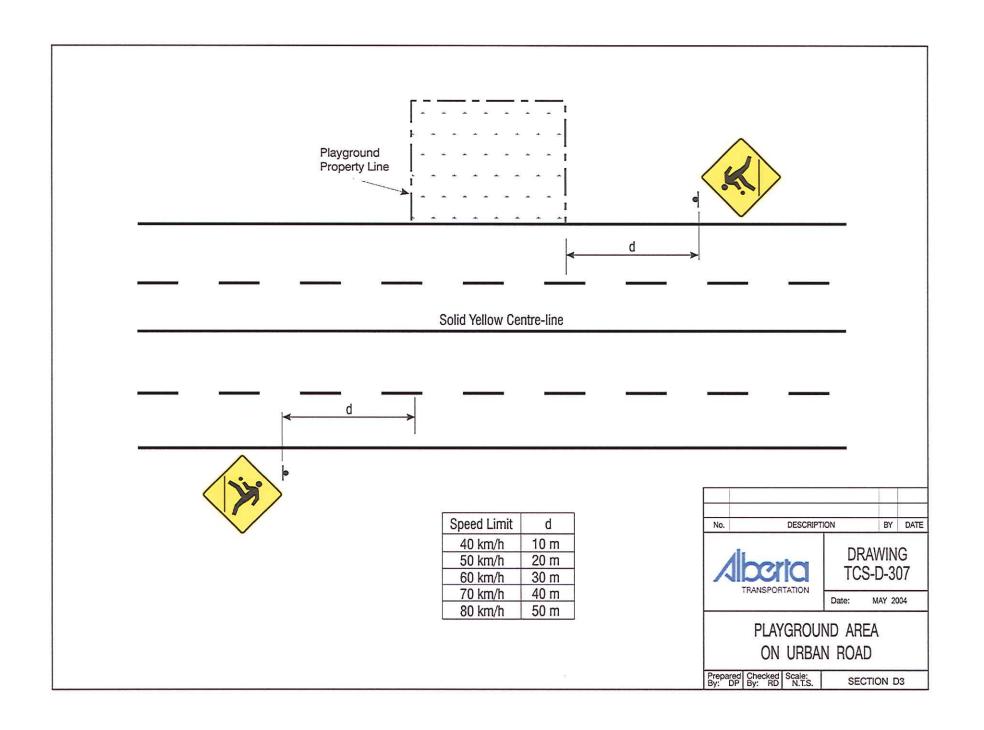


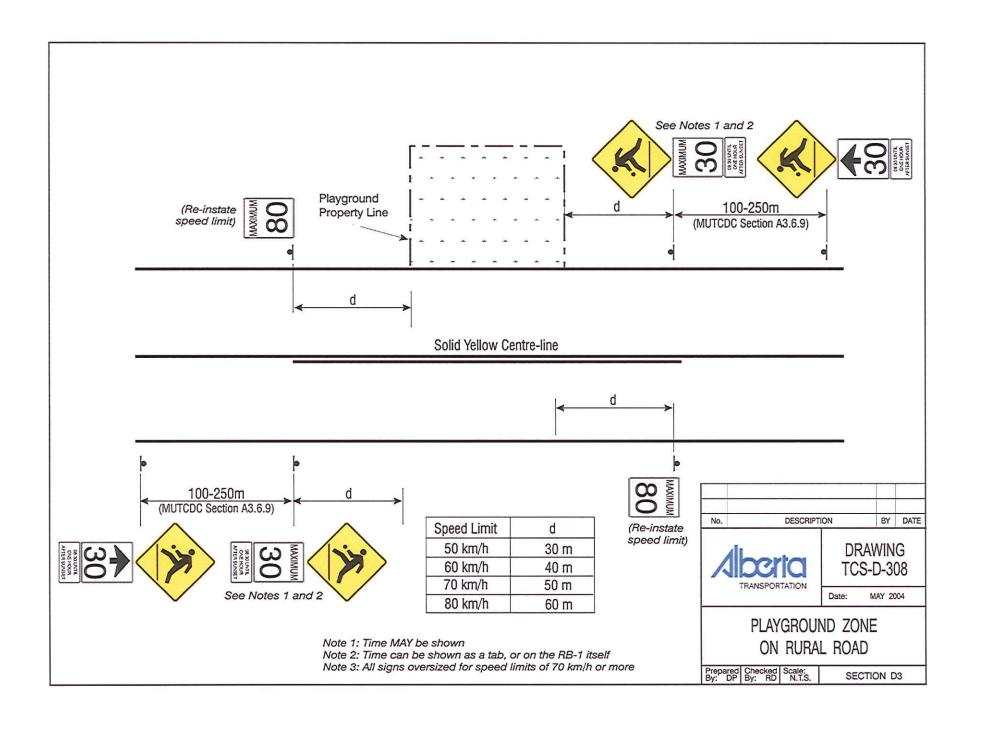


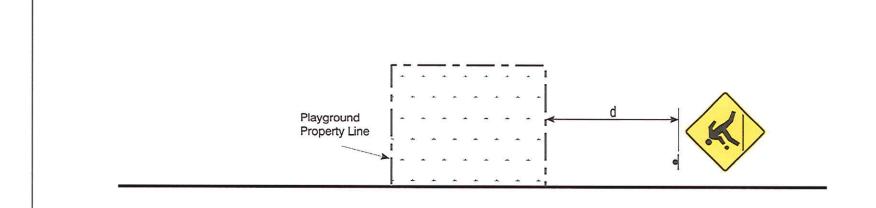


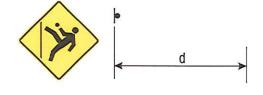






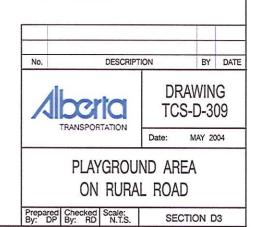


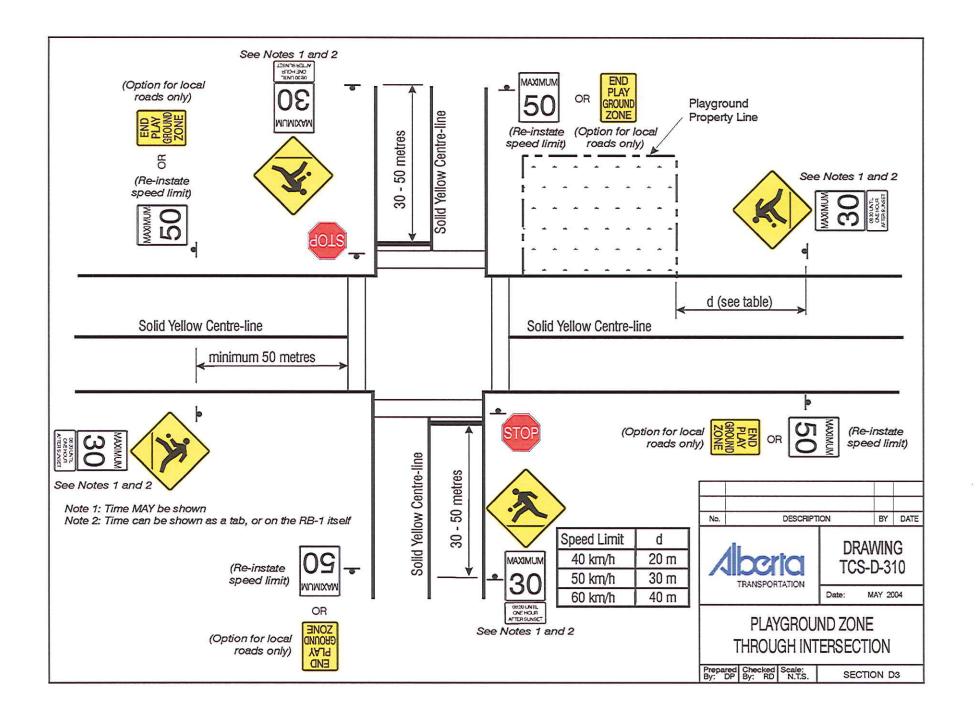


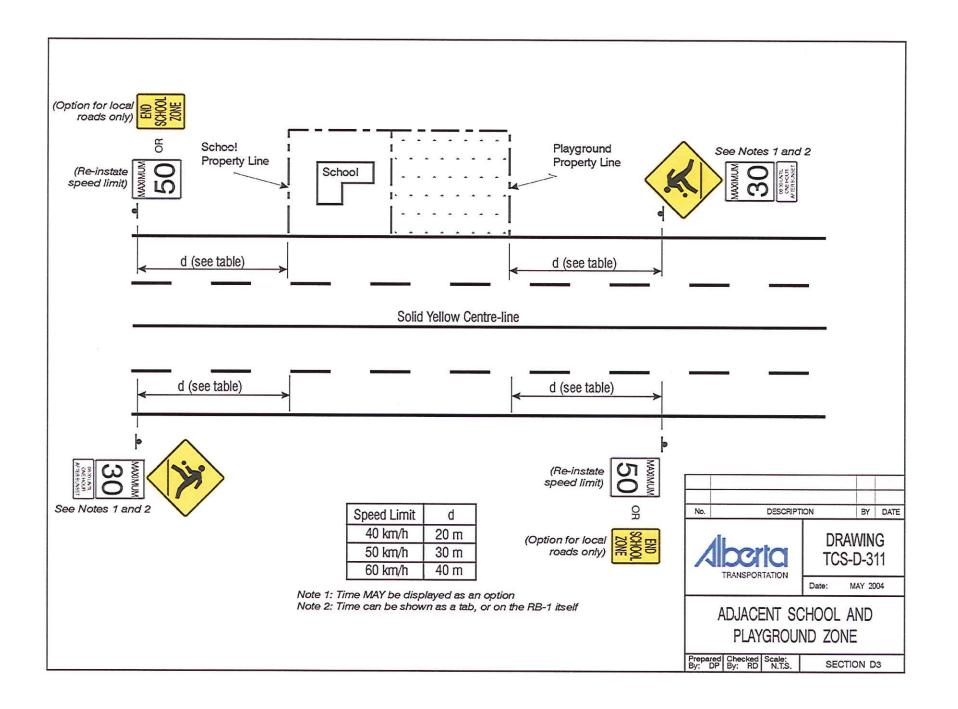


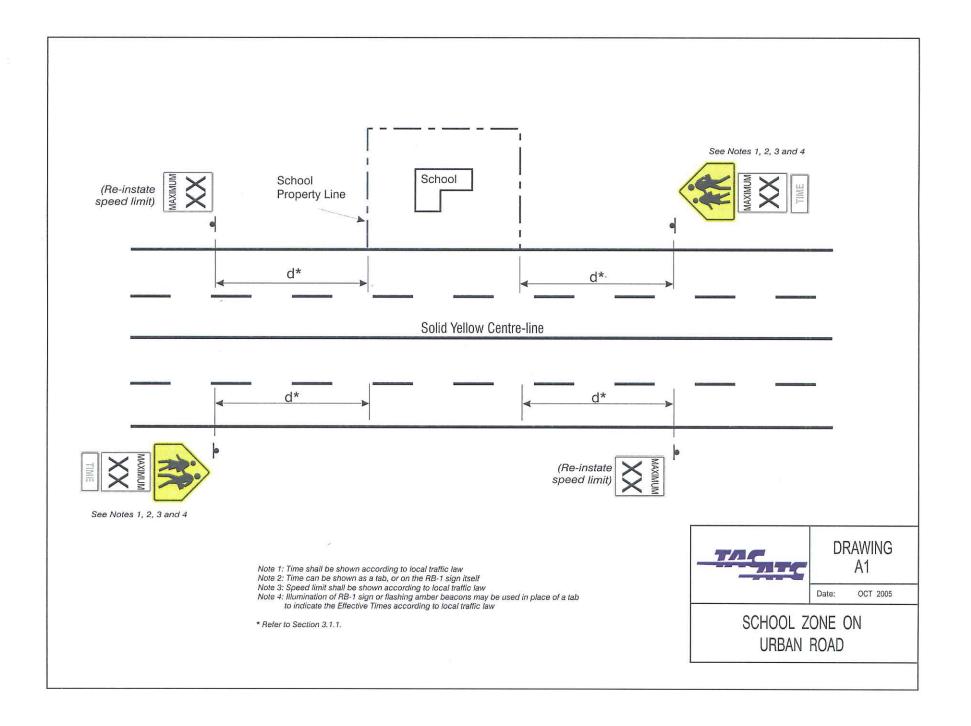
Speed Limit	d
50 km/h	20 m
60 km/h	30 m
70 km/h	40 m
80 km/h	50 m
90 km/h	60 m
100 km/h	70 m

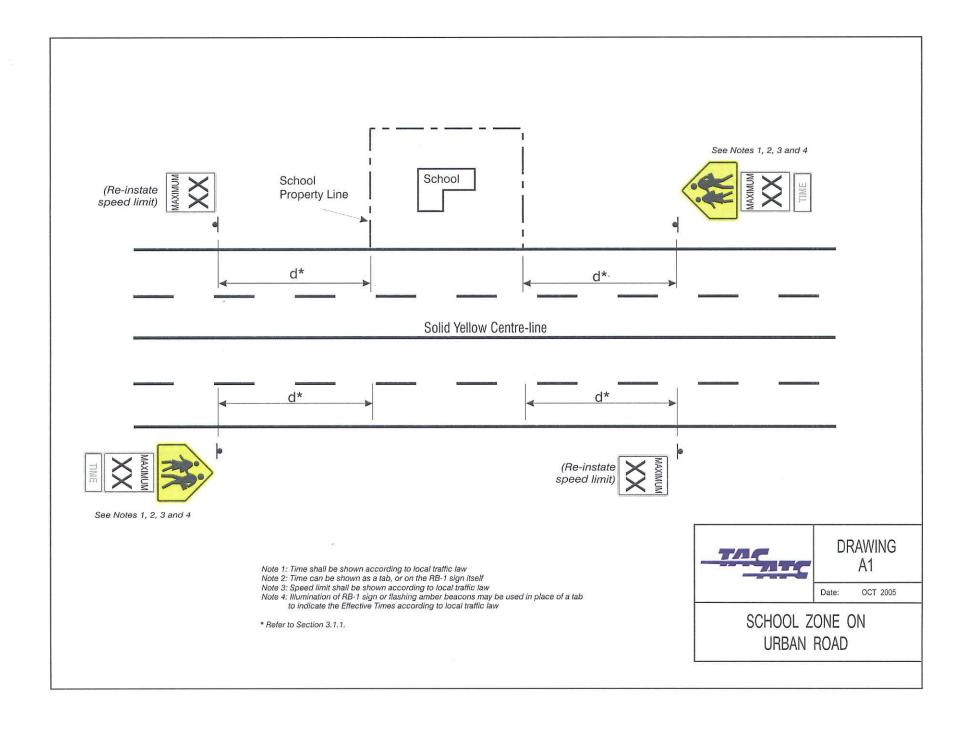
Note: All signs oversized for speed limits of 70 km/h or more

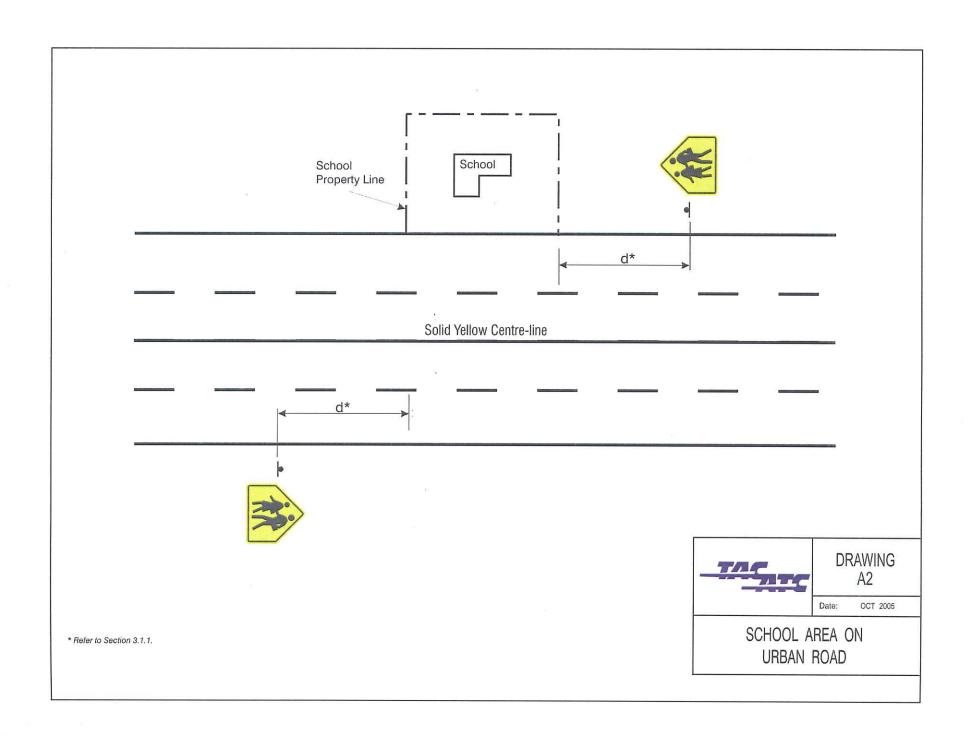


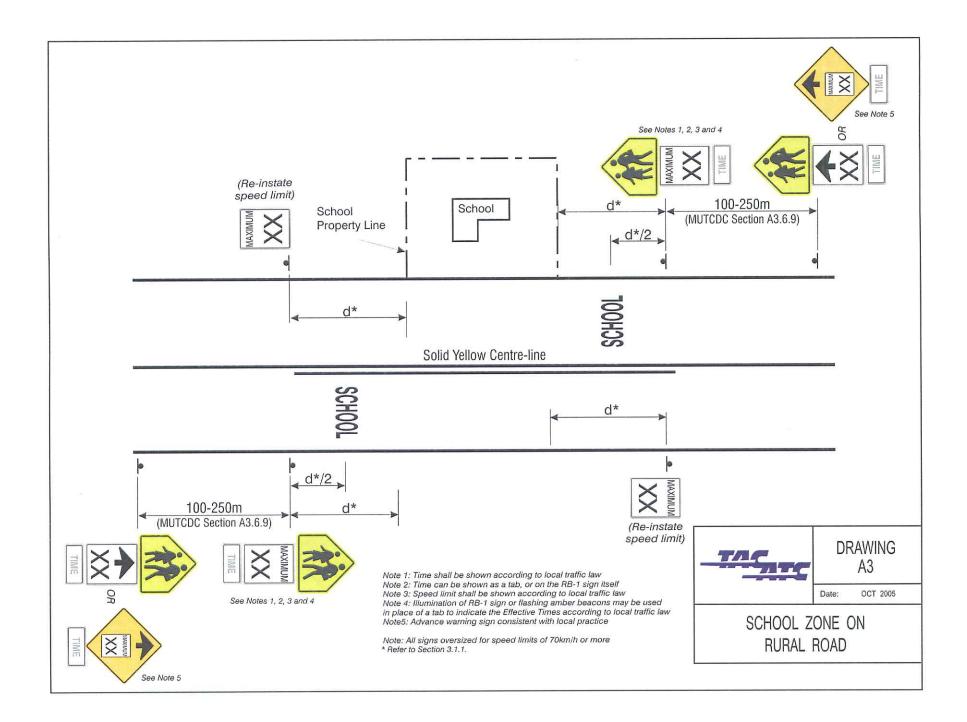


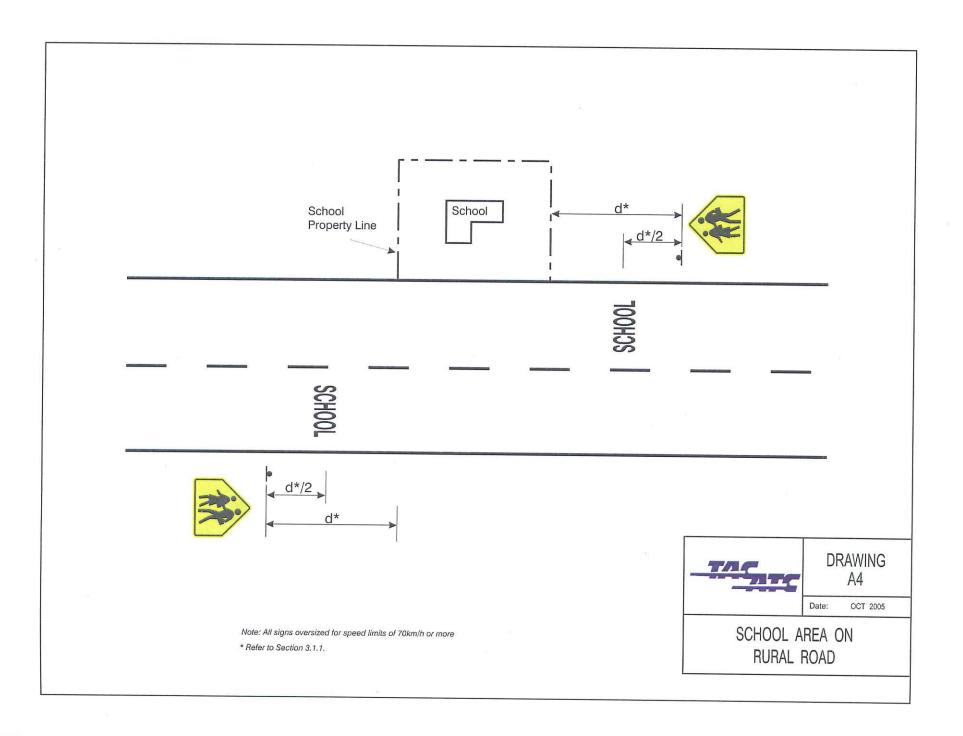


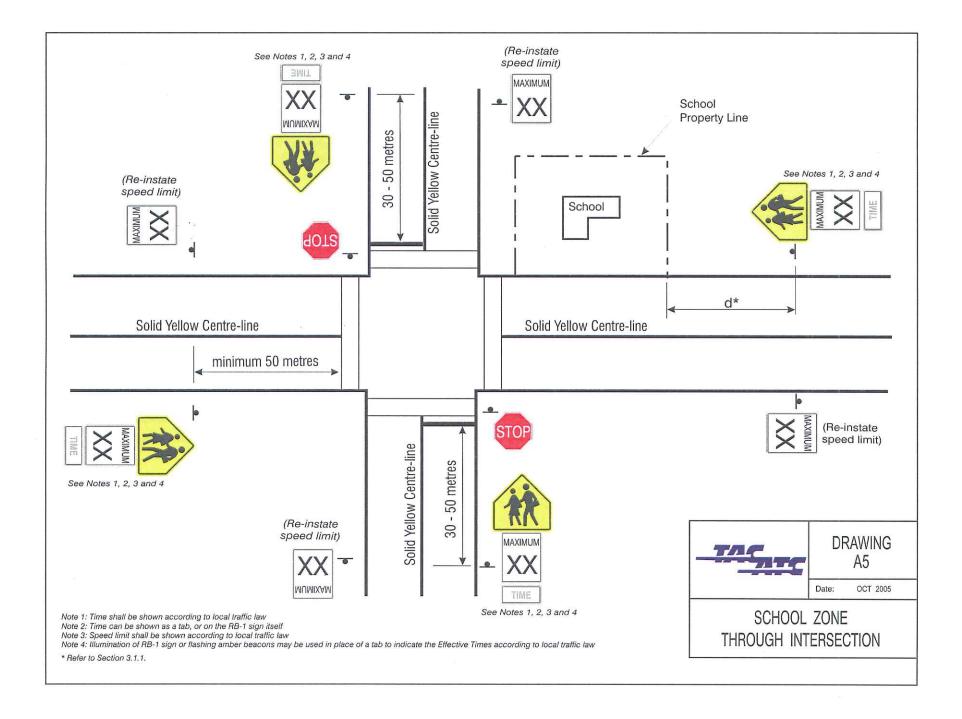


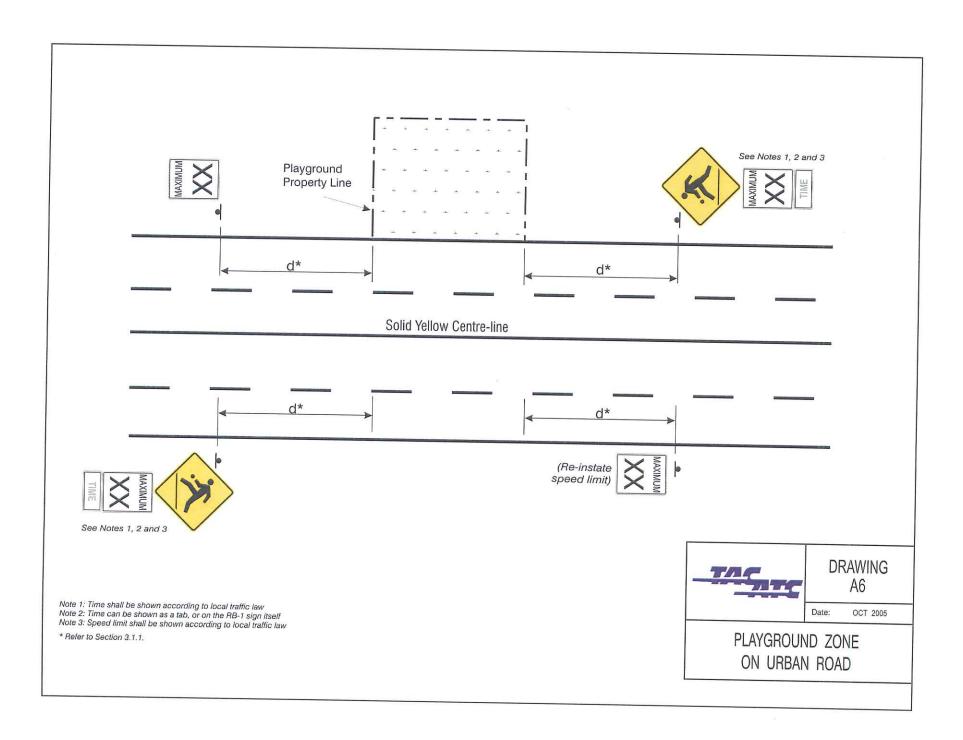


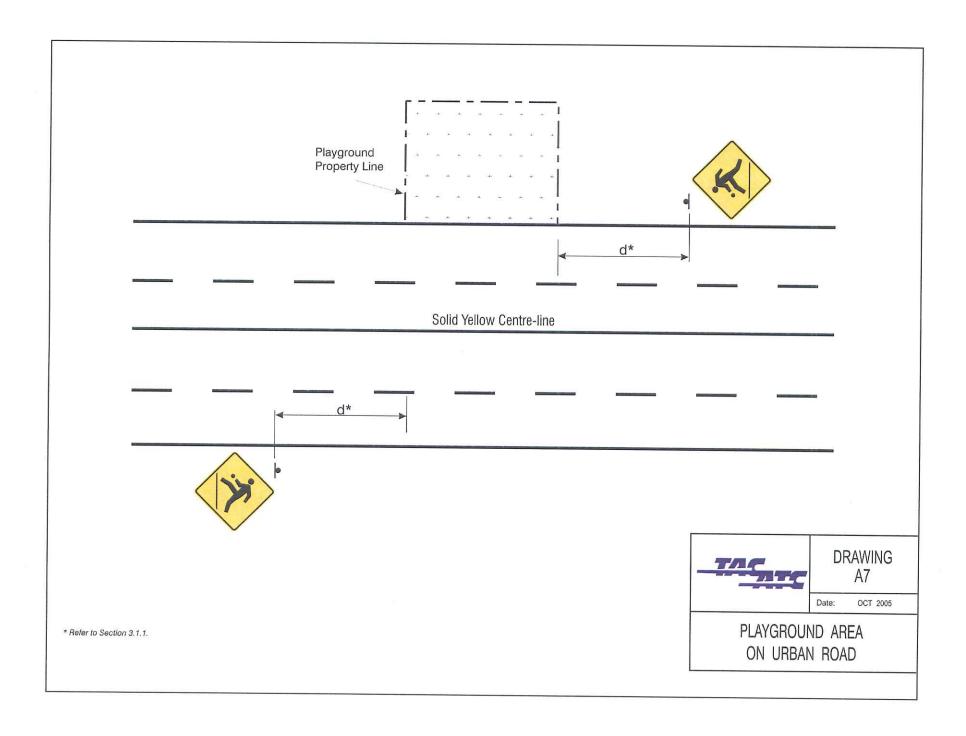


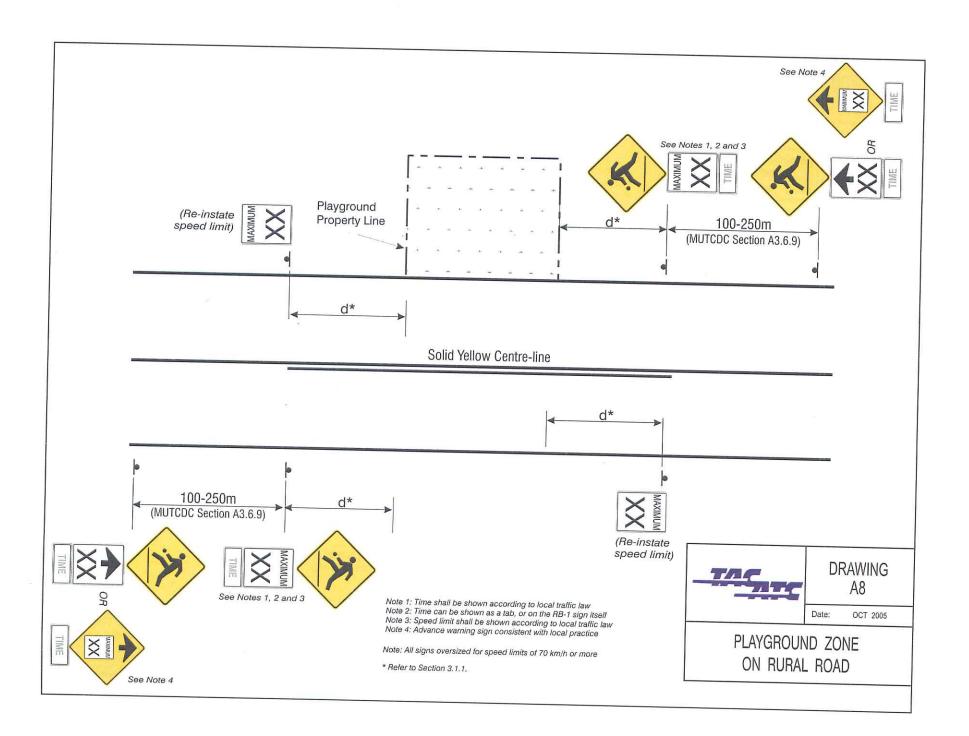


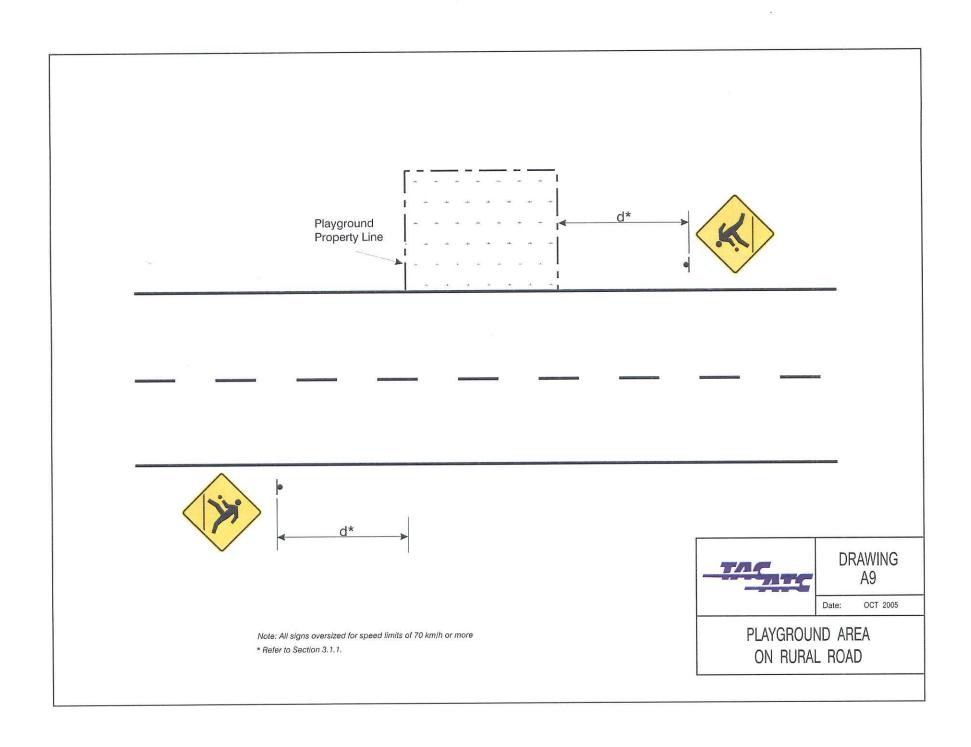


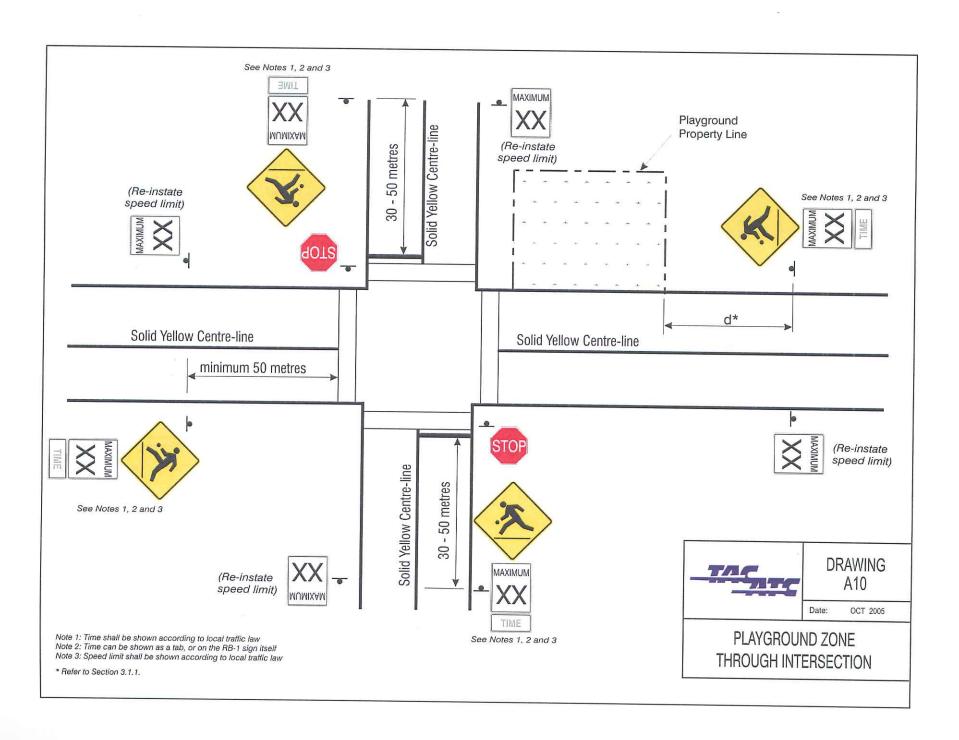


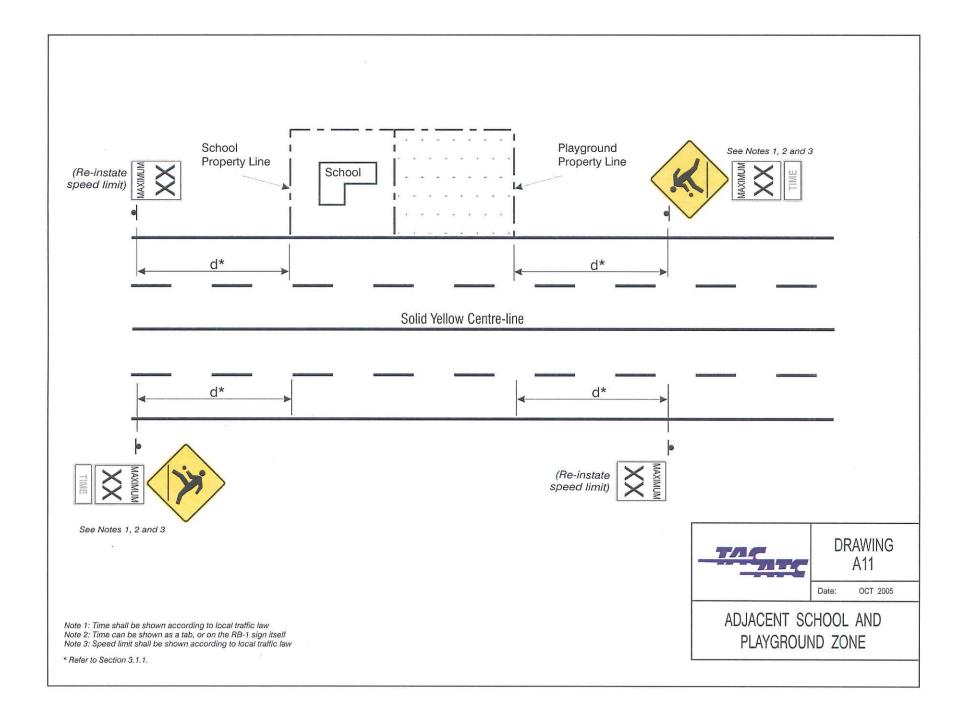












# **FINAL REPORT**

# **Appendix I - Unit Rate Development**

Asphalt (m³ to Tonne) = 2.60 Granular (m<sup>3</sup> to Tonne) = 2.35  $1m^2 = 0.0001$ 

### **Unit Rate Development**

### 8 Avenue (10 Street to Lakeshore Drive) - Roadway Upgrade

Widen to provide centre median and 2 lanes in each direction. 2.5% normal crown, asphalt placed in two lifts, tack coat between lifts.

				Unit Prices		
Pavement Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit
Sawcut Existing Asphalt	2.00	lm	\$40.00	Sawcutting	\$20.00	lm
Remove Existing Curb & Gutter	2.00	lm	\$34.00	Curb & Gutter Removal	\$17.00	lm
Remove Existing Sidewalk - 2m wide along both sides	4.00	m <sup>2</sup>	\$80.00	Sidewalk Removal	\$20.00	m <sup>2</sup>
Construct 5.0m Centre Median - Pin curb and gutter on existing asphalt	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm
Construct Additional Pavement Structure						
125mm Asphalt	1.67	m <sup>3</sup>	\$804.91	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Tack Coat	13.37	m <sup>2</sup>	\$26.74	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>
Prime Coat	13.37	m <sup>2</sup>	\$26.74	Prime Coat	\$2.00	m <sup>2</sup>
200mm Granular Base	2.92	m <sup>3</sup>	\$198.90	3/4 Granular	\$29.00	Tonne
300mm Granular Subbase	4.49	m <sup>3</sup>	\$305.75	3/4 Granular	\$29.00	Tonne
150mm Subgrade Preparation	14.95	m <sup>2</sup>	\$134.59	Subgrade Preparation	\$9.00	m <sup>2</sup>
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm

Total Cost for 8 Avenue Roadway Upgrade, per metre of Roadway = \$2,131.63

### **Various Locations - Install Traffic Signals**

Component	Quantity	Unit	Cost
Traffic Signals	1.00	int	\$350,000.00
Tota	Cost of Traffic Signals, per	Intersection -	\$350,000,00

# Channelize Right Turn Lane (Along Expressway/Arterials) Based on conceptual design for Highway 28 and 43 Avenue right turn channelization. Cost estimate does not include cost to remove existing curb & gutter and sidewalks

				Unit Prices		
Component	Quantity	Unit	Cost	Description	Unit Price	Unit
Sawcut Existing Asphalt	185.00	lm	\$3,700.00	Sawcutting	\$20.00	lm
Construct Pavement Structure						
125mm Asphalt	72.50	m <sup>3</sup>	\$34,872.50	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Tack Coat	580.00	m <sup>2</sup>	\$1,160.00	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>
Prime Coat	580.00	m <sup>2</sup>	\$1,160.00	Prime Coat	\$2.00	m <sup>2</sup>
200mm Granular Base	238.00	m <sup>3</sup>	\$16,219.70	3/4 Granular	\$29.00	Tonne
300mm Granular Subbase	357.00	m <sup>3</sup>	\$24,329.55	3/4 Granular	\$29.00	Tonne
150mm Subgrade Preparation	1,190.00	m <sup>2</sup>	\$10,710.00	Subgrade Preparation	\$9.00	m <sup>2</sup>
Curb & Gutter	170.00	lm	\$20,400.00	Curb & Gutter	\$120.00	lm
Concrete Island	35.00	m <sup>2</sup>	\$4,015.90	AT X350 - Solid Concrete Islands	\$114.74	m <sup>2</sup>
Pavement Marking - Solid Lines	115.00	lm	\$92.06	AT S350 - Roadway Lines - Supplying Paint and Painting (Direction Dividing)	\$800.53	km
Pavement Marking - Dashed Lines	75.00	lm	\$61.07	AT S351 - Roadway Lines - Supplying Paint and Painting (Lane Dividing)	\$814.31	km
Crosswalk Marking	1.00	message	\$239.78	AT S327 - Pavement Messages - Pedestrian Crossing	\$239.78	message

Total Cost to Channelize Right Turn Lane = \$116,960.56

### Channelize Right Turn Lane (Along Local)

Based on conceptual design for Highway 28 and 43 Avenue right turn channelization.

Cost estimate does not include cost to remove existing curb & gutter and sidewa	dikS						
					Unit Prices		
Component		Quantity	Unit	Cost	Description	Unit Price	Unit
Sawcut Existing Asphalt		185.00	lm	\$3,700.00	Sawcutting	\$20.00	lm
Construct Pavement Structure							
90mm Asphalt		52.20	m <sup>3</sup>	\$25,108.20	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Prime Coat		580.00	m <sup>2</sup>	\$1,160.00	Prime Coat	\$2.00	m <sup>2</sup>
250mm Granular Base		297.50	m <sup>3</sup>	\$20,274.63	3/4 Granular	\$29.00	Tonne
150mm Subgrade Preparation		1,190.00	m <sup>2</sup>	\$10,710.00	Subgrade Preparation	\$9.00	m <sup>2</sup>
Curb & Gutter		170.00	lm	\$20,400.00	Curb & Gutter	\$120.00	lm
Concrete Island		35.00	m <sup>2</sup>	\$4,015.90	AT X350 - Solid Concrete Islands	\$114.74	m <sup>2</sup>
Pavement Marking - Solid Lines		115.00	lm	\$92.06	AT S350 - Roadway Lines - Supplying Paint and Painting (Direction Dividing)	\$800.53	km
Pavement Marking - Dashed Lines		75.00	lm	\$61.07	AT S351 - Roadway Lines - Supplying Paint and Painting (Lane Dividing)	\$814.31	km
Crosswalk Marking		1.00	message	\$239.78	AT S327 - Pavement Messages - Pedestrian Crossing	\$239.78	message

Total Cost to Channelize Right Turn Lane = \$85,761.64

Asphalt (m<sup>3</sup> to Tonne) = 2.60 Granular (m<sup>3</sup> to Tonne) = 2.35

 $1m^2 = 0.0001$ 

### **Unit Rate Development**

### Highway 28 (53 Avenue to 52 Avenue, 52 Street to 47 Avenue) - Roadway Upgrade

Widen to provide centre median.

				Unit Prices		
Pavement Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit
Sawcut Existing Asphalt	2.00	lm	\$40.00	Sawcutting	\$20.00	lm
Remove Existing Curb & Gutter	2.00	lm	\$34.00	Curb & Gutter Removal	\$17.00	lm
Construct 5.0m Centre Median - Pin curb and gutter on existing asphalt	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm
Construct Additional Pavement Structure						
125mm Asphalt	0.75	m <sup>3</sup>	\$359.79	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Tack Coat	5.97	m <sup>2</sup>	\$11.93	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>
Prime Coat	5.97	m <sup>2</sup>	\$11.93	Prime Coat	\$2.00	m <sup>2</sup>
200mm Granular Base	1.44	m <sup>3</sup>	\$98.00	3/4 Granular	\$29.00	Tonne
300mm Granular Subbase	2.27	m <sup>3</sup>	\$154.41	3/4 Granular	\$29.00	Tonne
150mm Subgrade Preparation	7.55	m <sup>2</sup>	\$67.97	Subgrade Preparation	\$9.00	m <sup>2</sup>
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm

Total Cost for Highway 28 Roadway Upgrade, per metre of Roadway = \$1,258.04

## <u>Various Locations - Painted On-Street Parallel Parking Stalls</u> Assumed: 6.0m solid white lines for each parking stall.

				Unit Prices		
Component	Quantity	Unit	Cost	Description	Unit Price	Unit
Pavement Markings	6.00	lm	\$4.80	AT S350 - Roadway Lines - Supplying Paint and Painting (Directional Dividing)	\$800.53	km
Total Cost of Pavement Mark	king, per On-Street Parallel P	arking Stall =	\$4.80			

Kinosoo Beach - Pave and Paint Gravel Lot Parking Lot
Pavement will be graded for draining; slopes to vary. Assumed same 2.5% slope for roadway.
Assumed: Same pavement structure as Collector roads. 13.0m solid white lines for each parking stall.

				Unit Prices			
Component	Quantity	Unit	Cost	Description	Unit Price	Unit	
Pave Parking Lot - 2,280m <sup>2</sup>							
100mm Asphalt	228.00	m <sup>3</sup>	\$109,668.00	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne	
Prime Coat	2,280.00	m <sup>2</sup>	\$4,560.00	Prime Coat	\$2.00	m <sup>2</sup>	
300mm Granular Base	684.00	m <sup>3</sup>	\$46,614.60	3/4 Granular	\$29.00	Tonne	
150mm Granular Subbase	342.00	m <sup>3</sup>	\$23,307.30	3/4 Granular	\$29.00	Tonne	
150mm Subgrade Preparation	2,280.00	m <sup>2</sup>	\$20,520.00	Subgrade Preparation	\$9.00	m <sup>2</sup>	
Pavement Markings - 58 Stalls	754.00	lm	\$603.60	AT S350 - Roadway Lines - Supplying Paint and Painting (Directional Dividing)	\$800.53	km	

Total Cost to Pave and Paint Gravel Lot Parking Lot = \$205,273.50

### 1 Avenue - Install Parking Control Signs

				Unit Prices		
Component	Quantity	Unit	Cost	Description	Unit Price	Unit
Parking Control (RB-51, RB-52) Signs	4.00	signs	\$256.56	AT S288 - Install Sign - Less than 1 m <sup>2</sup>	\$64.14	sign
Sign Posts	4.00	posts	\$731.52	AT S772 - Supply and Install Post (100mm x 150mm)	\$182.88	post
Т	otal Cost to Install Parking C	control Signs=	\$988.08			

## 1 Avenue - Repave Corridor (Residential Collector)

Mill and replace full depth of asphalt layer.

Assumed: 13.1m road width along 1 Avenue (from In-Service Road Safety Review).

	Unit Prices					
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit
Coldmilling	13.10	m <sup>2</sup>	\$196.50	Cold Milling Asphalt Pavement	\$15.00	m <sup>2</sup>
100mm Asphalt	1.31	m <sup>3</sup>	\$630.11	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Total Cost of Repaying, per meter of Roadway = \$826.61				,		,

Asphalt (m³ to Tonne) = 2.60 Granular (m<sup>3</sup> to Tonne) = 2.35  $1m^2 = 0.0001$ 

### **Unit Rate Development**

### 19 Street Crosswalk - Provide Pavement Marking and Signage

				Unit Prices		
Component	Quantity	Unit	Cost	Description	Unit Price	Unit
Pedestrian Crosswalk - Pavement Markings	1.00	message	\$239.78	AT S327 - Pavement Messages - Pedestrian Crossing	\$239.78	message
Pedestrian Crosswalk (RA-4) Signs	2.00	signs	\$128.28	AT S288 - Install Sign - Less than 1 m <sup>2</sup>	\$64.14	sign
Sign Posts	2.00	posts	\$365.76	AT S772 - Supply and Install Post (100mm x 150mm)	\$182.88	post

Total Cost for Pavement Marking and Signage at 19 Street Crosswalk = \$733.82

Highway 55 (28 Street to Highway 28) - Roadway Upgrade

Build pavement structure to Arterial Standard (centre median with 2 lanes in each direction). 2.5% normal crown, asphalt placed in two lifts, tack coat between lifts.

Median assumed to have same pavement structure as roadway, in preparation for designated turn lanes.

Assumed: Existing pavement width = 13.0m (from MESS).

About to a case of paronon water = 10.011 (1011 In 200).				Unit Prices			
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit	
Remove Existing Pavement							
Remove Asphalt	13.00	m <sup>2</sup>	\$260.00	Asphalt Removal	\$20.00	m <sup>2</sup>	
Remove Base Course	3.90	m <sup>3</sup>	\$66.30	Excavation to Waste	\$17.00	m <sup>3</sup>	
Construct 5.0m Centre Median							
125mm Asphalt	0.63	m <sup>3</sup>	\$300.67	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne	
Tack Coat	5.00	m <sup>2</sup>	\$10.00	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>	
Prime Coat	5.00	m <sup>2</sup>	\$10.00	Prime Coat	\$2.00	m <sup>2</sup>	
200mm Granular Base	1.00	m <sup>3</sup>	\$68.17	3/4 Granular	\$29.00	Tonne	
300mm Granular Subbase	1.50	m <sup>3</sup>	\$102.26	3/4 Granular	\$29.00	Tonne	
150mm Subgrade Preparation	5.00	m <sup>2</sup>	\$45.01	Subgrade Preparation	\$9.00	m <sup>2</sup>	
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm	
Construct Pavement Structure							
125mm Asphalt	1.85	m <sup>3</sup>	\$889.08	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne	
Tack Coat	14.77	m <sup>2</sup>	\$29.54	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>	
Prime Coat	14.77	m <sup>2</sup>	\$29.54	Prime Coat	\$2.00	m <sup>2</sup>	
200mm Granular Base	3.20	m <sup>3</sup>	\$217.98	3/4 Granular	\$29.00	Tonne	
300mm Granular Subbase	4.91	m <sup>3</sup>	\$334.38	3/4 Granular	\$29.00	Tonne	
150mm Subgrade Preparation	16.36	m <sup>2</sup>	\$147.20	Subgrade Preparation	\$9.00	m <sup>2</sup>	
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm	

Total Cost for Highway 55 Roadway Upgrade, per metre of Roadway = \$2,990.14

16 Avenue (Highway 28 to 16 Street) - Roadway Upgrade
Build pavement structure to Arterial Standard (2 lanes in each direction), 2.5% normal crown, asphalt placed in two lifts, tack coat between lifts.
Assumed Fixtion pa

Assumed: Existing pavement width = 13.0m (from MESS).							
				Unit Prices			
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit	
Remove Existing Pavement					-	-	
Remove Asphalt	13.00	m <sup>2</sup>	\$260.00	Asphalt Removal	\$20.00	m <sup>2</sup>	
Remove Base Course	3.90	m <sup>3</sup>	\$66.30	Excavation to Waste	\$17.00	m <sup>3</sup>	
Construct Pavement Structure							
125mm Asphalt	1.85	m <sup>3</sup>	\$889.08	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne	
Tack Coat	14.77	m <sup>2</sup>	\$29.54	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>	
Prime Coat	14.77	m <sup>2</sup>	\$29.54	Prime Coat	\$2.00	m <sup>2</sup>	
200mm Granular Base	3.20	m <sup>3</sup>	\$217.98	3/4 Granular	\$29.00	Tonne	
300mm Granular Subbase	4.91	m <sup>3</sup>	\$334.38	3/4 Granular	\$29.00	Tonne	
150mm Subgrade Preparation	16.36	m <sup>2</sup>	\$147.20	Subgrade Preparation	\$9.00	m <sup>2</sup>	
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm	

Total Cost for 16 Avenue Roadway Upgrade, per metre of Roadway = \$2,214.01

Asphalt (m³ to Tonne) = 2.60 Granular (m<sup>3</sup> to Tonne) = 2.35

 $1m^2 = 0.0001$ 

### **Unit Rate Development**

## 16 Avenue (16 Street to 8 Street) - Roadway Upgrade

Widen to provide 2 lanes in each direction.

				Unit Prices		
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit
Sawcut Existing Asphalt	2.00	lm	\$40.00	Sawcutting	\$20.00	lm
Remove Existing Curb & Gutter	2.00	lm	\$34.00	Curb & Gutter Removal	\$17.00	lm
Remove Existing Sidewalk - 2m wide, assumed along both sides	4.00	m <sup>2</sup>	\$80.00	Sidewalk Removal	\$20.00	m <sup>2</sup>
Construct Pavement Structure						
100mm Asphalt	0.84	m <sup>3</sup>	\$403.46	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Prime Coat	8.38	m <sup>2</sup>	\$16.75	Prime Coat	\$2.00	m <sup>2</sup>
300mm Granular Base	2.87	m <sup>3</sup>	\$195.85	3/4 Granular	\$29.00	Tonne
150mm Granular Subbase	1.49	m <sup>3</sup>	\$101.75	3/4 Granular	\$29.00	Tonne
150mm Subgrade Preparation	9.95	m <sup>2</sup>	\$89.58	Subgrade Preparation	\$9.00	m <sup>2</sup>
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm

Total Cost for 16 Avenue Roadway Upgrade, per metre of Roadway = \$1,201.39

English Bay Road (Lake Avenue to Highway 28) - Roadway Upgrade
Build pavement structure to Arterial Standard (centre median with 2 lanes in each direction). 2.5% normal crown, asphalt placed in two lifts, tack coat between lifts.
Assumed: Existing pavement width = 13.0m (from MESS).

				Unit Prices				
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit		
Remove Existing Pavement								
Remove Asphalt	13.00	m <sup>2</sup>	\$260.00	Asphalt Removal	\$20.00	m <sup>2</sup>		
Remove Base Course	3.90	m <sup>3</sup>	\$66.30	Excavation to Waste	\$17.00	m <sup>3</sup>		
Remove Existing Curb & Gutter - Along east side, north of 1 Avenue	0.50	lm	\$8.50	Curb & Gutter Removal	\$17.00	lm		
Construct 5.0m Centre Median								
125mm Asphalt	0.63	m <sup>3</sup>	\$300.67	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne		
Tack Coat	5.00	m <sup>2</sup>	\$10.00	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>		
Prime Coat	5.00	m <sup>2</sup>	\$10.00	Prime Coat	\$2.00	m <sup>2</sup>		
200mm Granular Base	1.00	m <sup>3</sup>	\$68.17	3/4 Granular	\$29.00	Tonne		
300mm Granular Subbase	1.50	m <sup>3</sup>	\$102.26	3/4 Granular	\$29.00	Tonne		
150mm Subgrade Preparation	5.00	m <sup>2</sup>	\$45.01	Subgrade Preparation	\$9.00	m <sup>2</sup>		
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm		
Construct Pavement Structure								
125mm Asphalt	1.85	m <sup>3</sup>	\$889.08	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne		
Tack Coat	14.77	m <sup>2</sup>	\$29.54	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>		
Prime Coat	14.77	m <sup>2</sup>	\$29.54	Prime Coat	\$2.00	m <sup>2</sup>		
200mm Granular Base	3.20	m <sup>3</sup>	\$217.98	3/4 Granular	\$29.00	Tonne		
300mm Granular Subbase	4.91	m <sup>3</sup>	\$334.38	3/4 Granular	\$29.00	Tonne		
150mm Subgrade Preparation	16.36	m <sup>2</sup>	\$147.20	Subgrade Preparation	\$9.00	m <sup>2</sup>		
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm		

Total Cost for English Bay Road Roadway Upgrade, per metre of Roadway = \$2,998.64

## 28 Street - Remove Existing Roadway

Assumed. Existing pavement width = 13.0m (from wi255).	Unit Prices					
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit
Remove Asphalt	13.00	m <sup>2</sup>	\$260.00	Asphalt Removal	\$20.00	m <sup>2</sup>
Remove Base Course	3.90	m <sup>3</sup>	\$66.30	Excavation to Waste	\$17.00	m <sup>3</sup>
Top Soil	13.00	m <sup>2</sup>	\$51.22	AT G320 - Topsoil (Supply and Place)	\$3.94	m <sup>2</sup>
Seeding	13.00	m <sup>2</sup>	\$2.00	AT E608 - Broad Cast Seeding	\$1,542.17	ha
Total Cost for Par	\$379.52					

Relocate stop signs 2.00 331.88 AT S275 - Removal and Reinstallation or Disposal of Existing Signs 165.94 sians signs

Asphalt (m<sup>3</sup> to Tonne) = 2.60 Granular (m<sup>3</sup> to Tonne) = 2.35

 $1m^2 = 0.0001$ 

### **Unit Rate Development**

## 28 Street (English Bay Road to Highway 55) - Build Roadway Realign 28 Street and build pavement structure to Arterial Standard.

				Unit Prices			
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit	
Construct Pavement Structure							
125mm Asphalt	0.92	m <sup>3</sup>	\$444.01	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne	
Tack Coat	7.37	m <sup>2</sup>	\$14.74	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>	
Prime Coat	7.37	m <sup>2</sup>	\$14.74	Prime Coat	\$2.00	m <sup>2</sup>	
200mm Granular Base	1.72	m <sup>3</sup>	\$117.08	3/4 Granular	\$29.00	Tonne	
300mm Granular Subbase	2.69	m <sup>3</sup>	\$183.04	3/4 Granular	\$29.00	Tonne	
150mm Subgrade Preparation	8.95	m <sup>2</sup>	\$80.58	Subgrade Preparation	\$9.00	m <sup>2</sup>	
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm	

Total Cost for 28 Street Construction, per metre of Roadway = \$1,094.18

## 16 Street (16 Avenue to 75 Avenue) - Upgrade Roadway Build pavement structure to Arterial standard

Assumed: Existing pavement width = 10.0m (from MESS).

				Unit Prices			
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit	
Remove Existing Pavement							
Remove Asphalt	10.00	m <sup>2</sup>	\$200.00	Asphalt Removal	\$20.00	m <sup>2</sup>	
Remove Base Course	2.50	m <sup>3</sup>	\$42.50	Excavation to Waste	\$17.00	m <sup>3</sup>	
Construct Pavement Structure							
125mm Asphalt	1.01	m <sup>3</sup>	\$488.02	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne	
Tack Coat	8.83	m <sup>2</sup>	\$17.66	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>	
Prime Coat	8.83	m <sup>2</sup>	\$17.66	Prime Coat	\$2.00	m <sup>2</sup>	
200mm Granular Base	1.99	m <sup>3</sup>	\$135.95	3/4 Granular	\$29.00	Tonne	
300mm Granular Subbase	3.85	m <sup>3</sup>	\$262.36	3/4 Granular	\$29.00	Tonne	
150mm Subgrade Preparation	14.55	m <sup>2</sup>	\$130.92	Subgrade Preparation	\$9.00	m <sup>2</sup>	

Total Cost for 16 Street Roadway Upgrade, per metre of Roadway = \$1,295.08

## Future Arterial (75 Avenue to 50 Avenue) - Build Roadway Build out as per 20-year Horizon

• •				Unit Prices			
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit	
Construct Pavement Structure							
125mm Asphalt	1.01	m <sup>3</sup>	\$488.02	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne	
Tack Coat	8.83	m <sup>2</sup>	\$17.66	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>	
Prime Coat	8.83	m <sup>2</sup>	\$17.66	Prime Coat	\$2.00	m <sup>2</sup>	
200mm Granular Base	1.99	m <sup>3</sup>	\$135.95	3/4 Granular	\$29.00	Tonne	
300mm Granular Subbase	3.85	m <sup>3</sup>	\$262.36	3/4 Granular	\$29.00	Tonne	
150mm Subgrade Preparation	14.55	m <sup>2</sup>	\$130.92	Subgrade Preparation	\$9.00	m <sup>2</sup>	
Total Cost for Future Arte	\$1,052.58						

### 10 Street - Signage for Vertical Curve at 3 Avenue

	Unit Prices					
Component	Quantity	Unit	Cost	Description	Unit Price	Unit
Concealed Roadway (WA-13) signs	2.00	signs	\$128.28	AT S288 - Install Sign - Less than 1 m <sup>2</sup>	\$64.14	sign
Sign posts	2.00	posts	\$365.76	AT S772 - Supply and Install Post (100mm x 150mm)	\$182.88	post

### 10 Street - Repave (Residential Collector)

Mill and replace full depth of asphalt layer.

Assumed: 15.2m road width along 10 Street (from In-Service Road Safety Review)

				Unit Prices		
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit
Coldmilling	15.20	m <sup>2</sup>	\$228.00	Cold Milling Asphalt Pavement	\$15.00	m <sup>2</sup>
100mm Asphalt	1.52	m <sup>3</sup>	\$731.12	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne

Total Cost of Repaving, per meter of Roadway = \$959.12

Asphalt (m<sup>3</sup> to Tonne) = 2.60 Granular (m<sup>3</sup> to Tonne) = 2.35

 $1m^2 = 0.0001$ 

### **Unit Rate Development**

## 8 Street (16 Avenue to 75 Avenue) - Upgrade Roadway

Build pavement structure to Collector standard.

Assumed: Existing pavement width = 10.0m (from MESS).

			Unit Prices			
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit
Remove Existing Pavement						
Remove Asphalt	10.00	m <sup>2</sup>	\$200.00	Asphalt Removal	\$20.00	m <sup>2</sup>
Remove Base Course	2.50	m <sup>3</sup>	\$42.50	Excavation to Waste	\$17.00	m <sup>3</sup>
Construct Pavement Structure						
100mm Asphalt	0.74	m <sup>3</sup>	\$355.36	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Prime Coat	7.37	m <sup>2</sup>	\$14.75	Prime Coat	\$2.00	m <sup>2</sup>
300mm Granular Base	2.57	m <sup>2</sup>	\$74.64	3/4 Granular	\$29.00	Tonne
150mm Granular Subbase	1.34	m <sup>3</sup>	\$91.52	3/4 Granular	\$29.00	Tonne
150mm Subgrade Preparation	8.95	m <sup>3</sup>	\$189.35	Subgrade Preparation	\$9.00	m <sup>2</sup>
Curb & Gutter	2.00	m <sup>2</sup>	\$240.00	Curb & Gutter	\$120.00	lm

Total Cost for 8 Street Roadway Upgrade, per metre of Roadway = \$1,208.12

# 20 Avenue (12 Street to 8 Street) - Build Roadway Build out as per 20-year Horizon

			Unit Prices		
Quantity	Unit	Cost	Description	Unit Price	Unit
					ı
0.74	m <sup>3</sup>	\$355.36	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
7.37	m <sup>2</sup>	\$14.75	Prime Coat	\$2.00	m <sup>2</sup>
2.57	m <sup>2</sup>	\$74.64	3/4 Granular	\$29.00	Tonne
1.34	m <sup>3</sup>	\$91.52	3/4 Granular	\$29.00	Tonne
8.95	m <sup>3</sup>	\$189.35	Subgrade Preparation	\$9.00	m <sup>2</sup>
2.00	m <sup>2</sup>	\$240.00	Curb & Gutter	\$120.00	lm
	0.74 7.37 2.57 1.34 8.95 2.00	0.74 m <sup>2</sup> 7.37 m <sup>2</sup> 2.57 m <sup>2</sup> 1.34 m <sup>3</sup> 8.95 m <sup>3</sup> 2.00 m <sup>2</sup>	0.74 m <sup>3</sup> \$355.36 7.37 m <sup>2</sup> \$14.75 2.57 m <sup>2</sup> \$74.64 1.34 m <sup>3</sup> \$91.52 8.95 m <sup>3</sup> \$18.935 2.00 m <sup>2</sup> \$240.00	0.74 m³ \$355.36 Asphalt Concrete Pavement - PG Grade (-40) 7.37 m² \$14.75 Prime Coat 2.57 m² \$74.64 3/4 Granular 1.34 m³ \$91.52 3/4 Granular 8.95 m³ \$189.35 Subgrade Preparation 2.00 m² \$240.00 Curb & Gutter	0.74         m³         \$355.36         Asphalt Concrete Pavement - PG Grade (-40)         \$185.00           7.37         m²         \$14.75         Prime Coat         \$2.00           2.57         m²         \$74.64         3/4 Granular         \$29.00           1.34         m³         \$91.52         3/4 Granular         \$29.00           8.95         m³         \$189.35         Subgrade Preparation         \$9.00

Total Cost for 20 Avenue Construction, per metre of Roadway = \$965.62

## **Lakeshore Drive - Install Parking Control Signs**

			Unit Prices			
Component	Quantity	Unit	Cost	Description	Unit Price	Unit
Parking control (RB-51, RB-52) signs	11.00	signs	\$705.54	AT S288 - Install Sign - Less than 1 m <sup>2</sup>	\$64.14	sign
Sign posts	11.00	posts	\$2,011.68	AT S772 - Supply and Install Post (100mm x 150mm)	\$182.88	post
Т	otal Cost to Install Parking C	ontrol Signs=	\$2,717.22			

### Lakeshore Drive - Repave Corridor (Residential Local)

Mill and replace full depth of asphalt layer.

Assumed: 13.4 m road width along Lakeshore Drive (from In-Service Road Safety Review).

				Unit Prices		
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit
Coldmilling	13.40	m <sup>2</sup>	\$201.00	Cold Milling Asphalt Pavement	\$15.00	m <sup>2</sup>
90mm Asphalt	1.21	m <sup>3</sup>	\$580.09	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Total	Cost of Repaving, per meter	of roadway =	\$781.09			

### Lakeshore Drive - Improve Pedestrian Crosswalks

Lakeshore Brive Improve reacstrian Grosswants						
				Unit Prices		
Component	Quantity	Unit	Cost	Description	Unit Price	Unit
Pedestrian Crossing - Pavement marking (8 Ave, midblock 8 Ave & 7 Ave, 7 Ave, 6 Ave, 2 Ave)	5.00	message	\$1,198.90	AT S327 - Pavement Messages - Pedestrian Crossing	\$239.78	message
Pedestrian Crosswalk (RA-4) signs at crosswalks	10.00	signs	\$641.40	AT S288 - Install Sign - Less than 1 m <sup>2</sup>	\$64.14	sign
Sign posts at crosswalks	10.00	posts	\$1,828.80	AT S772 - Supply and Install Post (100mm x 150mm)	\$182.88	post
Curb extensions at 6 Avenue Crosswalk						ı
Curb	30.40	lm	\$3,648.00	Curb & Gutter	\$120.00	lm
Concrete	18.80	m <sup>2</sup>	\$2,157.11	AT X350 - Solid Concrete Islands	\$114.74	m <sup>2</sup>
Fill - Assume 0.305m depth (height of curb)	7.50	m <sup>3</sup>	\$511.13	3/4 Granular	\$29.00	Tonne

Total Cost to Improve Pedestrian Crosswalks along Lakeshore Drive = \$9,985.34

Asphalt (m³ to Tonne) = 2.60 Granular (m<sup>3</sup> to Tonne) = 2.35  $1m^2 = 0.0001$ 

## **Unit Rate Development**

# 75 Avenue (Highway 28 to Future Arterial) - Roadway Upgrade Build pavement structure to Collector standard. Assumed: Existing pavement width = 10.0m (from MESS).

				Unit Prices		
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit
Remove Existing Pavement						
Remove Asphalt	10.00	m <sup>2</sup>	\$200.00	Asphalt Removal	\$20.00	m <sup>2</sup>
Remove Base Course	2.50	m <sup>3</sup>	\$42.50	Excavation to Waste	\$17.00	m <sup>3</sup>
Construct Pavement Structure						
100mm Asphalt	1.48	m <sup>3</sup>	\$711.40	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Prime Coat	14.78	m <sup>2</sup>	\$29.55	Prime Coat	\$2.00	m <sup>2</sup>
300mm Granular Base	4.79	m <sup>3</sup>	\$326.74	3/4 Granular	\$29.00	Tonne
150mm Granular Subbase	2.45	m <sup>3</sup>	\$167.19	3/4 Granular	\$29.00	Tonne
150mm Subgrade Preparation	16.36	m <sup>2</sup>	\$147.20	Subgrade Preparation	\$9.00	m <sup>2</sup>
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm

Total Cost for 75 Avenue Roadway Upgrade, per metre of Roadway = \$1,864.57

## 69 Avenue (Glenwood to Highway 28) - Build Roadway Build out as per 20-year Horizon

				Unit Prices		
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit
Construct Pavement Structure						
125mm Asphalt	1.01	m <sup>3</sup>	\$488.02	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Tack Coat	8.83	m <sup>2</sup>	\$17.66	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>
Prime Coat	8.83	m <sup>2</sup>	\$17.66	Prime Coat	\$2.00	m <sup>2</sup>
200mm Granular Base	1.99	m <sup>3</sup>	\$135.95	3/4 Granular	\$29.00	Tonne
300mm Granular Subbase	3.85	m <sup>3</sup>	\$262.36	3/4 Granular	\$29.00	Tonne
150mm Subgrade Preparation	14.55	m <sup>2</sup>	\$130.92	Subgrade Preparation	\$9.00	m <sup>2</sup>

Total Cost for 69 Avenue Construction, per metre of Roadway = \$1,052.58

## 54 Avenue (56 Street to 49 Street) - Roadway Upgrade Widen to provide 2 lanes in each direction.

				Unit Prices		
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit
Sawcut Existing Asphalt	2.00	lm	\$40.00	Sawcutting	\$20.00	lm
Remove Existing Curb & Gutter	2.00	lm	\$34.00	Curb & Gutter Removal	\$17.00	lm
Remove Existing Sidewalk - 2m wide, assumed along both sides	4.00	m <sup>2</sup>	\$80.00	Sidewalk Removal	\$20.00	m <sup>2</sup>
Construct Pavement Structure						
100mm Asphalt	0.84	m <sup>3</sup>	\$403.46	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Prime Coat	8.38	m <sup>2</sup>	\$16.75	Prime Coat	\$2.00	m <sup>2</sup>
300mm Granular Base	2.87	m <sup>3</sup>	\$195.85	3/4 Granular	\$29.00	Tonne
150mm Granular Subbase	1.49	m <sup>3</sup>	\$101.75	3/4 Granular	\$29.00	Tonne
150mm Subgrade Preparation	9.95	m <sup>2</sup>	\$89.58	Subgrade Preparation	\$9.00	m <sup>2</sup>
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm

Total Cost for 54 Avenue Roadway Upgrade, per metre of Roadway = \$1,201.39

## 54 Avenue (49 Street to Future Arterial) - Build Roadway

Build out as per 20-year Horizon						
				Unit Prices		
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit
Construct Pavement Structure						
100mm Asphalt	1.48	m <sup>3</sup>	\$711.40	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Prime Coat	14.78	m <sup>2</sup>	\$29.55	Prime Coat	\$2.00	m <sup>2</sup>
300mm Granular Base	4.79	m <sup>3</sup>	\$326.74	3/4 Granular	\$29.00	Tonne
150mm Granular Subbase	2.45	m <sup>3</sup>	\$167.19	3/4 Granular	\$29.00	Tonne
150mm Subgrade Preparation	16.36	m <sup>2</sup>	\$147.20	Subgrade Preparation	\$9.00	m <sup>2</sup>
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm

Total Cost for 54 Avneue Construction, per metre of Roadway = \$1,622.07

Asphalt (m<sup>3</sup> to Tonne) = 2.60 Granular (m<sup>3</sup> to Tonne) = 2.35  $1m^2 = 0.0001$ 

#### **Unit Rate Development**

#### 52 Avenue (57 Street to Highway 28) - Roadway Upgrade Widen to provide 2 lanes in each direction.

				Unit Prices		
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit
Sawcut Existing Asphalt	2.00	lm	\$40.00	Sawcutting	\$20.00	lm
Remove Existing Curb & Gutter	2.00	lm	\$34.00	Curb & Gutter Removal	\$17.00	lm
Remove Existing Sidewalk - 2m wide, assumed along one side	2.00	m <sup>2</sup>	\$40.00	Sidewalk Removal	\$20.00	m <sup>2</sup>
Construct Pavement Structure						
100mm Asphalt	0.84	m <sup>3</sup>	\$403.46	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Prime Coat	8.38	m <sup>2</sup>	\$16.75	Prime Coat	\$2.00	m <sup>2</sup>
300mm Granular Base	2.87	m <sup>3</sup>	\$195.85	3/4 Granular	\$29.00	Tonne
150mm Granular Subbase	1.49	m <sup>3</sup>	\$101.75	3/4 Granular	\$29.00	Tonne
150mm Subgrade Preparation	9.95	m <sup>2</sup>	\$89.58	Subgrade Preparation	\$9.00	m <sup>2</sup>
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm

Total Cost for 52 Avenue Roadway Upgrade, per metre of Roadway = \$1,161.39

#### 51 Avenue (56 Street to Service Road) - Relocate Crosswalk

				Unit Prices		
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit
Remove existing pavement marking	70.00	lm	\$533.40	AT S375 - Removal of Existing Painted Lines	\$7.62	m
Pedestrian Crossing - Pavement marking	1.00	message	\$239.78	AT S327 - Pavement Messages - Pedestrian Crossing	\$239.78	message
	Total Cost for Crosswalk Relocation = \$773.18		\$773.18			

Centre Avenue (59 Street to 57 Street) - Roadway Upgrade
Widen to provide centre median and 2 lanes in each direction. 2.5% normal crown, asphalt placed in two lifts, tack coat between lifts.

				Unit Prices			
Pavement Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit	
Sawcut Existing Asphalt	2.00	lm	\$40.00	Sawcutting	\$20.00	lm	
Construct 5.0m Centre Median - Pin curb and gutter on existing asphalt	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm	
Construct Additional Pavement Structure							
125mm Asphalt	1.67	m <sup>3</sup>	\$804.91	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne	
Tack Coat	13.37	m <sup>2</sup>	\$26.74	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>	
Prime Coat	13.37	m <sup>2</sup>	\$26.74	Prime Coat	\$2.00	m <sup>2</sup>	
200mm Granular Base	2.92	m <sup>3</sup>	\$198.90	3/4 Granular	\$29.00	Tonne	
300mm Granular Subbase	4.49	m <sup>3</sup>	\$305.75	3/4 Granular	\$29.00	Tonne	
150mm Subgrade Preparation	14.95	m <sup>2</sup>	\$134.59	Subgrade Preparation	\$9.00	m <sup>2</sup>	
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm	

Total Cost for Centre Avenue Roadway Upgrade, per metre of Roadway = \$2,017.63

#### Centre Avenue (57 Avenue to Highway 28) - Roadway Upgrade Widen to provide centre median

widen to provide centre median.							
				Unit Prices			
Pavement Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit	
Sawcut Existing Asphalt	2.00	lm	\$40.00	Sawcutting	\$20.00	lm	
Remove Existing Curb & Gutter	2.00	lm	\$34.00	Curb & Gutter Removal	\$17.00	lm	
Construct 5.0m Centre Median - Pin curb and gutter on existing asphalt	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm	
Construct Additional Pavement Structure							
125mm Asphalt	0.75	m <sup>3</sup>	\$359.79	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne	
Tack Coat	5.97	m <sup>2</sup>	\$11.93	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>	
Prime Coat	5.97	m <sup>2</sup>	\$11.93	Prime Coat	\$2.00	m <sup>2</sup>	
200mm Granular Base	1.44	m <sup>3</sup>	\$98.00	3/4 Granular	\$29.00	Tonne	
300mm Granular Subbase	2.27	m <sup>3</sup>	\$154.41	3/4 Granular	\$29.00	Tonne	
150mm Subgrade Preparation	7.55	m <sup>2</sup>	\$67.97	Subgrade Preparation	\$9.00	m <sup>2</sup>	
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm	

Total Cost for Centre Avenue Roadway Upgrade, per metre of Roadway = \$1,258.04

Asphalt (m<sup>3</sup> to Tonne) = 2.60 Granular (m<sup>3</sup> to Tonne) = 2.35  $1m^2 = 0.0001$ 

#### **Unit Rate Development**

#### 50 Avenue - Provide Back-in Angle Parking Stalls

Assumed: City would implement back-in angle parking over parallel parking. 15.0m solid white lines for each parking stall.

				Unit Prices		
Component	Quantity	Unit	Cost	Description	Unit Price	Unit
Remove existing pavement marking	70.00	m	\$533.40	AT S375 - Removal of Existing Painted Lines	\$7.62	э
Pavement markings	1.00	lm	\$0.80	AT S350 - Roadway Lines - Supplying Paint and Painting (Directional Dividing)	\$800.53	km
Total Cost to Remove Existing and Provide New Pavement Marking Lines, per Angle Parking Stall = \$534.			\$534.20			

#### 50 Avenue - Repaint Pavement Marking (Highway 28 to 49 Street)

Assumed: 6.0m solid white lines for each parallel parking stall

				Unit Prices			
Component	Quantity	Unit	Cost	Description	Unit Price	Unit	
Repaint centre line	632.00	m	\$505.93	AT S350 - Roadway Lines - Supplying Paint and Painting (Directional Dividing)	\$800.53	km	
Repaint stop bar	20.00	message	\$2,578.60	AT S315 - Pavement Message - Stop Bar	\$128.93	message	
Painted on-street parking (parallel stalls) - 53 stalls	318.00	m	\$254.57	AT S350 - Roadway Lines - Supplying Paint and Painting (Directional Dividing)	\$800.53	km	
Pedestrian Crossing - Pavement marking (At 53 St, 52 St, 51 St, midblock 51 St/50 St, 50 St, @ 49 St)	20.00	message	\$4,795.60	AT S327 - Pavement Messages - Pedestrian Crossing	\$239.78	message	

Total Cost to Repaint Pavement Marking along 50 Avenue = \$8,134.70

#### 50 Avenue - Provide Curb Extensions and Signage at Crosswalks

				Unit Prices			
Component	Quantity	Unit	Cost	Description	Unit Price	Unit	
Curb extensions - 18 (At 53 St, 52 St, 51 St, midblock 51 St/50 St, 50 St and 49 St)							
Curb	273.60	lm	\$32,832.00	Curb & Gutter	\$120.00	lm	
Concrete	169.20	m <sup>2</sup>	\$19,414.01	AT X350 - Solid Concrete Islands	\$114.74	m <sup>2</sup>	
Fill - Assume 0.305m depth (height of curb)	67.50	m <sup>3</sup>	\$4,600.13	3/4 Granular	\$29.00	Tonne	
Pedestrian Crosswalk (RA-4) signs at 49 Street and 53 Street	4.00	signs	\$256.56	AT S288 - Install Sign - Less than 1 m <sup>2</sup>	\$64.14	sign	
Sign posts at 49 Street and 53 Street	4.00	posts	\$731.52	AT S772 - Supply and Install Post (100mm x 150mm)	\$182.88	post	

Total Cost to Provide Curb Extensions and Signage at Crosswalks = \$57,834.21

#### Highway 28 and 43 Avenue Intersection Improvements

				Alberta Transportation Unit Price (2010)			
Component	Quantity	Unit	Cost	Description	Unit Price	Unit	
Pavement							
125mm Asphalt	2,390.00	m <sup>3</sup>	\$1,149,590.00	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne	
Tack Coat	19,120.00	m <sup>2</sup>	\$38,240.00	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>	
Prime Coat	19,120.00	m <sup>2</sup>	\$38,240.00	Prime Coat	\$2.00	m <sup>2</sup>	
200mm Granular Base	4,656.00	m <sup>3</sup>	\$317,306.40	3/4 Granular	\$29.00	Tonne	
300mm Granular Subbase	6,984.00	m <sup>3</sup>	\$475,959.60	3/4 Granular	\$29.00	Tonne	
150mm Subgrade Preparation	23,280.00	m <sup>2</sup>	\$209,520.00	Subgrade Preparation	\$9.00	m <sup>2</sup>	
Curb & Gutter	3,150.00	lm	\$378,000.00	Curb & Gutter	\$120.00	lm	
Median Fill - Assume 0.305m depth (height of curb)	1,232.20	m <sup>3</sup>	\$83,974.43	3/4 Granular	\$29.00	Tonne	
Concrete Island	290.00	m <sup>2</sup>	\$33,274.60	AT X350 - Solid Concrete Islands	\$114.74	m <sup>2</sup>	
Concrete Sidewalks	1,190.00	lm	\$142,800.00	Concrete Sidewalk	\$120.00	lm	
Pavement Marking - Solid Lines	1,080.00	lm	\$864.57	AT S350 - Roadway Lines - Supplying Paint and Painting (Direction Dividing)	\$800.53	km	
Pavement Marking - Dashed Lines	2,415.00	lm	\$1,966.56	AT S351 - Roadway Lines - Supplying Paint and Painting (Lane Dividing)	\$814.31	km	
Repaint stop bar	4.00	message	\$515.72	AT S315 - Pavement Message - Stop Bar	\$128.93	message	
Crosswalk Marking	7.00	message	\$1,678.46	ATS327 - Pavement Messages - Pedestrian Crossing	\$239.78	message	

Total Cost for Highway 28 and 43 Avenue Improvements = \$2,871,930.34

#### 43 Avenue (Highway 28 to 45 Street) - Upgrade Roadway Build pavement structure to Collector standard.

Assumed: Existing pavement width = 10.0m (from MESS).

				Unit Prices			
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit	
Remove Existing Pavement							
Remove Asphalt	10.00	m <sup>2</sup>	\$200.00	Asphalt Removal	\$20.00	m <sup>2</sup>	
Remove Base Course	2.50	m <sup>3</sup>	\$42.50	Excavation to Waste	\$17.00	m <sup>3</sup>	
Remove Existing Curb & Gutter	2.00	lm	\$34.00	Curb & Gutter Removal	\$17.00	lm	
Construct Pavement Structure							
100mm Asphalt	1.48	m <sup>3</sup>	\$711.40	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne	
Prime Coat	14.78	m <sup>2</sup>	\$29.55	Prime Coat	\$2.00	m <sup>2</sup>	
300mm Granular Base	4.79	m <sup>3</sup>	\$326.74	3/4 Granular	\$29.00	Tonne	
150mm Granular Subbase	2.45	m <sup>3</sup>	\$167.19	3/4 Granular	\$29.00	Tonne	
150mm Subgrade Preparation	16.36	m <sup>2</sup>	\$147.20	Subgrade Preparation	\$9.00	m <sup>2</sup>	
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm	

Total Cost for 43 Avenue Roadway Upgrade, per meter of Roadway = \$1,898.57

Asphalt (m<sup>3</sup> to Tonne) = 2.60 Granular (m<sup>3</sup> to Tonne) = 2.35  $1m^2 = 0.0001$ 

#### **Unit Rate Development**

#### Kingsway (Medley Gate to 59 Street) - Roadway Upgrade

Widen to provide centre median and 2 lanes in each direction. 2.5% normal crown, asphalt placed in two lifts, tack coat between lifts.

				Unit Prices		
Pavement Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit
Sawcut Existing Asphalt	2.00	lm	\$40.00	Sawcutting	\$20.00	lm
Construct 5.0m Centre Median - Pin curb and gutter on existing asphalt	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm
Construct Additional Pavement Structure						
125mm Asphalt	1.76	m <sup>3</sup>	\$848.87	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Tack Coat	14.83	m <sup>2</sup>	\$29.67	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>
Prime Coat	14.83	m <sup>2</sup>	\$29.67	Prime Coat	\$2.00	m <sup>2</sup>
200mm Granular Base	3.20	m <sup>3</sup>	\$217.77	3/4 Granular	\$29.00	Tonne
300mm Granular Subbase	5.65	m <sup>3</sup>	\$385.10	3/4 Granular	\$29.00	Tonne
150mm Subgrade Preparation	20.55	m <sup>2</sup>	\$184.96	Subgrade Preparation	\$9.00	m <sup>2</sup>

Total Cost for Kingsway Roadway Upgrade, per metre of Roadway = \$1,976.04

Kingsway (Glenwood to Medley Gate) - Roadway Upgrade
Widen to provide centre median and 2 lanes in each direction. 2.5% normal crown, asphalt placed in two lifts, tack coat between lifts.

				Unit Prices		
Pavement Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit
Sawcut Existing Asphalt	2.00	lm	\$40.00	Sawcutting	\$20.00	lm
Construct 5.0m Centre Median - Pin curb and gutter on existing asphalt	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm
Construct Additional Pavement Structure						
125mm Asphalt	1.67	m <sup>3</sup>	\$804.91	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Tack Coat	13.37	m <sup>2</sup>	\$26.74	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>
Prime Coat	13.37	m <sup>2</sup>	\$26.74	Prime Coat	\$2.00	m <sup>2</sup>
200mm Granular Base	2.92	m <sup>3</sup>	\$198.90	3/4 Granular	\$29.00	Tonne
300mm Granular Subbase	4.49	m <sup>3</sup>	\$305.75	3/4 Granular	\$29.00	Tonne
150mm Subgrade Preparation	14.95	m <sup>2</sup>	\$134.59	Subgrade Preparation	\$9.00	m <sup>2</sup>
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm

Total Cost for Kingsway Roadway Upgrade, per metre of Roadway = \$2,017.63

Kingsway (Timberline to Glenwood) - Roadway Upgrade
Build pavement structure to Arterial Standard (centre median with 2 lanes in each direction). 2.5% normal crown, asphalt placed in two lifts, tack coat between lifts.
Median assumed to have same pavement structure as roadway, in preparation for designated turn lanes.
Assumed: Existing pavement width = 13.0m (from MESS).

Assumed: Existing pavement width = 13.0m (from MESS).				Unit Prices		
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description Unit Prices	Unit Price	Unit
Remove Existing Pavement	Quantity	Oilit	0031	Description	Olik i lice	Oint
Remove Asphalt	13.00	m <sup>2</sup>	\$260.00	Asphalt Removal	\$20.00	m <sup>2</sup>
Remove Base Course	3.90	m <sup>3</sup>	\$66.30	Excavation to Waste	\$17.00	m <sup>3</sup>
Remove Existing Curb & Gutter	2.00	lm	\$34.00	Curb & Gutter Removal	\$17.00	lm
Remove Existing Sidewalk - 2m wide along both sides	4.00	m <sup>2</sup>	\$80.00	Sidewalk Removal	\$20.00	m <sup>2</sup>
Construct 5.0m Centre Median						
125mm Asphalt	0.63	m <sup>3</sup>	\$300.67	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Tack Coat	5.00	m <sup>2</sup>	\$10.00	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>
Prime Coat	5.00	m <sup>2</sup>	\$10.00	Prime Coat	\$2.00	m <sup>2</sup>
200mm Granular Base	1.00	m <sup>3</sup>	\$68.17	3/4 Granular	\$29.00	Tonne
300mm Granular Subbase	1.50	m <sup>3</sup>	\$102.26	3/4 Granular	\$29.00	Tonne
150mm Subgrade Preparation	5.00	m <sup>2</sup>	\$45.01	Subgrade Preparation	\$9.00	m <sup>2</sup>
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm
Construct Pavement Structure						
125mm Asphalt	1.85	m <sup>3</sup>	\$889.08	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Tack Coat	14.77	m <sup>2</sup>	\$29.54	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>
Prime Coat	14.77	m <sup>2</sup>	\$29.54	Prime Coat	\$2.00	m <sup>2</sup>
200mm Granular Base	3.20	m <sup>3</sup>	\$217.98	3/4 Granular	\$29.00	Tonne
300mm Granular Subbase	4.91	m <sup>3</sup>	\$334.38	3/4 Granular	\$29.00	Tonne
150mm Subgrade Preparation	16.36	m <sup>2</sup>	\$147.20	Subgrade Preparation	\$9.00	m <sup>2</sup>
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm

Total Cost for Kingsway Roadway Upgrade, per metre of Roadway = \$3,104.14

Asphalt (m<sup>3</sup> to Tonne) = 2.60 Granular (m<sup>3</sup> to Tonne) = 2.35  $1m^2 = 0.0001$  ha

#### **Unit Rate Development**

Glenwood (69 Avenue to Kingsway) - Roadway Upgrade
Build pavement structure to Arterial Standard (2 lanes in each direction). 2.5% normal crown, asphalt placed in two lifts, tack coat between lifts.
Assumed: Existing pavement width = 13.0m (from MESS).

				Unit Prices		
Component - 1.0m Section of Roadway	Quantity	Unit	Cost	Description	Unit Price	Unit
Remove Existing Pavement					-	-
Remove Asphalt	13.00	m <sup>2</sup>	\$260.00	Asphalt Removal	\$20.00	m <sup>2</sup>
Remove Base Course	3.90	m <sup>3</sup>	\$66.30	Excavation to Waste	\$17.00	m <sup>3</sup>
Construct Pavement Structure						
125mm Asphalt	1.85	m <sup>3</sup>	\$889.08	Asphalt Concrete Pavement - PG Grade (-40)	\$185.00	Tonne
Tack Coat	14.77	m <sup>2</sup>	\$29.54	Tack Coat Between Asphalt Lifts	\$2.00	m <sup>2</sup>
Prime Coat	14.77	m <sup>2</sup>	\$29.54	Prime Coat	\$2.00	m <sup>2</sup>
200mm Granular Base	3.20	m <sup>3</sup>	\$217.98	3/4 Granular	\$29.00	Tonne
300mm Granular Subbase	4.91	m <sup>3</sup>	\$334.38	3/4 Granular	\$29.00	Tonne
150mm Subgrade Preparation	16.36	m <sup>2</sup>	\$147.20	Subgrade Preparation	\$9.00	m <sup>2</sup>
Curb & Gutter	2.00	lm	\$240.00	Curb & Gutter	\$120.00	lm

Total Cost for Glenwood Roadway Upgrade, per metre of Roadway = \$2,214.01

# **Technical Memorandum**

## **City of Cold Lake**

**Cold Lake Transportation Study Traffic Volume Forecast and Analysis** 

### **April 2011**



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#### City of Cold Lake

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## Introduction

The City of Cold Lake (City) is located 287 km northeast of the City of Edmonton and was formed in 1996 by merging three municipalities, namely Grand Centre, Medley (Canadian Forces Base W4) and Cold Lake. Grand Centre was subsequently renamed Cold Lake South (CLS) and the original Cold Lake is now known as Cold Lake North (CLN).

The City has experienced noticeable growth in recent years. According to municipal census the City had a population of 11,991 in 2006 and 13,924 in 2009. This corresponds to a 5.4% linear growth annually. Current transportation improvements within the City have been based on the previous transportation study completed in 2000 (2000 Transportation Study) and is no longer considered representative of the actual transportation network required to address current and future transportation needs.

In light of the continuing accelerated pace of development in the region and the need to rationalize and identify the transportation network requirements for the City, including surrounding rural municipalities and counties, the existing transportation plan requires a comprehensive update.

#### 1.1 STUDY BACKGROUND

Associated Engineering (AE) was retained by the City to update the existing transportation study. The purpose of the transportation study is to provide a comprehensive long-range plan that integrates the transportation infrastructure with requirements of the existing and future land uses. The transportation study will provide the City with a blueprint on which to plan and implement specific transportation network improvement projects over the next 20 years in 5-year, 10-year, 15-year, and 20-year planning horizons.

One component of the transportation study was to forecast the future traffic volumes for the next 20 years. Traffic volumes were forecasted for the 5-year, 10-year, 15-year, and 20-year planning horizons, and analyzed to determine the future road classification and the number of lanes required to accommodate the future traffic volumes. This technical memorandum presents the methodology used and the results from the traffic volume forecast and analysis.

#### 1.2 STUDY AREA

The traffic volume forecast will encompass the area bounded by the current city limits. Figure 1.1 presents the study area.



#### 1.3 STUDY OBJECTIVE

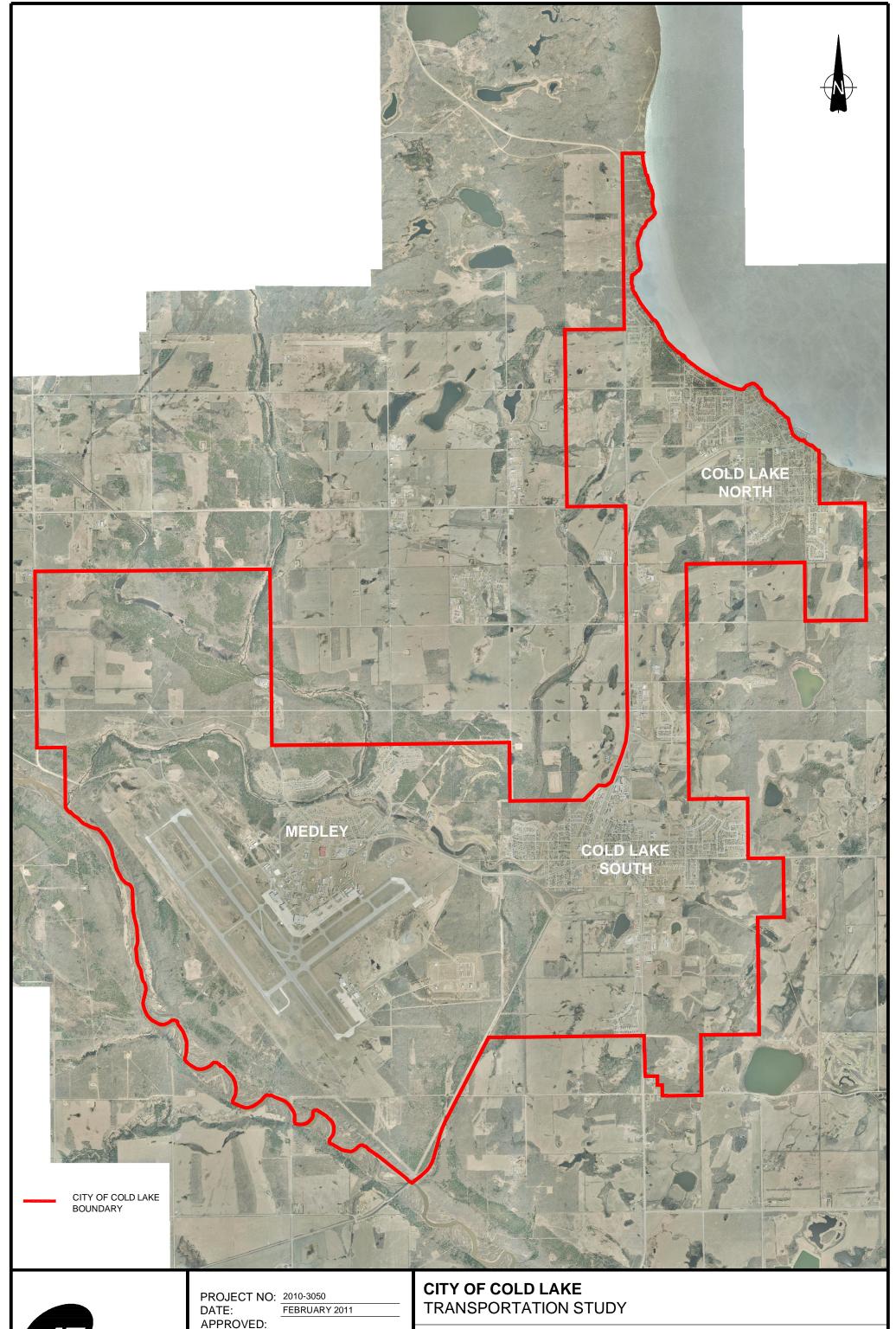
The objectives for the traffic volume forecast and analysis were:

- Forecast the future traffic volumes over the next 20 years, in 5-year planning horizons
- Establish the required road classification for the City's major road network over the next 20 years
- Determine the number of lanes required for the City's major road network over the next 20 years.

For the purpose of the study, the afternoon (p.m.) peak hour traffic volume forecast was considered to be the most critical and was selected for the analysis. The following planning horizons were analyzed:

- 5-year (2015) horizon
- 10-year (2020) horizon
- 15-year (2025) horizon
- 20-year (2030) horizon.

The road classification and number of lanes required to accommodate traffic under the 20-year (2030) horizon will be used by the City to preserve the right-of-way required to accommodate future roadway expansion.



Associated Engineering

APPROVED: SCALE:

NTS DWG NO:

FIGURE 1.1 TRAFFIC FORECAST STUDY AREA 2

## **Traffic Forecast: Methodology**

A spreadsheet model, following the four-step planning process, was used to forecast the future traffic volumes. To complete the spreadsheet model, a skeletal road network was developed for each planning horizon to represent the anticipated road network. The major road network (collectors and arterials) identified for each planning horizon in the 2000 Transportation Study were used to represent the skeletal road network for the respective planning horizon. Aside from alignment changes to English Bay Road/28 Street/25 Street in the northwest quadrant and the classification of Centre Avenue, between 57 Street and Highway 28, as a four-lane arterial, the major road networks presented in the 2000 Transportation Study were considered to be valid. Changes were made to the 5-year and 10-year road networks to reflect the current alignment of English Bay Road/28 Street/25 Street.

Future traffic within the City will be comprised of background traffic and development traffic. Background traffic represents the growth in existing traffic reflecting the additional trips generated in the surrounding areas and in the City's existing subdivisions. Future background traffic volumes for each planning horizon were estimated by applying an annual growth rate of 2% to the existing (2010) traffic volumes, over a 5-year, 10-year, 15-year, and 20-year period.

Development traffic represents traffic generated by new subdivisions or area redevelopment. The information about future development or redevelopment within the City was obtained from the City's Area Structure Plans (ASP), Area Redevelopment Plans (ARP) and Outline Pans, and from the Municipal District (MD) of Bonnyville's Intermunicipal Development Plan (IDP). Future development traffic volumes for each planning horizon were estimated using a four-step process, which involved:

- Trip Generation: Estimate the number of trips generated from and attracted to each development/redevelopment
- Trip Distribution: Estimate the origin and destination of trips to and from each development/redevelopment
- Modal Split: Not within the scope of the study
- **Trip Assignment**: Select the routes to and from the developments/redevelopments and assign the development traffic volumes to the City's road network.



3

## **Traffic Forecast: Background Traffic Volumes**

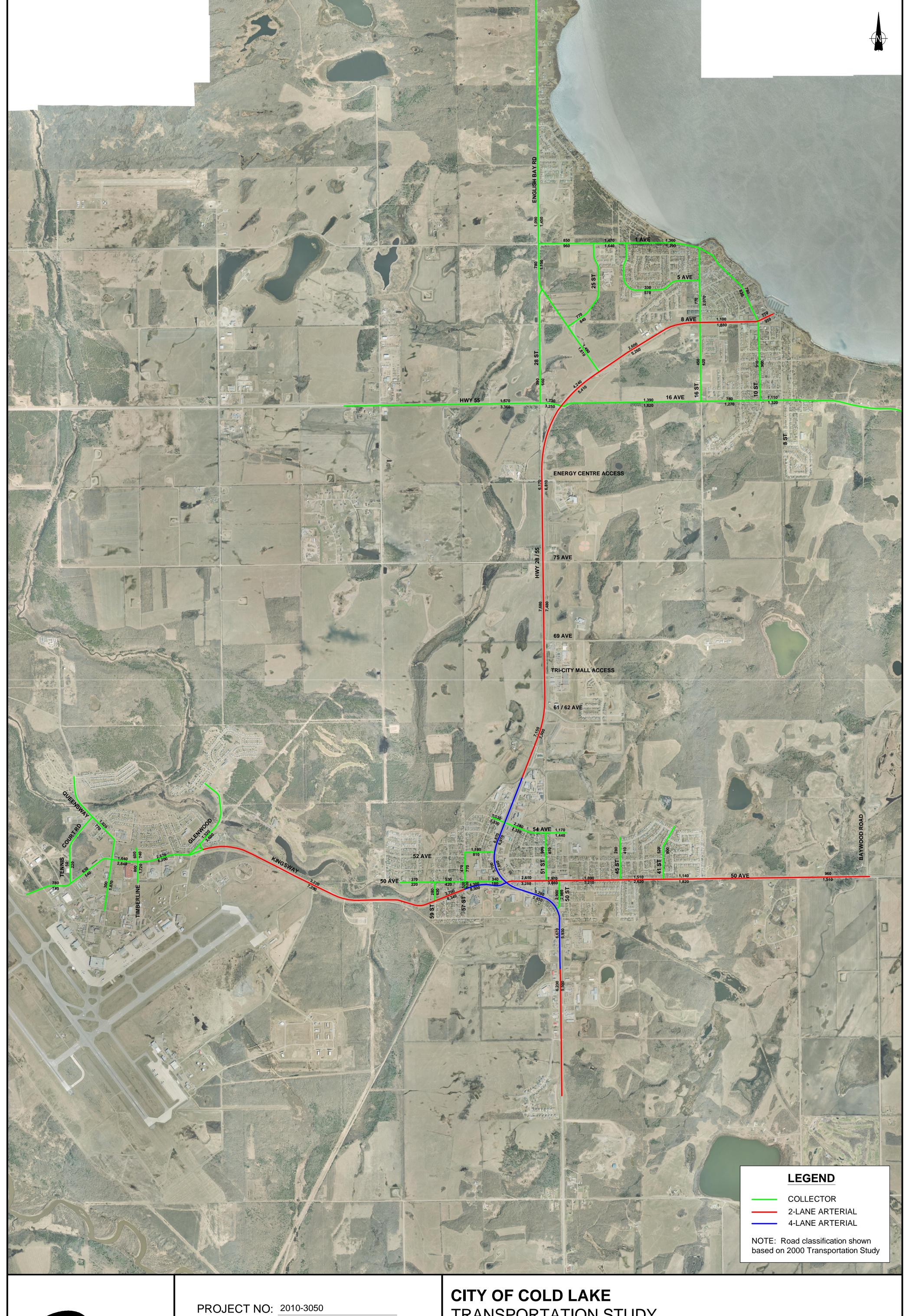
Growth rate calculations were completed as part of the existing condition analysis for the transportation study update and an annual, non-compounding, growth rate of 2.0% was established. The growth rate was selected after discussions with the City. The detailed growth rate calculations can be referenced in the technical memorandum titled Existing (2010) Traffic Operational Analysis.

#### 3.1 FORECASTED BACKGROUND TRAFFIC VOLUMES

Background traffic volumes for the planning horizons were estimated by applying the 2.0% annual growth rate to the existing (2010) traffic volumes, over a 5-year, 10-year, 15-year and 20-year period. The existing (2010) daily traffic volumes were obtained from the Existing (2010) Traffic Operational Analysis technical memorandum and presented in Figure 3.1.

Figure 3.2 through Figure 3.5 present the forecasted daily background traffic volumes for the 5-year, 10-year, 15-year and 20-year horizons.





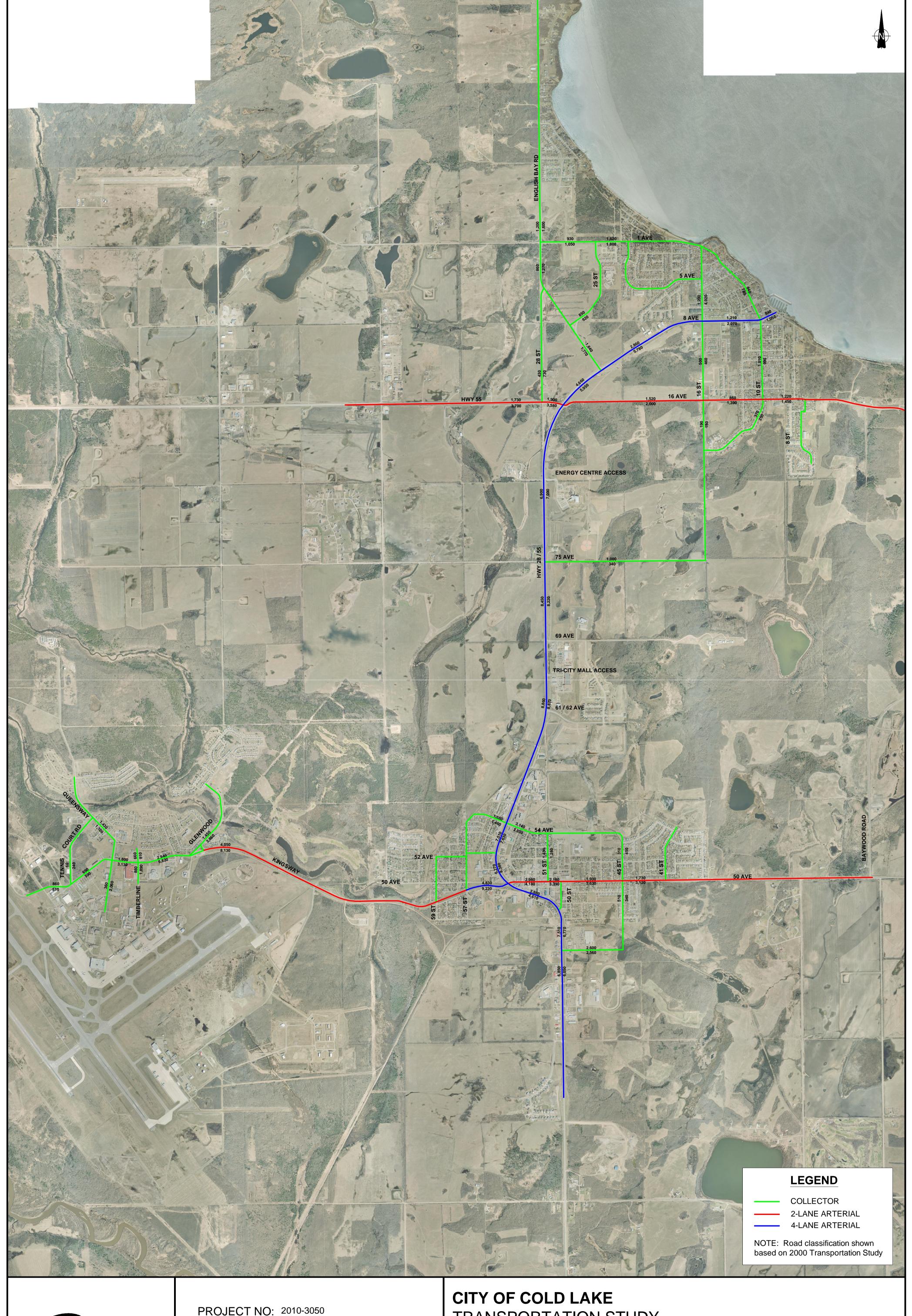


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## TRANSPORTATION STUDY

FIGURE 3.1 EXISTING (2010) DAILY TRAFFIC VOLUMES



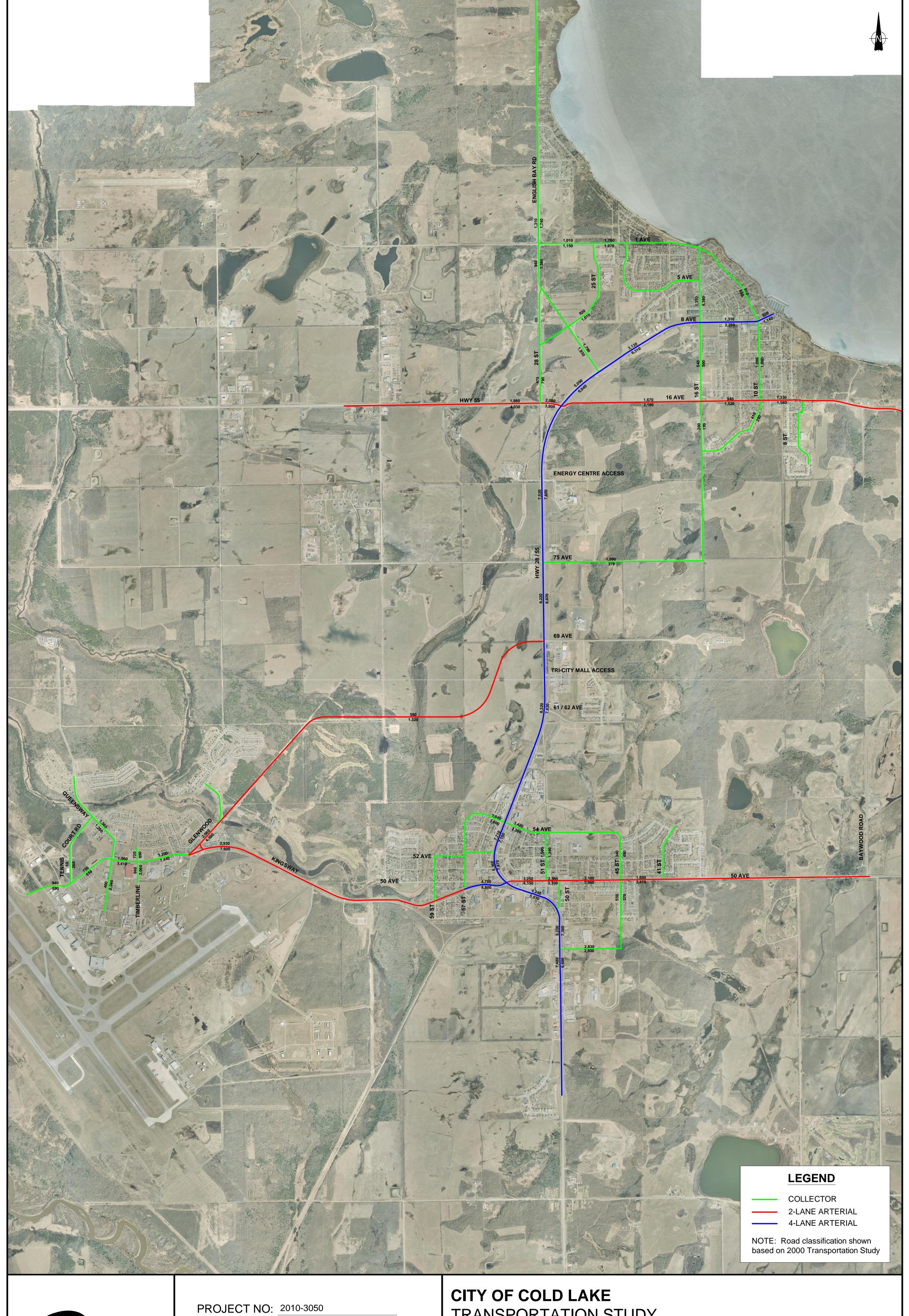


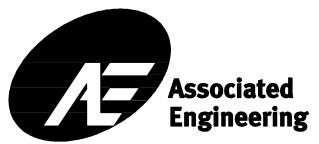
PROJECT NO: 2010-3050 DATE: **APRIL 2011** APPROVED:

NTS SCALE: DWG NO:

TRANSPORTATION STUDY

FIGURE 3.2 5 YEAR (2015) DAILY BACKGROUND TRAFFIC VOLUMES



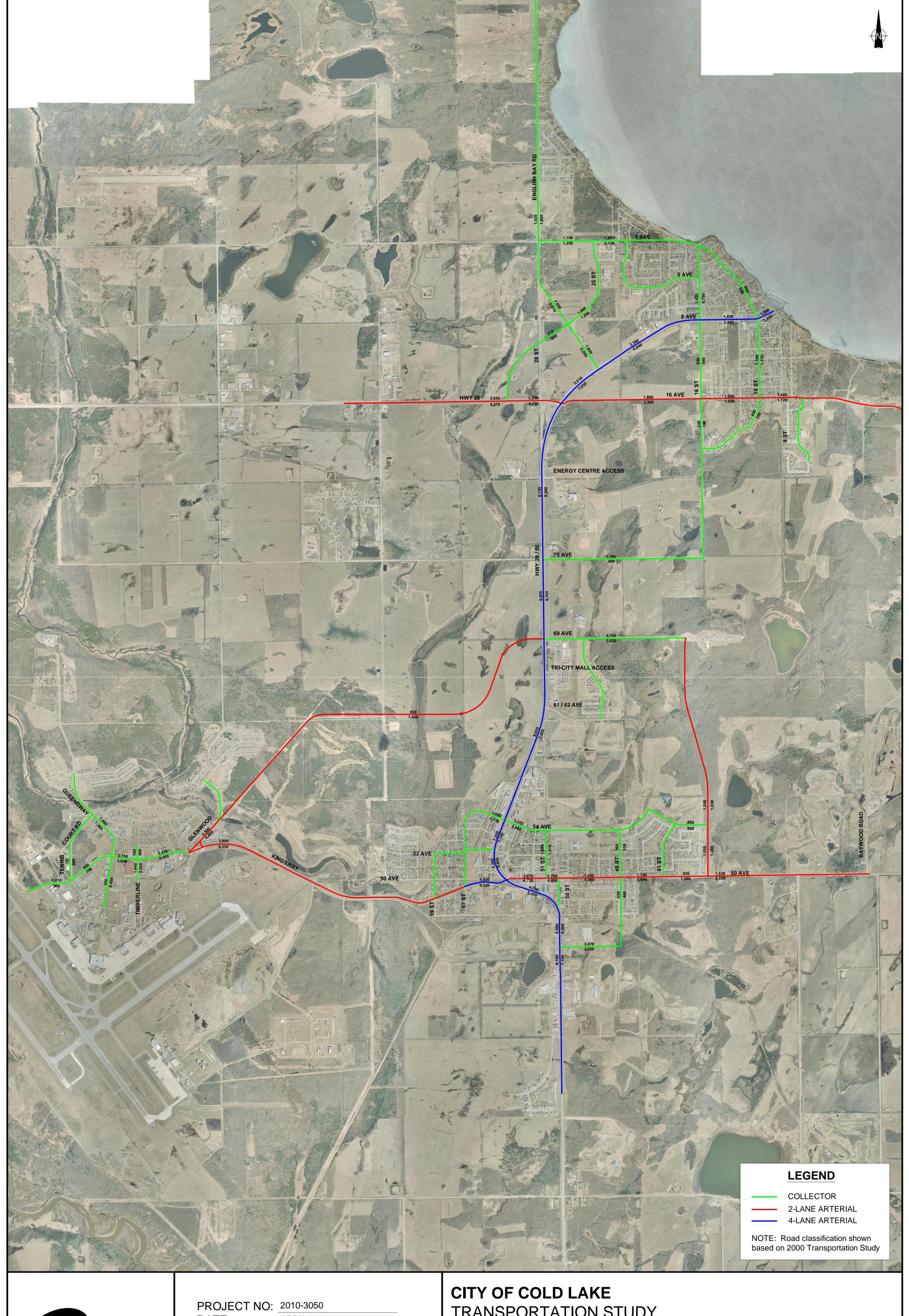


DATE: **APRIL 2011** APPROVED:

NTS SCALE: DWG NO:

# TRANSPORTATION STUDY

FIGURE 3.3 10 YEAR (2020) DAILY BACKGROUND TRAFFIC VOLUMES



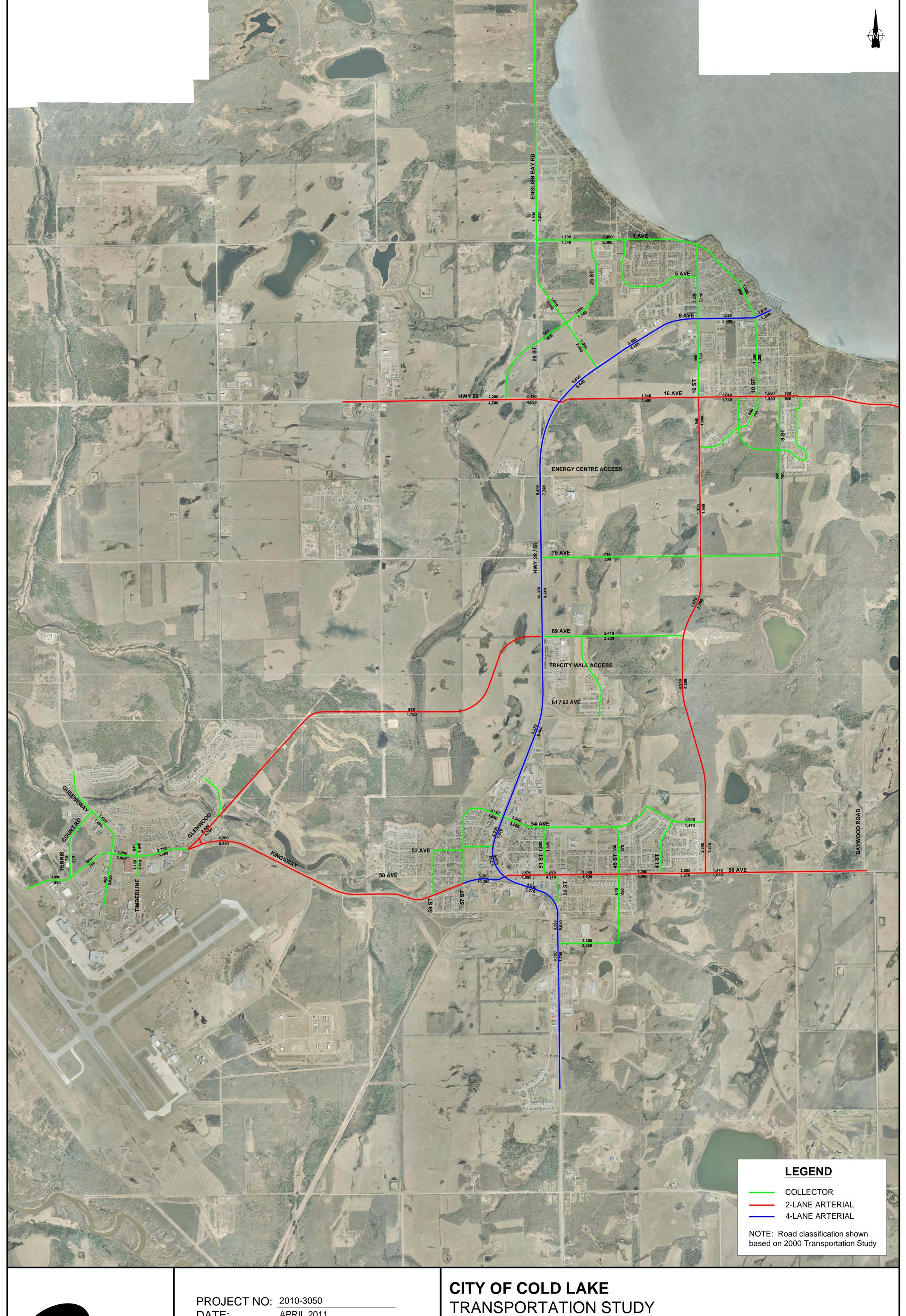


DATE: **APRIL 2011** APPROVED: NTS SCALE:

DWG NO:

TRANSPORTATION STUDY

FIGURE 3.4 15 YEAR (2025) DAILY BACKGROUND TRAFFIC VOLUMES





DATE: APPROVED:

DWG NO:

**APRIL 2011** NTS SCALE:

FIGURE 3.5 20 YEAR (2030) DAILY BACKGROUND TRAFFIC VOLUMES

## 4

## **Traffic Forecast: Future Development Traffic Volumes**

#### 4.1 FUTURE DEVELOPMENTS

There are 20 development/redevelopment projects identified for the City and surrounding area in the next 20 years; 13 are located within the City and seven are located outside the City, in the MD of Bonnyville. The development/redevelopment projects are shown in Figure 4.1 and are discussed in the following sections.

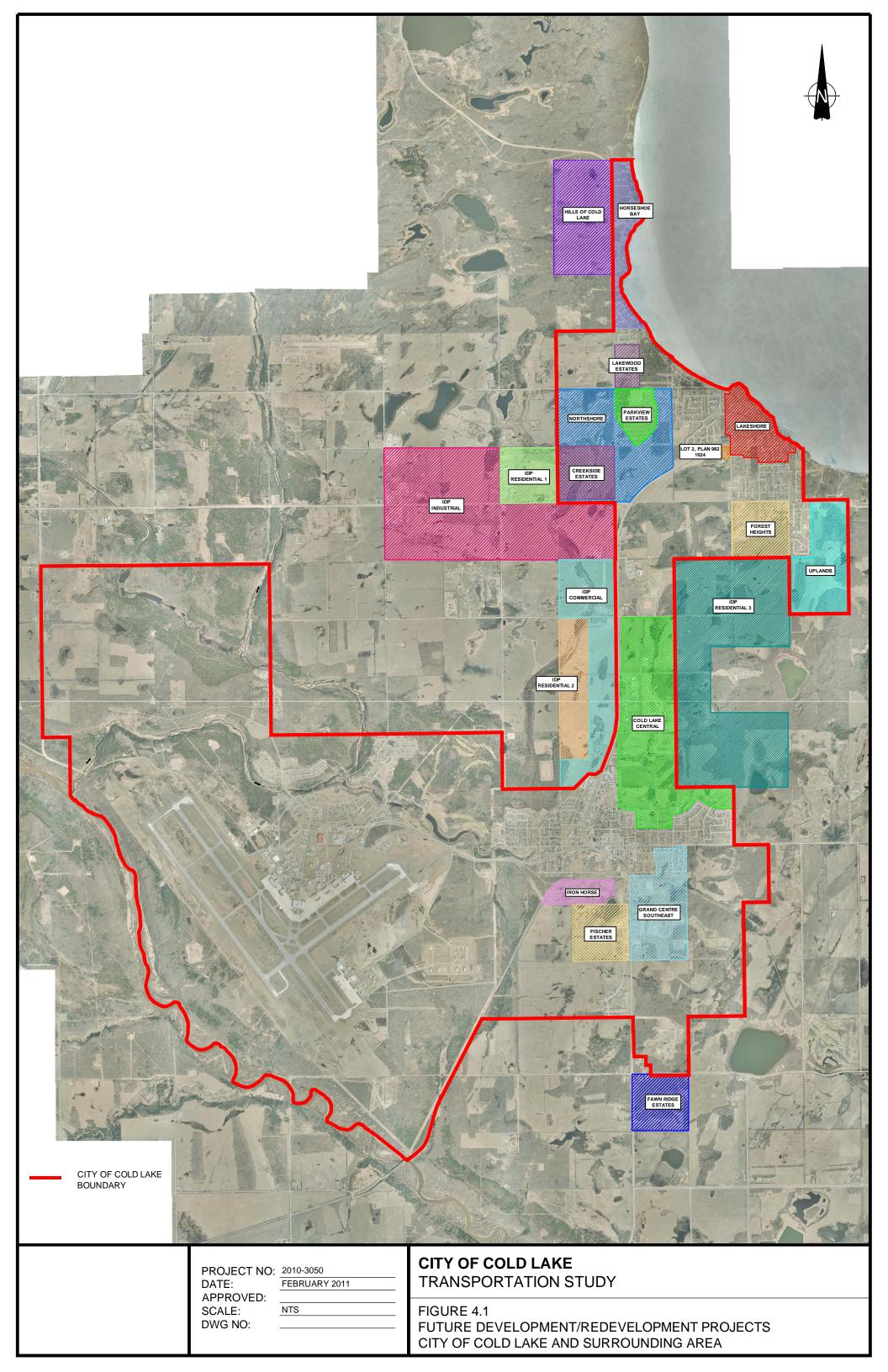
AE contacted the Department of Defence in Medley to obtain information about future land use growth or change within the base, and was informed that there would be no expected changes or growth within the 20-year planning horizon of the transportation study.

#### 4.1.1 City of Cold Lake

The 13 development/redevelopment projects expected within the City are:

- Fischer Estates: 63.5 hectares located in Cold Lake South (SE 1/4 34-62-2-4)
- Iron Horse: 30.77 hectares located in Cold Lake South (N ½ f 34-62-2-4)
- Cold Lake Central: 248.0 hectares located between Cold Lake North and Cold Lake South (W ½ 11-63-2-4, W ¼ 2-63-2-4, and S ½ 2-63-11-4)
- Grand Centre SE: 105.0 hectares located in Cold Lake South (W ½ 35-62-2-4)
- Forest Heights: 64.0 hectares located in Cold Lake North (NW ¼ o13-63-2-4)
- Northshore: 244.0 hectares located in Cold Lake North (NE ¼ 22-63-2-4, SE ¼ 22-63-2-4, SW ¼ 23-63-2-4, and NW ¼ 23-63-2-4)
- Lot 2, Plan 982 1024: 1.81 hectares located in Cold Lake North (SE ½ 23-63-2-4)
- Horseshoe Bay: 77.7 hectares located in Cold Lake North (NW ¼ 26-63-2-4, SW ¼ 35-63-2-4, and NW ¼ 35-63-2-4)
- Uplands: 101.9 hectares located in Cold Lake north (NE 13-63-2-4 and SE 13-63-2-4)
- Lakeshore Redevelopment: 66.0 hectares located in Cold Lake North.
- Lakewood Estates: 21.3 hectares located in Cold Lake North (SW ½ 26-63-2-4)
- Creekside Estates: 60.5 hectares located in Cold Lake North (SE 1/4 22-63-2-4)
- Parkview Estates: 36.8 hectares located in Cold Lake North (NW 23-63-2-4).





#### 4.1.2 MD of Bonnyville

The seven development projects expected outside the City, in the MD of Bonnyville are:

- **Hills of Cold Lake**: 119.3 hectares located northwest of Cold Lake North (SE, NE ¼ 34-63-2-4)
- Fawn Ridge Estates: 34.9 hectares located south of Cold Lake South (NW ½ 23-62-2-4)
- **IDP Residential Development 1:** 63 hectares located along the north side of Highway 55, west of the City
- **IDP Residential Development 2:** 84 hectares located west of the IDP Commercial Development, from 75 Avenue and south of 61/62 Avenue
- **IDP Residential Development 3:** 418 hectares located east of Cold Lake Central, from Energy Centre and 55 Avenue
- IDP Industrial Development: 392 hectares located along both sides of Highway 55, west of the City
- **IDP Commercial Development:** 157 hectares located along the west side of Highway 28, from Energy Centre to 55 Avenue.

#### 4.1.3 Development Phasing

The ASP, ARP and Outline Plans for the development/redevelopment projects provided by the City and the IDP did not discuss the expected timing or staging for the projects. Most of the documents stated that the timing would be dictated by market conditions and the availability of municipal servicing capacity.

To forecast the future development traffic volumes for each planning horizon, the following assumptions were made:

- Each development/redevelopment plan will experience 25% growth in each planning horizon with full build-out by 2030 except for Fischer Estates, Iron Horse, Forest Heights and the IDP developments
- Development of Fischer Estates, Iron Horse and Forest Heights will be delayed until 2020.
   By 2030, these three developments will be 50% developed
- Development of the residential land from the IDP will be delayed until 2015. By 2030, the three residential developments will be 30% developed
- Development of the industrial land from the IDP will be delayed until 2015. By 2030, the industrial development will be 20% developed
- Development of the commercial land from the IDP will be delayed until 2015. By 2030, the commercial development will be 30% developed.

The development phasing assumptions were established through discussions with the City's Planning Department. Table 4.1 summarizes the development phasing assumed for each planning horizon.



Table 4-1
Development Phasing Assumption

Development / Redevelopment	Land Use	5-year (2015) Horizon	10-year (2020) Horizon	15-year (2025) Horizon	20-year (2030) Horizon
Fischer Estates	Residential	0%	0%	25%	50%
FISCHEL ESTATES	Commercial	0%	0%	25%	50%
Iron Horse	Residential	0%	0%	25%	50%
Cold Loke Control	Residential	25%	50%	75%	100%
Cold Lake Central	Commercial	50%	100%	100%	100%
Grand Centre	Residential	25%	50%	75%	100%
Southeast	Industrial	25%	50%	75%	100%
Forest Heights	Residential	0%	0%	25%	50%
	Residential	25%	50%	75%	100%
Nowholowa	Commercial	25%	50%	75%	100%
Northshore	Institutional	25%	50%	75%	100%
	School	0%	0%	100%	100%
Lot 2, Plan 982 1024	Commercial	100%	100%	100%	100%
Horseshoe Bay	Residential	50%	100%	100%	100%
	Residential	25%	50%	75%	100%
Uplands	Health Services & Mixed Use	25%	50%	75%	100%
Lakeshore Area Redevelopment	All	25%	50%	75%	100%

Development / Redevelopment	Land Use	5-year (2015) Horizon	10-year (2020) Horizon	15-year (2025) Horizon	20-year (2030) Horizon
Lakewood Estates	Residential	25%	50%	75%	100%
Creekside Estates	Residential	25%	50%	75%	100%
Parkview Estates	Residential	25%	50%	75%	100%
Parkview Estates	Commercial	25%	50%	75%	100%
Hills of Cold Lake	Residential	25%	50%	75%	100%
Fawn Ridge Estates Development	Residential	25%	50%	75%	100%
IDP Residential Development 1	Residential	0%	10%	20%	30%
IDP Residential Development 2	Residential	0%	10%	20%	30%
IDP Residential Development 3	Residential	0%	10%	20%	30%
IDP Industrial Development	Industrial	0%	5%	10%	20%
IDP Commercial Development	Commercial	0%	10%	20%	30%

#### 4.2 TRIP GENERATION

The ASP, ARP and Outline Plans for the development/redevelopment projects were reviewed to obtain information regarding the future land uses and the associated developable area. Of particular relevance was the residential, commercial, industrial and institutional land uses.

Portions of the above mentioned subdivisions are currently developed. Traffic volumes from the developed portions are captured by existing (2010) traffic volumes; therefore, these areas were not included in the forecast for the future development traffic volumes. The breakdown of the future land uses and areas for each development/redevelopment project are presented in Appendix A.

Table 4.2 summarizes the trip generation calculations for each development/redevelopment project. The Institute of Transportation Engineer (ITE) Trip Generation Handbook (7th Edition) was referenced to obtain trip rates for each land use. The maximum site coverage assumptions listed in Table 4.2 reflect those stated in the Traffic Demand Forecast Work Plan established at project initiation and attached in Appendix B. Some site coverage assumptions were revised using engineering judgement to reflect more practical trip estimates.



#### City of Cold Lake

Figure 4.2 through Figure 4.5 present the expected p.m. peak hour trips generated by each development/redevelopment project, for the 5-year, 10-year, 15-year and 20-year horizon, respectively.

In order to establish the trip distribution within the City, the study area was broken into nine traffic analysis zones (TAZ), and is illustrated in Figure 4.6. Zone boundaries were selected to encompass areas with relatively homogenous land use types (e.g., business zones and residential zones).

107,639.1 City of Cold Lake Transportation Study 1 hectare = sq.ft. Project No: 2010-3050 1 hectare = 2.4711 acre Date: April 9, 2011

## **Table 4.2 Future Developments Trip Generation**Trip Generation - ITE Trip Generation Handbook

						Independent	Variable			ITE	Data					Trips	(T)		
Fischer Multi Family Resi Commercial - Art	Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM P	eak		PM P	Peak			AM Peak			PM Peak	
Development	Description	# 01	Onic	Land Ose Description	Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
Low Density R  How Density R  Multi Family R  Commercial -	Low Density Residential	449	du	210: Single Family Residential	-	Dwelling Units	449	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	324	81	243	414	261	153
	Multi Family Residential	295	du	230: Residential Condo/Townhouse	-	Dwelling Units	295	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	123	21	102	146	98	48
	Commercial - Arterial	0.0	ha	770: Business Park	80%	1000 sq.ft	0.0	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	-	-	-	-	-	-
Low Density R  How Density R  Multi Family R  Commercial -	Commercial - Neighbourhood	5.8	ha	770: Business Park	50%	1000 sq.ft	313.3	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	438	368	70	431	99	332
					Tota	I Dwelling Units	744						Total Trips:	884	470	415	992	458	534
	Total Commercial/Industrial (1000 sq.ft) 313							]					•						

						Independent	Variable			ITE	Data					Trips	, (T)		
Development  Low Density Residential  Iron Horse  Medium Density Residential  High Density Residential	Description	# of	Unit	Land Use Description		independent	variable	AM Po	eak		PM Pe	eak		ı	AM Peak		ı	PM Peak	
					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
	Low Density Residential	323	du	210: Single Family Residential	Coverage - C	Dwelling Units	323	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	236	59	177	308	194	114
Iron Horse	Medium Density Residential	18	du	230: Residential Condo/Townhouse	-	Dwelling Units	18	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	13	2	11	15	10	5
	High Density Residential	45	du	223: Mid-Rise Apartment	-	Dwelling Units	45	T = 0.41(X) - 13.06	31%	69%	T = 0.48(X) - 11.07	58%	42%	5	2	4	11	6	4
					Tota	I Dwelling Units	386				·		Total Trips:	254	63	191	333	210	123

						Independent	Variable			ITE I	Data					Trips	(T)		
Development	Description	dium Density Residential         578         du         230           n Density Residential         602         du         223	Land Use Description	Maximum Lot	independent	variable	AM P	eak		PM P	Peak			AM Peak			PM Peak		
Low Density Resid  Cold Lake Central Area  Medium Density R  High Density Resid					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	ln	Out	Total	In	Out
	Low Density Residential	1,354	du	Land Use Description  Cove  210: Single Family Residential  230: Residential Condo/Townhouse  223: Mid-Rise Apartment	-	Dwelling Units	1,354	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	957	239	718	1,118	705	414
Cold Lake Central Area	Medium Density Residential	578	du	230: Residential Condo/Townhouse	-	Dwelling Units	578	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	210	36	174	253	170	84
Cold Lake Certifal Area	High Density Residential	602	du	223: Mid-Rise Apartment	-	Dwelling Units	602	T = 0.41(X) - 13.06	31%	69%	T = 0.48(X) - 11.07	58%	42%	234	72	161	278	161	117
	Commercial - Arterial	18.7	ha	770: Business Park	80%	1000 sq.ft	1,612.7	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	2,182	1,833	349	1,949	448	1,500
					Tota	l Dwelling Units	2.534						Total Trips:	3.583	2.180	1.403	3.598	1.484	2.114

Total Dwelling Units 2,534

Total Commercial/Industrial (1000 sq.ft) 1,612.7

						Independent	Variable			ITE	Data					Trip	s (T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	maepenaem	Variable	AM Peak   PM Peak   AM Peak   PM Peak   AM Peak   AM Peak   PM Peak   AM Peak   PM Peak   AM Peak   PM P		PM Peak									
Low Density Grand Centre SE Mobile home					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	ln	Out
	Low Density Residential	281	du	210: Single Family Residential	-	Dwelling Units	281	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	206	52	155	272	171	101
Grand Centre SE	Mobile home	150	du	240: Mobile Home Park	-	Dwelling Units	150	Ln(T) = 0.64Ln(X) + 0.96	20%	80%	T = 0.57(X) + 2.06	62%	38%	65	13	52	88	54	33
	Industrial - Light Industrial	5.9	ha	130: Industrial Park	60%	1000 sq.ft	380.1	Ln(T) = 0.77Ln(X) + 1.09	82%	18%	T = 0.77(X) + 42.11	21%	79%	288	236	52	335	70	265
					Tota	l Dwelling Units	431						Total Trips:	559	301	258	694	296	398
				Total Co	ommercial/Indus	trial (1000 sq.ft)	380.1												

						Independent	Variable			ITE	Data					Trips	(T)		
Development  Single Famil	Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM P	eak		PM P	Peak			AM Peak		ı	PM Peak	
					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	in	Out
Forest Heighte	Single Family Residential	345	du	210: Single Family Residential	-	Dwelling Units	345	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	251	63	188	327	206	121
Forest Heights	Multi Family Residential	248	du	230: Residential Condo/Townhouse	-	Dwelling Units	248	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	107	18	89	127	85	42
					Total Dwelli								Total Trips:	358	81	277	453	291	163

						Independent	Variable			ITE	Data					Trip	s (T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM Po	eak		PM F	Peak			AM Peak			PM Peak	
<b>-</b>					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
	Low Density Residential	537	du	210: Single Family Residential	-	Dwelling Units	537	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	385	96	289	487	307	180
Medium Density Residential  Mixed Use Commercial <sup>1</sup> Mixed Use Institutional <sup>2</sup> School Site  Commercial - Neighbourhood  Commercial - Arterial  laximum lot coverage for commercial portion of mix use commercial as	475	du	230: Residential Condo/Townhouse	-	Dwelling Units	475	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	180	31	149	216	145	71	
	Mixed Hee Commercial 1	547	du	223: Mid-Rise Apartment	-	Dwelling Units	547	T = 0.41(X) - 13.06	31%	69%	T = 0.48(X) - 11.07	58%	42%	211	65	146	251	146	106
	iviixed use commercial	6.5	ha	770: Business Park	25%	1000 sq.ft	174.9	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	247	208	40	252	58	194
	Miyad Has Institutional 2	157	du	223: Mid-Rise Apartment	-	Dwelling Units	157	T = 0.41(X) - 13.06	31%	69%	T = 0.48(X) - 11.07	58%	42%	51	16	35	64	37	27
	iviixed Ose iristitutionai	2.6	ha	720: Medical-Dental Office Building	15%	1000 sq.ft	42.0	2.48	79%	21%	3.72	27%	73%	104	82	22	156	42	114
Low Density Residential  Medium Density Residential  Mixed Use Commercial   Mixed Use Institutional   School Site  Commercial - Neighbourhood	1,958	students	520: Elementary School	-	Students	1,958	Ln(T) = 1.11Ln(X) - 1.73	55%	45%	Ln(T) = 1.08Ln(X) - 1.90	45%	55%	799	439	360	537	242	295	
	Commercial - Neighbourhood	1.8	ha	770: Business Park	50%	1000 sq.ft	96.9	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	139	116	22	147	34	113
	Commercial - Arterial	11.9	ha	770: Business Park	80%	1000 sq.ft	1,024.7	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	1,399	1,175	224	1,284	295	989
ium lot coverage for co					Total	Dwelling Units	1,716						Total Trips:	3,516	2,229	1,286	3,394	1,305	2,08

Maximum lot coverage for commercial portion of mix use commercial assumed to be 25%
 Maximum lot coverage for institutional portion of mix use insitutuional assumed to be 15%

Total Dwelling Units 1,716
Total Commercial/Instutional (1000 sq.ft) 1,338.5 School Site (Students)

City of Cold Lake Transportation Study 1 hectare = 107,639.1 sq.ft. Project No: 2010-3050 1 hectare = 2.4711 acre Date: April 9, 2011

#### **Table 4.2 Future Developments Trip Generation**

Trip Generation - ITE Trip Generation Handbook

						Independent	Variable			ITE	Data					Trips	s (T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM P	eak		PM P	eak			AM Peak		F	PM Peak	
					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	ln	Out	Total	In	Out
Lot 2	Commercial - Arterial	15,365	sq.ft	770: Business Park	-	1000 sq.ft	15.4	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	23	19	4	27	6	21
				Total Co	mmercial/Indus	strial (1000 sq.ft)	15.4						Total Trips:	23	19	4	27	6	21

						Independent	Variable			ITE	Data					Trips	s (T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM Pe	eak		PM P	eak			AM Peak		F	PM Peak	
					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	ln	Out	Total	In	Out
Horseshoe Bay	Low Density Residential	42	du	210: Single Family Residential	-	Dwelling Units	42	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	39	10	29	49	31	18
	-			_	Tota	I Dwelling Units	42						Total Trips:	39	10	29	49	31	18

						Independent	Variable			ITE	Data					Trips	(T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM F	Peak		PM P	eak			AM Peak			PM Peak	
201000p		0.	J		Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
	Single Family Residential	904	du	210: Single Family Residential	-	Dwelling Units	904	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	642	161	482	778	490	288
Uplands	Multi Family Residential	480	du	230: Residential Condo/Townhouse	-	Dwelling Units	480	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	181	31	150	218	146	72
	Health Services and Mixed Use 3,4	5.0	ha	620: Nursing Home	50%	1000 sq.ft	269.1	0.38	53%	47%	0.42	47%	53%	102	54	48	113	53	60
3. Maximum site coverage for Heal	th Care & Mixed Use assumed to be 50%				Tota	Dwelling Units	1,384						Total Trips:	926	246	680	1,108	689	419

#### Total Health Care & Mixed Use (1000 sq.ft) 269.1 4. AM directional split for Nursing Home, by 1000 sq. ft., not available. Assume reverse of PM directional split

#### TO SUBTRACT

						Independent	Variable			ITE	Data					Trips	(T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	muependent	Variable	AM F	Peak		PM P	eak			AM Peak			PM Peak	
					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	ln	Out	Total	ln	Out
	902 10 Street	0.11	hec	770: Business Park	50%	1000 sq.ft	5.9	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	9	8	1	11	3	9
	904 10 Street	0.07	hec	770: Business Park	50%	1000 sq.ft	3.7	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	6	5	1	7	2	6
Lakeshore ARP	901 9 Avenue	0.11	hec	770: Business Park	50%	1000 sq.ft	6.0	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	9	8	1	11	3	9
	803 10 Avenue	0.22	hec	770: Business Park	50%	1000 sq.ft	12.1	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	18	15	3	22	5	17
	Fire Hall / Community Hall	0.34	hec	495: Recreational Community Center 5	50%	1000 sq.ft	18.4	22.88	50%	50%	10 % of <i>i</i>	All Day		422	211	211	42	21	21
5. For Fire Hall, assumed same tri	p rate as future community centre. Trips will remain	the same esse	entially.		Total Commer	cial (1000 sq.ft)	27.7						Total Trips:	464	246	218	94	33	61

<sup>5.</sup> For Fire Hall, assumed same trip rate as future community centre. Trips will remain the same essentially.

## Total Commercial (1000 sq.ft) 27.7 Total Fire Hall / Community Hall (1000 sq.ft) 18.4

#### TO ADD

						Independent	Variable			ITE	Data					Trips	s (T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM F	Peak		PM P	Peak			AM Peak			PM Peak	
					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	ln	Out	Total	In	Out
	Vacant parcel on 12 Street and 8 Avenue	38	du	230: Residential Condo/Townhouse	-	Dwelling Units	38	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	24	4	20	27	18	9
	902 10 Street	15	du	223: Mid-Rise Apartment <sup>6</sup>	=	Dwelling Units	15	0.3	31%	69%	0.39	58%	42%	4	1	3	6	3	2
	904 10 Street	9	du	223: Mid-Rise Apartment	=	Dwelling Units	9	0.3	31%	69%	0.39	58%	42%	3	1	2	4	2	2
	901 9 Avenue	15	du	223: Mid-Rise Apartment	-	Dwelling Units	15	0.3	31%	69%	0.39	58%	42%	5	1	3	6	3	2
Lakeshore ARP	803 10 Avenue	3	du	210: Single Family Residential	-	Dwelling Units	3	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	12	3	9	5	3	2
	Triangle Park	0.11	hec	411: City Park 7	100%	Acres	0.3	1.59	50%	50%	10 % of .	All Day		0	0	0	0	0	0
	Bibeau Park	1.16	hec	411: City Park	100%	Acres	2.9	1.59	50%	50%	10 % of a	All Day		5	2	2	0	0	0
	Centoaph Park	0.26	hec	411: City Park	100%	Acres	0.6	1.59	50%	50%	10 % of .	All Day		1	1	1	0	0	0
	Fire Hall / Community Hall	0.34	hec	495: Recreational Community Center 8	50%	1000 sq.ft	18.4	22.88	50%	50%	10 % of .	All Day		422	211	211	42	21	21
verage rate used instead of	f equation since equation results in negative trip valu	es.			Tota	I Dwelling Units	80						Total Trips:	475	225	251	90	51	38

<sup>7.</sup> Trip rate for park is for all-day, weekday. PM trips assumed to be 10% of all-day trips (presented under AM Trips)
8. Trip rate for community centre is for all-day, weekday. PM trips assumed to be 10% of all-day trips (presented under AM Trips). To be used for special events only.

Total City Park (Acres)	3.8
Total Fire Hall / Community Hall (1000 sq.ft)	18.4

						Independent	Variable			ITE	Data					Trips	(T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	muepenuem	variable	AM P	eak		PM P	eak			AM Peak			PM Peak	
					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
Lakewood	Low Density Residential	153	du	210: Single Family Residential	-	Dwelling Units	153	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	117	29	87	157	99	58
					Tota	al Dwelling Units	153						Total Trips:	117	29	87	157	99	58

						Independent	Variable			ITE	Data					Trips	(T) ذ		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM Pe	eak		PM P	eak			AM Peak			PM Peak	
					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
Creekside	Low Density Residential	594	du	210: Single Family Residential	-	Dwelling Units	594	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	425	106	319	533	336	197
Creekside	Medium Density Residential	196	du	230: Residential Condo/Townhouse	-	Dwelling Units	196	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	88	15	73	104	70	34
					Total	<b>Dwelling Units</b>	790			-			Total Trips:	514	121	392	637	406	232

City of Cold Lake Transportation Study 107,639.1 1 hectare = sq.ft. Project No: 2010-3050
Date: April 9, 2011 1 hectare = 2.4711 acre

## **Table 4.2 Future Developments Trip Generation**Trip Generation - ITE Trip Generation Handbook

						Independent	Variable			ITE	Data					Trips	(T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM P	eak		PM P	Peak			AM Peak			PM Peak	
2010iopinoin	2-3-3-1-piloti		J		Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
Parkview	Low Density Residential	367	du	210: Single Family Residential	-	Dwelling Units	367	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	266	67	200	345	218	128
Parkview	Commercial	2.6	ha	770: Business Park	50%	1000 sq.ft	141.7	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	201	169	32	208	48	160
					Tota	l Dwelling Units	367						Total Trips:	467	236	232	553	265	288
				Total C	commercial/Indus	trial (1000 sq.ft)	141.7						-						

				10	otal Commercial/Indus	iriai (1000 Sq.it)	141.7	<u>l</u>											
						Independent	Variable			ITE	Data					Trips	s (T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM F	Peak		PM P	Peak			AM Peak			PM Peak	
2010.00	20001.p.1011	,, ,,	J		Coverage	Description	Units (X)	Equation or Average Rate	% Inbound %	6 Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
MD - Hills of Cold Lake	Option 2 - 300 1/2 Acre Lot Subdivision	300	du	210: Single Family Residential	-	Dwelling Units	300	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	219	55	165	288	182	107
	- <del>-</del>				Tota	I Dwelling Units	300						Total Trips:	219	55	165	288	182	107
						•													
						Independent	Variable			ITE	Data					Trips	s (T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	maepenaem	variable	AM F	Peak		PM P	Peak			AM Peak			PM Peak	
					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound %	6 Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
MD - Fawn Ridge Estates <sup>9</sup>	Country Residential Lots	0	du	210: Single Family Residential	-	Dwelling Units	54	0.77	26%	74%	1.02	64%	36%	42	11	31	55	35	20
9. Information, including dwelling u	nits, trip generation rates and percentage splits, as	s per 2010 TIA			Tota	I Dwelling Units	54				<u> </u>		Total Trips:	42	11	31	55	35	20

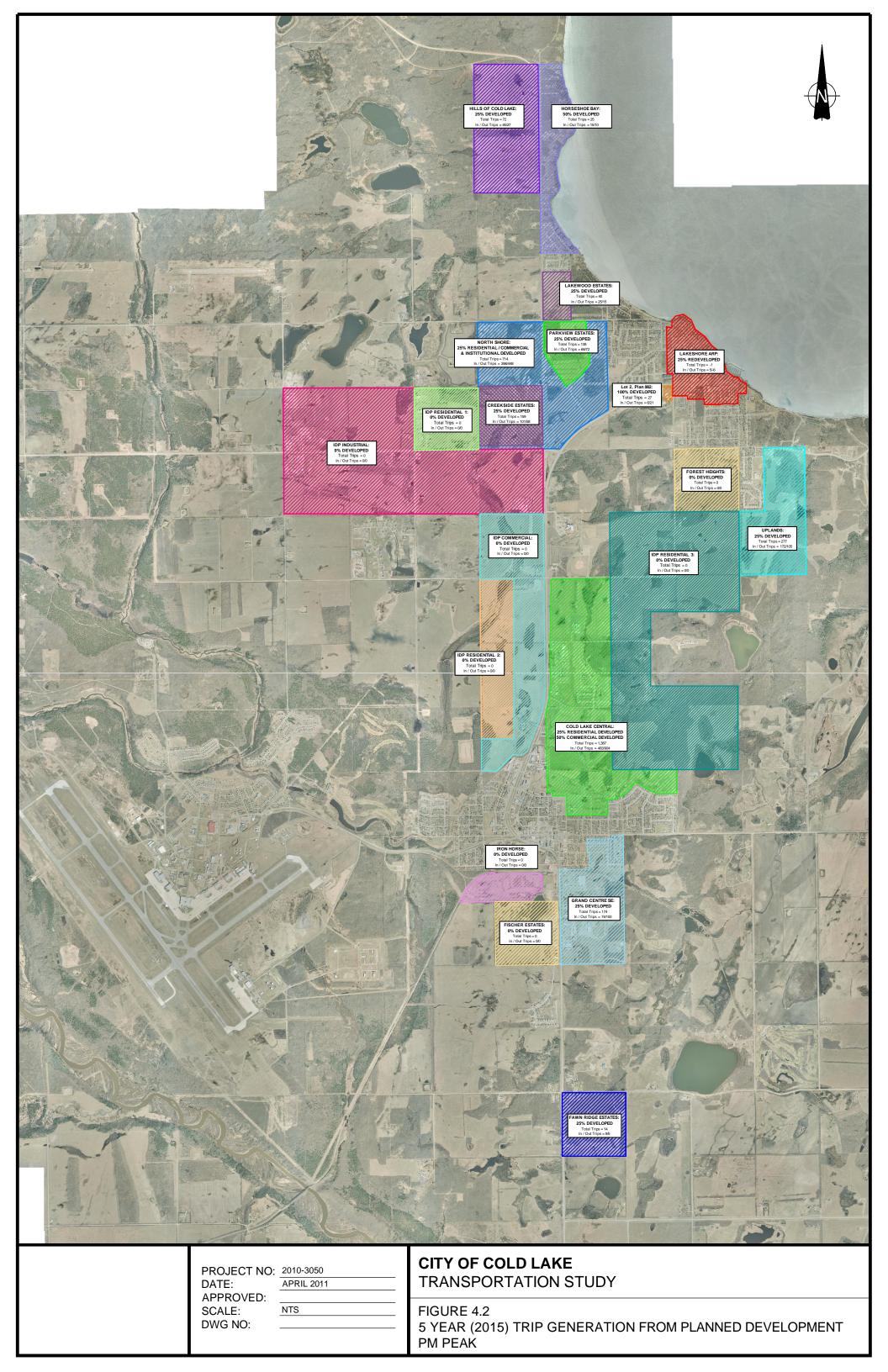
						Independent	Variable			ITE	Data					Trips	(T) د		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	independent	variable	AM P	eak		PM F	Peak			AM Peak			PM Peak	
2010iopinoin	20001.p.1011	0.	0		Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
IDD Residential Development 1	Low Density Residential	283	du	210: Single Family Residential	-	Dwelling Units	283	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	208	52	156	273	172	101
IDF - Residential Development 1	Multi Family Residential	236	du	230: Residential Condo/Townhouse	-	Dwelling Units	236	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	103	17	85	122	81	40
					Tota	I Dwelling Units	519						Total Trips:	310	69	241	395	254	141

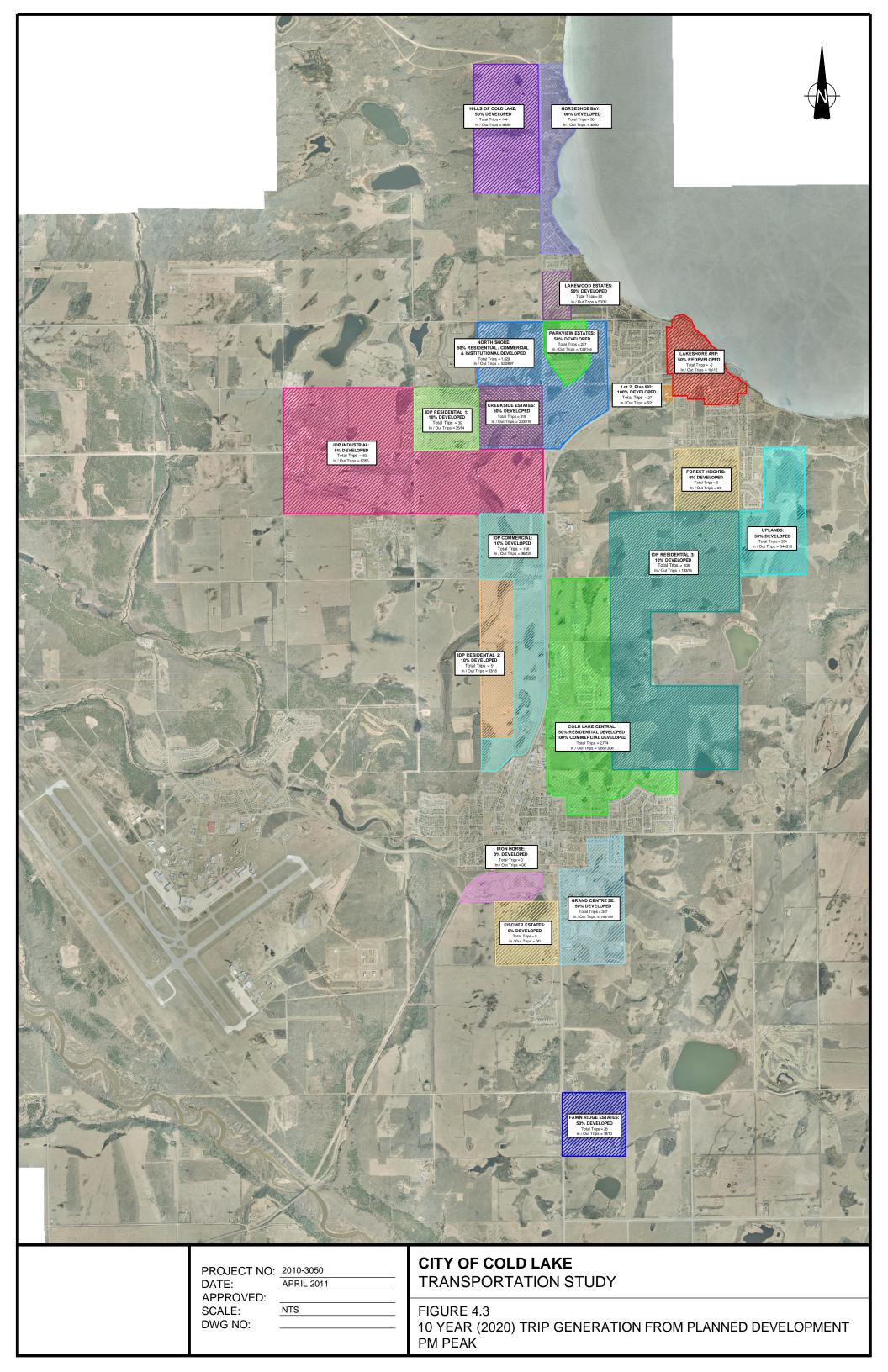
						Independent	Variable			ITE I	Data					Trips	(T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	maepenaem	variable	AM P	eak		PM P	Peak			AM Peak			PM Peak	
					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out
IDD Desidential Development 2	Low Density Residential	379	du	210: Single Family Residential	-	Dwelling Units	379	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	275	69	206	356	224	132
IDP - Residential Development 2	Multi Family Residential	316	du	230: Residential Condo/Townhouse	-	Dwelling Units	316	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	130	22	108	154	104	51
	_				Tota	l Dwelling Units	696						Total Trips:	405	91	314	510	328	183

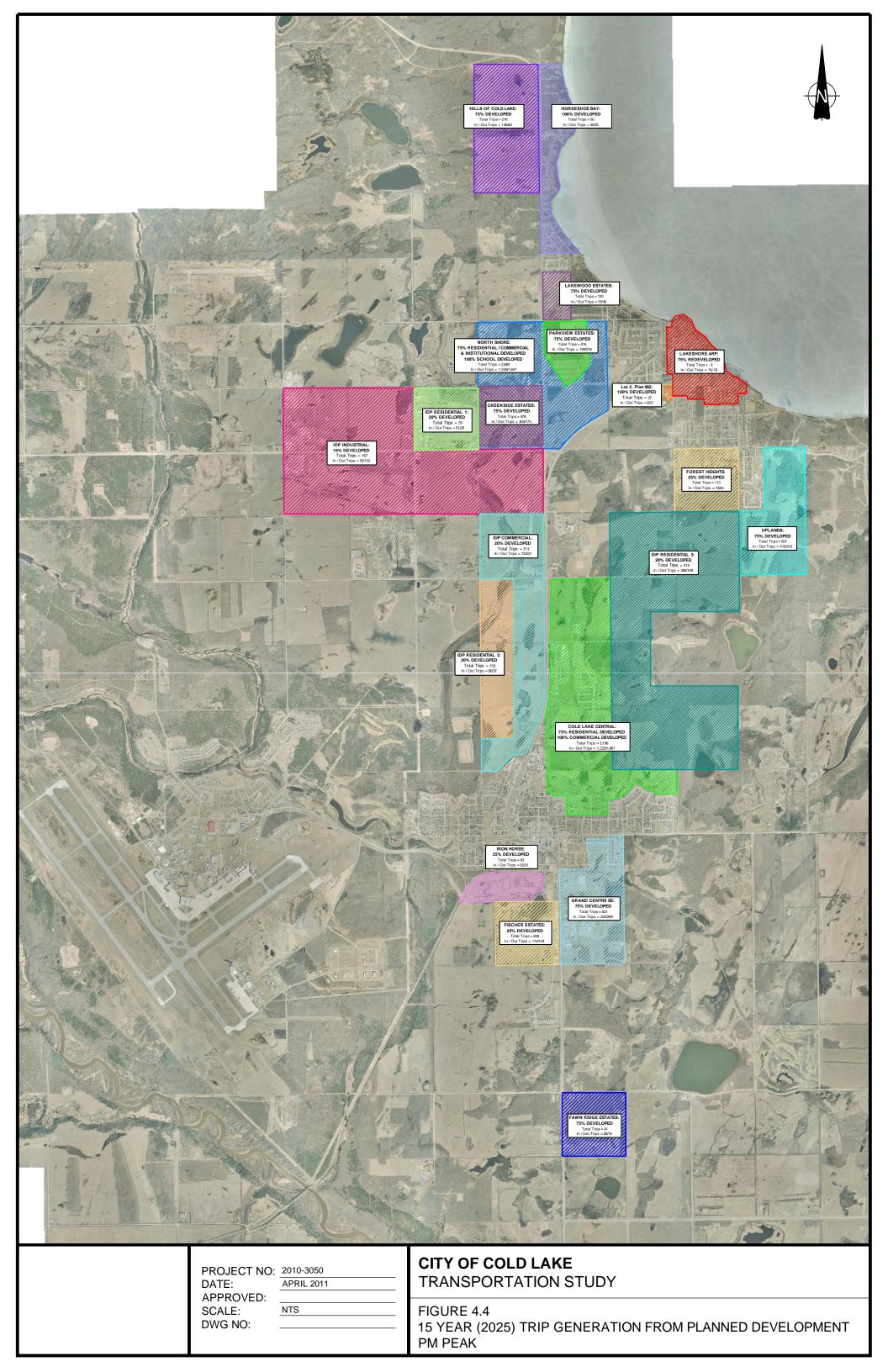
						Independent Variable		ITE Data							Trip			ips (T)		
Development	Description	# of	Unit	Land Use Description	Maximum Lot	n Lot AM Peak			PM Peak			AM Peak		PM Peak						
					Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	In	Out	Total	In	Out	
IDP - Residential Development 3	Low Density Residential	1,880	du	210: Single Family Residential	-	Dwelling Units	1,880	T = 0.70X + 9.43	25%	75%	Ln(T) = 0.90Ln(X) + 0.53	63%	37%	1326	331	994	1503	947	556	
IDF - Residential Development 3	Multi Family Residential	1,567	du	230: Residential Condo/Townhouse	-	Dwelling Units	1,567	Ln(T) = 0.80Ln(X) + 0.26	17%	83%	Ln(T) = 0.82Ln(X) + 0.32	67%	33%	467	79	387	574	385	189	
					Tota	I Dwelling Units	3,447						Total Trips:	1,792	411	1,381	2,077	1,331	746	

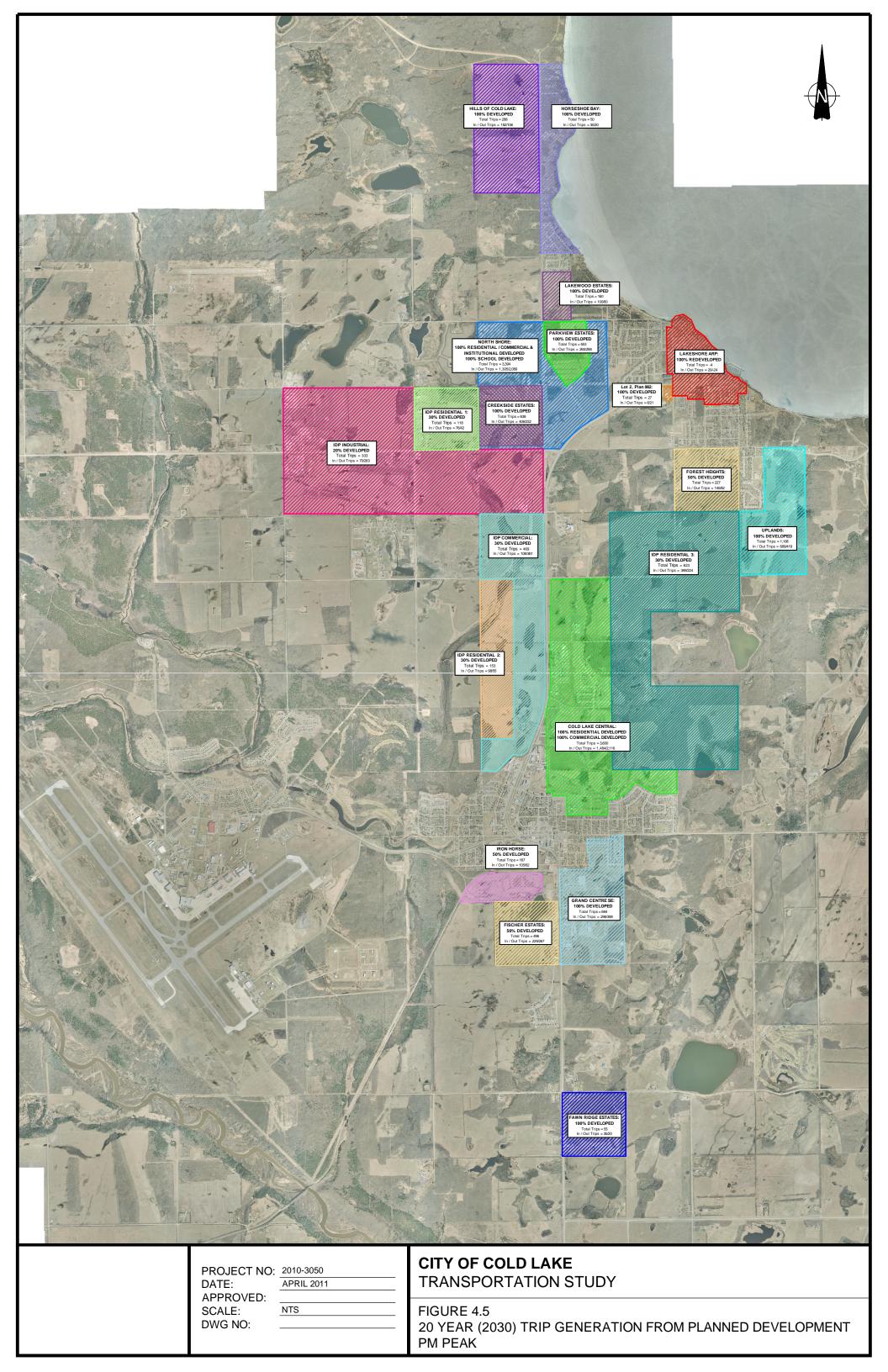
Development							Independent Variable		ITE Data						Trips			s (T)		
	Description	# of	Unit		Land Use Description	Maximum Lot	independen	variable	AM Peak			PM Peak		AM Peak		PM Peak				
						Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	ln	Out	Total	ln	Out
IDP - Industrial Developments 10	Industrial - Light & Heavy Industrial	78	ha	130: Industrial Park		25%	1000 sq.ft	2,109.4	Ln(T) = 0.77Ln(X) + 1.09	82%	18%	T = 0.77(X) + 42.11	21%	79%	1,079	885	194	1,666	350	1,316
10. Maximum site coverage for IDP	- Industial Developments assumed to be 25%				Total C	ommercial/Indus	trial (1000 sq.ft)	2,109						Total Trips:	1,079	885	194	1,666	350	1,316

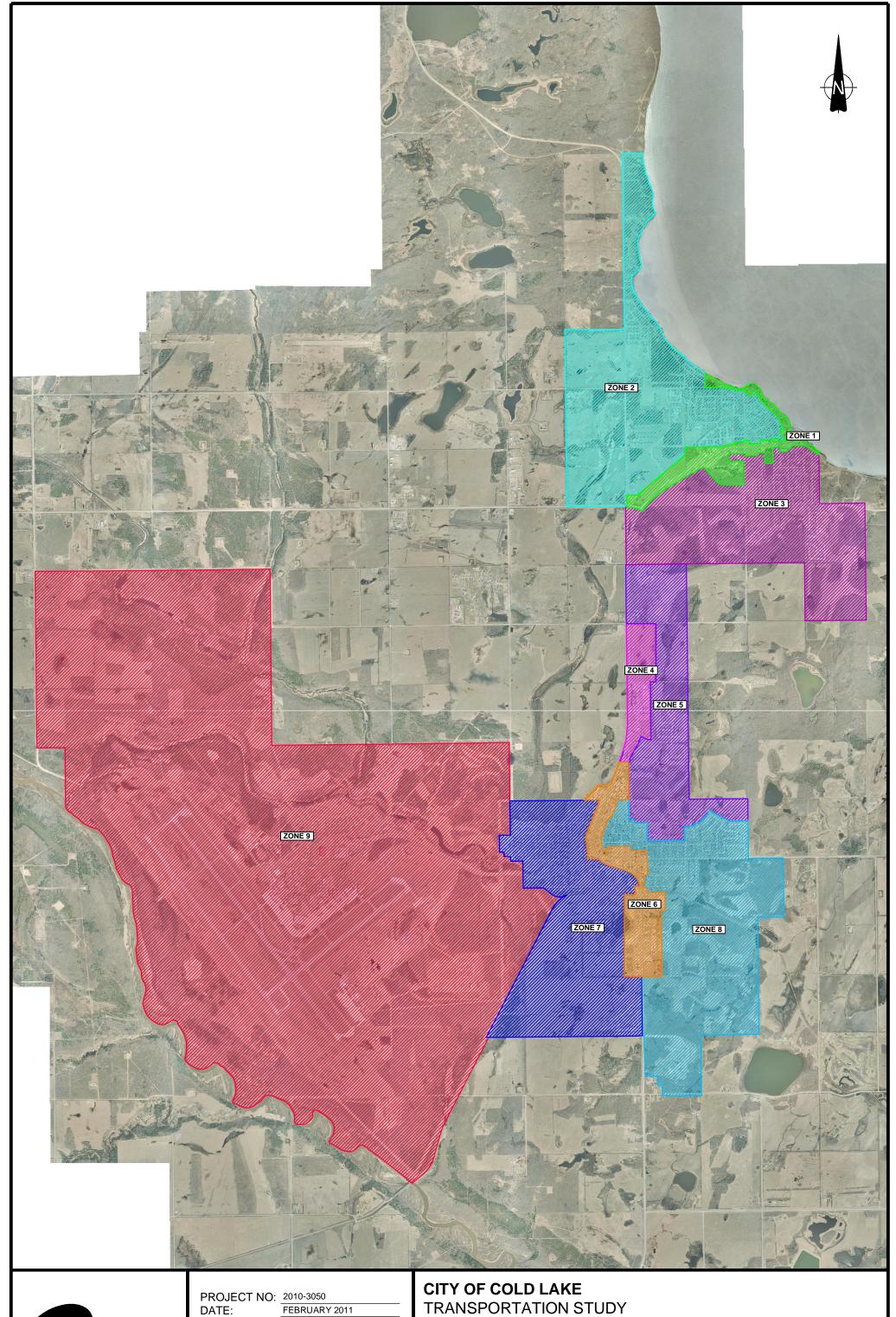
Development					Independ		Variable	ITE Data						Trips (T)					
	Description	# of	Unit	Land Use Description	Maximum Lot	maepenaen	variable	AM Peak			PM Peak			AM Peak		PM Peak			
	22200				Coverage	Description	Units (X)	Equation or Average Rate	% Inbound	% Outbound	Equation or Average Rate	% Inbound	% Outbound	Total	ln	Out	Total	In	Out
IDP - Commercial 11	Commercial - Arterial	47.2	ha	770: Business Park	25%	1000 sq.ft	1,270.8	Ln(T) = 0.98Ln(X) + 0.45	84%	16%	Ln(T) = 0.92Ln(X) + 0.78	23%	77%	1,727	1,451	276	1,565	360	1,205
11. Maximum site coverage for IDP	- Industial Developments assumed to be 25%			Total Co	ommercial/Indus	strial (1000 sq.ft)	1,271		·	•		·	Total Trips:	1,727	1,451	276	1,565	360	1,205













DATE: APPROVED: SCALE:

DWG NO:

FEBRUARY 2011

NTS

FIGURE 4.6 TRAFFIC ANALYSIS ZONES WITHIN CITY OF COLD LAKE The development trips presented above in Figure 4.2 through Figure 4.5 were broken down by location and the corresponding TAZ. Table 4.3 through Table 4.6 summarizes the p.m. peak hour trips generated by each zone, for the 5-year, 10-year, 15-year, and 20-year planning horizons respectively.

Table 4.3 5-year (2015) - Trip Generation from Planned Developments by Zone

Zone	Total Trips	Inbound Trips	Outbound Trips	
1	347	85	262	
2	755	755 400		
3	277	172	105	
4	974	224	750	
5	412	259	154	
6	84	18	66	
7	0	0	0	
8	90	56	33	
9	0	0	0	
External Zones	86	54	32	
Total	3,025	1,268	1,757	

Table 4.4
10-year (2020) - Trip Generation from Planned Developments by Zone

Zone	Total Trips	Inbound Trips	Outbound Trips
1	667	163	504
2	1,510	800	710
3	554	210	
4	1,949	448	1,500
5	825	518	307
6	167	35	132
7	0	0	0
8	180	113	67
9	0	0	0
External Zones	710	353	356
Total	6,023	2,530	3,493

Table 4.5
15-year (2025) - Trip Generation from Planned Developments by Zone

Zone	Total Trips	Inbound Trips	Outbound Trips
1	987	241	745
2	2,777	1,426	1,351
3	944	589	355
4	1,949	448	1,500
5	1,237	777	461
6	359	78	281
7	223	142	81
8	270	169	100
9	0	0	0
External Zones	1,334	652	681
Total	9,003	4,033	4,970

Table 4.6 20-year (2030) - Trip Generation from Planned Developments by Zone

Zone	Total Trips	Inbound Trips	Outbound Trips
1	1,307	320	987
2	3,507	1,811	1,696
3	1,335	834	501
4	1,949	448	1,500
5	1,650	1,036	614
6	551	120	431
7	447	284	162
8	360	226	134
9	0	0	0
External Zones	2,041	969	1,072
Total	11,447	5,295	6,152

#### 4.3 TRIP DISTRIBUTION

A simplified gravity model was originally selected to determine the distribution of trips within the City generated by the proposed development/redevelopment. The gravity model assumes that the number of trips between two zones is directly proportional to the trips produced and attracted by both zones and inversely proportional to the square of travel time between the two zones. The procedure for the simplified gravity model was illustrated in detail in the Traffic Demand Forecast Work Plan (included in Appendix B) and the trip distribution calculations for the 5-year planning horizon are presented in Appendix C.

A trip distribution table was established for the 5-year planning horizon using the simplified gravity model. The trip distribution established for each TAZ was not reflective of the local travel patterns within the City; therefore, the trip distribution was revised after discussions with the City to reflect the local characteristic of the City. Table 4.7 presents the trip distribution used for the traffic volume forecast. The same trip distribution was used for each planning horizon (5-year, 10-year, 15-year, and 20-year).

Table 4.7
Trip Distribution Table (Within City Limits)

Erom					То					CLIM
From	1	2	3	4	5	6	7	8	9	SUM
1	5%	23%	23%	5%	8%	5%	10%	8%	13%	100%
2	20%	15%	10%	15%	10%	8%	7%	5%	10%	100%
3	20%	10%	15%	15%	10%	8%	7%	5%	10%	100%
4	6%	15%	15%	5%	15%	8%	12%	12%	12%	100%
5	16%	12%	12%	15%	15%	8%	7%	5%	10%	100%
6	5%	5%	5%	5%	15%	5%	25%	25%	10%	100%
7	10%	5%	5%	15%	10%	18%	12%	15%	10%	100%
8	10%	5%	5%	15%	10%	20%	10%	15%	10%	100%
9	8%	10%	10%	10%	7%	20%	10%	10%	15%	100%
SUM	100%	100%	100%	100%	100%	100%	100%	100%	100%	-

The trip distribution presented above is only applicable to development trips within the City. The developments located outside the City, within the MD of Bonnyville, were distributed using the trip distribution presented in Table 4.8.

Table 4.8

Trip Distribution Table (Outside City Limits)

Development	Inbound Trips	Outbound Trips
Hills of Cold	20% from Cold Lake North business area	5% to Cold Lake North business area
Lake	20% from Tri-City Mall area	25% to Tri-City Mall area
	20% from Cold Lake South business area	25% to Cold Lake South business area
	20% from the commercial area in the south	40% to the commercial area in the south
	(near 43 Avenue)	(near 43 Avenue)
	20% from Medley	5% to Medley
Fawn Ridge	20% from Cold Lake North business area	5% to Cold Lake North business area
Estates	20% from Tri-City Mall area	25% to Tri-City Mall area
	20% from Cold Lake South business area	25% to Cold Lake South business area
	20% from the commercial area in the south	40% to the commercial area in the south
	(near 43 Avenue)	(near 43 Avenue)
	20% from Medley	5% to Medley
IDP	25% Internal	30% Internal
Residential 1	15% from Cold Lake North business area	5% to Cold Lake North business area
IDP	15% from Tri-City Mall area	20% to Tri-City Mall area
Residential 2	15% from Cold Lake South business area	20% to Cold Lake South business area
IDP	15% from the commercial area in the south	20% to the commercial area in the south
Residential 3	(near 43 Avenue)	(near 43 Avenue)
	15% from Medley	5% to Medley
IDP Industrial	25% Internal	25% Internal
	25% Cold Lake North residential	25% Cold Lake North residential
	10% Residential behind Tri-City Mall	10% Residential behind Tri-City Mall
	35% Cold Lake South residential	35% Cold Lake South residential
	5% Medley	5% Medley
IDP	25% Internal	25% Internal
Commercial	25% Cold Lake North residential	25% Cold Lake North residential
	10% Residential behind Tri-City Mall	10% Residential behind Tri-City Mall
	35% Cold Lake South residential	35% Cold Lake South residential
	5% Medley	5% Medley

#### 4.4 TRIP ASSIGNMENT

The development trips were assigned onto the future road network by considering the logical routes that would be taken by the commuters between the origin and destinations, on the basis of impedance and travel time. To capture worst-case traffic scenarios, the development trips were primarily assigned to the skeletal road network established for the planning horizon.

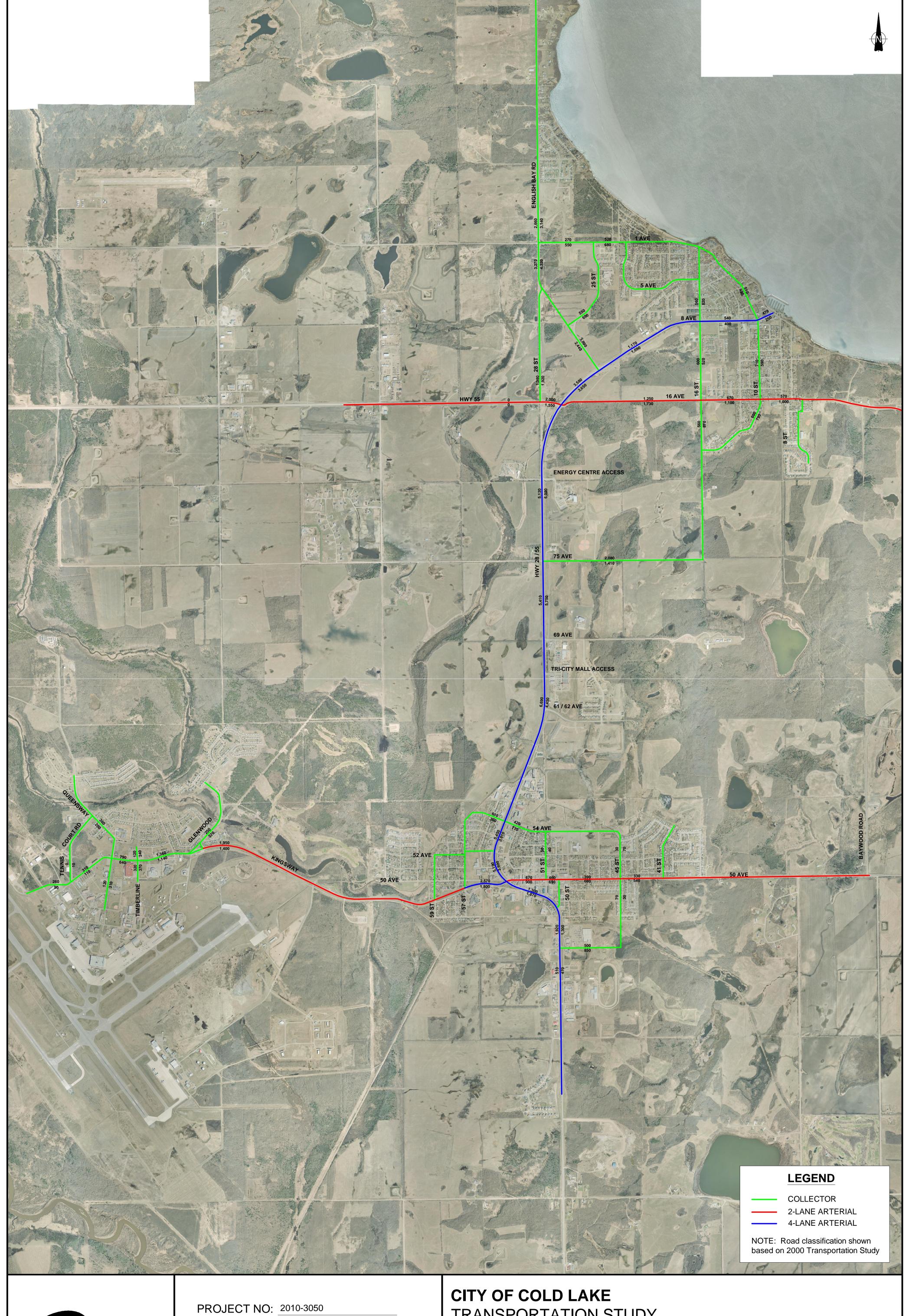
#### 4 - Traffic Forecast: Future Development Traffic Volumes

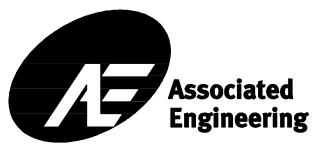
To simplify the trip assignment process, selected intersections were used to represent each study zone and development trips were assumed to enter/exit the zone from those intersections. As a result, some roadways within the skeletal road network do not appear to have background or development traffic assigned to it. The traffic volumes on these roadways were forecasted using growth patterns established from the other roadways.

#### 4.5 FORECASTED DEVELOPMENT TRAFFIC VOLUMES

Figure 4.7 presents the forecasted daily development traffic volumes for the 5-year (2015) Horizon. Figure 4.8 presents the forecasted daily development traffic volumes for the 10-year (2020) Horizon. Figure 4.9 presents the forecasted daily development traffic volumes for the 15-year (2025) Horizon. Figure 4.10 presents the forecasted daily development traffic volumes for the 20-year (2030) Horizon.







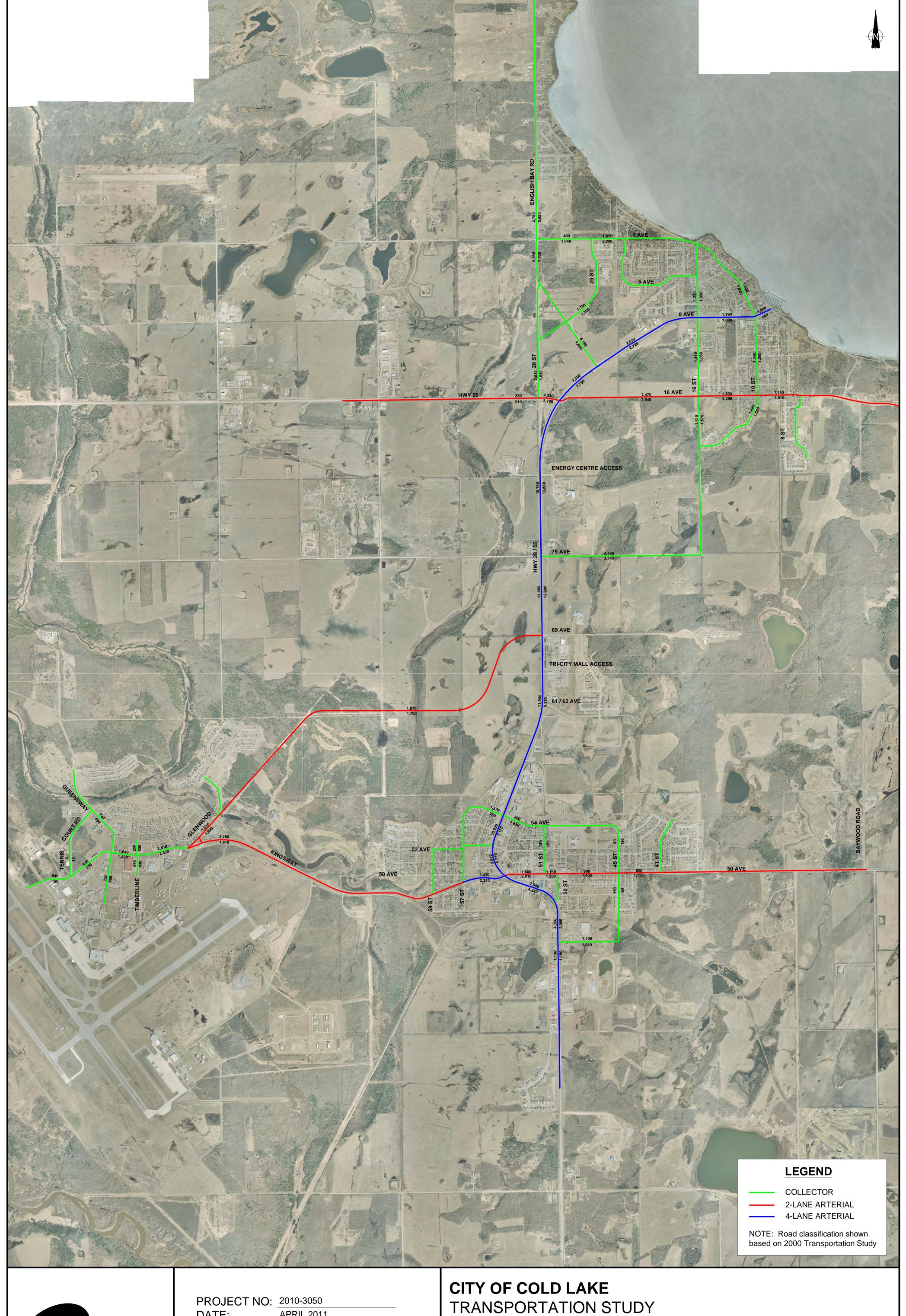
DATE: **APRIL 2011** APPROVED:

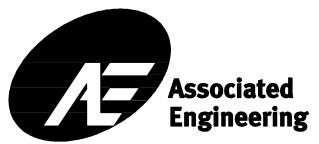
DWG NO:

NTS SCALE:

TRANSPORTATION STUDY

FIGURE 4.7 5 YEAR (2015) DAILY DEVELOPMENT TRAFFIC VOLUMES



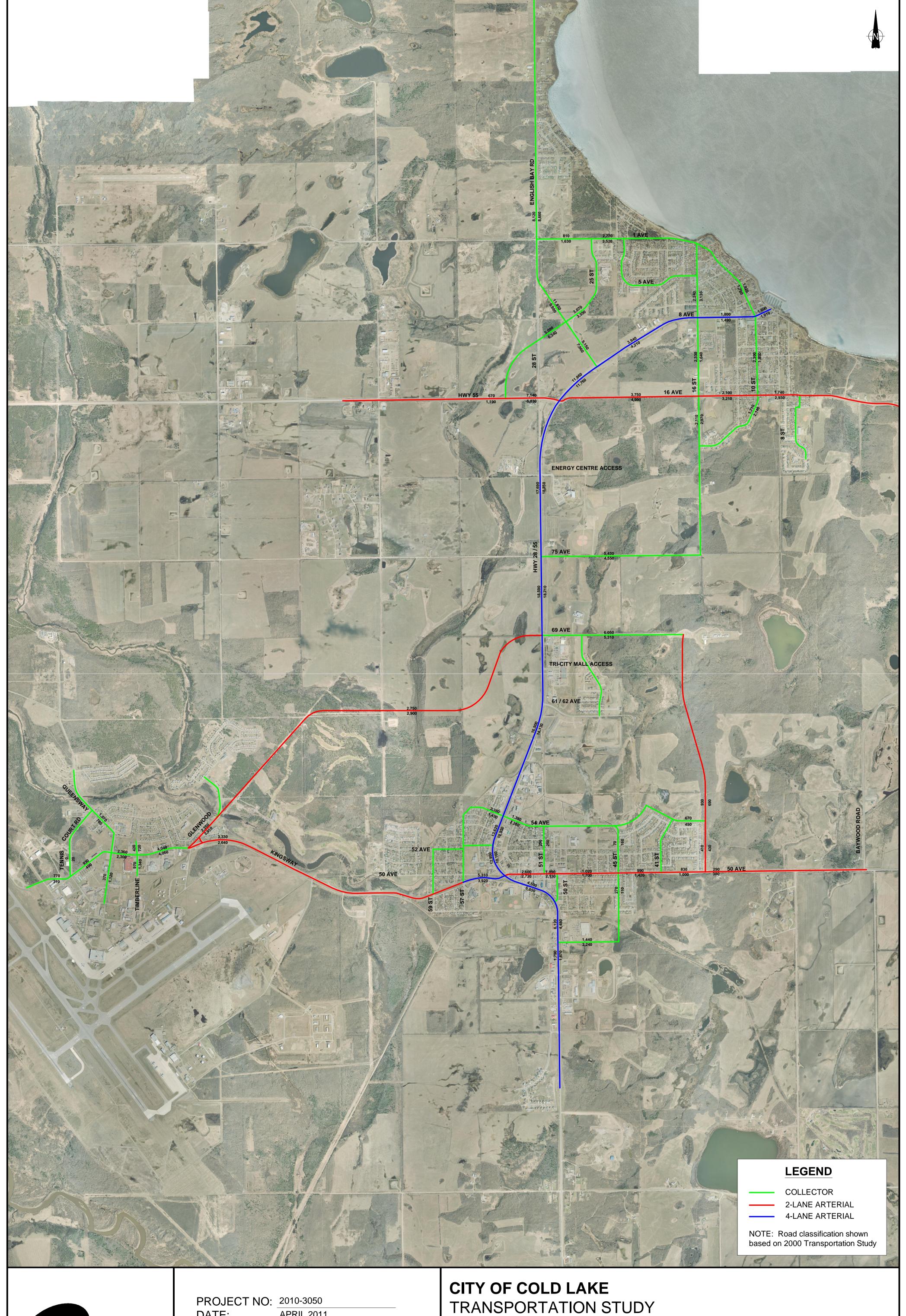


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DWG NO:

**APRIL 2011** 

FIGURE 4.8 10 YEAR (2020) DAILY DEVELOPMENT TRAFFIC VOLUMES



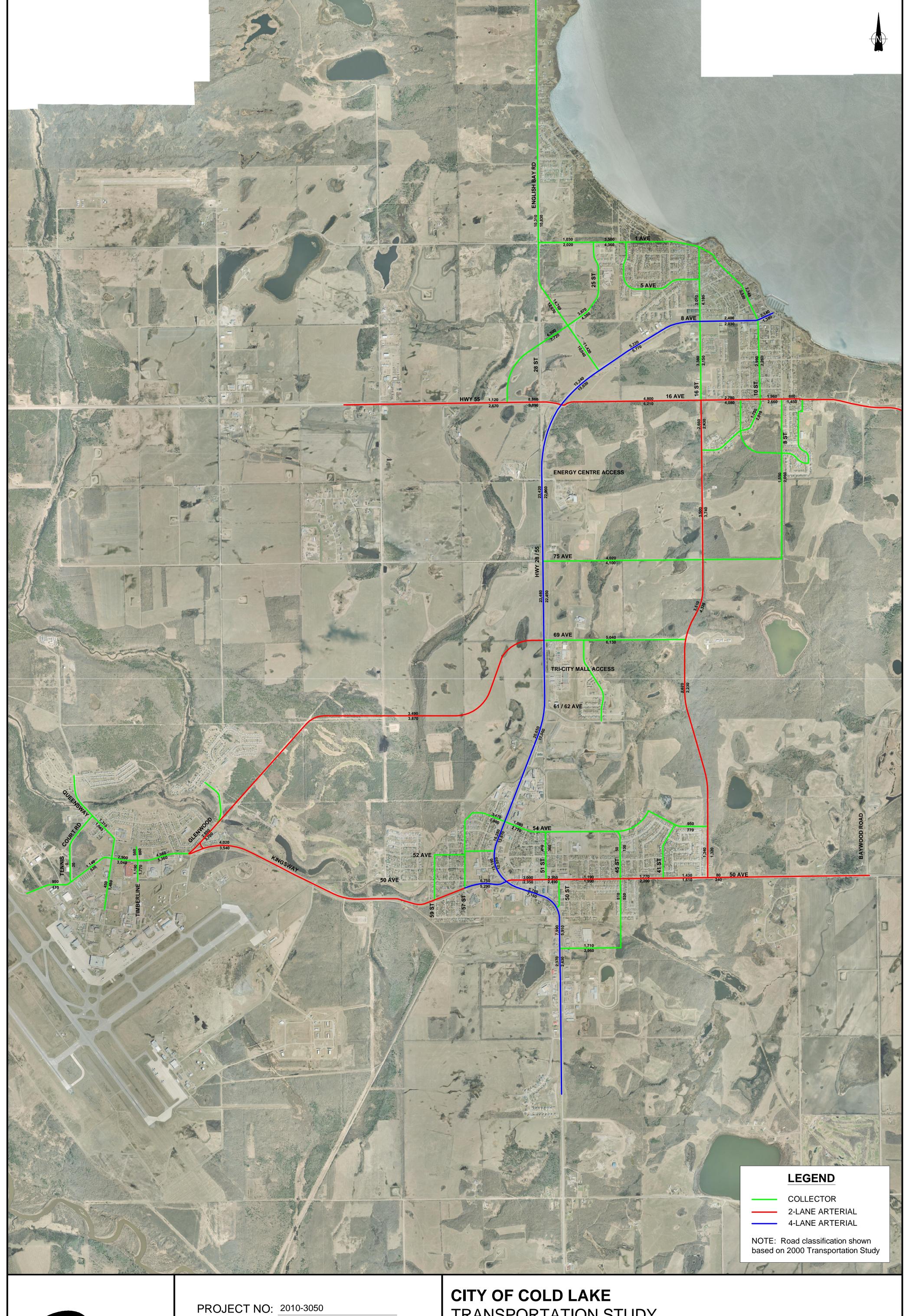


DATE: **APRIL 2011** APPROVED:

DWG NO:

NTS SCALE:

FIGURE 4.9 15 YEAR (2025) DAILY DEVELOPMENT TRAFFIC VOLUMES





DATE: **APRIL 2011** APPROVED: SCALE:

NTS DWG NO:

TRANSPORTATION STUDY

FIGURE 4.10 20 YEAR (2030) DAILY DEVELOPMENT TRAFFIC VOLUMES 5

## **Traffic Forecast: Total Traffic Volumes**

Total traffic volumes were calculated by combining the background traffic volumes with the development traffic volumes for common planning horizons. The average traffic growth for each road classification established in the 2000 Transportation Study (collectors, two-lane arterials, and four-lane arterials) was used to forecast future traffic volumes on the roadways which were not included in the trip assignment. The total traffic volumes for each planning horizon are presented in the following section.

#### 5.1 FORECASTED TOTAL TRAFFIC VOLUMES

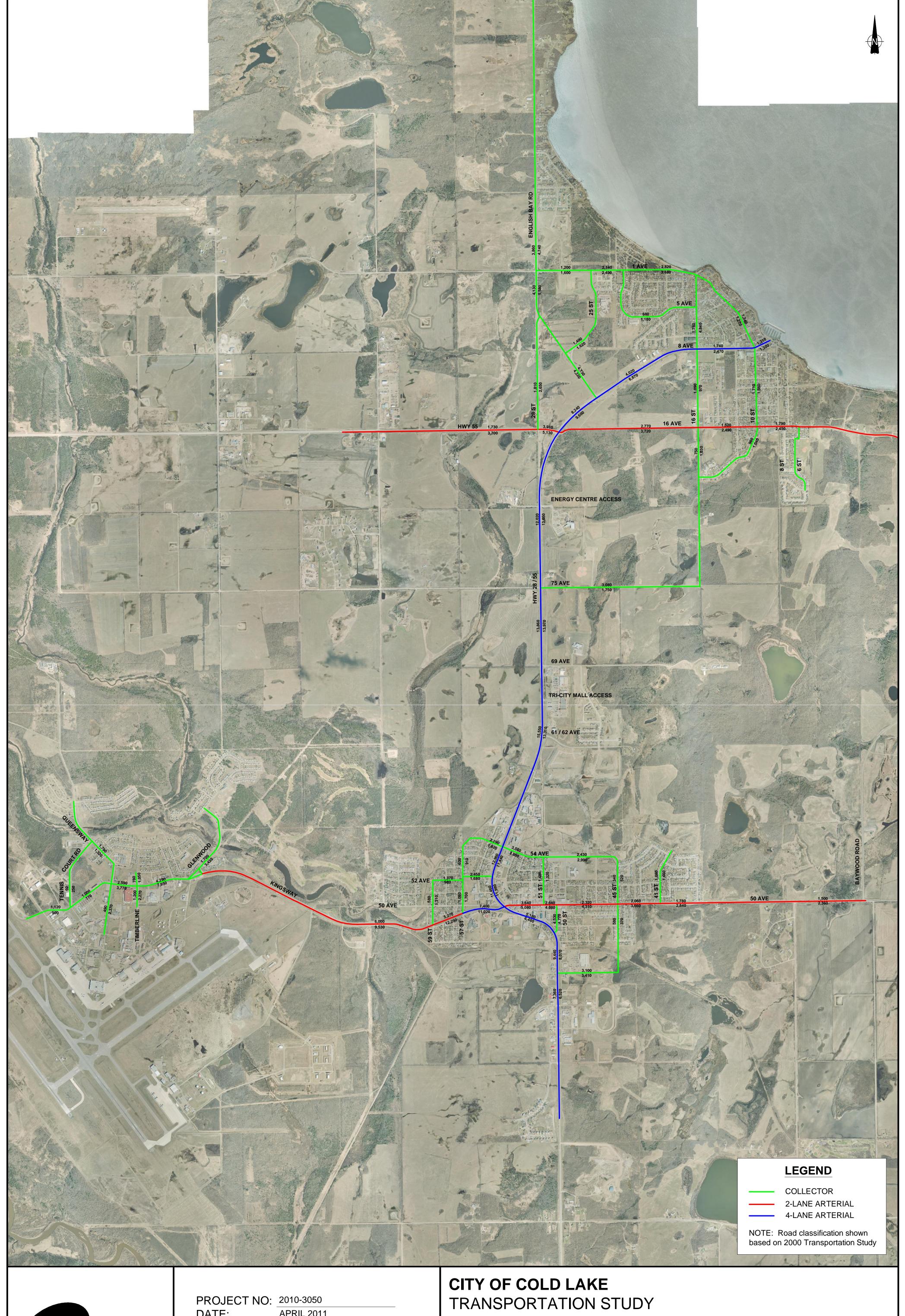
Figure 5.1 presents the forecasted daily total traffic volumes for the 5-year (2015) Horizon.

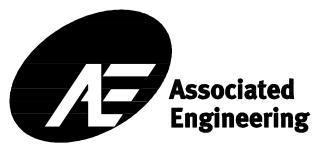
Figure 5.2 presents the forecasted daily total traffic volumes for the 10-year (2020) Horizon.

Figure 5.3 presents the forecasted daily total traffic volumes for the 15-year (2025) Horizon.

Figure 5.4 presents the forecasted daily total traffic volumes for the 20-year (2030) Horizon



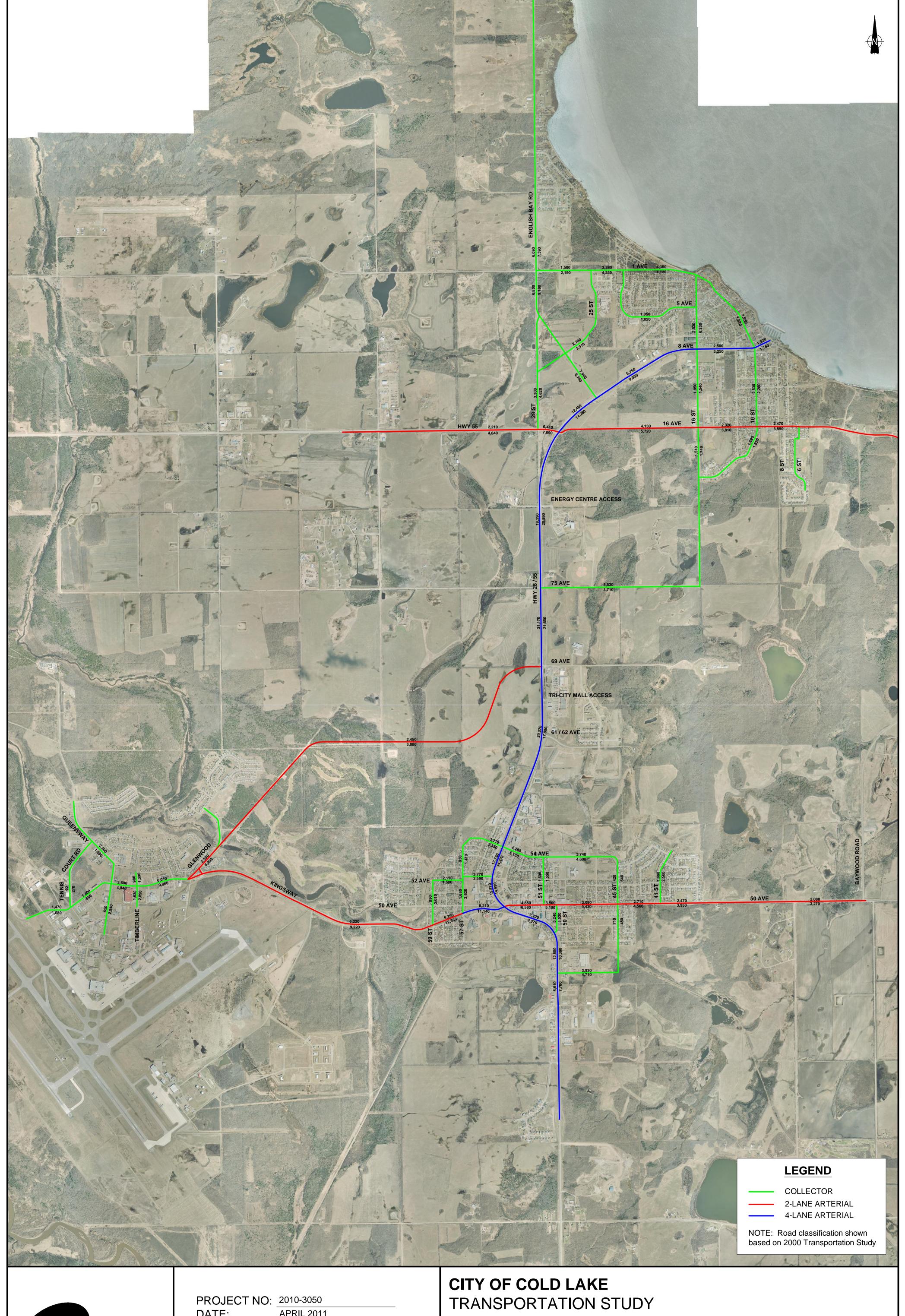


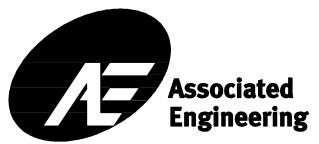


DATE: **APRIL 2011** APPROVED:

NTS SCALE: DWG NO:

FIGURE 5.1 5 YEAR (2015) DAILY TOTAL TRAFFIC VOLUMES

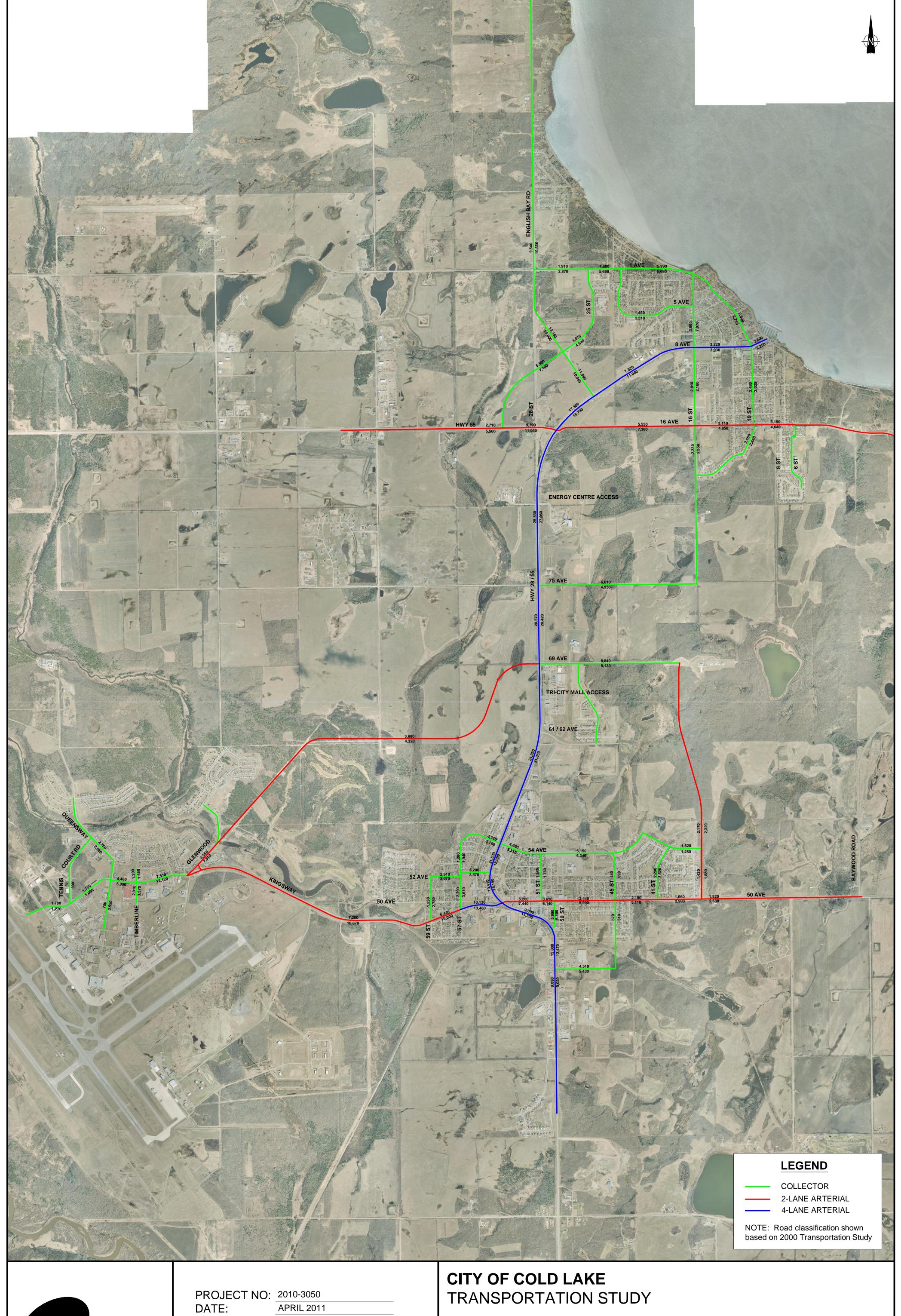




DATE: **APRIL 2011** APPROVED:

NTS SCALE: DWG NO:

FIGURE 5.2 10 YEAR (2020) DAILY TOTAL TRAFFIC VOLUMES

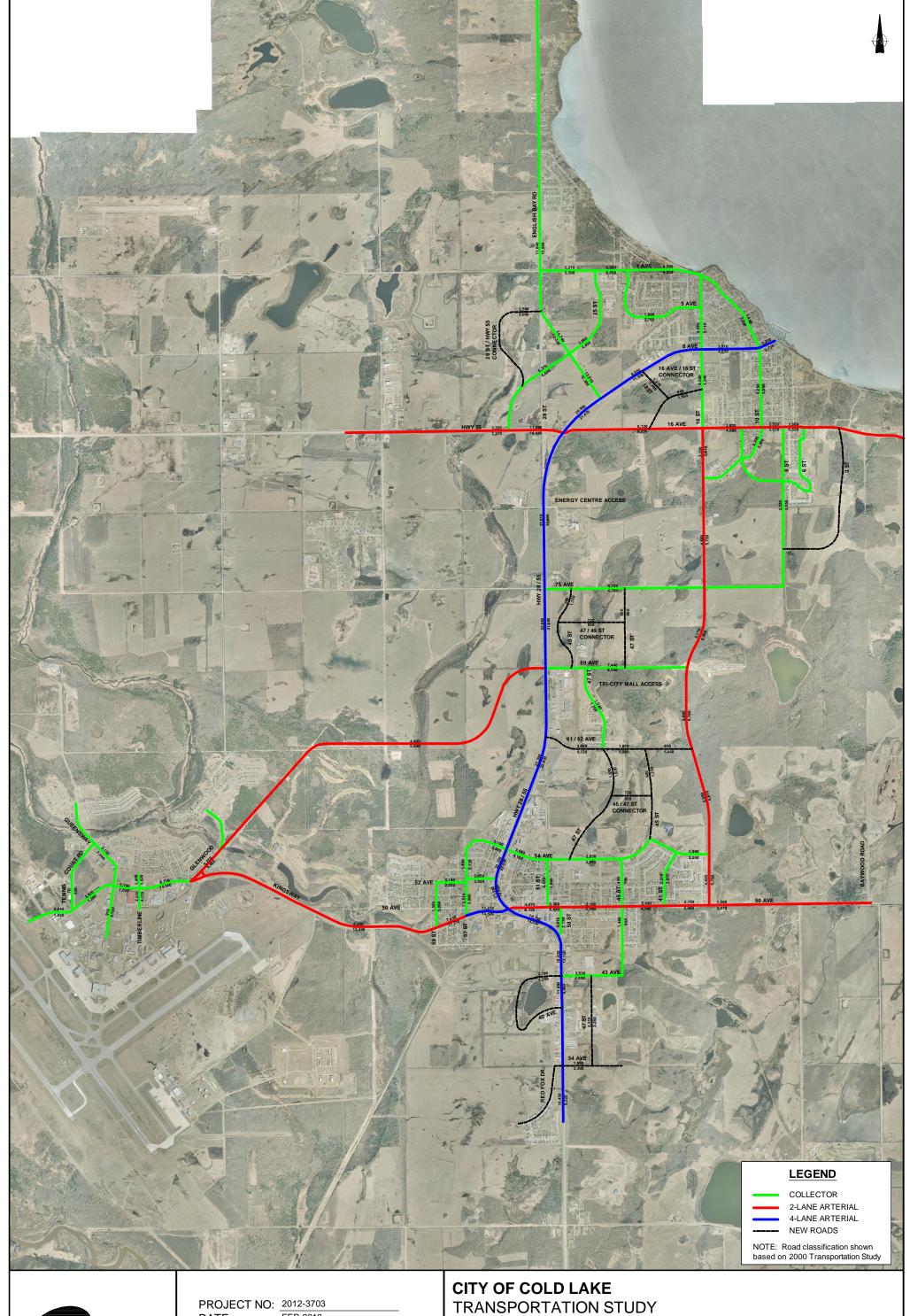




APPROVED:

NTS SCALE: DWG NO:

FIGURE 5.3 15 YEAR (2025) DAILY TOTAL TRAFFIC VOLUMES





PROJECT NO: 2012-3703

DATE: FEB 2013

APPROVED: NTS

DWG NO: NTS

FIGURE 5.4 (REVISED) 20 YEAR (2030) DAILY TOTAL TRAFFIC VOLUMES



# **Roadway Requirements**

#### 6.1 METHODOLOGY

Table 6.1 presents the City's roadway design standards obtained from the Municipal Engineering Servicing Standards And Standard Construction Specifications (2008). The table presents the City's roadway designation/classification and the daily service volumes for each roadway classification.

Table 6-1
City of Cold Lake – Roadway Classification and Daily Service Volumes

Roadway Designation	Daily Service Volume (vpd)	Daily Service Volume Range (vpd)	
Urban Expressway	>30,000	>30,000	
Divided Arterial	>20,000	20,000 - 30,000	
Undivided Arterial	<20,000	10,000 - 20,000	
Divided Residential Collector	<10,000	3,000 - 10,000	
Undivided Residential Collector	<10,000	3,000 - 10,000	
Divided Residential Local	<3,000	500 - 3,000	
11m Undivided Residential Local	<3,000	500 - 3,000	
10m Undivided Residential Local	<3,000	500 - 3,000	
Rural Industrial Collector	<10,000	3,000 - 10,000	
Urban Industrial Collector	<10,000	3,000 - 10,000	
Rural Industrial Local	<3,000	500 - 3,000	
Urban Industrial Local	<3,000	500 - 3,000	
Frontage (Service) Road	<3,000	500 - 3,000	
Lanes	<500	<500	

Table 6.2 presents typical lane capacities, in vehicles per hour, for various road classifications.



Table 6-2
Lane Capacity by Road Classification

Road Classification	City of Cold Lake Road Classification	Capacity (vehicles per hour, per lane)	Capacity (vehicles per day, per lane)	
Provincial Controlled Access Highway	Expressway	1,800	18,000	
County Arterial Road	Divided Arterial	1,000	10,000	
Local Major and Minor Arterial Roads	Undivided Arterial	800	8,000	
Local Collector Road	Collector (Residential or Industrial)	400	4,000	
Local Road (Other)	Local (Residential or Industrial)	100	1,000	

NOTE: Capacities are generalized based on typical engineering design standards.

Lane capacity per day based on assumption that peak hour traffic volumes are 10% of daily traffic volumes.

The forecasted total traffic volumes for each planning horizon were compared with the daily service volumes provided in Table 6.1 to determine the required roadway classification, as per the City's standards. The lane volumes were also compared with the lane capacity for the given road classification provided in Table 6.2, to determine the number of lanes required along each roadway.

#### 6.2 RESULTS

The required road classification and number of lanes for the future road network are summarized and provided in Appendix D for each planning horizon. The results presented in the appendix were determined from the forecasted traffic volumes and does not account for continuous roadway functionality and lane balancing along a single corridor.

As mentioned, the road classification and number of lanes required to accommodate traffic under the 20-year (2030) horizon will be used by the City to retain the right-of-way required to accommodate future roadway expansion. All the major corridors under the 20-year (2030) horizon were reviewed independently to establish continuous roadway function and lane balance along the corridor, where possible.

Figure 6.1 presents the recommended road classification and number of lanes, for the 20-year (2030) horizon.

According to the preliminary analysis completed for the 20-year planning horizon, 1 Avenue (25 Street to 16 Street) should be classified as a two-lane undivided arterial roadway. However, in order to maintain continuous roadway functionality, AE recommends that the road segment be classified as a collector roadway. This portion of 1 Avenue is adjacent to Kinosoo Beach, a major tourist attraction in Cold Lake North, and has been identified in the In-Service Road Safety Review technical memorandum, as an ideal location to implement traffic calming and beautification measures. A collector road classification would better suit the functionality of the area. Table 6.3 summarizes the road corridors in the 20-year planning horizon, along with the recommended road classification, number of lanes, and expected capacities. Table 6.4 summarizes the major road network in the 20-year planning horizon, along with a comparison of the existing and future road classification and number of lanes. The table also summarizes the improvements required to upgrade the corridors from the existing horizon to the 20-year planning horizon.



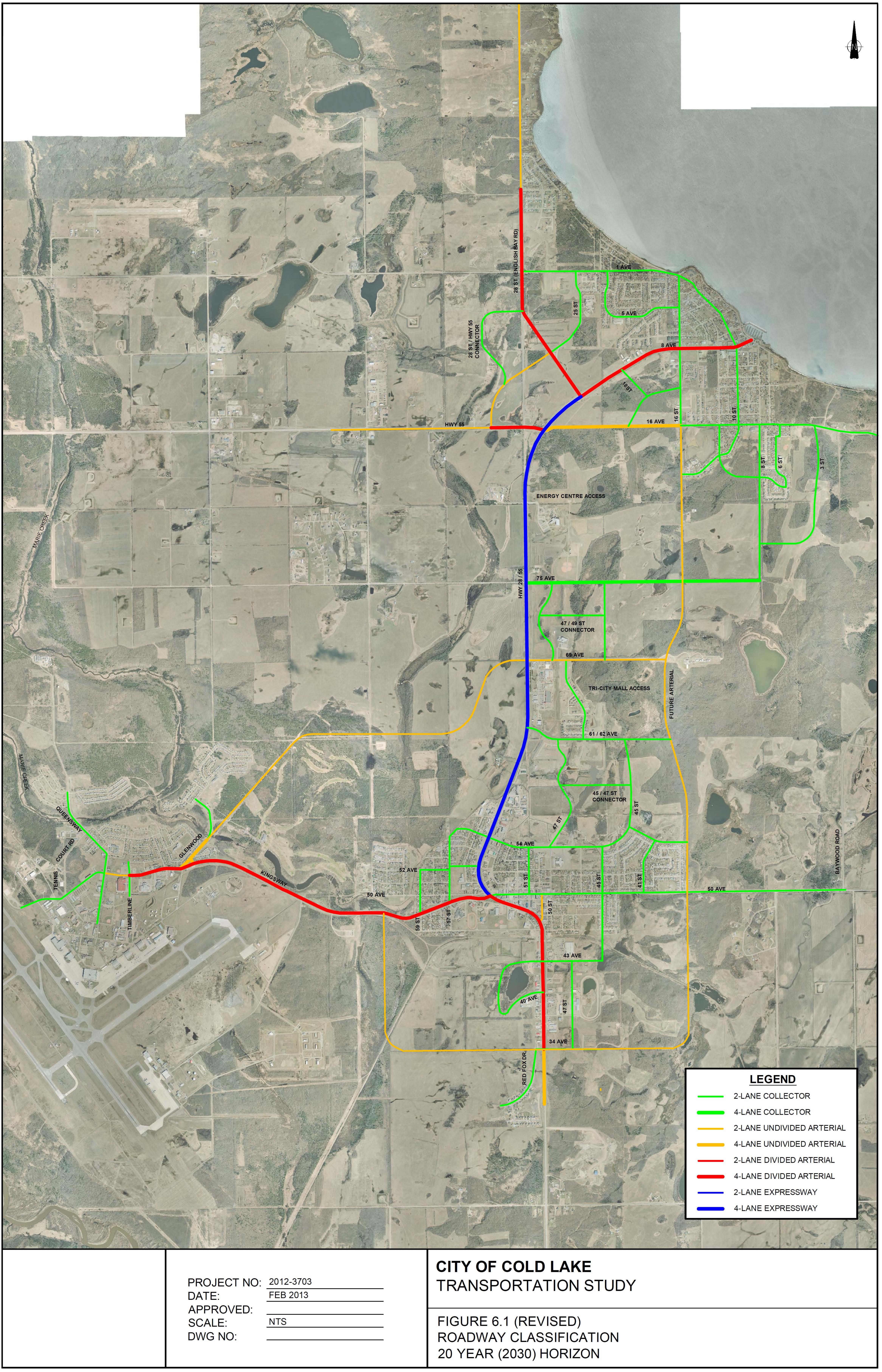


TABLE 6.3: CAPACITY ANALYSIS - 20 YEAR (2030) HORIZON - ROAD CLASSIFICATION, NUMBER OF LANES AND CAPACITIES

									Lane Capacity	Lane Capacity	Number of Lanes
	Corridor			Direction					for Road Classification	for Road Classification	Required (One
Tomos	1 Avenue						Collector				1
Column											
Prop				Westbound	5,560					,	
Prop   Program				Westbound	6,580	** *				-,	
Victor   V	Hwy 28	· ·		Southbound	20,900	42,370					2
	8 Avenue	25 Street	16 Street	Westbound	8,060	20,050	4-Lane Arterial	Divided Arterial	1,000	10,000	1
March   Marc	8 Avenue	16 Street	10 Street	Westbound	3,910	8,540	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	
March   Marc	8 Avenue	10 Street	Lakeshore Drive	Westbound	3,210	5,930	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	
Michael   Mich	Hwy 55	West City Limit	25 Street	Westbound	3,320	10,690	2-Lane Arterial	Undivided Arterial	800	8,000	1
1.	Hwy 55	25 Street	Hwy 28			25,720	2-Lane Arterial	Divided Arterial	1,000	10,000	
M. Service   19 Stand	16 Avenue	Hwy 28	16 Street			14,370	2-Lane Arterial	Undivided Arterial	800	8,000	
15   Security   15   Securit	16 Avenue <sup>5</sup>	16 Street	10 Street	Eastbound	4,880	8,700	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	
State   Stat	16 Avenue	10 Street	8 Street	Eastbound	3,670	7,190	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
	16 Avenue	8 Street	East City Limit	Eastbound	1,530	3,190	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
Seption   Design	English Bay Road <sup>6</sup>	North City Limit	Lake Avenue	Northbound	6,430	12,350	Collector	Undivided Arterial	800	8,000	
Popular Beynolds	* '		1 Avenue	Northbound	12,860	24.700	Collector	Divided Arterial	1,000	10.000	
Company   Comp				Northbound	14,740						
2   Street   Company   C											2
15 Stant   1-	- '				12,300 6,690				,	-,	
Secure   Content											
Select Concessor   Opt				Southbound	3,960						1
10   10   10   10   10   10   10   10	Street Connector			Westbound	2,080					,	1
18 Based   Punnes   P	Nelson Street			Westbound	1,600	,					1
1. Billion   1.	16 Street <sup>8</sup>	1 Avenue	8 Avenue	Southbound	3,570	8,680	Collector	Collector (Residential or Industrial)	400	4,000	1
1.	16 Street	8 Avenue	16 Avenue	Southbound	3,580	6,820	Collector	Collector (Residential or Industrial)	400	4,000	1
Concession   Survey		16 Avenue	75 Avenue	Southbound	3,200	7,010	Collector	Collector (Residential or Industrial)	400	4,000	1
Comment   15 Annual   15 Street   Machinery   15 Annual   15 Street   15 Annual   15 Ann	Connector	8 Avenue	16 Avenue		1,490	2,760	-	Local (Residential or Industrial)	150	1,500	
1		16 Avenue	16 Street			2,390	-	Local (Residential or Industrial)	150	1,500	
10 Servel   16 Annua   15 Annua		1 Avenue	8 Avenue			6,720	Collector	Collector (Residential or Industrial)	400	4,000	
10 Street   16 Annual   16 Street   17 Annual   27 Annual   2,200   4,700   Calector   Calector Residented in Industrial)   400   4,000   1	10 Street <sup>9</sup>	8 Avenue	16 Avenue	Northbound		7,810	Collector	Collector (Residential or Industrial)	400	4,000	•
8 Breef   19 Annum   75 Annum   25 Annum   35500   4.780   Calector   Calector   Calector (Residented or Inclusions)   450   4.500   1	10 Street	16 Avenue	16 Street	Northbound	2,380	4,680	Collector	Collector (Residential or Industrial)	400	4,000	1
6 State     16 American   24 American   2-2800   4,880   Collector   Collector (Residential or Industrial)   400   4,000   1	8 Street	16 Avenue	75 Avenue	Northbound	2,550	4,780	Collector	Collector (Residential or Industrial)	400	4,000	1
Hely 2855	6 Street 10	16 Avenue	21 Avenue	Northbound	2,380	4,680	Collector	Collector (Residential or Industrial)	400	4,000	1
Normal   Personal		Hwy 55/16 Avenue	75 Avenue	Northbound	30,680	62,490	4-Lane Arterial	Expressway	1.800	18.000	
Normal   Hay 2865   Futus Andread   22,000   50,210   4-Lane Andread   Egressway   1,800   18,000   2   2   2   2   2   2   2   2   2	•		69 Avenue	Northbound	31,690	65 550	4-I ane Arterial				2 2
Seminor   14					22,820						2 2
69 Avenue	,										
On Annual   Hay 2855   Faura Artifal   Eatliboard   4.480   15,700   Collector   Undivided Anterial   800   8,000   2   1   1   1   1   1   1   1   1   1		,				-, -				,	
49 Street   77 Avenue				Westbound	4,480						
47 Street   75 Avenue				Westbound	7,440					,	
# Street   61 Avenue   62 Avenue   63 Avenue   64 Avenue   64 Avenue   65 Aven	49 Street	75 Avenue	69 Avenue	Southbound	680	1,800	Collector	Local (Residential or Industrial)	150	1,500	
A Street   A Street		75 Avenue	69 Avenue	Southbound	600	1,560	Collector	Local (Residential or Industrial)	150	1,500	1
# 7 Street   61 Avenue®		47 Street		Westbound	720	1,670	-	Local (Residential or Industrial)	150	1,500	1
All Street   Street	47 Street	69 Avenue		Southbound	1,750	3,270	Collector	Collector (Residential or Industrial)	400	4,000	1
State   Manual   State   Manual   State   Manual   State   Manual   State   Manual   State   Manual   State   Manual   State	47 Street <sup>11</sup>	61 Avenue/62 Avenue	54 Avenue	Southbound	1,590	2,620	Collector	Local (Residential or Industrial)	150	1,500	1
## Annune@2 Avenue ## Af Street ## Street ## Street ## Eastbound ## 1.810 ## Annune@2 Avenue ## Af Street ## Future Anterial ## Eastbound ## 1.810	61 Avenue/62 Avenue <sup>12</sup>	Hwy 28/55	47 Street	Eastbound		8,000	Collector	Collector (Residential or Industrial)	400	4,000	1
## Street ## Str	61 Avenue/62 Avenue	47 Street	45 Street	Eastbound	3,990	5,800	Collector	Collector (Residential or Industrial)	400	4,000	1
47 Street	61 Avenue/62 Avenue	45 Street	Future Arterial	Eastbound	1,440	2,350	Collector	Collector (Residential or Industrial)	400	4,000	
45 Street   61 Avenue <sup>62</sup>   Avenue   54 Avenue   54 Avenue   54 Avenue   56 Street   Hwy 28/55   Eastbound   800   2,080   -   Local (Residential or Industrial)   150   1,500   1		47 Street	45 Street	Eastbound	950	1,670	-	Local (Residential or Industrial)	150	1,500	1
SourceOuth   Sou		61 Avenue/62 Avenue	54 Avenue	Northbound	1,280	2,080	-	Local (Residential or Industrial)	150	1,500	1
Standard   Standard   Street   Street   Street   Street   Standard   Street   Standard				Eastbound	3,380		Collector				1
Standard   Street				Eastbound	4,160						1
State				Eastbound	3,950						1
Section				Westbound Eastbound	2,850 3,095	-,		,		,	1
Section   Sect											
Section   Sect				Westbound	1,990						1
Section   Sect				Westbound	3,190	-,				,	1
Centre Avenue   59 Street   57 Street   Westbound   7,430   24,440   2-Lane Anterial   Divided Anterial   1,000   10,000   1			,	Westbound	3,850						i
Centre Avenue   57 Street   Five 265   Westbound   11,410   2,910   4-Laine Arterial   Division Arterial   1,000   10,000   2	Centre Avenue	59 Street	57 Street	Westbound	7,430	24,740	2-Lane Arterial	Divided Arterial	1,000	10,000	1
50 Avenue	Centre Avenue	57 Street	Hwy 28/55	Westbound	11,410	25,910	4-Lane Arterial	Divided Arterial	1,000	10,000	2
50 Avenue   51 Street   50 Street   Westbound   3,300   8,740   2-Lane Arterial   Collector (Residential or Industrial)   400   4,000   1	50 Avenue <sup>16</sup>	Hwy 28/55	51 Street	Westbound	4,475	10,830	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue <sup>19</sup> 50 Street 45 Street Eastbound 5,290 7,470 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue <sup>19</sup> 45 Street 41 Street Eastbound 2,180 7,470 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue <sup>19</sup> 45 Street 41 Street Westbound 2,440 7,380 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue 41 Street Future Arterial Eastbound 3,900 6,650 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue 41 Street Future Arterial 3,900 6,650 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue 41 Street Future Arterial 2,900 6,650 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue 41 Street Future Arterial 2,900 6,650 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue 41 Street Future Arterial 2,900 6,650 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue 41 Street Future Arterial 2,900 6,650 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue 41 Street Future Arterial 2,900 6,650 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue 41 Street Future Arterial 2,900 6,650 2-Lane Arterial 2,900 6,650 2-Lane Arterial 3,900 6,650 2-	50 Avenue <sup>17</sup>	51 Street	50 Street	Eastbound Westbound	5,440 3,300	8,740	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue <sup>13</sup> 45 Street 41 Street Eastbound 4,940 7,380 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue 41 Street Future Arterial 3,900 6,650 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1  50 Avenue 41 Street Future Arterial 3,900 6,650 2-Lane Arterial Collector (Residential or Industrial) 400 4,000 1	50 Avenue <sup>18</sup>	50 Street	45 Street	Eastbound	5,290	7,470	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	
50 Avenue 41 Street Future Arterial Eastbound 3,900 6,650 2,Lane Arterial Collector (Recidential or Industrial) 400 4,000 1	50 Avenue <sup>19</sup>	45 Street	41 Street	Eastbound	4,940	7,380	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
Westbound 2,750 Vertuel Vestbound 2,750 Vertuel Vestbound 1	50 Avenue	41 Street	Future Arterial	Eastbound	3,900	6,650	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1

TABLE 6.3: CAPACITY ANALYSIS - 20 YEAR (2030) HORIZON - ROAD CLASSIFICATION, NUMBER OF LANES AND CAPACITIES

:	Intersec	Intersection		Forecasted Vo		Road Classification Road Classification		Lane Capacity	Lane Capacity	Number of Lanes
Corridor	From	То	Direction	Daily Traffic -	Daily Traffic - Two Way	2000 TPS <sup>1</sup>	City of Cold Lake <sup>2</sup>	for Road Classification (veh/hour/lane) 3	for Road Classification (veh/day/lane) 4	Required (One Direction)
50 Avenue	Future Arterial	Baywood Road	Eastbound	2,070	3,630	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 1
43 Avenue <sup>20</sup>	Hwy 28/55	45 Street	Westbound Eastbound	1,560 4,830	8.360	Collector	Collector (Residential or Industrial)	400	4.000	1
59 Street	52 Avenue	Centre Avenue	Westbound Northbound	3,530 3,050	4.400	Collector	Collector (Residential or Industrial)	400	4.000	1
			Southbound Northbound	1,350 2,130	, , , ,				,	1
57 Street	54 Avenue	52 Avenue	Southbound Northbound	1,400 3,960	3,530	Collector	Collector (Residential or Industrial)	400	4,000	1
57 Street	52 Avenue	Centre Avenue	Southbound Northbound	2,510 20,930	6,470	Collector	Collector (Residential or Industrial)	400	4,000	1
Hwy 28/55	54 Avenue	52 Avenue	Southbound	22,150	43,080	4-Lane Arterial	Expressway	1,800	18,000	2
Hwy 28/55	52 Avenue	50 Avenue	Northbound Southbound	18,500 21,950	40,450	4-Lane Arterial	Expressway	1,800	18,000	2 2
Hwy 28/55	50 Avenue	50 Street	Northbound Southbound	10,700 13,030	23,730	4-Lane Arterial	Divided Arterial	1,000	10,000	2 2
Hwy 28/55	50 Street	43 Avenue	Northbound Southbound	13,120 16,420	29,540	4-Lane Arterial	Divided Arterial	1,000	10,000	2 2
Hwy 28/55	43 Avenue	34 Avenue	Northbound Southbound	9,960 11,690	21,650	4-Lane Arterial	Divided Arterial	1,000	10,000	1 2
Hwy 28/55	34 Avenue	South City Limit	Northbound Southbound	9,220 10,450	19,670	4-Lane Arterial	Undivided Arterial	800	8,000	2 2
40 Avenue	43 Avenue	Hwy 28/55	Eastbound	2,260	5,050	-	Collector (Residential or Industrial)	400	4,000	1
34 Avenue	Hwy 28/55	47 Street	Westbound Eastbound	2,790 2,400	4.390	-	Collector (Residential or Industrial)	400	4.000	1
47 Street	43 Avenue	34 Avenue	Westbound Northbound	1,990 2,280	4.490	_	Collector (Residential or Industrial)	400	4.000	1
			Southbound Northbound	2,210 1,980	, , , ,		, , , , , , , , , , , , , , , , , , , ,		,	1
51 Street	54 Avenue	50 Avenue	Southbound Northbound	2,230 7.170	4,210	Collector	Collector (Residential or Industrial)	400	4,000	1
50 Street	50 Avenue	Hwy 28/55	Southbound Northbound	6,810	13,980	Collector	Undivided Arterial	800	8,000	1 1
45 Street	54 Avenue	50 Avenue	Southbound	450	1,150	Collector	Local (Residential or Industrial)	150	1,500	1
45 Street	50 Avenue	43 Avenue	Northbound Southbound	960 1,460	2,420	Collector	Local (Residential or Industrial)	150	1,500	1 1
41 Street	54 Avenue	50 Avenue	Northbound Southbound	3,870 2,510	6,380	Collector	Collector (Residential or Industrial)	400	4,000	1
Future Arterial	75 Avenue	69 Avenue	Northbound Southbound	6,900 5.170	12,070	2-Lane Arterial	Undivided Arterial	800	8,000	1
Future Arterial	69 Avenue	61 Avenue/62 Avenue	Northbound Southbound	6,760 5,900	12,660	2-Lane Arterial	Undivided Arterial	800	8,000	1 1
Future Arterial	61 Avenue/62 Avenue	54 Avenue	Northbound Southbound	6,810 5,990	12,800	2-Lane Arterial	Undivided Arterial	800	8,000	1 1
Future Arterial	54 Avenue	50 Avenue	Northbound	5,730	10.350	2-Lane Arterial	Undivided Arterial	800	8.000	1
Kingsway	59 Street	Glenwood	Southbound Eastbound	4,620 12,400	20,620	2-Lane Arterial	Divided Arterial	1,000	10,000	1 2
Kingsway	Timberline	Glenwood	Westbound Eastbound	8,220 14,040	22,750	Collector	Divided Arterial	1,000	10,000	1 2
		Timberline	Westbound Eastbound	8,710 7,020	12.210	Collector	Undivided Arterial	800	8.000	1
Kingsway	Queensway		Westbound Eastbound	5,190 1,280	, ,				-,	1 1
Kingsway	Tennis Court Road	Queensway	Westbound	2,020	3,300	Collector	Collector (Residential or Industrial)	400	4,000	1 1
Kingsway	End of Road	Tennis Court Road	Westbound Westbound	2,040	3,480	Collector	Collector (Residential or Industrial)	400	4,000	1
Tennis Court Road	Queensway	Kingsway	Northbound Southbound	330 70	400	Collector	Lane	N/A	N/A	N/A N/A
Queensway	Tennis Court Road	Kingsway	Northbound Southbound	3,130 2,440	5,570	Collector	Collector (Residential or Industrial)	400	4,000	1
Queensway	Kingsway	Hanger Ln	Northbound Southbound	4,100 910	5,010	Collector	Collector (Residential or Industrial)	400	4,000	2
Timberline	Juniper Avenue	Kingsway	Northbound Southbound	1,920 1,400	3,320	Collector	Collector (Residential or Industrial)	400	4,000	1
Timberline	Kingsway	Athabasca Road	Northbound	4,170 2,310	6,480	Collector	Collector (Residential or Industrial)	400	4,000	2
Glenwood Drive	Glenwood	Kingsway	Northbound Southbound	8,460 5,270	13.730	2-Lane Arterial	Undivided Arterial	800	8,000	2
Road classification be Based on Lane Capas Based on assumption A 2-lane collector crox A 2-lane crox A 2-lane crox A 2-lane crox A 2-lane crox A 2-lane crox A 2-lane crox A 2-lane crox A 2-lane crox A 2-lane crox A 2-lane crox A 2-lane crox A 2-lane crox A 2-lane crox A 2-lane crox A 2-lane cr	zity Table (attached). Using that PM peak hour traffic i ss section would be approg for English Bay Road (Nort ss section would be approg ss section would be approg s section would be approg to fo Street to be similar to section would be apprograted poss section soupprograted poss section would be apprograted cost section would be apprograted to for 54 Nornue (45 Street).	nes stipulated in City's road classification acc s 10% of the daily traff riate for 16 Avenue be th City Limit to Lake Av riate 25 Street betwee riate for 16 Street betwee trate for 10 Street between 10 Street (16 Avenue priate for 47 Street betwor 61 Avenue 62 Avenue priate for 54 Avenue b to 41 Street) to be aver thy 28 and 51 Street in 51 Street her hyz 8 and 51 Street in 51 Street her hyz 8 and 51 Street in 51 St	cording to City's ic tween 16 Street venue) to be half in 1 Avenue and veen 1 Avenue are and 16 Street) tween 62 Avenue use between 47 Setween 56 Street etween 16 July 1aff is 10,830, a 2-lair si 10,830, a 2-lair si 10,830, a 2-lair si 10 Street etween 56 Street etween 56 Street etween 56 Street etween 56 Street etween 56 Street etween 56 Street etween 56 Street etween 56 Street etween 56 Street etween 56 Street etween 56 Street etween 56 Street etween 16 July 1aff is 10,830, a 2-lair si 10,830, a 2-l	standards.  and 10 Street as the A of daily traffic on Engli English Bay Road as the AF and 16 Avenue as the AF and 16 Avenue as the AF at and 45 Avenue as the AF at and 45 Street as the AF and 15 Street as the Constant of Street as the constant of Street as the constant of Street as the constant of Street as the constant of Street as the constant of Street as the constant of Street as the constant of Street as the constant of Street as the constant of Street as the constant of Street as the constant of Street as the constant of Street as the constant of Street as the constant of Street as the constant of Street as the Stree	ADT is less than 90 ish Bay Road (Lake / he AADT is less than 4DT is less than 90 AADT is less than 90 is less than 90 is less than 90 is less than 90 is less than 90 is less than 90 is less than 90 is less than 90 is less than 190 is less than	00. (Nenue to 1 Avenue) 9 9000 0 0 0 0 0 0 0 0 0 0 0 0	ard Construction Specifications, Jan 2008)  Future Anterial)  Soft the adjacent land use i.e. Downtown			

TABLE 6.4: COMPARISON OF EXISTING AND 20-YEAR ROAD NETWORK

	Interse	ation				Recommended 20-Year	
Corridor	From	То	Existing (2010) Road Classification <sup>1</sup>	Existing (2010) Number of Lanes (One Direction)	Recommended 20-Year (2030) Road Classification	(2030) Number of Lanes (One Direction)	Improvements Required
8 Avenue	10 Street	Lakeshore Drive	Undivided Arterial	1 1	Divided Arterial	2 2	Widen to provide centre median and 2 travel lanes in each direction
8 Avenue	25 Street	10 Street	Divided Arterial	2	Divided Arterial	2	-
Hwy 28/55	Hwy 55/16 Avenue	53 Avenue	Divided Arterial	2	Expressway	2	
Hwy 28/55	53 Avenue	52 Avenue	Undivided Arterial	2	Expressway	2	Widen to provide centre median
Hwy 28/55	52 Avenue	50 Avenue	Divided Arterial	2 2	Expressway	2	
Hwy 28/55	50 Avenue	52 Street	Divided Arterial	2 2	Divided Arterial	2 2	
Hwy 28/55	52 Street	47 Avenue	Undivided Arterial	2 2	Divided Arterial	2 2	Widen to provide centre median
Hwy 28/55	47 Avenue	40 Avenue	Divided Arterial	2 2	Divided Arterial	2 2	_
Hwy 28/55	40 Avenue	South City Limit	Undivided Arterial	2	Undivided Arterial	2	
1 Avenue	28 Street	1 Avenue	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1	
		28 Street		1	Undivided Arterial	1	
Hwy 55 <sup>2</sup> Hwy 55	West City Limit 28 Street	Hwy 28	Collector (Residential or Industrial)  Collector (Residential or Industrial)	1	Divided Arterial	1 2	Build pavement structure to Arterial standard (centre median and 2 trave
·				1		2 2	lanes in each direction)  Build pavement structure to Arterial standard (2 travel lanes in each
16 Avenue	Hwy 28	16 Street	Collector (Residential or Industrial)	1	Undivided Arterial	2	direction)
16 Avenue	16 Street	8 Street	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	2	Widen to provide 2 travel lanes in each direction
16 Avenue	8 Street	East City Limit	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1	•
English Bay Road	North City Limit	Lake Avenue	Collector (Residential or Industrial)	1 1	Undivided Arterial	1	- Build pavement structure to Arterial standard (centre median and 2 trave
English Bay Road	Lake Avenue	Hwy 28	Collector (Residential or Industrial)	1	Divided Arterial	2	Build pavement structure to Arterial standard (centre median and 2 trave lanes in each direction)
28 Street	English Bay Road	Hwy 55	Collector (Residential or Industrial)	1 1	Undivided Arterial	1 1	Realign 28 Street and build pavement structure to Arterial standard
25 Street	1 Avenue	English Bay Road	Collector (Residential or Industrial)	1	Undivided Arterial	1	-
Nelson Street	1 Avenue	16 Street	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1	-
16 Street	1 Avenue	16 Avenue	Collector (Residential or Industrial)	1	Undivided Arterial	1	-
16 Street	16 Avenue	75 Avenue	Local	1	Undivided Arterial	1	Build pavement structure to Arterial standard
Future Arterial	75 Avenue	50 Avenue	Non-existant		Undivided Arterial	1	Build out as per 20-year horizon
10 Street	1 Avenue	16 Avenue	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1	-
10 Street	16 Avenue	16 Street	Local	1	Collector (Residential or Industrial)	1	-
8 Street	16 Avenue	75 Avenue	Local	1	Collector (Residential or Industrial)	1	Build pavement structure to Collector standard
6 Street	16 Avenue	21 Avenue	Local	1	Collector (Residential or Industrial)	1	
20 Avenue	12 Street	8 Street	Non-existant		Collector (Residential or Industrial)	1	Build out as per 20-year horizon
75 Avenue	Hwy 28/55	Future Arterial	Local	1	Collector (Residential or Industrial)	2	Build pavement structure to Collector standard (2 travel lanes in each direction)
69 Avenue	Glenwood	Hwy 28/55	Non-existant	:	Undivided Arterial	1	Build out as per 20-year horizon
69 Avenue	Hwy 28/55	Future Arterial	Local	1	Undivided Arterial	1	
47 Street	69 Avenue	61/62 Avenue	Local	1	Collector (Residential or Industrial)	1	
54 Avenue	56 Street	49 Street	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	2 2	Widen to provide 2 travel lanes in each direction
54 Avenue	49 Street	Future Arterial	Non-existant	-	Collector (Residential or Industrial)	2 2	Build out as per 20-year horizon
52 Avenue	59 Street	57 Street	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1	
52 Avenue	57 Street	Hwy 28/55	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1 2	Widen to provide 2 travel lanes in each direction
Centre Avenue	59 Street	57 Street	Undivided Arterial	1	Divided Arterial	2 2	Widen to provide centre median and 2 travel lanes in each direction
Centre Avenue	57 Street	Hwy 28/55	Undivided Arterial	1 2	Divided Arterial	2 2	Widen to provide centre median
50 Avenue	Hwy 28/55	Future Arterial	Undivided Arterial	2 1	Undivided Arterial	2	
50 Avenue	Future Arterial	Baywood Road	Undivided Arterial	1	Collector (Residential or Industrial)	1	
43 Avenue	Hwy 28/55	45 Street	Local	1	Collector (Residential or Industrial)	1 2	Build pavement structure to Collector standard (2 travel lanes in each
				1		2	direction)
59 Street	52 Avenue	Centre Avenue	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1	
57 Street	54 Avenue	52 Avenue	Local	1	Collector (Residential or Industrial)	1	-
57 Street	52 Avenue	Centre Avenue	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1	-
51 Street	54 Avenue	50 Avenue	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1	-
50 Street	50 Avenue	Hwy 28/55	Collector (Residential or Industrial)	1	Undivided Arterial	1	•
45 Street	54 Avenue	50 Avenue	Collector (Residential or Industrial)	1 1	Collector (Residential or Industrial)	1 1	-
45 Street	50 Avenue	43 Avenue	Local	1 1	Collector (Residential or Industrial)	1	-
41 Street	54 Avenue	50 Avenue	Collector (Residential or Industrial)	1 1	Collector (Residential or Industrial)	1	
Kingsway	59 Street	Glenwood	Undivided Arterial	1	Divided Arterial	2 2	Widen to provide centre median and 2 travel lanes in each direction  Build payement structure to Arterial standard (centre median and 2 trave
Kingsway	Timberline	Glenwood	Collector (Residential or Industrial)	1	Divided Arterial	2 2	Build pavement structure to Arterial standard (centre median and 2 trave lanes in each direction)
Kingsway	Queensway	Timberline	Collector (Residential or Industrial)	1	Undivided Arterial	1	-
Kingsway	Queensway	End of Road	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1	-
Queensway	Tennis Court Road	Hanger Ln	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1	-
Timberline	Juniper Avenue	Athabasca Road	Collector (Residential or Industrial)	1	Collector (Residential or Industrial)	1	-
Glenwood Drive	Glenwood	Kingsway	Collector (Residential or Industrial)	1	Undivided Arterial	2 2	Build pavement structure to Arterial standard (2 travel lanes in each direction)
4 D 0000 T	ortotion Cturky Bood (	Placeifications with c	consideration for Highway 28 Twinning	(10 Street and 54 Avenue)			

 <sup>1.</sup> Based on 2000 Transportation Study Road Classifications with consideration for Highway 28 Twinning (10 Street and 54 Avenue)
 2. Following the reclassification of a roadway, no improvements are required to upgrade the pavement structure unless widening is also required.

#### TECHNICAL MEMORANDUM

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## **Summary of Findings**

AE was retained by the City to forecast the future traffic volumes for the next 20 years. Traffic volumes were forecasted for the 5-year, 10-year, 15-year, and 20-year planning horizons and analyzed to determine roadway classification and number of lanes required to accommodate the future traffic volumes.

Figure 5.1 through Figure 5.4 present the forecasted daily traffic volumes for the 5-year, 10-year, 15-year, and 20-year planning horizons respectively.

The forecasted total traffic volumes for each planning horizon were compared with the City's daily service volumes to determine the required roadway classification. The lane volumes were also compared with the lane capacity for the given road classification, to determine the required number of lanes required along each roadway. The results of the analysis are summarized in Appendix D for each planning horizon.

The 20-year (2030) road classification and number of lanes will be used by the City to determine the right-of-way that should be retained to accommodate future expansion of the road network. The major corridors in the 20-year road network were reviewed independently to establish consistent road classification and numbers of lanes along the corridor, where possible. The recommended road classification and number of lanes is presented in Figure 6.1.

Table 6.4 summarizes the major road network in the 20-year planning horizon, along with a comparison of the existing and future road classification and number of lanes. The table also summarizes the improvements required to upgrade the corridors from the existing horizon to the 20-year planning horizon.



## **TECHNICAL MEMORANDUM**



# **Appendix A - ASP, ARP and Outline Plan Information**



Project No: 2010-3050 Date: January 28, 2011

#### FISCHER ESTATES - LAND USE INFORMATION

Land Use Type	Total Dev	velopable	Develope	ed in 2010	Undeveloped in 2010	
Land Ose Type	Area (ha.)	Dwelling Units	Areas (ha)	Dwelling Units	Area (ha)	Dwelling Units
Low-Density Residential	25.0	449	-	0	-	449
Multi-Family Residential	5.9	295	-	0	-	295
Commercial - Arterial	3.6	-	3.6	-	0.0	-
Commercial - Neighbourhood	6.7	-	0.9	-	5.8	-
Municipal Reserve	5.0	-	0.0	-	5.0	-
Stormwater	4.7	-	0.0	-	4.7	-
Other (Roadway/Pathway)	12.6	-	0.0	-	12.6	-
Total	63.5	744	4.5	0	28.1	744

Project No: 2010-3050 Date: January 28, 2011

#### **IRON HORSE - LAND USE INFORMATION**

Land Use	Total Dev	/elopable	Develope	ed in 2010	Undeveloped in 2010	
Land USe	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units
Low-Density Residential	19.7	323	-	0	-	323
Medium-Density Residential	0.6	18	-	0	-	18
High-Density Residential	0.9	45	-	0	-	45
Municipal Reserve	2.24	-	0.0	-	0.0	-
Other (Roadways)	7.36	-	0.0	-	0.0	-
Total	30.8	386	0.0	0	0.0	386

City of Cold Lake Transportation Study Traffic Volume Forecast Project No: 2010-3050

Date: January 28, 2011

#### **COLD LAKE CENTRAL - LAND USE INFORMATION**

	Total Developable		Develope	ed in 2010	Undeveloped in 2010	
Land Use	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units
Low-Density Residential	90.1	1,559	•	205	-	1,354
Medium-Density Residential	20.7	622	•	44	•	578
High-Density Residential	10.5	1,046	•	444	•	602
Manufactured Housing	12.3	243	•	243	-	0
Commercial - Arterial	37.7	-	18.9	-	18.7	-
Institutional	2.6	-	2.6	-	0.0	-
Parks/Municipal Reserve	25.8	-	0.0	-	0.0	-
Stormwater Facility/PUL (Sanitary Forcemain)	13.9	-	0.0	-	0.0	-
Circulation	36.9	=	0.0	-	0.0	-
Total	250.6	3,470	21.6	936	18.7	2,534

Project No: 2010-3050 Date: January 28, 2011

#### **GRAND CENTRE SE - LAND USE INFORMATION**

	Total Developable		Develope	ed in 2010	Undeveloped in 2010	
Land Use	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units
Low-Density Residential	21.6	370	-	89	-	281
Mobile Home	8.4	240	-	90	-	150
Commercial - Arterial	10.1	-	10.1	-	0.0	-
Industrial	15.8	-	9.9	-	5.9	-
Utility	6.4	-	0.0	-	6.4	-
Open Space	1.8	-	0.0	-	1.8	-
Fairgrounds	40.1	-	0.0	-	40.1	-
Cementary	0.8	-	0.8	-	0.0	-
Total	105.0	610	20.8	179	54.3	431

City of Cold Lake Transportation Study

Traffic Volume Forecast Project No: 2010-3050 Date: January 28, 2011

#### FOREST HEIGHTS - LAND USE INFORMATION

	Total Developable		Develope	ed in 2010	Undeveloped in 2010	
Land Use	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units
Developed in 2007 (120 Residential Lots & School Site)	19.6	120	19.6	120	0.0	0
Single-Family Residential	20.6	345	0.0	0	20.6	345
Multi-Family Residential	8.3	248	0.0	0	8.3	248
Municipal Reserve	4.4	-	0.0	-	4.4	-
Storm Water Management	1.7	-	0.0	-	1.7	-
Roadways	9.4	-	0.0	-	9.4	-
Total Residential	64.0	713	19.6	120	44.4	593

City of Cold Lake Transportation Study Traffic Volume Forecast Project No: 2010-3050 Date: January 28, 2011

#### **NORTHSHORE - LAND USE INFORMATION**

\* Land uses highlighted in yellow will be used to generate development traffic volumes \*

#### **TOTAL AREA**

Land Use	Total Area <sup>1</sup> (ha)	Creekside ASP <sup>2</sup> (ha)	Parkview ASP <sup>3</sup> (ha)	Remaining Area (ha)
		60.5	` '	` ,
Gross Area	244.1		36.8	146.8
Non-Residential Subtotal	125.6	17.8	13.2	94.6
Linear Parks (Parkways/Trails)	4.9	0.0	0.0	4.9
Local Parks	10.4	1.3	5.7	3.5
Special Study Area	20.8	9.8	0.0	11.0
Stormwater Management Facilities	10.1	3.6	0.0	6.5
Public Utility Lots	1.6	0.0	0.0	1.6
Roads	53.6	2.8	7.5	43.3
School Site	4.6	0.0	0.0	4.6
Institutional	5.6	0.0	0.0	5.6
Religious Assembly	0.3	0.3	0.0	0.0
Neighbourhood Commercial	1.8	0.0	0.0	1.8
Highway Commercial	11.9	0.0	0.0	11.9
Residential	118.5	42.7	23.6	52.2
Low Density Residential	91.9	38.4	21.0	32.6
Medium Density Residential	14.9	4.4	0.0	10.6
Mixed Use Commercial	9.1	0.0	2.6	6.5
Mixed Use Institutional	2.6	0.0	0.0	2.6

<sup>1.</sup> From Northshore ASP

#### 2007 HORIZON - DEVELOPED

Land Use	Total Area (ha)	Creekside ASP (ha)	Parkview ASP (ha)	Remaining Area (ha)
Gross Area	244.1	60.5	36.8	146.8
Non-Residential Subtotal	5.9	0.3	0.0	5.6
Linear Parks (Parkways/Trails)	0.0	0.0	0.0	0.0
Local Parks	0.0	0.0	0.0	0.0
Special Study Area	0.0	0.0	0.0	0.0
Stormwater Management Facilities	0.0	0.0	0.0	0.0
Public Utility Lots	0.0	0.0	0.0	0.0
Roads	0.0	0.0	0.0	0.0
School Site	0.0	0.0	0.0	0.0
Institutional	5.6	0.0	0.0	5.6
Religious Assembly	0.3	0.3	0.0	0.0
Neighbourhood Commercial	0.0	0.0	0.0	0.0
Highway Commercial	0.0	0.0	0.0	0.0
Residential	5.1	0.0	0.0	5.1
Low Density Residential	5.1	0.0	0.0	5.1
Medium Density Residential	0.0	0.0	0.0	0.0
Mixed Use Commercial	0.0	0.0	0.0	0.0
Mixed Use Institutional	0.0	0.0	0.0	0.0
Developable Area	233.1	60.2	36.8	136.1

<sup>2.</sup> From Creekside ASP

<sup>3.</sup> From Parkview ASP

Project No: 2010-3050 Date: January 28, 2011

#### **NORTHSHORE - LAND USE INFORMATION**

\* Land uses highlighted in yellow will be used to generate development traffic volumes \*

#### 2010 HORIZON - DEVELOPED

Land Use	Total Area (ha)	Creekside ASP (ha)	Parkview ASP (ha)	Remaining Area (ha)
Gross Area	233.1	60.2	36.8	136.1
Non-Residential Subtotal	0.9	0.9	0.0	0.0
Linear Parks (Parkways/Trails)	0.0	0.0	0.0	0.0
Local Parks	0.0	0.0	0.0	0.0
Special Study Area	0.0	0.0	0.0	0.0
Stormwater Management Facilities	0.9	0.9	0.0	0.0
Public Utility Lots	0.0	0.0	0.0	0.0
Roads	0.0	0.0	0.0	0.0
School Site	0.0	0.0	0.0	0.0
Institutional	0.0	0.0	0.0	0.0
Religious Assembly	0.0	0.0	0.0	0.0
Neighbourhood Commercial	0.0	0.0	0.0	0.0
Highway Commercial	0.0	0.0	0.0	0.0
Residential	7.4	5.7	1.7	0.0
Low Density Residential	7.4	5.7	1.7	0.0
Medium Density Residential	0.0	0.0	0.0	0.0
Mixed Use Commercial <sup>1</sup>	0.0	0.0	0.0	0.0
Mixed Use Institutional	0.0	0.0	0.0	0.0
Developable Area	224.8	53.7	35.1	136.1

#### 2010 - DEVELOPABLE

	Total Area	Creekside ASP	Parkview ASP	Remaining Area
Land Use	(ha)	(ha)	(ha)	(ha)
Gross Area	224.8	53.7	35.1	136.1
Non-Residential Subtotal	118.8	16.6	13.2	89.0
Linear Parks (Parkways/Trails)	4.9	0.0	0.0	4.9
Local Parks	10.4	1.3	5.7	3.5
Special Study Area	20.8	9.8	0.0	11.0
Stormwater Management Facilities	9.2	2.8	0.0	6.5
Public Utility Lots	1.6	0.0	0.0	1.6
Roads	53.6	2.8	7.5	43.3
School Site	4.6	0.0	0.0	4.6
Institutional	0.0	0.0	0.0	0.0
Religious Assembly	0.0	0.0	0.0	0.0
Neighbourhood Commercial	1.8	0.0	0.0	1.8
Commercial - Arterial	11.9	0.0	0.0	11.9
Residential	106.0	37.1	21.9	47.1
Low Density Residential	79.4	32.7	19.2	27.5
Medium Density Residential	14.9	4.4	0.0	10.6
Mixed-Use Commercial 4	9.1	0.0	2.6	6.5
Mixed-Use Institutional	2.6	0.0	0.0	2.6

<sup>4.</sup> To maintain consistency with Northshore ASP, Parkview's Commercial as been considered as Mixed Use Commercial - To account for total land area

Project No: 2010-3050 Date: January 28, 2011

#### **NORTHSHORE - LAND USE INFORMATION**

\* Land uses highlighted in yellow will be used to generate development traffic volumes \*

#### TOTAL DEVELOPABLE - SCHOOL/RESIDENTIAL

	Total	Total Area		Parkview ASP	Remaining Area
Land Use	Unit	# of	# of	# of	# of
School	Students	1,958	0	0	1,958
Low-Density Residential	Dwelling Units	1,654	659	401	594
Medium-Density Residential	Dwelling Units	671	196	0	475
Mixed-Use Commercial	Dwelling Units	547	0	0	547
Mixed-Use Institutional	Dwelling Units	157	0	0	157

#### 2007 DEVELOPED - SCHOOL/RESIDENTIAL

	Total	Area	Creekside ASP	Parkview ASP	Remaining Area
Land Use	Unit	# of	# of	# of	# of
School	Students	0	0	0	0
Low-Density Residential	Dwelling Units	57	0	0	57
Medium-Density Residential	Dwelling Units	0	0	0	0
Mixed-Use Commercial	Dwelling Units	0	0	0	0
Mixed-Use Institutional	Dwelling Units	0	0	0	0

#### 2010 DEVELOPED - SCHOOL/RESIDENTIAL

	Total Area		Creekside ASP	Parkview ASP	Remaining Area
Land Use	Unit # of		# of	# of	# of
School	Students	0	0	0	0
Low-Density Residential	Dwelling Units	99	65	34	0
Medium-Density Residential	Dwelling Units	0	0	0	0
Mixed-Use Commercial	Dwelling Units	0	0	0	0
Mixed-Use Institutional	Dwelling Units	0	0	0	0

#### 2010 DEVELOPABLE - SCHOOL/RESIDENTIAL

	Total	Total Area		Parkview ASP	Remaining Area
Land Use	Unit	# of	# of	# of	# of
School	Students	1,958	0	0	1,958
Low-Density Residential	Dwelling Units	1,498	594	367	537
Medium-Density Residential	Dwelling Units	671	196	0	475
Mixed-Use Commercial	Dwelling Units	547	0	0	547
Mixed-Use Institutional	Dwelling Units	157	0	0	157

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#### LOT 2, PLAN 982 1024 - LAND USE INFORMATION

	Total Developable		Develope	ed in 2010	Undeveloped in 2010	
Land Use	Area (s.ft.)	Dwelling Units	Area (s.ft.)	Dwelling Units	Area (s.ft.)	Dwelling Units
Residential						
Building 1	5,506.0	12	5,506.0	12	0.0	0
Building 2	5,506.0	12	5,506.0	12	0.0	0
Building 3	19,394.0	54	19,394.0	54	0.0	0
Building 6 - Will not be built	0.0	0	0.0	0	0.0	0
Total Residential	30,406.0	78	30,406.0	78	0.0	0
Commercial						
Building 4	11,295.9	0	0.0	0	11,295.9	0
Building 5	4,068.6	0	0.0	0	4,068.6	0
Total Commercial	15,364.5	0	0.0	0	15,364.5	0

City of Cold Lake Transportation Study

Traffic Volume Forecast Project No: 2010-3050 Date: January 28, 2011

#### **HORSESHOE BAY - LAND USE INFORMATION**

Land Use	Area (acres)	Area (ha)	% of Planned Area	Lots	Population Estimate
Existing 50 ft width lot (Beach Avenue)	4.0	1.6	2.0%	21	65
Existing 0.5 acre lots	3.7	1.5	2.0%	7	22
Existing 1.0 acre lots	14.0	5.7	7.4%	11	34
Potential Serviced Residential Estates	105.0	42.5	55.3%	182	564
Natural Area Park	5.0	2.0	2.6%		
Lakeshore Trail System	5.0	2.0	2.6%		
Environmental Reserve	26.0	10.5	13.7%		
English Bay Road	7.0	2.8	3.7%		
Local Roads (by dedication)	20.0	8.0	10.5%		
Total	190.0	77.0	100.0%	219	651

	Total Developable	Developed in 2010	Undeveloped in 2010
Land Use	(Dwelling Unit)	(Dwelling Unit)	(Dwelling Unit)
Low Density Residential	219	177	42

Project No: 2010-3050 Date: January 28, 2011

#### **UPLANDS - LAND USE INFORMATION**

	Total Developable		Develope	ed in 2010	Undeveloped in 2010	
Land Use	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units
Single-Family Residential	45.2	904	0.0	0	45.2	904
Multi-Family Residential	9.6	480	0.0	0	9.6	480
Health Services and Mixed Use	5.0	•	0.0	-	5.0	-
Municipal Reserve	12.7	•	0.0	-	12.7	-
SWMF and Existing Wetlands	7.9	•	0.0	-	7.9	-
Roads and Lanes	21.5	-	0.0	-	21.5	-
Total	101.9	1,384	0.0	0	101.9	1,384

City of Cold Lake Transportation Study Traffic Volume Forecast Project No: 2010-3050

Date: January 28, 2011

#### LAKESHORE REDEVELOPMENT - LAND USE INFORMATION

	Description	Existing Land Use	Area (s.m.)	Area (hec)	Max. Site Coverage	Developable Area (hec)	Area (sq.ft)
1	Vacant parcel on 12 Street and 8 Avenue	Vacant	16,938.6	1.7	0%	0.0	0.0
2	902 10 Street <sup>1</sup>	Commercial	1,097.2	0.1	50%	0.1	5,905.0
3	904 10 Street	Commercial	690.4	0.1	50%	0.0	3,715.9
4	901 9 Avenue	Commercial	1,118.8	0.1	50%	0.1	6,021.3
5	803 10 Avenue	Commercial	2,248.6	0.2	50%	0.1	12,102.1
6	Triangle Park <sup>2</sup>	Park / Open Space	1,135.7	0.1	100%	0.1	12,224.7
7	Bibeau Park	Park / Open Space	11,648.9	1.2	100%	1.2	125,387.8
8	Centoaph Park	Park / Open Space	2,605.4	0.3	100%	0.3	28,044.2
9	Fire Hall <sup>3</sup>	Fire Hall	3,427.9	0.3	50%	0.2	18,449.1

<sup>1.</sup> Assume maximum site coverage for HDR is the same for MDR (50%)

<sup>3.</sup> From address map, fire hall building is approximately 50% of site.

	Description	Future Land Use	Area (s.m.)	Area (hec)	Max. Site Coverage	Developable Area (hec)	Area (sq.ft)	Dwelling Units
1	Vacant parcel on 12 Street and 8 Avenue <sup>4</sup>	Medium Density Residential	16,938.6	1.7	50%	0.8	91,162.6	38
2	902 10 Street <sup>5</sup>	High Density Residential	1,097.2	0.1	50%	0.1	5,905.0	15
3	904 10 Street <sup>5</sup>	High Density Residential	690.4	0.1	50%	0.0	3,715.9	9
4	901 9 Avenue <sup>5</sup>	High Density Residential	1,118.8	0.1	50%	0.1	6,021.3	15
5	803 10 Avenue <sup>6</sup>	Low Density Residential	2,248.6	0.2	45%	0.1	10,891.9	3
6	Triangle Park	Park / Open Space	1,135.7	0.1	100%	0.1	12,224.7	-
7	Bibeau Park	Park / Open Space	11,648.9	1.2	100%	1.2	125,387.8	-
8	Centoaph Park	Park / Open Space	2,605.4	0.3	100%	0.3	28,044.2	-
9	Fire Hall	Community Hall	3,427.9	0.3	50%	0.2	18,449.1	-

<sup>4.</sup> Maximum Density of 45 units/ha

<sup>2.</sup> Will not include in existing trip generation since not currently used. Following redevelopment, parks will be used and generate traffic.

<sup>5.</sup> HDR assumed to have 1 dwelling unit per 400 sq.ft of building footprint. Derived using ratio from Lot 2 buildings.

<sup>6. 803 10</sup> Avenue can be subdivided into three single family lots

City of Cold Lake Transportation Study

Traffic Volume Forecast Project No: 2010-3050 Date: January 28, 2011

#### **LAKEWOOD ESTATES - LAND USE INFORMATION**

		Total Developable		Develop	ed in 2010	Undeveloped in 2010	
Land Use	Area (s.m.)	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units
Total ASP Area	213,281.7	21.3	198				
Low-Density Residential	103,315.2	10.3					
Phase I		-	45	-	45	-	0
Phase II		-	32	-	0	-	32
Phase III		-	21	-	0	-	21
Phase IV		-	31	-	0	-	31
Phase V		-	21	-	0	-	21
Phase VI		-	28	-	0	-	28
Phase VII		-	20	-	0	-	20
Municipal Reserve	25,619.5	2.6	-	0.0	-	2.6	-
Others (Roadway/Pathways)	84,347.0	8.4	-	0.0	-	8.4	-
Total	213,281.7	21.3	198	0.0	45	11.0	153

City of Cold Lake Transportation Study Traffic Volume Forecast Project No: 2010-3050

Date: January 28, 2011

#### **CREEKSIDE ESTATES - LAND USE INFORMATION**

		Total D	evelopable		Developed in 2010		Undeveloped in 2010	
Land Use	Area (s.m.)	Area (ha)	Density (Units/ha) <sup>1</sup>	Dwelling Units	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units
Total ASP Area	605,034.5	60.5						
Developed - In 2010								
Low-Density Residential	56,734.3	5.7	-	65	-	65	-	0
SWMF	-	0.9	-	-	0.9	-		
Undeveloped - In 2010								
Low-Density Residential	330,059.3	33.0	18.0	594	-	0	-	594
Medium-Density Residential	-	4.4	45.0	196	-	0	-	196
Park	-	1.3	-	-	0.0	-	1.3	-
SWMF	-	2.8	-		0.0	-	2.8	-
Special Study Area	-	9.8	-	-	0.0	-	9.8	-
Other (Roadways / Pathways)	-	2.8					2.8	
Total		60.5		855	0.9	65	16.6	790

<sup>1.</sup> From Northshore ASP

City of Cold Lake Transportation Study Traffic Volume Forecast Project No: 2010-3050

Date: January 28, 2011

#### PARKVIEW ESTATES - LAND USE INFORMATION

	Total Developable			2010 Developed		2010 Undeveloped		
Land Use	Area (s.m.)	Area (ha)	Density (Units/ha) <sup>1</sup>	Dwelling Units	Area (ha)	Dwelling Units	Area (ha)	Dwelling Units
Total ASP Area	367,975.6	36.8	-	-				
Developed - In 2010								
Low-Density Residential (R1B)	17,287.5	1.7	-	34	-	34	-	0
Undeveloped - In 2010								
Low-Density Residential - Divided Lots	51,906.5	5.2	-	114	-	0		114
Low-Density Residential - Undivided Lots	140,448.2	14.0	18.0	253	-	0		253
Neighbourhood Commercial <sup>2</sup>	26,325.8	2.6	-	-	0.0	-	2.6	
Open Space	56,814.5	5.7	-	-	0.0	-	5.7	
Other (Roadways / Pathways)	75,193.1	7.5	-	-	0.0	-	7.5	
Total	292,782.5	29.3		401	0.0	34	15.8	367

<sup>1.</sup> From Northshore ASP

<sup>2.</sup> Outline Plan shows this area as C3 - Neighbourhood Commercial but Northshore ASP shows as Mixed Use Commercial

Project No: 2010-3050 Date: January 28, 2011

#### **HILLS OF COLD LAKE - LAND USE INFORMATION**

\* Land uses highlighted in yellow will be used to generate development traffic volumes \*

Description of Land Use	Unserviced Lots (Acres)	Serviced Lots (Acres)
Total area available for development	294.9	294.9
Land to be allocated to the MD. Land marked as MR reserve.	45.7	64.5
Area of road reserve and public utility lanes	46.3	59.5
Area planned for establishment of building lots	202.9	170.9

**Phasing** 

Phase	Unserviced Lot Subdivision	Serviced Lot Subdivision
Phase A - Year 1	40	40
Phase B - Year 2-3	40	40
Phase C - Year 4-5	40	40
Phase D - Year 6-8	60	60
Phase E - Year 9	20	20
Phase F - Year 10-11	-	40
Phase G - Year 12-13	-	40
Phase H - Year 14	-	20
Total	200	300

City of Cold Lake Transportation Study Traffic Volume Forecast Project No: 2010-3050 Date: January 28, 2011

### FAWN RIDGE ESTATES - LAND USE INFORMATION

		Developable Area	
Subdivisions and Legal Description	Land Use	(Acres)	Dwelling Units
NW 23-62-3-4 (Fawn Ridge Estates Subdivision)	Country Residential (CR)	86.3	54

			Direction Distribution (%)		Direction Distribution (%)	
	Trip Generation Rate (Trips per Dwelling					
Time Period	Units)	Generated Trips	Inbound	Outbound	Inbound	Outbound
	NW 23-62-3-4 (Fawn R	idge Estates Subdivis	sion)			
	Country Residential (Fawn Ridg	ge Estates Subdivisio	n) Code 210			
Weekday (AADT)	Weekday (AADT) 9.57 517 50% 50% 258 258					
AM Peak Hour 0.77 42 26% 74% 11 31						31
PM Peak Hour	1.02	55	64%	36%	35	20

City of Cold Lake Transportation Study Project No: 2010-3050

Date: April 9, 2011

### MD BONNYVILLE - RESIDENTIAL

Source: Intermunicipal Development Plan (Feb 2009)

### Future Land Uses:

- Residential developments

Land Use	Location	Developable Area (m²)	Developable Area (Hec)	Developed Area by 2030 (Hec)
Residential Development 1 - 30% Developed by 2030	Along north side of Highway 55, west of Cold Lake	629,116.85	63	19
Residential Development 2 - 30% Developed by 2030	West of IDP Commercial Development, between 75 Avenue and south of 61/62 Avenue	843,132.02	84	25
Residential Development 3 - 30% Developed by 2030	East of Cold Lake Central, between Energy Centre to 55 Avenue	4,178,346.10	418	125

#### Assumed:

- Single family: 20 dwelling units/ha
- Multi family: 50 dwelling units/ha
- 75/25 split between single family and multi family residential developments

Land Use	Building Type	Developable Area (Hec)	Dwelling Units
	75% - Single Family Residential	14	283
Residential Development 1	25% - Multi Family Residential	5	236
	Total	19	519
	75% - Single Family Residential	19	379
Residential Development 2	25% - Multi Family Residential	6	316
	Total	25	696
	75% - Single Family Residential	94	1,880
Residential Development 3	25% - Multi Family Residential	31	1,567
	Total	125	3,447

Project No: 2010-3050 Date: April 9, 2011

# **MD BONNYVILLE - IDP INDUSTRIAL**

Source: Intermunicipal Development Plan (Feb 2009)

### Location:

- Either side of Highway 55, west of Cold Lake

## Future Land Uses:

- Industrial

			Developed Area
Land Use	Developable Area (m²)	Developable Area (Hec)	by 2030 (Hec)
Industrial Development - 20% Developed by 2030	3,919,353.21	392	78

### Assumed:

- 60% max site coverage (as per City of Cold Lake Bylaw)

Project No: 2010-3050 Date: April 9, 2011

## MD BONNYVILLE - IDP COMMERCIAL

Source: Intermunicipal Development Plan (Feb 2009)

### Location:

- Along west side of Highway 28, from Energy Centre to 55 Avenue

### **Future Land Uses:**

- Commercial

Land Use	Developable Area (m²)	Developable Area (Hec)	Developed Area by 2030 (Hec)
Commercial Development - 30% developed by 2030	1,574,100.10	157	47

### Assumed:

- Arterial Commercial: 80% site coverage (as per City of Cold Lake Bylaw)

# **TECHNICAL MEMORANDUM**



# **Appendix B - Traffic Demand Forecast Work Plan**





**Date:** February 25, 2011 **File:** 20103050.00.01.10

To: Bob Kitchen

From: Rohit Vij

**Project:** City of Cold Lake Transportation Study

Subject: Traffic Demand Forecast Work Plan

**MEMO** 

### 1 OBJECTIVE

The objective of this work plan is to develop a methodology by which to forecast future traffic demand within Cold Lake, using the ASP, ARP and Outline Plans. **Highlighted text illustrates our assumptions for Cold Lake Transportation Study.** Please review the assumptions and provide your consensus.

### 2 BACKGROUND INFORMATION

Associated Engineering (AE) has obtained the following information from the City of Cold Lake.

### 2.1 AREA STRUCTURE PLANS (ASP)

- Fischer Estates
- Horseshoe Bay
- Iron Horse
- Cold Lake Central
- Southeast
- Forest Heights
- North Shore
- Lot 2, Plan 982 1024
- Uplands

### 2.2 AREA REDEVELOPMENT PLAN (ARP)

Cold Lake Downtown (Cold Lake North)

### 2.3 OUTLINE PLANS

- Lakewood Estates
- Creekside Estates
- Parkview Estates

**Figure 1** presents the Outline Plan for Lakewood Estates. The Outline Plan presents the breakdown of the subdivision to parcels and indicates the phasing anticipated; however, the land use is not indicated. Based on the layout, **it will be assumed that subdivision will be solely low-density residential**.



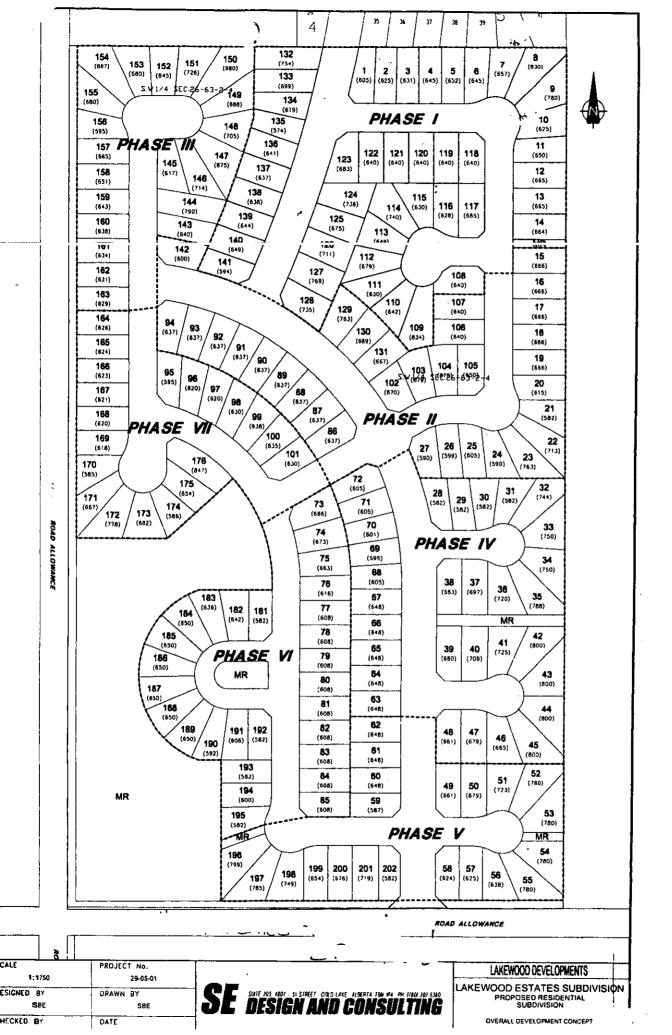
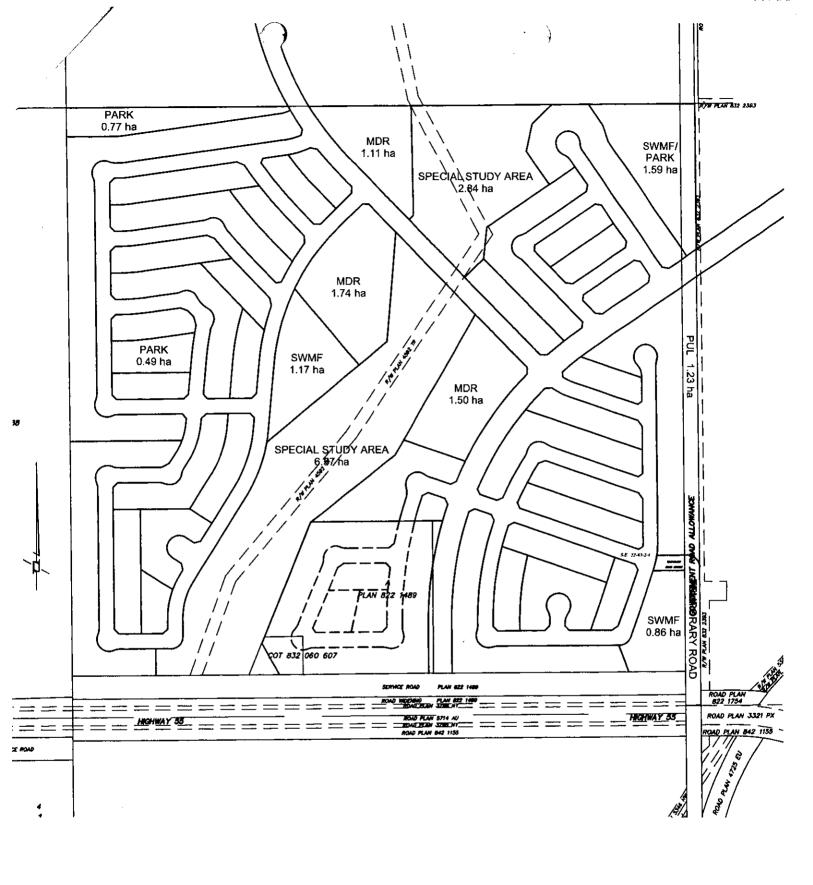


Figure 1

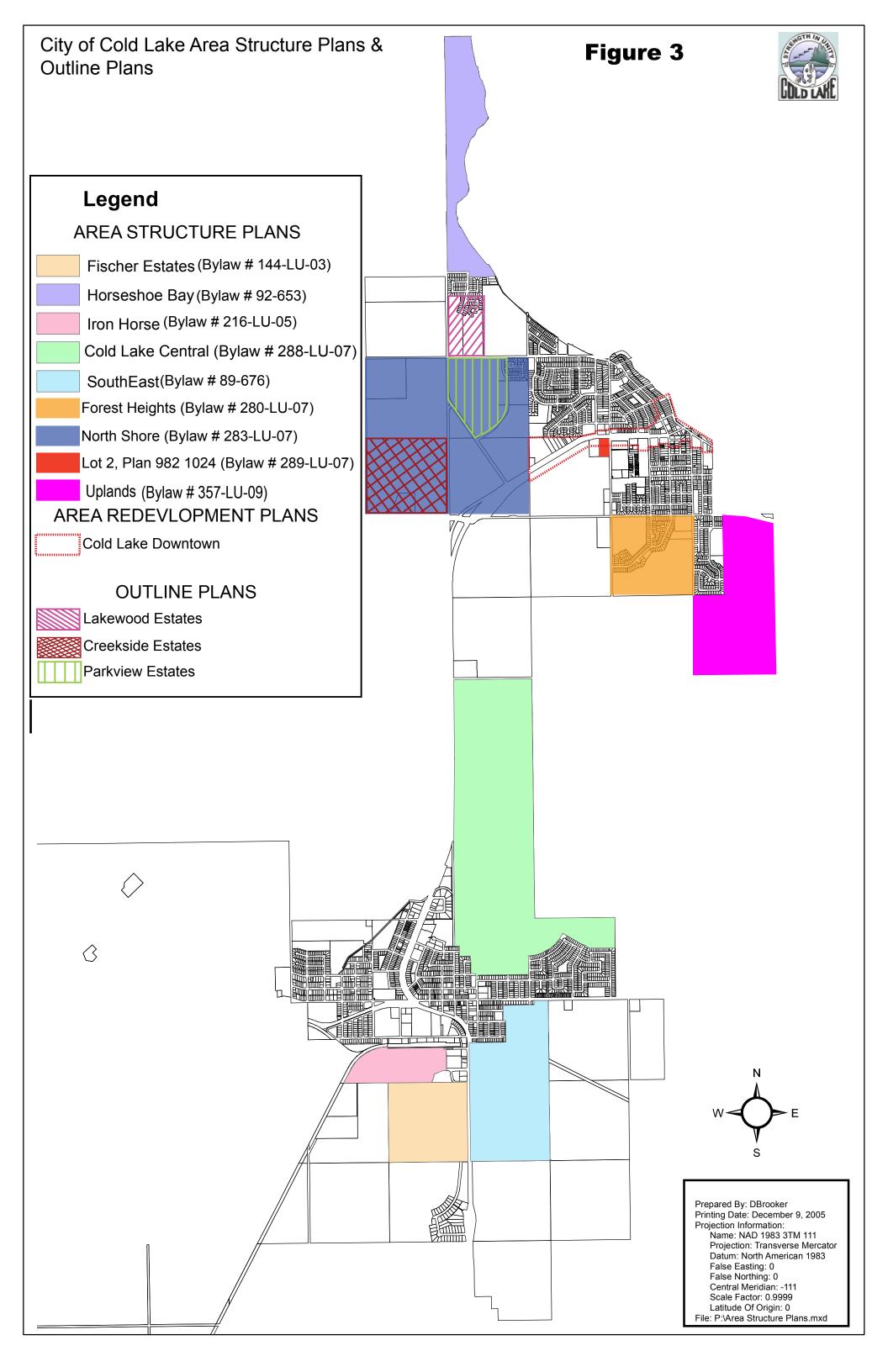


CREEKSIDE

OVERALL DESIGN CONCEPT

Figure 2







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**Figure 2** presents the Outline Plan for Creekside Estates. The Outline Plan presents the breakdown of the subdivision into parcels and indicates the land use. **For parcels where the land use is not indicated, low-density residential will be assumed.** 

The available ASP, ARP and Outline Plans are shown in Figure 3.

### 2.4 CITY OF COLD LAKE LAND USE BY-LAW

The different land use districts within the City of Cold Lake was presented and described in the Land Use By-law. The following table summarizes the information of interest for the purpose of the Traffic Demand Forecast.

Table 2.1 - Information from City of Cold Lake Land Use Bylaw

Land Use District	Maximum Lot Coverage	Minimum Floor Area (m²)	Maximum Floor Area	Maximum Density
RE – Residential Estates District	35%	108.0	-	1 unit/lot
R1A – Residential District (Single Detached)	45%	84.0	-	1 unit/lot
R1B – Residential District (Single Detached – Small Lots)	45%	72.0	-	1 unit/lot
R1B-1 – Residential District (Single Detached – Small Lots)	45%	72.0	-	1 unit/lot
R2 – Residential District (Semi- Detached/Duplex)	45%	72.0	-	2 units/lot
R3 – Medium Density Residential (Row Housing)	50%	63.0	-	42 units/ha
R4 – High Density Residential	-	-	1.3	95 units/ha
RMX – Residential Mixed Use	-	At discretion of Development Authority	-	-





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Land Use District	Maximum Lot Coverage	Minimum Floor Area (m²)	Maximum Floor Area	Maximum Density
RMHC – Residential Manufactured Home Community District	40%	a) single wide – 65.0 b) double wide – 85.0	-	16 units/ha
RMHS – Residential Manufactured Home Subdivision	40%	49.5	-	-
C1 – Downtown Commercial (Central Business District)	80%	At discretion of Development Authority	-	-
C2 – Arterial Commercial (Along Major Arterial Roads, Highway 28)	80%	At discretion of Development Authority	-	-
C3 – Neighbourhood Commercial	50%	Permitted Use – 250.0 Discretionary Use – 1000.0	-	-
LC – Lakeshore Commercial	80%	Commercial – Min. 30% of all floors, 50% of ground floor Residential – Max. 70% of all, 50% of ground floor	-	-
BD – Beach District	At discretion of Development Authority			
LI – Light Industrial	60%	-	-	-
HI – Heavy Industrial	60%	-	-	-





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Land Use District	Maximum Lot Coverage	Minimum Floor Area (m²)	Maximum Floor Area Ratio	Maximum Density	
PS – Public Service (Educational, government, health care and recreational services)	At discretion of Development Authority				
IP – Imperial Park District		At discretion of Development Authority			
UR – Urban Reserve	At discretion of Development Authority				
CON - Conservation	At discretion of Development Authority				
DC – Direct Control District	-	-	-	-	
DC-SR – Spinnaker Ridge Direct Control District	-	-	-	45 units/ha 8 units/row house	
DC-TCE – Tri City Estates Direct Control District	40% 63.0 - 40 units/h				
DC-RMHC – Residential Manufactured Home Community Direct Control District	45%	49.5	-	25.2 units/ha or 19.76 per gross ha.	
FW – National Defense	At discretion of Department of National Defense				

### 2.5 MD OF BONNYVILLE NO. 87 LAND USE BYLAW

The different land use districts within the MD of Bonnyville was presented and described in the Land Use Bylaw. The following table summarizes the information of interest for the purpose of the Traffic Demand Forecast.

Table 2.2 - Information from MD of Bonnyville Land Use Bylaw

Land Use District	Maximum Lot Coverage	Minimum Floor Area (m²)	Maximum Floor Area Ratio	Maximum Density
A – Agricultural	-	-	-	1 unit/lot
CR – Country Residential (Resort)	-	-	-	1 unit/lot





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Land Use District	Maximum Lot Coverage	Minimum Floor Area (m²)	Maximum Floor Area Ratio	Maximum Density
CR1 – Country Residential	-	-	-	1 unit/lot
CR2 – Country Residential (Large Lot)	-	-	-	1 unit/lot
CUD – Controlled Urban Development	-	-	-	-
DC – Direct Control	-	-	-	-
HG – Hamlet General	-	Unserviced – 1860.0 Serviced – 420.0 Sewer only – 930.0 Water only – 1400.0	-	1 unit/lot
HR1 – Hamlet Single Family Residential	-	Unserviced – 1860.0 Serviced – 560.0 Sewer only – 930.0 Water only – 1400.0	-	1 unit/lot
HR2 – Hamlet Multi Family Residential (Duplex)	35%	Interior Site – 697.0 Corner Site – 744.0	-	2 units/lot
HR2 – Hamlet Multi Family Residential (Triplex/Fourplex)	-	297.0 / unit	At discretion of Development Authority	At discretion of Development Authority
HR2 – Hamlet Multi Family Residential (Townhouse)	At discretion of Development Authority	Interior Lot – 185.5 Corner Lot – 297.0	At discretion of Development Authority	30 units/ha
HR2 – Hamlet Multi Family Residential (Apartment)	30%	800.0	0.60	At discretion of Development Authority
HUR – Hamlet Urban Reserve District	-	-	-	-
IR – Intensive Recreation		At discretion of Deve	elopment Authority	





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Land Use District	Maximum Lot Coverage	Minimum Floor Are	Maximum Floor Area Ratio	Maximum Density
MHC – Manufactured Home Community	-	a) single wide – 465.0 b) double wide – 510.0	-	20 units/ha
RC – Rural Commercial		At discretion of [	Development Authority	
RI – Rural Industrial		At discretion of [	Development Authority	

### 3 WORKPLAN

A spreadsheet model will be utilized to forecast the future traffic demand of Cold Lake in the 5-year, 10-year, 15-year, and 20-year horizons. The spreadsheet model will comprise of 9 steps, which are explained in detail below.

### 3.1 STEP 1: DEVELOP NETWORK

- Draw road network. The road network for the City of Cold Lake will consist of collector and arterial roads within the existing City limits.
- Divide study areas into traffic analysis zones (TAZ). The City will be divided into different TAZs that are homogenous in terms of land use. **AE anticipates that nine TAZs will be established for Cold Lake to represent the following:** 
  - TAZ 1: Cold Lake North Commercial
  - TAZ 2: Cold Lake North Residential North/West
  - TAZ 3: Cold Lake North Residential South/East
  - TAZ 4: Central Corridor Commercial (between Cold Lake North and Cold Lake South, including 75 Avenue and 61/62 Avenue)
  - TAZ 5: Central Corridor Residential (between Cold Lake North and Cold Lake South, including Energy Centre Access and 61/62 Avenue)
  - TAZ 6: Cold Lake South Commercial
  - TAZ 7: Cold Lake South Residential West
  - TAZ 8: Cold Lake South Residential East
  - TAZ 9: Medley
- Identify intersections with counts within each zone.
- Determine the centroid for each zone. The centroid may be chosen to be the geographic center or the "center" of the road network.



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- Calculate the area of each zone (A<sub>1</sub>, A<sub>2</sub>, etc.)
- Calculate the distance between the centroid of each zone and the furthest point of the zone (d<sub>ii</sub>) and calculate the distance between each centroid (d<sub>ij</sub>). Where i denotes the study zone and j denotes the destination zone.

### 3.2 STEP 2 – EXISTING VOLUME

• Build a spreadsheet in Excel to summarize the existing traffic volumes. The table would be similar to the following table.

### **Existing Traffic Volumes (2010 Horizon)**

Intersection		Zone 1		Zone 2		Zone		те 9	
		I <sub>1,1</sub>	I <sub>1,2</sub>	I <sub>1,x</sub>	l <sub>2,1</sub>	l <sub>2,2</sub>	l <sub>i,x</sub>	<b>I</b> <sub>9,1</sub>	I <sub>9,x</sub>
	Left	5	9	15					
NB	Through	85	211	150					
	Right	3	15	7					
SB	Left								
	Through								
	Right								
	Left								
EB	Through								
	Right								
	Left								
WB	Through								
	Right								

Where  $I_{i,x}$  denotes intersection number x in Zone i.





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### 3.3 STEP 3 – FUTURE BACKGROUND VOLUMES

- Grow the existing traffic volumes using an annual growth factor to the 5-year, 10-year, 15-year, and 20-year horizons.
- An annual growth rate of 2.0% has been chosen to since it was used in the Municipal Development Plan and represents the median between the moderate (1.5%) and high (2.5%) projection growth in the Inter-municipal Development Plan.
- Future traffic volume n years = Existing traffic volumes + (Existing traffic volume x n x growth %).

  Therefore for n = 5 years, Future traffic volume = Existing traffic volume + (Existing traffic volume x 5 x 0.02).
- Build spreadsheets similar to Step 2 to summarize the Future Background Traffic.
- Four future background traffic volume spreadsheets will be developed for Cold Lake to represent the future background traffic volumes in the 5-year, 10-year, 15-year, and 20-year horizons.

5-year Background Traffic Volumes (2015)





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Intersection		Zone 1			Zone 2		Zone		ne 9
		I <sub>1,1</sub>	I <sub>1,2</sub>	I <sub>1,x</sub>	<b>I</b> <sub>2,1</sub>	l <sub>2,2</sub>	l <sub>i,x</sub>	<b>I</b> <sub>9,1</sub>	l <sub>9,x</sub>
NB	Left	6 = 5+(5x5x 0.02)	10 = 9+(9x5x 0.02)	17 =15+(15 x5x0.02)					
	Through	94	232	165					
	Right	3	17	8					
SB	Left								
	Through								
	Right								
	Left								
EB	Through								
	Right								
	Left								
WB	Through								
	Right								

### 3.4 STEP 4 – FUTURE PRODUCTION

- Calculate the trip production for each horizon using the information provided by the City in the ASPs, ARPs and Outline Plans.
- The City does not have projected growth information available for the four horizons. The City of Cold Lake
  is subject to boom/bust cycles of population growth or contraction tied to the resource section. This
  makes growth forecasting difficult.
- For the transportation study, growth assumptions are necessary. **Associated Engineering assumed** that the following development staging would be implemented for each study horizon:





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# **Assumed Development Staging by Study Horizon**

Development / Redevelopment	Land Use	5-Year (2015) Horizon	10-Year (2020) Horizon	15-Year (2025) Horizon	20-Year (2030) Horizon	Total % Developed
Final on Fototon	Residential	0%	0%	25%	25%	50%
Fischer Estates	Commercial	0%	0%	25%	25%	50%
Iron Horse	Residential	0%	0%	25%	25%	50%
Cold Lake Central	Residential	25%	25%	25%	25%	100%
Cold Lake Certifal	Commercial	50%	50%	0%	0%	100%
Grand Centre	Residential	25%	25%	25%	25%	100%
Southeast	Industrial	25%	25%	25%	25%	100%
Forest Heights	Residential	0%	0%	25%	25%	50%
	Residential	25%	25%	25%	25%	100%
Northshore	Commercial	25%	25%	25%	25%	100%
Nottristiore	Institutional	25%	25%	25%	25%	100%
	School	0%	0%	100%	0%	100%
Lot 2, Plan 982 1024	Commercial	100%	0%	0%	0%	100%
Horseshoe Bay	Residential	50%	50%	0%	0%	100%
Linianda	Residential	25%	25%	25%	25%	100%
Uplands	Health Services & Mixed Use	25%	25%	25%	25%	100%





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Development / Redevelopment	Land Use	5-Year (2015) Horizon	10-Year (2020) Horizon	15-Year (2025) Horizon	20-Year (2030) Horizon	Total % Developed
Lakeshore Area Redevelopment	All	25%	25%	25%	25%	100%
Lakewood Estates	Residential	25%	25%	25%	25%	100%
Creekside Estates	Residential	25%	25%	25%	25%	100%
Parkview Estates	Residential	25%	25%	25%	25%	100%
Parkview Estates	Commercial	25%	25%	25%	25%	100%
Hills of Cold Lake	Residential	25%	25%	25%	25%	100%
Fawn Ridge Estates Development	Residential	25%	25%	25%	25%	100%

- The trips produced by the new developments for each future horizon will be generated using the Institute of Transportation Engineers (ITE) Trip Generation Handbook, 7<sup>th</sup> Edition and summarized in a spreadsheet similar to the one below.
- Four trip production spreadsheets will be produced to represent the trip production in the 5-year, 10-year, 15-year, and 20-year horizons.

**Trip Production (Pi) - Horizon** 

	,
Zone (i)	Trip Produced
1	
2	
3	
4	
5	
6	



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7	
8	
9	
Total (∑Pi)	

### 3.5 STEP 5 – FUTURE ATTRACTION

- Calculate attraction from land use and origin-destination (OD) surveys.
- For the spreadsheet model this is normally not available and this step is bypassed.
- This is the case for the Cold Lake project.

### 3.6 STEP 6 – TRIP TABLE

- Calculate the trips T<sub>ij</sub> between origin zone i and destination zone j, using the gravity model illustrated in the following table.
- The gravity model states that the interaction (trips) between two zones declines with increasing distance between them.

From/To Zone	Weight	%	Final Trip
Zone 1 to Zone 1	$A_1/(d_{11})^2$	$[A_1/(d_{11})^2] / \sum_1$	$P_1 \times [A_1/(d_{11})^2] / \sum_1$
Zone 1 to Zone 2	$A_1/(d_{12})^2$	$[A_1/(d_{12})^2] / \sum_1$	$P_1 \times [A_1/(d_{12})^2] / \sum_1$
Zone 1 to Zone 3	A <sub>1</sub> /(d <sub>13</sub> ) <sup>2</sup>	$[A_1/(d_{13})^2] / \sum_1$	$P_1 \times [A_1/(d_{13})^2] / \sum_1$
Zone 1 to Zone 4	A <sub>1</sub> /(d <sub>14</sub> ) <sup>2</sup>	$[A_1/(d_{14})^2] / \sum_1$	$P_1 \times [A_1/(d_{14})^2] / \sum_1$
Zone 1 to Zone 5	A <sub>1</sub> /(d <sub>15</sub> ) <sup>2</sup>	$[A_1/(d_{15})^2] / \sum_1$	$P_1 \times [A_1/(d_{15})^2] / \sum_1$
Zone 1 to Zone 6	A <sub>1</sub> /(d <sub>16</sub> ) <sup>2</sup>	$[A_1/(d_{16})^2] / \sum_1$	$P_1 \times [A_1/(d_{16})^2] / \sum_1$
Zone 1 to Zone 7	A <sub>1</sub> /(d <sub>17</sub> ) <sup>2</sup>	$[A_1/(d_{17})^2] / \sum_1$	$P_1 \times [A_1/(d_{17})^2] / \sum_1$
Zone 1 to Zone 8	A <sub>1</sub> /(d <sub>18</sub> ) <sup>2</sup>	$[A_1/(d_{18})^2] / \sum_1$	$P_1 \times [A_1/(d_{18})^2] / \sum_1$
Zone 1 to Zone 9	A <sub>1</sub> /(d <sub>19</sub> ) <sup>2</sup>	$[A_1/(d_{19})^2] / \sum_1$	$P_1 \times [A_1/(d_{19})^2] / \sum_1$





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Total $\Sigma_1$ 100% $P_1$
-----------------------------

- Repeat above table for all zones (Zone 1 through Zone 9).
- Compile the final trip table, similar to the one shown below, for each time horizon.
- Four trip tables will be produced to represent the trip distribution in the 5-year, 10-year, 15-year and 20-year horizons.

0 D	1	2	3	4	5	6	7	8	9	Total
1	$P_1 x$ $[A_1/(d_{11})^2] / $ $\Sigma_1$	$P_1 x$ $[A_1/(d_{12})^2] / $ $\sum_1$	$P_1 x$ $[A_1/(d_{13})^2] / $ $\sum_1$	$P_1 x$ $[A_1/(d_{14})^2] / $ $\sum_1$	$P_1 x$ $[A_1/(d_{15})^2] / $ $\sum_1$	$P_1 x$ $[A_1/(d_{16})^2] / \sum_1$	$P_1 x$ $[A_1/(d_{17})^2] / \sum_1$	$P_1 x$ $[A_1/(d_{18})^2] / \sum_1$	$P_1 x$ $[A_1/(d_{19})^2] / $ $\Sigma_1$	$\Sigma = P_1$
2										$\Sigma = P_2$
3										$\sum = P_3$
4										$\sum = P_4$
5										$\sum = P_5$
6										$\Sigma = P_6$
7										$\Sigma = P_7$
8										∑ = P <sub>8</sub>
9										$\Sigma = P_9$
Total	$\sum = A_1$	$\Sigma = A_2$	$\Sigma = A_3$	$\Sigma = A_4$	$\Sigma = A_5$	$\Sigma = A_6$	$\Sigma = A_7$	∑ = A <sub>8</sub>	$\Sigma = A_9$	

• If Step 5 had been completed, the sum of the attraction for each zone should equal the sum of production for the same zone.

### 3.7 STEP 7 – ASSIGNMENT

Assign the trips to intersections using the minimum path algorithm for each zone, for each study horizon.

### 3.8 STEP 8 – EXTERNAL TRIP (IF AVAILABLE)

Collect external trips from Cordon points





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- Distribute external-external trips to intersections.
- This step will be bypassed for Cold Lake as there is no information available for external trips

### 3.9 STEP 9 – ADD DEVELOPMENT TRIPS TO BACKGROUND TRIPS

- Add the trip distribution from Step 7 to the future background traffic volumes established in Step 3.
- The volumes provided at each intersection are the total traffic volume anticipated for each of the study horizons and can be used to analysis the future intersection capacity.



# **TECHNICAL MEMORANDUM**



# **Appendix C - Gravity Model (Trip Distribution) Calculations**



Project No: 2010-3050 Date: February 28, 2011

### **Traffic Demand Model: Zones & Intersections**

Zone	Description		Intersection with Counts
20116	Description	Node #	Intersection
		104	1 Avenue & 16 Street
		105	1 Avenue / 2 Avenue & 10 Street
		106	8 Avenue & Lakeshore Drive
1	Cold Lake North - Commercial/Recreational	107	8 Avenue & 10 Street
		108	8 Avenue & 16 Street
		109	Highway 28 & 25 Street
		111	Highway 55/ 16 Avenue & Highway 28
		101	1 Avenue & 28 Street / English Bay Road
2	Cold Lake North - Residential (North of Hwy 28)	102	1 Avenue & 25 Street
2	Cold Lake North - Residential (North of Hwy 28)	103	1 Avenue & Nelson Street
		110	Highway 55 & 28 Street / English Bay Road
3	Cold Lake North Decidential (South of Hun, 29)	112	16 Avenue & 16 Street
3	Cold Lake North - Residential (South of Hwy 28)	113	16 Avenue & 10 Street
		202	Highway 28 / 55 & 75 Avenue
4	Cold Lake Central - Commercial	203	Highway 28 / 55 & 69 Avenue / Museum Road
4	Cold Lake Central - Commercial	204	Highway 28 / 55 & Tri-City Mall Access
		205	Highway 28 / 55 & 62 Avenue / 61 Avenue
5	Cold Lake Central - Residential	201	Highway 28 / 55 & Energy Centre Access
		301	Highway 28 / 55 & 54 Avenue
		302	Highway 28 / 55 & 52 Avenue
		303	Highway 28 / 55 & 50 Avenue
		304	Highway 28 / 55 & 52 Street
		305	Highway 28 / 55 & 51 Street
		306	Highway 28 / 55 & 50 Street
6	Cold Lake South - CBD/Commercial	307	Highway 28 / 55 & 46 Avenue
		308	Highway 28 / 55 & 43 Avenue
		316	50 Avenue & 53 Street
		317	50 Avenue & 52 Street
		318	50 Avenue & 51 Street
		319	50 Avenue & 50 Street
		320	50 Avenue & 49 Street
		309	57 Street & 52 Avenue (North)
		310	57 Street & 52 Avenue (South)
_		311	50 Avenue & 59 Street
7	Cold Lake South - Residential (West of Hwy 28)	312	50 Avenue & 57 Street
		313	Centre Avenue & 59 Street
		314	Centre Avenue & 57 Street
		315	54 Avenue & 51 Street
_	Cold Late Court Posts 11 /F 1 / 11 20	321	50 Avenue & 45 Street
8	Cold Lake South - Residential (East of Hwy 28)	322	50 Avenue & 41 Street
		323	50 Avenue / Twp Rd 630 & Baywood Road / RR 20
		401	Kingsway & Medley Road
		402	Kingsway & Glenwood Drive (West)
		403	Kingsway & Glenwood Drive (East)
9	Medley	404	Kingsway & Timberline Drive
_		405	Kingsway & Queensway
		406	Kingsway & Tennis Court Road
		407	Queensway & Tennis Court Road
		707	Gastiona, a Tollillo Court Road

# City of Cold Lake Transportation Study Project No: 2010-3050

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# **Traffic Demand Model: Zones & Areas**

Zone	Description	Area (m²)	Area (hec)
1	Cold Lake North - Commercial/Recreational	818,544	81.9
2	Cold Lake North - Residential (North of Hwy 28)	5,302,120	530.2
3	Cold Lake North - Residential (South of Hwy 28)	4,499,137	449.9
4	Cold Lake Central - Commercial	622,964	62.3
5	Cold Lake Central - Residential	2,710,956	271.1
6	Cold Lake South - CBD/Commercial	1,171,259	117.1
7	Cold Lake South - Residential (West of Hwy 28)	4,519,673	452.0
8	Cold Lake South - Residential (East of Hwy 28)	5,295,608	529.6
9	Medley	34,603,627	3,460.4
	Total	59,543,887	5,954.4

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**Traffic Demand Model: Distances** 

		Distance from Zone X to Zone Y (m)								
	1	2	3	4	5	6	7	8	9	
1	2,190	1,674	1,348	3,747	4,019	6,077	6,321	6,077	7,799	
2	1,674	3,277	2,896	4,055	4,535	6,504	6,521	6,691	7,336	
3	1,348	2,896	2,729	3,251	3,312	5,299	5,706	5,161	7,650	
4	3,747	4,055	3,251	1,144	649	2,450	2,576	2,688	4,476	
5	4,019	4,535	3,312	649	2,191	2,058	2,395	2,155	4,704	
6	6,077	6,504	5,299	2,450	2,058	1,701	888	747	3,890	
7	6,321	6,521	5,706	2,576	2,395	888	2,813	1,630	3,002	
8	6,077	6,691	5,161	2,688	2,155	747	1,630	3,068	4,632	
9	7,799	7,336	7,650	4,476	4,704	3,890	3,002	4,632	5,380	

Distance from Centroid to furthest point in same zone

### Traffic Demand Model - Distances^2

				Distance fro	om Zone X to	Zone Y (m)			
	1	2	3	4	5	6	7	8	9
1	4,796,930	2,801,310	1,817,598	14,039,176	16,154,592	36,928,963	39,957,131	36,929,607	60,825,891
2	2,801,310	10,741,309	8,388,242	16,441,732	20,568,375	42,307,265	42,528,271	44,762,992	53,815,958
3	1,817,598	8,388,242	7,449,558	10,572,188	10,970,754	28,074,900	32,557,480	26,631,180	58,527,941
4	14,039,176	16,441,732	10,572,188	1,309,087	421,173	6,002,192	6,637,264	7,225,079	20,033,666
5	16,154,592	20,568,375	10,970,754	421,173	4,801,520	4,235,053	5,734,169	4,645,231	22,125,180
6	36,928,963	42,307,265	28,074,900	6,002,192	4,235,053	2,893,811	789,007	557,471	15,134,766
7	39,957,131	42,528,271	32,557,480	6,637,264	5,734,169	789,007	7,911,770	2,658,492	9,014,252
8	36,929,607	44,762,992	26,631,180	7,225,079	4,645,231	557,471	2,658,492	9,410,559	21,456,287
9	60,825,891	53,815,958	58,527,941	20,033,666	22,125,180	15,134,766	9,014,252	21,456,287	28,949,736

Distance from Centroid to furthest point in same zone

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### Traffic Demand Model: Future Production, 5 Year (2015)

Zone	Total Trips	In Trips	Out Trips
1	347	85	262
2	755	400	355
3	277	172	105
4	974	224	750
5	412	259	154
6	84	18	66
7	0	0	0
8	90	56	33
9	0	0	0
Outside of City	86	54	32
Total	3,025	1,268	1,757

### Traffic Demand Model: Future Production, 10 Year (2020)

Zone	Total Trips	In Trips	Out Trips
1	667	163	504
2	1,510	800	710
3	554	344	210
4	1,949	448	1,500
5	825	518	307
6	167	35	132
7	0	0	0
8	180	113	67
9	0	0	0
Outside of City	172	108	63
Total	6,023	2,530	3,493

### Traffic Demand Model: Future Production, 15 Year (2025)

Zone	Total Trips	In Trips	Out Trips
1	987	241	745
2	2,777	1,426	1,351
3	944	589	355
4	1,949	448	1,500
5	1,237	777	461
6	359	78	281
7	223	142	81
8	270	169	100
9	0	0	0
Outside of City	257	163	95
Total	9,003	4,033	4,970

# Traffic Demand Model: Future Production, 20 Year (2030)

Zone	Total Trips	In Trips	Out Trips		
1	1,307	320	987		
2	3,507	1,811	1,696		
3	1,335	834	501		
4	1,949	448	1,500		
5	1,650	1,036	614		
6	551	120	431		
7	447	284	162		
8	360	226	134		
9	0	0	0		
Outside of City	343	217	126		
Total	11,447	5,295	6,152		

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# **Traffic Demand Model, 5 Year Horizon: Future Production**

Weight based on Gravity Model. Weight of Zone X to Zone Y = (Area of Zone Y) / (Distance between Zone X & Y  $^2$ )

From Zone	To Zone	Weight <sup>1</sup>	%	Final Trip
	1	0.17	3.0%	11
	2	1.89	33.8%	117
	3	2.48	44.1%	153
	4	0.04	0.8%	3
_	5	0.17	3.0%	10
1	6	0.03	0.6%	2
	7	0.11	2.0%	7
	8	0.14	2.6%	9
	9	0.57	10.1%	35
	SUM	5.61	100.0%	347
	1	0.29	12.2%	92
	2	0.49	20.7%	156
	3	0.54	22.5%	170
	4	0.04	1.6%	12
}	5	0.13	5.5%	42
2	6	0.03	1.2%	9
<u> </u>	7	0.03	4.5%	34
<u> </u>	8	0.12	5.0%	37
}	9	0.64	26.9%	203
	SUM	2.39	100.0%	<b>755</b>
	1	0.45	15.2%	42
	2	0.63	21.3%	59
	3	0.60	20.4%	56
	4	0.06	2.0%	6
	5	0.00	8.3%	23
3	6	0.23	1.4%	4
-	7	0.14	4.7%	13
	8	0.14	6.7%	19
	9	0.59	20.0%	55
-	SUM	2.96	100.0%	277
	1	0.06	0.5%	5
	2	0.32	2.9%	28
	3	0.43	3.8%	38
	4	0.48	4.3%	42
4	5	6.44	58.2%	567
	<u>6</u> 7	0.20	1.8%	17
	8	0.68	6.2% 6.6%	60 65
-		0.73		
-	9	1.73	15.6%	152
	SUM	11.06	100.0%	974
-	<u> </u>	0.05	0.8%	3
-		0.26	3.9%	16
-	3 4	0.41	6.3%	26
-		1.48	22.6%	93
5	5	0.56	8.6%	36
<u> </u>	6	0.28	4.2%	17
<u> </u>	7	0.79	12.1%	50
<u> </u>	8	1.14	17.5%	72
<u> </u>	9	1.56	23.9%	99
	SUM	6.53	100.0%	412

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**Traffic Demand Model, 5 Year Horizon: Future Production**Weight based on Gravity Model. Weight of Zone X to Zone Y = (Area of Zone Y) / (Distance between Zone X & Y ^ 2)

From Zone	To Zone	Weight <sup>1</sup>	%	Final Trip
	1	0.02	0.1%	0
	2	0.13	0.7%	1
	3	0.16	0.8%	1
	4	0.10	0.5%	0
6	5	0.64	3.4%	3
O	6	0.40	2.1%	2
	7	5.73	30.2%	25
	8	9.50	50.1%	42
	9	2.29	12.1%	10
	SUM	18.97	100.0%	84
	1	0.02	0.2%	0
	2	0.12	1.4%	0
	3	0.14	1.6%	0
	4	0.09	1.1%	0
7	5	0.47	5.4%	0
'	6	1.48	17.0%	0
	7	0.57	6.5%	0
	8	1.99	22.8%	0
	9	3.84	43.9%	0
	SUM	8.74	100.0%	0
	1	0.02	0.3%	0
	2	0.12	1.7%	2
	3	0.17	2.4%	2
	4	0.09	1.2%	1
8	5	0.58	8.4%	8
O	6	2.10	30.2%	27
	7	1.70	24.4%	22
	8	0.56	8.1%	7
	9	1.61	23.2%	21
	SUM	6.96	100.0%	90
	1	0.01	0.6%	0
	2	0.10	4.2%	0
	3	0.08	3.3%	0
	4	0.03	1.3%	0
9	5	0.12	5.2%	0
	6	0.08	3.3%	0
	7	0.50	21.2%	0
	8	0.25	10.4%	0
	9	1.20	50.6%	0
	SUM	2.36	100.0%	0

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# **Traffic Demand Model, 5 Year Horizon: Future Production**

					Trips					
From	1	2	3	4	5	6	7	8	9	SUM
1	11	117	153	3	10	2	7	9	35	347
2	92	156	170	12	42	9	34	37	203	755
3	42	59	56	6	23	4	13	19	55	277
4	5	28	38	42	567	17	60	65	152	974
5	3	16	26	93	36	17	50	72	99	412
6	0	1	1	0	3	2	25	42	10	84
7	0	0	0	0	0	0	0	0	0	0
8	0	2	2	1	8	27	22	7	21	90
9	0	0	0	0	0	0	0	0	0	0
SUM	154	379	445	157	688	78	211	251	576	

Note: Sum of column for Zone 1 must match sum of row for Zone 1 IF both production and attraction information available. No attraction information.

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## TRIP DISTRIBUTION FROM GRAVITY MODEL

					Trips					
From	1	2	3	4	5	6	7	8	9	SUM
1	3%	34%	44%	1%	3%	1%	2%	3%	10%	100%
2	12%	21%	22%	2%	6%	1%	4%	5%	27%	100%
3	15%	21%	20%	2%	8%	1%	5%	7%	20%	100%
4	1%	3%	4%	4%	58%	2%	6%	7%	16%	100%
5	1%	4%	6%	23%	9%	4%	12%	17%	24%	100%
6	0%	1%	1%	1%	3%	2%	30%	50%	12%	100%
7	0%	1%	2%	1%	5%	17%	7%	23%	44%	100%
8	0%	2%	2%	1%	8%	30%	24%	8%	23%	100%
9	1%	4%	3%	1%	5%	3%	21%	10%	51%	100%

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# **ADJUSTED TRIP DISTRIBUTION**

					Trips					
From	1	2	3	4	5	6	7	8	9	SUM
1	5%	23%	23%	5%	8%	5%	10%	8%	13%	100%
2	20%	15%	10%	15%	10%	8%	7%	5%	10%	100%
3	20%	10%	15%	15%	10%	8%	7%	5%	10%	100%
4	6%	15%	15%	5%	15%	8%	12%	12%	12%	100%
5	16%	12%	12%	15%	15%	8%	7%	5%	10%	100%
6	5%	5%	5%	5%	15%	5%	25%	25%	10%	100%
7	10%	5%	5%	15%	10%	18%	12%	15%	10%	100%
8	10%	5%	5%	15%	10%	20%	10%	15%	10%	100%
9	8%	10%	10%	10%	7%	20%	10%	10%	15%	100%
SUM	100%	100%	100%	100%	100%	100%	100%	100%	100%	

# **TECHNICAL MEMORANDUM**



# **Appendix D - Capacity Analysis**



### CAPACITY ANALYSIS - EXISTING (2010) HORIZON

	Inters	ection		Forecaste	ed Volumes	Band Classification	David Classification	Lane Capacity	Lane Capacity	
Corridor	From	То	Direction	Daily Traffic - Directional	Daily Traffic - Two Way	Road Classification 2000 TPS <sup>1</sup>	Road Classification City of Cold Lake <sup>2</sup>	for Road Classification (veh/hour/lane) 3	for Road Classification (veh/day/lane) 4	Number of Lanes Required (One Direction)
1 Avenue	28 Street	25 Street	Eastbound	960	1,810	Collector	Local (Residential or Industrial)	100	1,000	1
1 Avenue	25 Street	Nelson Street	Westbound Eastbound Westbound	850 1,640 1,470	3,110	Collector	Collector (Residential or Industrial)	400	4,000	1 1
1 Avenue	Nelson Street	16 Street	Eastbound Westbound	1,470 1,290 1,360	2,650	Collector	Local (Residential or Industrial)	100	1,000	2 2
Hwy 28	Hwy 55/16 Avenue	25 Street	Northbound	5,410	9,650	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2 2
8 Avenue	25 Street	16 Street	Southbound Eastbound	4,240 5,260	7,860	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
8 Avenue	16 Street	10 Street	Westbound Eastbound	2,600 1,880	2,980	2-Lane Arterial	Local (Residential or Industrial)	100	1,000	1 2
8 Avenue	10 Street	Lakeshore Drive	Westbound Eastbound	1,100 950	1,720	2-Lane Arterial	Local (Residential or Industrial)	100	1,000	2 1
Hwy 55	West City Limit	28 Street	Westbound Eastbound	770 3,360	4,930	Collector	Collector (Residential or Industrial)	400	4,000	1
Hwy 55	28 Street	Hwy 28	Westbound Eastbound	1,570 3,250	4,980	Collector	Collector (Residential or Industrial)	400	4,000	1 1
16 Avenue	Hwy 28	16 Street	Westbound Eastbound	1,730 1,820	3,210	Collector	Collector (Residential or Industrial)	400	4,000	1
16 Avenue	16 Street	10 Street	Westbound Eastbound	1,390 1,270	2,050	Collector	Local (Residential or Industrial)	100	1,000	1 2
16 Avenue	10 Street	8 Street	Westbound Eastbound	780 1,320	2,430	Collector		100	1,000	1 2
			Westbound Northbound	1,110 1,450			Local (Residential or Industrial)			2 2
English Bay Road	North City Limit	1 Avenue	Southbound Northbound	1,090 1,150	2,540	Collector	Local (Residential or Industrial)	100	1,000	2 2
English Bay Road	1 Avenue	25 Street	Southbound Northbound	780 1,490	1,930	Collector	Local (Residential or Industrial)	100	1,000	1 1
English Bay Road	25 Street	Hwy 28	Southbound Northbound	1,610 660	3,100	Collector	Collector (Residential or Industrial)	400	4,000	1
28 Street	English Bay Road	Hwy 55	Southbound Northbound	390 840	1,050	Collector	Local (Residential or Industrial)	100	1,000	1
25 Street	1 Avenue	English Bay Road	Southbound Eastbound	770 570	1,610	Collector	Local (Residential or Industrial)	100	1,000	1
Nelson Street	1 Avenue	16 Street	Westbound Northbound	330 2,070	900	Collector	Local (Residential or Industrial)	100	1,000	1 3
16 Street	1 Avenue	8 Avenue	Southbound Northbound	770 420	2,840	Collector	Local (Residential or Industrial)	100	1,000	1
16 Street	8 Avenue	16 Avenue	Southbound	450 780	870	Collector	Local (Residential or Industrial)	100	1,000	1
10 Street	1 Avenue	8 Avenue	Northbound Southbound	830 900	1,610	Collector	Local (Residential or Industrial)	100	1,000	1
10 Street	8 Avenue	16 Avenue	Northbound Southbound	910 910 6.810	1,810	Collector	Local (Residential or Industrial)	100	1,000	1
Hwy 28/55	Hwy 55/16 Avenue	75 Avenue	Northbound Southbound	6,170	12,980	2-Lane Arterial	Undivided Arterial	800	8,000	1
Hwy 28/55	75 Avenue	69 Avenue	Northbound Southbound	7,480 7,680	15,160	2-Lane Arterial	Undivided Arterial	800	8,000	1
Hwy 28/55	69 Avenue	54 Avenue	Northbound Southbound	7,500 7,150	14,650	2-Lane Arterial	Undivided Arterial	800	8,000	1
54 Avenue	56 Street	Hwy 28/55	Westbound	1,330 1,530	2,860	Collector	Local (Residential or Industrial)	100	1,000	2 2
54 Avenue	Hwy 28/55	51 Street	Westbound Westbound	2,320 2,280	4,600	Collector	Collector (Residential or Industrial)	400	4,000	1
54 Avenue	51 Street	45 Street	Westbound Westbound	1,440 1,170	2,610	Collector	Local (Residential or Industrial)	100	1,000	2 2
52 Avenue	57 Street	Hwy 28/55	Westbound Westbound	810 1,180	1,990	Collector	Local (Residential or Industrial)	100	1,000	1 2
50 Avenue	62 Street	59 Street	Westbound Westbound	220 370	590	Collector	Local (Residential or Industrial)	100	1,000	1 1
50 Avenue	59 Street	57 Street	Westbound Westbound	420 530	950	Collector	Local (Residential or Industrial)	100	1,000	1
50 Avenue	57 Street	55 Street	Westbound Westbound	180 340	520	Collector	Local (Residential or Industrial)	100	1,000	1
Centre Avenue	59 Street	57 Street	Westbound Westbound	8,340 3,700	12,040	2-Lane Arterial	Undivided Arterial	800	8,000	2 1
Centre Avenue	57 Street	Hwy 28/55	Eastbound Westbound	8,500 4,300	12,800	4-Lane Arterial	Undivided Arterial	800	8,000	2 1
50 Avenue	Hwy 28/55	51 Street	Eastbound Westbound	3,280 2,610	5,890	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue	51 Street	50 Street	Eastbound Westbound	3,080 1,970	5,050	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue	50 Street	45 Street	Eastbound Westbound	3,210 1,890	5,100	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue	45 Street	41 Street	Eastbound Westbound	2,620 1,510	4,130	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue	41 Street	Future Arterial	Eastbound Westbound	1,820 1,140	2,960	2-Lane Arterial	Local (Residential or Industrial)	100	1,000	2 2
50 Avenue	Future Arterial	Baywood Road	Eastbound Westbound	1,510 960	2,470	2-Lane Arterial	Local (Residential or Industrial)	100	1,000	2
59 Street	50 Avenue	Centre Avenue	Northbound Southbound	630 280	910	Collector	Local (Residential or Industrial)	100	1,000	1
57 Street	52 Avenue	50 Avenue	Northbound Southbound	770 470	1,240	Collector	Local (Residential or Industrial)	100	1,000	1
57 Street	50 Avenue	Centre Avenue	Northbound Southbound	820 520	1,340	Collector	Local (Residential or Industrial)	100	1,000	1
Hwy 28/55	54 Avenue	52 Avenue	Northbound	6,970	13,840	4-Lane Arterial	Undivided Arterial	800	8,000	1 1
Hwy 28/55	52 Avenue	50 Avenue	Northbound Southbound	6,870 7,640	13,340	4-Lane Arterial	Undivided Arterial	800	8,000	1 1
Hwy 28/55	50 Avenue	50 Street	Northbound Southbound	5,700 5,280	10,730	4-Lane Arterial	Undivided Arterial	800	8,000	1
Hwy 28/55	50 Street	43 Avenue	Northbound Southbound	5,450 5,930	12,600	4-Lane Arterial	Undivided Arterial	800	8,000	1
Hwy 28/55	43 Avenue	South City Limit	Northbound	6,670 5,500	11,730	2-Lane Arterial	Undivided Arterial	800	8,000	1
51 Street	54 Avenue	50 Avenue	Southbound Northbound	6,230 870	1,770	Collector	Local (Residential or Industrial)	100	1,000	1
50 Street	50 Avenue	Hwy 28/55	Southbound Northbound	900 2,980	5,880	Collector	Collector (Residential or Industrial)	400	4,000	1
45 Street	54 Avenue	50 Avenue	Southbound Northbound	2,900 410	690	Collector	Local (Residential or Industrial)	100	1,000	1
45 Street 41 Street	54 Avenue	50 Avenue	Southbound Northbound	280 800	1,320	Collector	Local (Residential or Industrial)  Local (Residential or Industrial)	100	1,000	1
41 Street	54 Avenue	50 Avenue	Southbound	520	1,320	Collector	Local (Residential or Industrial)	100	1,000	1

City of Cold Lake - Transportation Study Project No: 2010-3050 Date: April 11, 2011

### CAPACITY ANALYSIS - EXISTING (2010) HORIZON

Corridor	Interse	ection	Direction			Road Classification	Road Classification	Lane Capacity for Road Classification	Lane Capacity for Road Classification	Number of Lanes
Corridor	From To		Direction	Daily Traffic - Directional	Daily Traffic - Two Way	2000 TPS <sup>1</sup>	City of Cold Lake <sup>2</sup>	(veh/hour/lane) 3	(veh/day/lane) 4	Required (One Direction)
Kingsway	59 Street	Glenwood	Eastbound	7,290	10.510	2-Lane Arterial	Undivided Arterial	800	8.000	1
Kingsway	39 311661	Gleriwood	Westbound	3,220	10,510	Z*Lane Antenai	Oridivided Arterial	800	8,000	1
Kingsway	Timberline	Glenwood	Eastbound	6,200	8.870	Collector	Collector (Residential or Industrial)	400	4,000	2
Kiligsway	Timberine	Gieriwood	Westbound	2,670	0,070	Collector	Collector (Resideritial or Industrial)	400	4,000	1
Kingsway	Queensway	Timberline	Eastbound	2,840	4.480	Collector	Collector (Residential or Industrial)	400	4,000	1
Kiligsway	Queerisway	Tittibetilite	Westbound	1,640	4,400	Collector	Collector (Resideritial or Industrial)	400	4,000	1
Kingsway	Tennis Court Road	Queensway	Eastbound	540	1.170	Collector	Local (Residential or Industrial)	100	1,000	1
Kingsway	gsway Terriis Court Road Queerisway	Queensway	Westbound	630	1,170	Collector	Local (Residential of Industrial)	100	1,000	1
Vingowov	End of Road	Tennis Court Road	Road Eastbound	740	1,520	Collector	Local (Residential or Industrial)	100	1,000	1
Kiligsway	Kingsway End of Road Tennis	Termis Court Road	Westbound	780	1,320	Collector	Local (Residential of Industrial)	100	1,000	1
Tennis Court Road	Queensway	Kingsway	Northbound	220	270	Collector	Lane	N/A	N/A	N/A
Terms Court Road	Queerisway	KiligSway	Southbound	50	210	Collector	Laire	N/A	IVA	N/A
Queensway	Tennis Court Road	Kingsway	Northbound	1,020	1,790	Collector	Local (Residential or Industrial)	100	1,000	2
Queerisway	Termis Court Road	KiligSway	Southbound	770	1,750	Collector	Local (Residential of Industrial)		1,000	1
Queensway	Kingsway	Hanger Ln	Northbound	1,870	2,200	Collector	Local (Residential or Industrial)	100	1,000	2
Queensway	rungsway	rianger En	Southbound	330	2,200	Collector	Local (residential of Industrial)	100	1,000	1
Timberline	Juniper Avenue	Kingsway	Northbound	740	1,340	Collector	Local (Residential or Industrial)	100	1,000	1
THIDDINIO	Julipoi Averiue	rungsway	Southbound	600	1,540	Collector	Local (residential of Industrial)	100	1,000	1
Timberline	Kingsway	Athabasca Road	Northbound	1,720	2,520	Collector	Local (Residential or Industrial)	100	1.000	2
rimberline	niiysway	Alliabasca Road	Southbound	800	2,320	Conector	Local (Residential or Industrial)	100	1,000	1
Glenwood Drive	Glenwood	Kingsway	Northbound	2,600	3,920	Collector	Collector (Residential or Industrial)	) 400	4,000	1
Bend election be		• •	Southbound	1,320	3,920	Collector	Collector (Nesidefillal of Industrial)	400	4,000	1

<sup>1.</sup>Road classification based on 2000 Transportation Study
2. Road classification based on 2000 Transportation Study
2. Road classification based on Daily Service Volumes stipulated in City's Roadway Design Standards (Municipal Engineering Servicing Standards and Standard Construction Specifications, Jan 2008)
3. Based on Lane Capacity Table (lattached). Using road classification according to City's standards.
4. Based on assumption that PM peak hour traffic is 10% of the daily traffic

### CAPACITY ANALYSIS - 5 YEAR (2015) HORIZON

Corridor	Interse	ection	Direction	Forecasted		Road Classification	Road Classification	Lane Capacity for Road Classification	Lane Capacity for Road Classification	Number of Lanes
	From	То		Daily Traffic - Directional	Daily Traffic - Two Way	2000 TPS 1	City of Cold Lake <sup>2</sup>	(veh/hour/lane) 3	(veh/day/lane) 4	Required (One Direction)
1 Avenue	28 Street	25 Street	Westbound Westbound	1,600 1,200	2,800	Collector	Local (Residential or Industrial)	100	1,000	2 2
1 Avenue	25 Street	Nelson Street	Eastbound Westbound	2,490 2,140	4,630	Collector	Collector (Residential or Industrial)	400	4,000	1
1 Avenue	Nelson Street	16 Street	Eastbound Westbound	2,680 2,820	5,500	Collector	Collector (Residential or Industrial)	400	4,000	1
Hwy 28	Hwy 55/16 Avenue	25 Street	Northbound Southbound	9,600 8,240	17,840	4-Lane Arterial	Undivided Arterial	800	8,000	2
8 Avenue	25 Street	16 Street	Eastbound Westbound	6,870 4,020	10,890	4-Lane Arterial	Undivided Arterial	800	8,000	1
8 Avenue	16 Street	10 Street	Eastbound	2,470	4,210	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
8 Avenue	10 Street	Lakeshore Drive	Westbound Eastbound	1,740 1,300	2,610	4-Lane Arterial	Local (Residential or Industrial)	100	1,000	2
Hwy 55	West City Limit	28 Street	Westbound Eastbound	1,310 3,700	5,430	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2 1
Hwy 55	28 Street	Hwy 28	Westbound Eastbound	1,730 5,130	9,040	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2
16 Avenue	Hwy 28	16 Street	Westbound Eastbound	3,910 3,720	6,490	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
	-		Westbound Eastbound	2,770 2,490						1
16 Avenue	16 Street	10 Street	Westbound Eastbound	1,520 2,450	4,010	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
16 Avenue	10 Street	8 Street	Westbound Northbound	1,790 4,740	4,240	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2
English Bay Road	North City Limit	1 Avenue	Southbound Northbound	3,860 5,580	8,600	Collector	Collector (Residential or Industrial)	400	4,000	1 2
English Bay Road	1 Avenue	25 Street	Southbound	4,130 4,730	9,710	Collector	Collector (Residential or Industrial)	400	4,000	2 2
English Bay Road	25 Street	Hwy 28	Southbound	4,220	8,950	Collector	Collector (Residential or Industrial)	400	4,000	2
28 Street	English Bay Road	Hwy 55	Northbound Southbound	2,650 1,810	4,460	Collector	Collector (Residential or Industrial)	400	4,000	1
25 Street	1 Avenue	English Bay Road	Northbound Southbound	1,620 1,400	3,020	Collector	Collector (Residential or Industrial)	400	4,000	1
Nelson Street	1 Avenue	16 Street	Eastbound Westbound	1,180 680	1,860	Collector	Local (Residential or Industrial)	100	1,000	2
16 Street	1 Avenue	8 Avenue	Northbound Southbound	4,840 1,760	6,600	Collector	Collector (Residential or Industrial)	400	4,000	2
16 Street	8 Avenue	16 Avenue	Northbound Southbound	970 1,090	2,060	Collector	Local (Residential or Industrial)	100	1,000	1 2
16 Street	16 Avenue	10 Street	Northbound Southbound	1,030 1,030 750	1,780	Collector	Local (Residential or Industrial)	100	1,000	2
10 Street	1 Avenue	8 Avenue	Northbound	1,340	2.610	Collector	Local (Residential or Industrial)	100	1.000	2
10 Street	8 Avenue	16 Avenue	Southbound Northbound	1,270 1,580	3,290	Collector	Collector (Residential or Industrial)	400	4.000	2 1
10 Street	16 Avenue	16 Street	Southbound Northbound	1,710 1,040	2.020	Collector	Local (Residential or Industrial)	100	1.000	1 2
			Southbound Northbound	980 1,040	2,020	Collector	Local (Residential or Industrial)	100	1,000	1 2
6 Street 5	16 Avenue	21 Avenue	Southbound Northbound	980 13,050	,		,		,	1 2
Hwy 28/55	Hwy 55/16 Avenue	75 Avenue	Southbound Northbound	12,020 13,970	25,070	4-Lane Arterial	Divided Arterial	1,000	10,000	2
Hwy 28/55	75 Avenue	69 Avenue	Southbound Northbound	13,860 13,310	27,830	4-Lane Arterial	Divided Arterial	1,000	10,000	2
Hwy 28/55	69 Avenue	54 Avenue	Southbound	15,550	28,860	4-Lane Arterial	Divided Arterial	1,000	10,000	2
75 Avenue	Hwy 28/55	Future Arterial	Westbound Westbound	1,750 3,080	4,830	Collector	Collector (Residential or Industrial)	400	4,000	1
54 Avenue	56 Street	Hwy 28/55	Eastbound Westbound	1,820 2,290	4,110	Collector	Collector (Residential or Industrial)	400	4,000	1
54 Avenue	Hwy 28/55	51 Street	Eastbound Westbound	3,800 3,550	7,350	Collector	Collector (Residential or Industrial)	400	4,000	1
54 Avenue	51 Street	45 Street	Eastbound Westbound	2,990 2,430	5,420	Collector	Collector (Residential or Industrial)	400	4,000	1
52 Avenue	59 Street	57 Street	Eastbound Westbound	980 1,370	2,350	Collector	Local (Residential or Industrial)	100	1,000	1 2
52 Avenue	57 Street	Hwy 28/55	Eastbound Westbound	1,680 2,450	4,130	Collector	Collector (Residential or Industrial)	400	4,000	1
Centre Avenue	59 Street	57 Street	Eastbound	12,290	17,560	2-Lane Arterial	Undivided Arterial	800	8,000	2
Centre Avenue	57 Street	Hwy 28/55	Westbound Eastbound	5,270 11,020	18,420	4-Lane Arterial	Undivided Arterial	800	8,000	2
50 Avenue	Hwy 28/55	51 Street	Westbound Eastbound	7,400 5,080	8,720	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2
50 Avenue	51 Street	50 Street	Westbound Eastbound	3,640 4,080	6,720	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2
			Westbound Eastbound	2,640 4,310			Collector (Residential or Industrial)  Collector (Residential or Industrial)			1 2
50 Avenue	50 Street	45 Street	Westbound Eastbound	2,320 3,660	6,630	2-Lane Arterial	· · · · · · · · · · · · · · · · · · ·	400	4,000	1 1
50 Avenue	45 Street	41 Street	Westbound Eastbound	2,060 2,840	5,720	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue	41 Street	Future Arterial	Westbound	1,780	4,620	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue	Future Arterial	Baywood Road	Westbound	2,360 1,500	3,860	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 1
43 Avenue	Hwy 28/55	45 Street	Westbound	3,410 3,100	6,510	Collector	Collector (Residential or Industrial)	400	4,000	1
59 Street	52 Avenue	Centre Avenue	Northbound Southbound	1,310 580	1,890	Collector	Local (Residential or Industrial)	100	1,000	2 1
57 Street	54 Avenue	52 Avenue	Northbound Southbound	910 600	1,510	Collector	Local (Residential or Industrial)	100	1,000	1
57 Street	52 Avenue	Centre Avenue	Northbound Southbound	1,700 1,080	2,780	Collector	Local (Residential or Industrial)	100	1,000	2 2
Hwy 28/55	54 Avenue	52 Avenue	Northbound Southbound	11,440 12,960	24,400	4-Lane Arterial	Divided Arterial	1,000	10,000	2 2
Hwy 28/55	52 Avenue	50 Avenue	Northbound	11,980 11,460	23,440	4-Lane Arterial	Divided Arterial	1,000	10,000	2 2
Hwy 28/55	50 Avenue	50 Street	Northbound	5,180	11,660	4-Lane Arterial	Undivided Arterial	800	8,000	1
Hwy 28/55	50 Street	43 Avenue	Northbound	6,480 8,070	17,510	4-Lane Arterial	Undivided Arterial	800	8,000	2
Hwy 28/55	43 Avenue	South City Limit	Southbound Northbound	9,440 6,520	13,880	4-Lane Arterial	Undivided Arterial	800	8,000	2 1
		50 Avenue	Southbound Northbound	7,360 1,320						1 2
51 Street	54 Avenue		Southbound Northbound	1,480 4,770	2,800	Collector	Local (Residential or Industrial)	100	1,000	2 2
50 Street	50 Avenue	Hwy 28/55	Southbound Northbound	4,530 520	9,300	Collector	Collector (Residential or Industrial)	400	4,000	2
45 Street	54 Avenue	50 Avenue	Southbound Northbound	340 370	860	Collector	Local (Residential or Industrial)	100	1,000	1
45 Street	50 Avenue	43 Avenue	Southbound	580	950	Collector	Local (Residential or Industrial)	100	1,000	1
41 Street	54 Avenue	50 Avenue	Northbound Southbound	1,660 1,080	2,740	Collector	Local (Residential or Industrial)	100	1,000	2

City of Cold Lake - Transportation Study Project No: 2010-3050 Date: April 11, 2011

### CAPACITY ANALYSIS - 5 YEAR (2015) HORIZON

Corridor	Interse	ection	Direction	Forecasted Volumes		Road Classification	Road Classification	Lane Capacity for Road Classification	Lane Capacity for Road Classification	Number of Lanes
Corridor	From	То		Daily Traffic - Directional	Daily Traffic - Two Way	2000 TPS <sup>1</sup>	City of Cold Lake <sup>2</sup>	(veh/hour/lane) 3	(veh/day/lane) 4	Required (One Direction)
Kingsway	59 Street	Glenwood	Eastbound	9,530	15,530	2-I ane Arterial	Undivided Arterial	800	8.000	2
rungaway	33 011661	Cicilwood	Westbound	6,000	13,330	2-Lane Arterial	Charlaca / Itoria	000	0,000	1
Kingsway	Timberline	Glenwood	Eastbound	7,950	12.240	Collector	Undivided Arterial	800	8,000	1
rungaway	Tillibellille	Gieriwood	Westbound	4,290	12,240	Collector	Ondivided Arterial	000	8,000	1
Kingsway	Queensway	Timberline	Eastbound	3,770	6.360	Collector	Collector (Residential or Industrial)	400	4.000	1
rungaway	Quociisway	TITIDETITIE	Westbound	2,590	0,300	Collector	Collector (residential or industrial)	400	4,000	1
Kingsway	Tennis Court Road	Queensway	Eastbound	770	1.770	Collector	Local (Residential or Industrial)	100	1,000	1
rangonay	TOTALIO COURT TOUG	quoononay	Westbound	1,000	1,110	Colloctor	Lood (Nooldonida or madorida)	100	1,000	1
Kingsway	End of Road	Wes	Eastbound	900	2,020	Collector	Local (Residential or Industrial)	100	1,000	1
			Westbound	1,120				100	1,000	2
Tennis Court Road	Queensway	Kingsway	Northbound	250	310	Collector	Lane	N/A	N/A	N/A
		3,	Southbound	60						N/A
Queensway	Tennis Court Road	Kingsway	Northbound	1,790	3.110	Collector	Collector (Residential or Industrial)	400	4,000	1
		3,	Southbound	1,320	-, -				,	1
Queensway	Kingsway	Hanger Ln	Northbound	2,370	2,860	Collector	Local (Residential or Industrial)	100	1,000	3
	97		Southbound	490	_,				.,,	1
Timberline	Juniper Avenue	Kingsway	Northbound	1,050	1.830	Collector	Local (Residential or Industrial)	100	1.000	2
		3,	Southbound	780	,		(	.00	,	1
Timberline	Kingsway	Athabasca Road	Northbound	2,270	3,470	Collector	Collector (Residential or Industrial)	400	4.000	1
	· · ·		Southbound	1,200		2230101			.,500	1
Glenwood Drive	Glenwood	Kingsway	Northbound	3,430	5,130	Collector	Collector (Residential or Industrial)	400	4,000	1
L		3,	Southbound	1,700		Collector				1

Southbound 1,700 5,100 Concern

### CAPACITY ANALYSIS - 10 YEAR (2020) HORIZON

	From 28 Street	То	Direction	Forecasted		Road Classification	Road Classification	Lane Capacity for Road Classification	for Road Classification	Number of Lanes
1 Avenue	28 Street			Daily Traffic - Directional	Daily Traffic - Two Way	2000 TPS <sup>1</sup>	City of Cold Lake <sup>2</sup>	(veh/hour/lane) 3	(veh/day/lane) 4	Required (One Direction)
1 Avenue N		25 Street	Eastbound Westbound	2,190 1.500	3,690	Collector	Collector (Residential or Industrial)	400	4,000	1
	25 Street	Nelson Street	Eastbound Westbound	4,250 3,380	7,630	Collector	Collector (Residential or Industrial)	400	4,000	2
Hwy 28 Hwy	Nelson Street	16 Street	Eastbound Westbound	4,120 4,350	8,470	Collector	Collector (Residential or Industrial)	400	4,000	2 2
	wy 55/16 Avenue	25 Street	Northbound Southbound	14,280 12,480	26,760	4-Lane Arterial	Divided Arterial	1,000	10,000	2
8 Avenue	25 Street	16 Street	Eastbound Westbound	9,030 5,750	14,780	4-Lane Arterial	Undivided Arterial	800	8,000	2
8 Avenue	16 Street	10 Street	Eastbound	3,250	5,750	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
8 Avenue	10 Street	Lakeshore Drive	Westbound Eastbound	2,500 1,780	3,700	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 1
Hwy 55 W	West City Limit	28 Street	Westbound Eastbound Westbound	1,920 4,640 2,210	6,850	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
Hwy 55	28 Street	Hwy 28	Eastbound	7,690	14,100	2-Lane Arterial	Undivided Arterial	800	8,000	1
16 Avenue	Hwy 28	16 Street	Westbound Eastbound	6,410 5,720	9,850	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
16 Avenue	16 Street	10 Street	Westbound Eastbound	4,130 3,810	6,130	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
16 Avenue	10 Street	8 Street	Westbound Eastbound	2,320 3,590	6,060	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
English Bay Road N	North City Limit	1 Avenue	Westbound Northbound	2,470 7,390	13,480	Collector	Undivided Arterial	800	8,000	1
English Bay Road	1 Avenue	25 Street	Southbound Northbound	6,090 9,100	15,900	Collector	Undivided Arterial	800	8,000	1 2
English Bay Road	25 Street	Hwy 28	Southbound Northbound	6,800 7,980	14,720	Collector	Undivided Arterial	800	8,000	1
	inglish Bay Road	Hwy 55	Southbound Northbound	6,740 4,620	7.920	Collector	Collector (Residential or Industrial)	400	4.000	1 2
25 Street En		English Bay Road	Southbound Northbound	3,300 3,310	6,010	Collector	Collector (Residential or Industrial)  Collector (Residential or Industrial)	400	4,000	1
	1 Avenue		Southbound Eastbound	2,700 1,820						1 2
Nelson Street	1 Avenue	16 Street	Westbound Northbound	1,050 6,230	2,870	Collector	Local (Residential or Industrial)	100	1,000	2 2
16 Street	1 Avenue	8 Avenue	Southbound Northbound	2,530 1,540	8,760	Collector	Collector (Residential or Industrial)	400	4,000	1
16 Street	8 Avenue	16 Avenue	Southbound Northbound	1,950 1,740	3,490	Collector	Collector (Residential or Industrial)	400	4,000	1
16 Street	16 Avenue	10 Street	Southbound Northbound	1,510 1,950	3,250	Collector	Collector (Residential or Industrial)	400	4,000	1
10 Street	1 Avenue	8 Avenue	Southbound	1,870	3,820	Collector	Collector (Residential or Industrial)	400	4,000	1
10 Street	8 Avenue	16 Avenue	Northbound Southbound	2,280 2,530	4,810	Collector	Collector (Residential or Industrial)	400	4,000	1 1
10 Street	16 Avenue	16 Street	Northbound Southbound	1,820 1,690	3,510	Collector	Collector (Residential or Industrial)	400	4,000	1
6 Street <sup>5</sup>	16 Avenue	21 Avenue	Northbound Southbound	1,820 1,690	3,510	Collector	Collector (Residential or Industrial)	400	4,000	1
Hwy 28/55 Hwy	wy 55/16 Avenue	75 Avenue	Northbound Southbound	20,290 18,290	38,580	4-Lane Arterial	Expressway	1,800	18,000	2
Hwy 28/55	75 Avenue	69 Avenue	Northbound Southbound	21,650 21,170	42,820	4-Lane Arterial	Expressway	1,800	18,000	2
Hwy 28/55	69 Avenue	54 Avenue	Northbound Southbound	17,000 20,270	37,270	4-Lane Arterial	Expressway	1,800	18,000	1 2
75 Avenue	Hwy 28/55	Future Arterial	Eastbound Westbound	3,710 5,530	9,240	Collector	Collector (Residential or Industrial)	400	4,000	1 2
69 Avenue	Glenwood	Hwy 28/55	Eastbound Westbound	3,080 2,450	5,530	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
54 Avenue	56 Street	Hwy 28/55	Eastbound Westbound	2,350 3,210	5,560	Collector	Collector (Residential or Industrial)	400	4,000	1
54 Avenue	Hwy 28/55	51 Street	Eastbound Westbound	5,130 4,280	9,410	Collector	Collector (Residential or Industrial)	400	4,000	2 2
54 Avenue	51 Street	45 Street	Eastbound Westbound	4,600 3,740	8,340	Collector	Collector (Residential or Industrial)	400	4,000	2
52 Avenue	59 Street	57 Street	Eastbound Westbound	1,500 2,110	3,610	Collector	Collector (Residential or Industrial)	400	4,000	1
52 Avenue	57 Street	Hwy 28/55	Eastbound Westbound	2,590 3,770	6,360	Collector	Collector (Residential or Industrial)	400	4,000	1
Centre Avenue	59 Street	57 Street	Eastbound Westbound	12,560 5,390	17,950	2-Lane Arterial	Undivided Arterial	800	8,000	2
Centre Avenue	57 Street	Hwy 28/55	Eastbound Westbound	11,140 8.210	19,350	4-Lane Arterial	Undivided Arterial	800	8,000	2
50 Avenue	Hwy 28/55	51 Street	Eastbound Westbound	6,560 4,850	11,410	2-Lane Arterial	Undivided Arterial	800	8,000	1
50 Avenue	51 Street	50 Street	Eastbound Westbound	5,190	8,690	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
50 Avenue	50 Street	45 Street	Eastbound Westbound	3,500 5,420 3,090	8,510	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2 1
50 Avenue	45 Street	41 Street	Eastbound	4,560	7,270	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
50 Avenue	41 Street	Future Arterial	Westbound Eastbound	2,710 3,950	6,420	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue F	Future Arterial	Baywood Road	Westbound Eastbound	2,470 3,270	5,350	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
	Hwy 28/55	45 Street	Westbound Eastbound	2,080 4,710	8,640	Collector	Collector (Residential or Industrial)	400	4,000	2
	52 Avenue	Centre Avenue	Westbound Northbound	3,930 2,010	2.990	Collector	Local (Residential or Industrial)	100	1,000	1 3
	54 Avenue	52 Avenue	Southbound Northbound	980 1,410	2,340	Collector	Local (Residential or Industrial)	100	1,000	1 2
	52 Avenue	Centre Avenue	Southbound Northbound	930 2,620	4,280	Collector	Collector (Residential or Industrial)	400	4,000	1
	54 Avenue	52 Avenue	Southbound Northbound	1,660 14,270	32.000	4-Lane Arterial	Expressway	1,800	18,000	1
			Southbound Northbound	17,730 14,590	. ,					1
	52 Avenue	50 Avenue	Southbound Northbound	15,870 7,220	30,460	4-Lane Arterial	Expressway	1,800	18,000	1
<u> </u>	50 Avenue	50 Street	Southbound Northbound	9,270 10,360	16,490	4-Lane Arterial	Undivided Arterial	800	8,000	2
Hwy 28/55	50 Street	43 Avenue	Southbound	12,550 7,700	22,910	4-Lane Arterial	Divided Arterial	1,000	10,000	2
	43 Avenue	South City Limit	Northbound Southbound	8,610 1,550	16,310	4-Lane Arterial	Undivided Arterial	800	8,000	2
	54 Avenue	50 Avenue	Southbound Northbound	1,690 5,520	3,240	Collector	Collector (Residential or Industrial)	400	4,000	1 1
	50 Avenue	Hwy 28/55	Southbound	5,240	10,760	Collector	Undivided Arterial	800	8,000	1
45 Street	54 Avenue	50 Avenue	Northbound Southbound	640 420	1,060	Collector	Local (Residential or Industrial)	100	1,000	1
45 Street	50 Avenue	43 Avenue	Northbound Southbound	450 710	1,160	Collector	Local (Residential or Industrial)	100	1,000	1
41 Street	54 Avenue	50 Avenue	Northbound Southbound	2,560 1,660	4,220	Collector	Collector (Residential or Industrial)	400	4,000	1

City of Cold Lake - Transportation Study Project No: 2010-3050 Date: April 11, 2011

### CAPACITY ANALYSIS - 10 YEAR (2020) HORIZON

Corridor	Interse	Intersection		Forecasted	Volumes	Road Classification	Road Classification	Lane Capacity for Road Classification	Lane Capacity for Road Classification	Number of Lanes
Corridor	From	То	Direction	Daily Traffic - Directional	Daily Traffic - Two Way	2000 TPS <sup>1</sup>	City of Cold Lake <sup>2</sup>	(veh/hour/lane) 3	(veh/day/lane) 4	Required (One Direction)
Kingsway	59 Street	Glenwood	Eastbound	9,220	15,440	2-Lane Arterial	Undivided Arterial	800	8,000	2
Kingsway	39 311961	Gleriwood	Westbound	6,220	13,440	Z*Lane Antenai	Oridivided Arterial	000	8,000	1
Kingsway	Timberline	Glenwood	Eastbound	9,960	15,970	Collector	Undivided Arterial	800	8,000	2
Killysway	Timbelline	Gieriwood	Westbound	6,010	15,970	Collector	Olidivided Arterial	800	8,000	1
Kingsway	Queensway	Timberline	Eastbound	4,840	8.440	Collector	Collector (Residential or Industrial)	400	4,000	2
Kingsway	Queensway	Timberline	Westbound	3,600	8,440	Collector	Collector (Residential of Industrial)	400	4,000	1
Kingsway	Tennis Court Road	Queensway	Eastbound	890	2,290	Collector	Local (Residential or Industrial)	100	1.000	1
Killysway	igsway Tellilis Coult Road Queelisway	Queensway	Westbound	1,400	2,290	Collector	Local (Residential of Industrial)	100	1,000	2
Kingsway	End of Road	Tennis Court Road	Eastbound	1,080	2,550	Collector	Local (Residential or Industrial)	100	1,000	2
Killysway	sway End of Road Tennis Court Ro	Tellilis Coult Road	Westbound	1,470	2,330	Collector	Local (Residential of Industrial)	100	1,000	2
Tennis Court Road	Queensway	Kingsway	Northbound	270	330	Collector	Lane	N/A	N/A	N/A
Terris Court Road	Queensway	KillySway	Southbound	60			Laire	INA	147	N/A
Queensway	Tennis Court Road	Kingsway	Northbound	2,300	3,990	Collector	Collector (Residential or Industrial)	400	4,000	1
Queensway	Terris Court Road	KillySway	Southbound	1,690	3,990					1
0	Kingsway	Hanger Ln	Northbound	2,940	3,590	Collector	Collector (Residential or Industrial)	400	4,000	1
Queensway	Killysway	nanger Lit	Southbound	650	3,390	Collector	Collector (Residential or Industrial)	400	4,000	1
Timberline	Juniper Avenue	VI	Northbound	1,390	2,370	Collector	Local (Residential or Industrial)	100	1,000	2
i imberiine	Juniper Avenue	Kingsway	Southbound	980	2,370	Collector	Local (Residential or Industrial)	100	1,000	1
Timberline	Manager 1	Athabasca Road	Northbound	2,890	4.500	0-11	Callantes (Basidantial as Industrial)	400	4 000	1
i imperline	Kingsway	Athabasca Road	Southbound	1,630	4,520	Collector	Collector (Residential or Industrial)	400	4,000	1
Olemand Drive	01	Manager 1	Northbound	6,290	0.000	O Lance Administra	Collector (Residential or Industrial)	400	4,000	2
Glenwood Drive	Glenwood	Kingsway	Southbound	3,590	9,880	2-Lane Arterial				1

### CAPACITY ANALYSIS - 15 YEAR (2025) HORIZON

	Interse	ection	B1 11	Forecasted	Volumes	Road Classification	Road Classification	Lane Capacity	Lane Capacity	Number of Lanes
Corridor	From	То	Direction	Daily Traffic - Directional	Daily Traffic - Two Way	2000 TPS 1	City of Cold Lake <sup>2</sup>	for Road Classification (veh/hour/lane) 3	for Road Classification (veh/day/lane) 4	Required (One Direction)
1 Avenue	28 Street	25 Street	Eastbound Westbound	2,870 1,910	4,780	Collector	Collector (Residential or Industrial)	400	4,000	1
1 Avenue	25 Street	Nelson Street	Eastbound Westbound	5,660 4,680	10,340	Collector	Undivided Arterial	800	8,000	1
1 Avenue	Nelson Street	16 Street	Eastbound	5,680	11,670	Collector	Undivided Arterial	800	8,000	1
Hwy 28	Hwy 55/16 Avenue	25 Street	Westbound Northbound	5,990 18,790	36,170	4-Lane Arterial	Expressway	1,800	18,000	1 2
8 Avenue	25 Street	16 Street	Southbound Eastbound	17,380 11,040	18,360	4-Lane Arterial	Undivided Arterial	800	8,000	1 2
			Westbound Eastbound	7,320 3,930						1 1
8 Avenue	16 Street	10 Street	Westbound Eastbound	3,220 2,250	7,150	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
8 Avenue	10 Street	Lakeshore Drive	Westbound Eastbound	2,600 5,560	4,850	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2
Hwy 55	West City Limit	28 Street	Westbound	2,710	8,270	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
Hwy 55	28 Street	Hwy 28	Eastbound Westbound	11,050 9,390	20,440	2-Lane Arterial	Divided Arterial	1,000	10,000	2 1
16 Avenue	Hwy 28	16 Street	Westbound Westbound	7,360 5,550	12,910	2-Lane Arterial	Undivided Arterial	800	8,000	1
16 Avenue	16 Street	10 Street	Eastbound Westbound	4,850 3,110	7,960	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
16 Avenue	10 Street	8 Street	Eastbound Westbound	4,640 3.190	7,830	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
English Bay Road	North City Limit	1 Avenue	Northbound Southbound	10,550 9,540	20,090	Collector	Divided Arterial	1,000	10,000	2
English Bay road	1 Avenue	25 Street	Northbound	13,350	24,350	Collector	Divided Arterial	1,000	10,000	2
English Bay road	25 Street	Hwy 28	Southbound Northbound	11,000 11,090	21,150	Collector	Divided Arterial	1,000	10,000	2 2
28 Street	English Bay Road	Hwy 55	Southbound Northbound	10,060 7,100	12.690	Collector	Undivided Arterial	800	8.000	2
		,	Southbound Northbound	5,590 4,640	,,,,,				.,	1 2
25 Street	1 Avenue	English Bay Road	Southbound Eastbound	4,070 2,510	8,710	Collector	Collector (Residential or Industrial)	400	4,000	2
Nelson Street	1 Avenue	16 Street	Westbound Northbound	1,450 7,870	3,960	Collector	Collector (Residential or Industrial)	400	4,000	1
16 Street	1 Avenue	8 Avenue	Southbound	3,660	11,530	Collector	Undivided Arterial	800	8,000	1
16 Street	8 Avenue	16 Avenue	Northbound Southbound	2,180 2,910	5,090	Collector	Collector (Residential or Industrial)	400	4,000	1
16 Street	16 Avenue	10 Street	Northbound Southbound	2,550 2,330	4,880	Collector	Collector (Residential or Industrial)	400	4,000	1
10 Street	1 Avenue	8 Avenue	Northbound Southbound	2,800 2,710	5,510	Collector	Collector (Residential or Industrial)	400	4,000	1
10 Street	8 Avenue	16 Avenue	Northbound Southbound	3,020 3,480	6,500	Collector	Collector (Residential or Industrial)	400	4,000	1
10 Street	16 Avenue	16 Street	Northbound Southbound	2,440 2,150	4,590	Collector	Collector (Residential or Industrial)	400	4,000	1
6 Street <sup>5</sup>	16 Avenue	75 Avenue	Northbound	2,440	4,590	Collector	Collector (Residential or Industrial)	400	4,000	1
Hwy 28/55	Hwy 55/16 Avenue	75 Avenue	Southbound Northbound	2,150 27,060	52,890	4-Lane Arterial	Expressway	1,800	18,000	1 2
Hwy 28/55	75 Avenue	69 Avenue	Southbound Northbound	25,830 28,420	56.990	4-Lane Arterial	Expressway	1.800	18.000	2 2
•			Southbound Northbound	28,570 21,360				,,,,,	-7	2 2
Hwy 28/55	69 Avenue	54 Avenue	Southbound Eastbound	24,930 4,950	46,290	4-Lane Arterial	Expressway	1,800	18,000	2
75 Avenue	Hwy 28/55	Future Arterial	Westbound Eastbound	6,610 4,320	11,560	Collector	Undivided Arterial	800	8,000	1 2
69 Avenue	Glenwood	Hwy 28/55	Westbound Eastbound	3,680	8,000	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2
69 Avenue	Hwy 28/55	Future Arterial	Westbound	8,130 8,840	16,970	Collector	Undivided Arterial	800	8,000	2
47 Street <sup>6</sup>	69 Avenue	61/62 Avenue	Northbound Southbound	4,420 4,065	8,485	Collector	Collector (Residential or Industrial)	400	4,000	2 2
54 Avenue	56 Street	Hwy 28/55	Eastbound Westbound	3,160 4,360	7,520	Collector	Collector (Residential or Industrial)	400	4,000	1 2
54 Avenue	Hwy 28/55	51 Street	Eastbound Westbound	5,310 4.600	9,910	Collector	Collector (Residential or Industrial)	400	4,000	2 2
54 Avenue	51 Street	45 Street	Eastbound Westbound	6,340 5,150	11,490	Collector	Undivided Arterial	800	8,000	1
54 Avenue 7	45 Street	41 Street	Eastbound Westbound	3,795 3,335	7,130	Collector	Collector (Residential or Industrial)	400	4,000	1
54 Avenue	41 Street	Future Arterial	Eastbound Westbound	1,250	2,770	Collector	Local (Residential or Industrial)	100	1,000	2
52 Avenue	59 Street	57 Street	Eastbound	1,520 2,070	4,980	Collector	Collector (Residential or Industrial)	400	4,000	1
52 Avenue	57 Street	Hwy 28/55	Westbound Eastbound	2,910 3,570	8,770	Collector	Collector (Residential or Industrial)	400	4,000	1
Centre Avenue			Westbound Eastbound	5,200 15,000		2-Lane Arterial	Divided Arterial			2 2
	59 Street	57 Street	Westbound Eastbound	6,440 13,460	21,440			1,000	10,000	1 2
Centre Avenue	57 Street	Hwy 28/55	Westbound Eastbound	10,130 7,440	23,590	4-Lane Arterial	Divided Arterial	1,000	10,000	2
50 Avenue	Hwy 28/55	51 Street	Westbound	5,260 6,140	12,700	2-Lane Arterial	Undivided Arterial	800	8,000	1 2
50 Avenue	51 Street	50 Street	Westbound	3,610	9,750	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
50 Avenue	50 Street	45 Street	Eastbound Westbound	5,990 2,440	8,430	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2 1
50 Avenue	45 Street	41 Street	Eastbound Westbound	5,110 2,180	7,290	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
50 Avenue	41 Street	Future Arterial	Eastbound Westbound	2,980 1,660	4,640	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 1
50 Avenue	Future Arterial	Baywood Road	Eastbound Westbound	2,630 1,820	4,450	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
43 Avenue	Hwy 28/55	45 Street	Eastbound Westbound	5,430 4,510	9,940	Collector	Collector (Residential or Industrial)	400	4,000	2 2
59 Street	52 Avenue	Centre Avenue	Northbound	2,780	4,010	Collector	Collector (Residential or Industrial)	400	4,000	1
57 Street	54 Avenue	52 Avenue	Southbound Northbound	1,230 1,940	3,220	Collector	Collector (Residential or Industrial)	400	4,000	1
57 Street		Centre Avenue	Southbound Northbound	1,280 3,610			Collector (Residential or Industrial)	400		1 1
	52 Avenue		Southbound Northbound	2,290 18,090	5,900	Collector			4,000	1 2
Hwy 28/55	54 Avenue	52 Avenue	Southbound Northbound	21,530	39,620	4-Lane Arterial	Expressway	1,800	18,000	2 2
Hwy 28/55	52 Avenue	50 Avenue	Southbound	18,190 19,410	37,600	4-Lane Arterial	Expressway	1,800	18,000	2
Hwy 28/55	50 Avenue	50 Street	Northbound Southbound	9,090 11,300	20,390	4-Lane Arterial	Divided Arterial	1,000	10,000	1 2
Hwy 28/55	50 Street	43 Avenue	Northbound Southbound	12,470 15,000	27,470	4-Lane Arterial	Divided Arterial	1,000	10,000	2 2
Hwy 28/55	43 Avenue	South City Limit	Northbound Southbound	9,020 9,890	18,910	4-Lane Arterial	Undivided Arterial	800	8,000	2 2
	54 Avenue	50 Avenue	Northbound Southbound	1,760 1,940	3,700	Collector	Collector (Residential or Industrial)	400	4,000	1
51 Street			DOUBLING			L				
51 Street 50 Street	50 Avenue	Hwy 28/55	Northbound Southbound	6,300 5,990	12,290	Collector	Undivided Arterial	800	8,000	1

### CAPACITY ANALYSIS - 15 YEAR (2025) HORIZON

Corridor	Interse	ection	Direction Forecasted		Volumes	Road Classification	Road Classification	Lane Capacity for Road Classification	Lane Capacity for Road Classification	Number of Lanes
Corridor	From	То		Daily Traffic - Directional	Daily Traffic - Two Way	2000 TPS <sup>1</sup>	City of Cold Lake <sup>2</sup>	(veh/hour/lane) 3	(veh/day/lane) 4	Required (One Direction)
45 Street	50 Avenue	43 Avenue	Northbound Southbound	510 870	1,380	Collector	Local (Residential or Industrial)	100	1,000	1
41 Street	54 Avenue	50 Avenue	Northbound Southbound	3,520 2,290	5,810	Collector	Collector (Residential or Industrial)	400	4,000	1
Future Arterial	69 Avenue	54 Avenue	Northbound Southbound	2,320 2,170	4,490	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
Future Arterial	54 Avenue	50 Avenue	Northbound Southbound	1,880 1,420	3,300	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
Kingsway	59 Street	Glenwood	Eastbound Westbound	10,870 7,200	18,070	2-Lane Arterial	Undivided Arterial	800	8,000	2
Kingsway	Timberline	Glenwood	Eastbound Westbound	12,120 7,510	19,630	Collector	Undivided Arterial	800	8,000	2
Kingsway	Queensway	Timberline	Eastbound Westbound	5,990 4,480	10,470	Collector	Undivided Arterial	800	8,000	1
Kingsway	Tennis Court Road	Queensway	Eastbound Westbound	1,090 1,750	2,840	Collector	Local (Residential or Industrial)	100	1,000	2 2
Kingsway	End of Road	Tennis Court Road	Eastbound Westbound	1,270 1,780	3,050	Collector	Collector (Residential or Industrial)	400	4,000	1
Tennis Court Road	Queensway	Kingsway	Northbound Southbound	300 70	370	Collector	Lane	N/A	N/A	N/A N/A
Queensway	Tennis Court Road	Kingsway	Northbound Southbound	2,760 2,080	4,840	Collector	Collector (Residential or Industrial)	400	4,000	1
Queensway	Kingsway	Hanger Ln	Northbound Southbound	3,550 790	4,340	Collector	Collector (Residential or Industrial)	400	4,000	1 1
Timberline	Juniper Avenue	Kingsway	Northbound Southbound	1,680 1,200	2,880	Collector	Local (Residential or Industrial)	100	1,000	2 2
Timberline	Kingsway	Athabasca Road	Northbound Southbound	3,570 2,010	5,580	Collector	Collector (Residential or Industrial)	400	4,000	1
Glenwood Drive	Glenwood	Kingsway	Northbound Southbound	7,310 4,550	11,860	2-Lane Arterial	Undivided Arterial	800	8,000	1

Assumed to the Street to be also fall yraffic for 54 Avenue (45 Street to 41 Street) to be average of daily traffic on 54 Avenue (51 Street to 45 Street) and 54 Avenue (41 Street to Future Arterial)

### CAPACITY ANALYSIS - 20 YEAR (2030) HORIZON

	Interse	ection		Forecasted	I Volumes	Road Classification	Read Classification	Lane Capacity	Lane Capacity	Number of Lanes
Corridor	From	То	Direction	Daily Traffic - Directional	Daily Traffic - Two Way	2000 TPS <sup>1</sup>	Road Classification City of Cold Lake <sup>2</sup>	for Road Classification (veh/hour/lane) 3	for Road Classification (veh/day/lane) 4	Required (One Direction)
1 Avenue	28 Street	25 Street	Eastbound Westbound	3,360 2,210	5,570	Collector	Collector (Residential or Industrial)	400	4,000	1
1 Avenue	25 Street	Nelson Street	Eastbound Westbound	6,650 5,560	12,210	Collector	Undivided Arterial	800	8,000	1
1 Avenue	Nelson Street	16 Street	Eastbound Westbound	6,240 6,580	12,820	Collector	Undivided Arterial	800	8,000	1
Hwy 28	Hwy 55/16 Avenue	25 Street	Northbound	21,470	42,370	4-Lane Arterial	Expressway	1,800	18,000	2 2
8 Avenue	25 Street	16 Street	Southbound Eastbound	20,900 11,990	20,570	4-Lane Arterial	Divided Arterial	1,000	10,000	2
8 Avenue	16 Street	10 Street	Westbound Eastbound	8,580 4,630	8,540	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2
8 Avenue	10 Street	Lakeshore Drive	Westbound Eastbound	3,910 2,720	5,930	4-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
			Westbound Eastbound	3,210 7,370	.,		, ,		,	1
Hwy 55	West City Limit	28 Street	Westbound Eastbound	3,320 14,440	10,690	2-Lane Arterial	Undivided Arterial	800	8,000	1 2
Hwy 55	28 Street	Hwy 28	Westbound Eastbound	11,280 8,660	25,720	2-Lane Arterial	Divided Arterial	1,000	10,000	2 2
16 Avenue	Hwy 28	16 Street	Westbound Eastbound	6,650 5,820	15,310	2-Lane Arterial	Undivided Arterial	800	8,000	1 2
16 Avenue	16 Street	10 Street	Westbound Eastbound	3,820 4,510	9,640	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2
16 Avenue	10 Street	8 Street	Westbound	3,520	8,030	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 1
16 Avenue	8 Street	East City Limit	Westbound Westbound	2,370 1,660	4,030	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1
English Bay Road 5	North City Limit	Lake Avenue	Northbound Southbound	6,430 5,920	12,350	Collector	Undivided Arterial	800	8,000	1
English Bay Road	Lake Avenue	1 Avenue	Northbound Southbound	12,860 11,840	24,700	Collector	Divided Arterial	1,000	10,000	2 2
English Bay Road	1 Avenue	25 Street	Northbound Southbound	16,370 13,760	30,130	Collector	Expressway	1,800	18,000	1
English Bay Road	25 Street	Hwy 28	Northbound Southbound	13,510 12,300	25,810	Collector	Divided Arterial	1,000	10,000	2 2
28 Street	English Bay Road	Hwy 55	Northbound Southbound	8,640 7,040	15,680	Collector	Undivided Arterial	800	8,000	2
25 Street	1 Avenue	English Bay Road	Northbound	5,560 4,950	10,510	Collector	Undivided Arterial	800	8,000	1
Nelson Street	1 Avenue	16 Street	Eastbound Weathound	2,760	4,360	Collector	Collector (Residential or Industrial)	400	4,000	1
16 Street	1 Avenue	8 Avenue	Northbound	1,600 9,290	13,820	Collector	Undivided Arterial	800	8,000	1 2
16 Street	8 Avenue	16 Avenue	Southbound Northbound	4,530 3,850	8,130	Collector	Collector (Residential or Industrial)	400	4,000	1
16 Street	16 Avenue	10 Street	Southbound Northbound	4,280 3,810	7,010	Collector	Collector (Residential or Industrial)	400	4,000	2
			Southbound Northbound	3,200 5,720						1
16 Street	10 Street	75 Avenue	Southbound Northbound	4,600 3,400	10,320	Collector	Undivided Arterial	800	8,000	1
10 Street	1 Avenue	8 Avenue	Southbound Northbound	3,320 3,560	6,720	Collector	Collector (Residential or Industrial)	400	4,000	1
10 Street	8 Avenue	16 Avenue	Southbound	4,250	7,810	Collector	Collector (Residential or Industrial)	400	4,000	2
10 Street	16 Avenue	16 Street	Northbound Southbound	2,380 2,300	4,680	Collector	Collector (Residential or Industrial)	400	4,000	1
8 Street	16 Avenue	75 Avenue	Northbound Southbound	2,840 2,480	5,320	Collector	Collector (Residential or Industrial)	400	4,000	1
6 Street <sup>6</sup>	16 Avenue	21 Avenue	Northbound Southbound	2,380 2,300	4,680	Collector	Collector (Residential or Industrial)	400	4,000	1
Hwy 28/55	Hwy 55/16 Avenue	75 Avenue	Northbound Southbound	30,680 31.810	62,490	4-Lane Arterial	Expressway	1,800	18,000	2 2
Hwy 28/55	75 Avenue	69 Avenue	Northbound Southbound	31,690 33,860	65,550	4-Lane Arterial	Expressway	1,800	18,000	2 2
Hwy 28/55	69 Avenue	54 Avenue	Northbound Southbound	24,020 28,920	52,940	4-Lane Arterial	Expressway	1,800	18,000	2 2
75 Avenue	Hwy 28/55	Future Arterial	Eastbound	4,390	9,120	Collector	Collector (Residential or Industrial)	400	4,000	2
69 Avenue	Glenwood	Hwy 28/55	Westbound Eastbound	4,730 5,590	10,070	2-Lane Arterial	Undivided Arterial	800	8,000	1
69 Avenue	Hwy 28/55	Future Arterial	Westbound Eastbound	4,480 8,350	15,790	Collector	Undivided Arterial	800	8.000	1 2
47 Street 7	69 Avenue	61/62 Avenue	Westbound Northbound	7,440 3,720	7,895	Collector	Collector (Residential or Industrial)	400	4,000	1
54 Avenue	56 Street	Hwy 28/55	Southbound Eastbound	4,175 3,850	9,460	Collector	Collector (Residential or Industrial)	400	4,000	2
			Westbound Eastbound	5,610 5,850						2
54 Avenue	Hwy 28/55	51 Street	Westbound Eastbound	4,710 6,960	10,560	Collector	Undivided Arterial	800	8,000	1
54 Avenue	51 Street	45 Street	Westbound Eastbound	5,660 4,600	12,620	Collector	Undivided Arterial	800	8,000	1 2
54 Avenue 8	45 Street	41 Street	Westbound Eastbound	3,825 2,240	8,425	Collector	Collector (Residential or Industrial)	400	4,000	1 1
54 Avenue	41 Street	Future Arterial	Westbound	1,990	4,230	Collector	Collector (Residential or Industrial)	400	4,000	1
52 Avenue	59 Street	57 Street	Westbound	2,270 3,190	5,460	Collector	Collector (Residential or Industrial)	400	4,000	1
52 Avenue	57 Street	Hwy 28/55	Eastbound Westbound	3,920 5,710	9,630	Collector	Collector (Residential or Industrial)	400	4,000	1 2
Centre Avenue	59 Street	57 Street	Eastbound Westbound	17,310 7,430	24,740	2-Lane Arterial	Divided Arterial	1,000	10,000	2
Centre Avenue	57 Street	Hwy 28/55	Eastbound Westbound	15,540 11,950	27,490	4-Lane Arterial	Divided Arterial	1,000	10,000	2 2
50 Avenue	Hwy 28/55	51 Street	Eastbound Westbound	7,940 5,570	13,510	2-Lane Arterial	Undivided Arterial	800	8,000	1
50 Avenue	51 Street	50 Street	Eastbound Westbound	6,800 4,120	10,920	2-Lane Arterial	Undivided Arterial	800	8,000	1
50 Avenue	50 Street	45 Street	Eastbound Westbound	6,610	9,340	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
50 Avenue	45 Street	41 Street	Eastbound	2,730 6,170	9,220	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	2
50 Avenue	41 Street	Future Arterial	Westbound Eastbound	3,050 4,880	8.320	2-Lane Arterial	Collector (Residential or Industrial)	400	4,000	1 2
50 Avenue	Future Arterial		Westbound Eastbound	3,440 2,070	3,630	2-Lane Arterial	Collector (Residential or Industrial)  Collector (Residential or Industrial)	400	4,000	1
		Baywood Road	Westbound Eastbound	1,560 6,220						1
43 Avenue	Hwy 28/55	45 Street	Westbound Northbound	5,020 3,050	11,240	Collector	Undivided Arterial	800	8,000	1
59 Street	52 Avenue	Centre Avenue	Southbound Northbound	1,350 2,130	4,400	Collector	Collector (Residential or Industrial)	400	4,000	1
57 Street	54 Avenue	52 Avenue	Southbound	1,400 3,960	3,530	Collector	Collector (Residential or Industrial)	400	4,000	1 1
57 Street	52 Avenue	Centre Avenue	Northbound Southbound	2,510	6,470	Collector	Collector (Residential or Industrial)	400	4,000	1
Hwy 28/55	54 Avenue	52 Avenue	Northbound Southbound	20,930 24,610	45,540	4-Lane Arterial	Expressway	1,800	18,000	2 2
Hwy 28/55	52 Avenue	50 Avenue	Northbound Southbound	20,540 21,950	42,490	4-Lane Arterial	Expressway	1,800	18,000	2 2
Hwy 28/55	50 Avenue	50 Street	Northbound Southbound	10,700 13,030	23,730	4-Lane Arterial	Divided Arterial	1,000	10,000	2 2
Hwy 28/55	50 Street	43 Avenue	Northbound Southbound	14,520 17,550	32,070	4-Lane Arterial	Expressway	1,800	18,000	1
Hwy 28/55	43 Avenue	40 Avenue	Northbound Southbound	10,330 11,090	21,420	4-Lane Arterial	Divided Arterial	1,000	10,000	2 2
Hwy 28/55 9	40 Avenue	South City Limit	Northbound	5,165	10,710	4-Lane Arterial	Undivided Arterial	800	8,000	1
,		. ,	Southbound	5,545					.,	1

#### CAPACITY ANALYSIS - 20 YEAR (2030) HORIZON

Corridor	Interse	ection	Direction	Forecasted		Road Classification	Road Classification	Lane Capacity for Road Classification	Lane Capacity for Road Classification	Number of Lanes Required (One
Comuoi	From	То	Direction	Daily Traffic - Directional	Daily Traffic - Two Way	2000 TPS <sup>1</sup>	City of Cold Lake <sup>2</sup>	(veh/hour/lane) 3	(veh/day/lane) 4	Direction)
51 Street	54 Avenue	50 Avenue	Northbound Southbound	1,980 2,230	4,210	Collector	Collector (Residential or Industrial)	400	4,000	1
			Northbound	7.170						1
50 Street	50 Avenue	Hwy 28/55	Southbound	6,810	13,980	Collector	Undivided Arterial	800	8,000	1
45 Street	54 Avenue	50 Avenue	Northbound	700	1.150	Collector	Local (Residential or Industrial)	100	1.000	1
40 011001	0471401100	oo / wando	Southbound	450	1,100	Concolor	Ecota (residential or maderia)	100	1,000	
45 Street	50 Avenue	43 Avenue	Northbound Southbound	960 1.460	2,420	Collector	Local (Residential or Industrial)	100	1,000	2
			Northbound	3.870		0.11.				1
41 Street	54 Avenue	50 Avenue	Southbound	2,510	6,380	Collector	Collector (Residential or Industrial)	400	4,000	1
Future Arterial	75 Avenue	69 Avenue	Northbound Southbound	6,900 5.170	12,070	2-Lane Arterial	Undivided Arterial	800	8,000	1 1
			Northbound	6,510						1
Future Arterial	69 Avenue	54 Avenue	Southbound	5,570	12,080	2-Lane Arterial	Undivided Arterial	800	8,000	i
Future Arterial	54 Avenue	50 Avenue	Northbound	4,730	8.570	2-Lane Arterial	Collector (Residential or Industrial)	400	4.000	2
T didio 7 interior	0471401100	oo / wando	Southbound	3,840	0,010	E Edito / titorida	Collector (Neoderlian or Madourlas)	400	4,000	1
Kingsway	59 Street	Glenwood	Eastbound Westbound	12,400 8.220	20,620	2-Lane Arterial	Divided Arterial	1,000	10,000	2
Manager	These ratios	Olemand	Eastbound	14,040	20.750	0-11	Divided Arterial	4.000	10.000	2
Kingsway	Timberline	Glenwood	Westbound	8,710	22,750	Collector	Divided Arterial	1,000	10,000	1
Kingsway	Queensway	Timberline	Eastbound	7,020	12,210	Collector	Undivided Arterial	800	8,000	1
	•		Westbound Eastbound	5,190 1,280						1
Kingsway	Tennis Court Road	Queensway	Westbound	2.020	3,300	Collector	Collector (Residential or Industrial)	400	4,000	1
Kingsway	End of Road	Tennis Court Road	Eastbound	1,440	3.480	Collector	Collector (Residential or Industrial)	400	4.000	1
rungaway	Life of Road	Terrina Court Road	Westbound	2,040	3,400	Collector	Collector (Residential or Industrial)	400	4,000	1
Tennis Court Road	Queensway	Kingsway	Northbound Southbound	330 70	400	Collector	Lane	N/A	N/A	N/A N/A
			Northbound	3.130						1
Queensway	Tennis Court Road	Kingsway	Southbound	2,440	5,570	Collector	Collector (Residential or Industrial)	400	4,000	1
Queensway	Kingsway	Hanger Ln	Northbound	4,100	5.010	Collector	Collector (Residential or Industrial)	400	4.000	2
	5,		Southbound Northbound	910 1.920	-,				,	1 1
Timberline	Juniper Avenue	Kingsway	Southbound	1,400	3,320	Collector	Collector (Residential or Industrial)	400	4,000	1
Timberline	Kingsway	Athabasca Road	Northbound	4,170	6.480	Collector	Collector (Residential or Industrial)	400	4.000	2
rimbellille	KingSway	Autabastd Rudu	Southbound	2,310	0,400	Conector	Collector (residefittal of fridustrial)	400	4,000	1
Glenwood Drive	Glenwood	Kingsway	Northbound Southbound	8,460 5,270	13,730	2-Lane Arterial	Undivided Arterial	800	8,000	2

Glerwood Drive Glerwood Kingsway Southbound 5.270 13,730 2-Lane Afterial Undivided Arterial

1. Road classification based on Cool Transportation Study

2. Road classification based on Daily Service Volumes stipulated in City's Roadway Design Standards (Municipal Engineering Servicing Standards and Standard Construction Specifications, Jan 2008)

3. Based on Lane Capacity Table (attached). Using road cessification according to City's standards.

4. Based on assumption that PM peak hour traffic is 10% of the daily traffic

5. Assumed daily traffic for English Bay Road (North City Limit to Lake Avenue) to be half of daily traffic on English Bay Road (Lake Avenue to 1 Avenue)

6. Assumed daily traffic for Street to be similar to 0 Street (15 Avenue and 16 Street)

7. Assumed daily traffic for Street to be similar to 15 Street to 6 Street)

8. Assumed daily traffic for Streen (45 Street to 15 Street)

9. Assumed daily traffic for Highway 28 (40 Avenue to South City Limit) to be half of daily traffic on Highway 28 (43 Avenue to 40 Avenue)

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